

102 Abbie Court

5228

SFR

MASTER PERMIT NO. N/A

TOWN OF SEWALL'S POINT

Date 1/16/01

BUILDING PERMIT NO. 5228

Building to be erected for FOGLIA CONTRACTING CORP Type of Permit BLDG. - S.F.R.

Applied for by FOGLIA CONTRACTING CORP (Contractor) Building Fee \$1,844.35

Subdivision SEWALL'S MEADOW Lot 22 Block _____ Radon Fee 37.72

Address 102 ABBIE CT Impact Fee 4,024.92

Type of structure S.F.R. A/C Fee 120.00

Parcel Control Number: 13-38-41-013-000-00220-00000 Electrical Fee 120.00

Amount Paid \$6,571.42 Check # 4326 Cash _____ Other Fees (PLAD) 184.43

Total Construction Cost \$ 192,120.00 TOTAL Fees \$6,571.42

Signed [Signature]
Applicant

Signed [Signature]
Town Building Inspector OFFICIAL

BUILDING PERMIT

FORM BOARD SURVEY DATE _____
COMPACTION TESTS DATE _____
GROUND ROUGH DATE _____
SOIL POISONING DATE _____
FOOTINGS / PIERS DATE _____
SLAB ON GRADE DATE _____
TIE-BEAMS & COLUMNS DATE _____
STRAPS AND ANCHORS DATE _____
DRIVEWAY DATE _____
AS-BUILT SURVEY DATE _____

SHEATHING DATE _____
FRAMING DATE _____
INSULATION DATE _____
ROOF DRY-IN DATE _____
ROOF FINAL DATE _____
METER FINAL DATE _____
AS BUILT SURVEY DATE _____
STORM PANELS DATE _____
LANDCAPE & GRADE DATE _____
FINAL INSPECTION DATE _____

FLOOD ZONE _____

LOWEST HABITABLE FLOOR ELEV. _____

24 HOURS NOTICE REQUIRED FOR INSPECTIONS.

CALL 287-2455

WORK HOURS – 8:00 AM UNTIL 5:00 PM

MONDAY THROUGH SATURDAY

☐ New Construction ☐ Remodel ☐ Addition ☐ Demolition

This permit must be visible from the street, accessible to the inspector.

FURTHER CONDITIONS ARE SET FORTH IN THE APPLICATION FOR PERMIT,
NOTATIONS ON THE APPROVED SUBMITTALS, AND ATTACHMENTS IN THE PERMIT FILE.
DO NOT FASTEN THIS OR ANY OTHER SIGN TO A TREE!



TEL: (954) 975-3384
FAX: (954) 978-8980
3120 N.W. 16TH TERRACE
POMPANO BEACH, FL. 33064

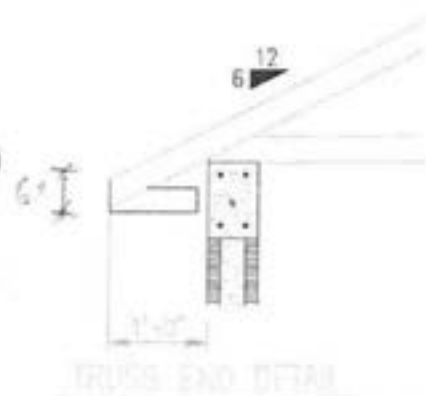
Customer FOGILA CUSTOM HOMES

Job # 00469

PROJECT LOT 22

TOWN COPY
102 ABBIE CT.

PN 5228



Designer : MH

MASTER PERMIT NO. 5228

TOWN OF SEWALL'S POINT

Date 8/29/01

BUILDING PERMIT NO. 5521

Building to be erected for FOGLIA CONTR. CORP.

Type of Permit IRRIGATION

Applied for by FOGLIA CONTR. CORP.

(Contractor)

Building Fee \$130.00

Subdivision SEWALL'S MEADOW Lot 22

Block

Radon Fee 60.00

Address 102 ARBIE COURT

Impact Fee

Type of structure S.F.R. (UNDER CONST.)

A/C Fee

8/29/01 "AFTER PART" PERMIT
- DOUBLE FEE

Electrical Fee

Parcel Control Number:

13-38-41-013-000-00220-00000

Plumbing Fee

Amount Paid \$66.00 Check # 4581 Cash

Roofing Fee 6.00

Total Construction Cost \$2,450.00

Other Fees (REB) 3.00

TOTAL Fees \$33.00

\$66.00

Signed [Signature]

Applicant

Signed

[Signature]

Town Building Inspector OFFICIAL

BUILDING PERMIT

FORM BOARD SURVEY DATE _____
COMPACTION TESTS DATE _____
GROUND ROUGH DATE _____
SOIL POISONING DATE _____
FOOTINGS / PIERS DATE _____
SLAB ON GRADE DATE _____
TIE-BEAMS & COLUMNS DATE _____
STRAPS AND ANCHORS DATE _____
DRIVEWAY DATE _____
AS-BUILT SURVEY DATE _____

SHEATHING DATE _____
FRAMING DATE _____
INSULATION DATE _____
ROOF DRY-IN DATE _____
ROOF FINAL DATE _____
METER FINAL DATE _____
AS BUILT SURVEY DATE _____
STORM PANELS DATE _____
LANDCAPE & GRADE DATE _____
FINAL INSPECTION DATE 9/3/01

FLOOD ZONE _____

LOWEST HABITABLE FLOOR ELEV. _____

24 HOURS NOTICE REQUIRED FOR INSPECTIONS.

CALL 287-2455

WORK HOURS - 8:00 AM UNTIL 5:00 PM

MONDAY THROUGH SATURDAY

☐ New Construction ☐ Remodel ☐ Addition ☐ Demolition

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TOWN OF SEWALL'S POINT

Building Department - Inspection Log

Date of Inspection: ☐ Mon ☐ Wed ☒ Fri AUGUST 31, 2001; Page 3 of 3.

PERMIT	OWNER/ADDRESS/CONTR.	INSPECTION TYPE	RESULTS	NOTES/COMMENTS:
✓ 5380	GIFFORD	POOL DECK	Passed	~~Co. per. Test.~~
(16)	85 N. SEWALL'S POINT RD A&G CONCRETE POOLS			Waterfall area only INSPECTOR: <u>J 8/31</u>
✓ 5489	STRACUZZI (285-2815) (260-1443 con)	U/G PLUMBING	Passed	
S (7)	12 RIO VISTA DR. RUCCOLO			INSPECTOR: <u>J 8/31</u>
✓ 5324	HECKENBERG	MAS. WALL - FINAL	Passed	PN 5405 - ENTRY COL. (STATUS?)
(13)	SLAGOON ISLAND CT. U/B (JAMIE 718-2777)	ENTRY COL. - FINAL	Passed	INSPECTOR: <u>J 8/31</u>
✓ T/R	TENNY	FIELD VERIF.	Passed	(AMENDED PLAN -)
S (9)	105 SEWALL'S POINT RD GORDON'S TREE SERVICE		PN 0494 8/31/01	(W/O GUMBO LIMBO) INSPECTOR: <u>J 8/31</u>
✓ T/R	AYRES	FIELD VERIF.	Passed	
S (8)	155 RIVER RD. MONTE'S TREE SERVICE		PN 0493 8/31/01	INSPECTOR: <u>J 8/31</u>
✓ 5482	RAO	IRRIGATION - FINAL	Passed	MPN 5192
(15)	30 CASTLE HILL WAY FRITZ IRRIGATION			INSPECTOR: <u>J 8/31</u>
✓ 5521	FOGLIA	IRRIGATION - FINAL	Passed	MPN 5228
S (12)	102 ABBIE COURT FOGLIA CONTRACTING			INSPECTOR: <u>J 8/31</u>

OTHER: _____

AC 5910992

STATE OF FLORIDA
DEPARTMENT OF BUSINESS AND PROFESSIONAL REGULATION
CONSTRUCTION INDUSTRY LICENSING BOARD

DATE BATCH NUMBER LICENSE NO.

07/03/2002 00900145
The Board will conduct a
random review of the
licensee's records to
verify the provisions of the
Florida Statutes.
Expiration date: AUG 31, 2002

FOGLIA, JOSEPH N.
FOGLIA CONTRACTING CORP
7428 WILES RD
CORAL SPRINGS

JEB BUSH
GOVERNOR

DISPLAY AS REQUIRED BY LAW

CYNTHIA A. HENDERSON
SECRETARY

CG-C038434

FLORIDA QUALITY TRUSS INC.

Customer:
Fagila Custom Homes
Project:
Lot 22.....

Truss ID.

1	CJ7C
2	C1
3	C3
4	C5
5	J7
6	H8
7	H1
8	H2
9	S2
10	S3
11	S4
12	S5
13	S6
14	GT25
15	FG3
16	GT36
17	S26
18	S25
19	S24
20	S23
21	S22
22	HH2
23	CJ9
24	J6
25	FG2
26	C4
27	NG2
28	C1A
29	C3A
30	C5A

31	H4
32	GT7
33	T2
34	T3
35	S27
36	S28
37	T4
38	H5
39	H6
40	T5
41	T6
42	S30
43	S31
44	S32
45	T7
46	T7A
47	S33
48	S34
49	S42
50	S41
51	H40A
52	S40
53	G1
54	G1A
55	S39
56	S38A
57	S38
58	S37
59	S36
60	S35
61	S14
62	CJ7A
63	NG1
64	CE5
65	CF5
66	S21

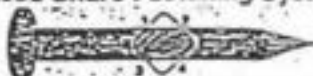
67	S21A
68	S21B
69	FG1
70	C57
71	GT10
72	S17
73	J10
74	S15
75	S18
76	S19
77	S9
78	GT16
79	C3B
80	CJ5C
81	C3E
82	GT6
83	S13
84	S12
85	S11
86	H3
87	CJ5
88	J4
89	GT6A
90	CA5
91	V4
92	V8
93	V12
94	V16
95	T9A
96	T9
97	T8
98	T1
99	J02
100	T10
101	S1
102	CJ1A

103	CC1
104	CC3
105	C2
106	FG8
107	CJ5A
108	CC5
109	CT14
110	CC2
111	FG5
112	CJ4
113	C04
114	FG4
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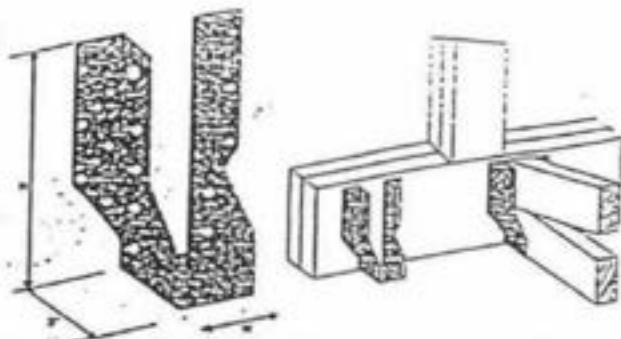
TRUSS/JOIST HANGERS, TUH - TYPE

- Featuring Load Share Fastening System
- Develops full Truss/Joist capacity
- 16 gauge Galvanized Steel
- Engineered and Tested

Load Share Fastening System



Diamond shaped opening allows for angular "Load Share" fastening. Four angled bolts distribute double the bearing load to the connector. Conventional connectors transfer the bearing load to a single point.

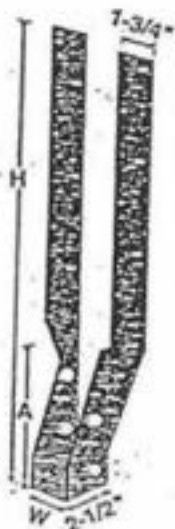
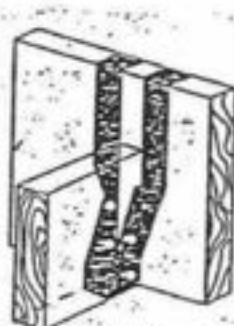


Load Share Nailin

Product Code	Joist Size	Dimensions		Qty/ Box	Fasteners		Design Loads (lbs)			
		W	H		Girder Header	Truss/ Joist	Normal 100%	Snow 115%	Roof 125%	Uplift
TUH 26	2X6	1-9/16"	5-5/16"	50	(16) 16d	(8) 16d	2500	3500	2500	2000

JOIST/TRUSS HANGERS, ADJUSTABLE

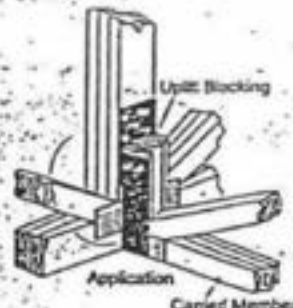
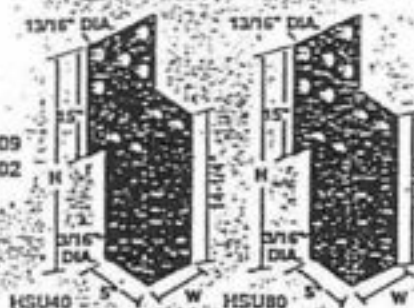
- Extra long straps allow maximum number of nails for maximum strength
- Can be field formed for top flange hanger convenience
- A few sizes satisfies all your hanger requirements
- No bolts required
- A full 2-1/2" seat bearing depth
- Galvanized steel
- SBCCI PST & ESI No. 94139
- Dade County Approval No. 95-1023.02



Product Code	Gauge	Qty/ Box	Wgt/ Box	Dimensions			Fasteners		Design Loads (lbs)	
				W	H	A	Header	Joist	Normal	Uplift
JHA 413	18 ga	50	40	3-5/8"	12-1/4"	4-3/4"	16-16d	6-10d	2440	820
JHA 418	16 ga	30	41	3-5/8"	16-1/4"	4-3/4"	24-16d	6-10d	3250	820

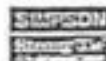
GIRDER HANGERS (HEAVY)

- Material: 7 gauge (3/16") H. R. Steel
- 3/4" diameter bolts required with 3" of wood penetration and minimum vertical width 5-1/2"
- Used where heavy design loads and uplift are required
- SBCCI PST & ESI No. 94139
- Dade County Approval No. 94-0706.09
- Dade County Approval No. 94-0328.02



Product Code	Description	W	H	Fasteners			Design Loads (lbs)		
				Bolts		Nails	Normal	Max	Uplift
				Girder	Block	Carried Member			
HSU 40	Girder Hgr(Heavy)	3-1/4"	23"	(5)-3/4"	(2)-3/4"	4-10d	6250	6250	9110
HSU 80	Girder Hgr(Heavy)	4-7/8"	23-1/2"	(6)-3/4"	(2)-3/4"	4-10d	9280	10670	10000
HSU 8016	Girder Hgr(Heavy)	4-7/8"	16"	(6)-3/4"	(2)-3/4"	4-10d	9280	10670	10000

LUS/HUS/HHUS/HGUS DOUBLE SHEAR JOIST HANGERS

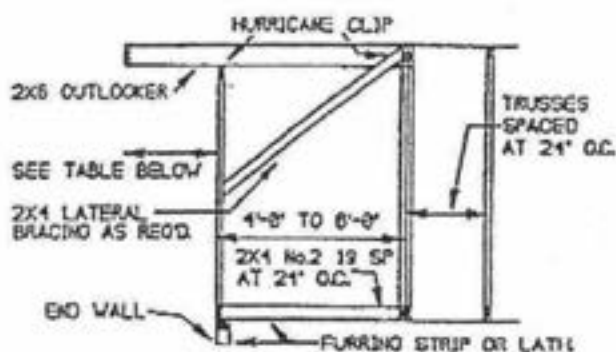
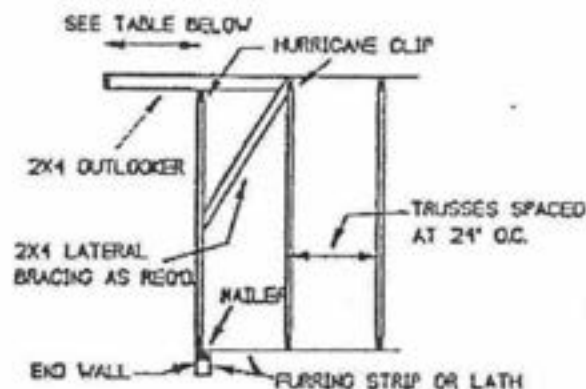
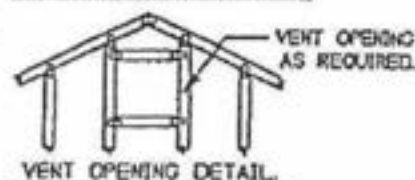
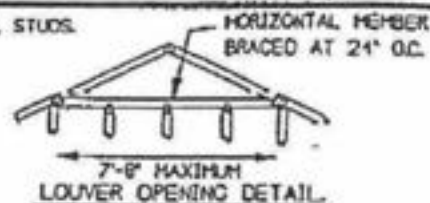
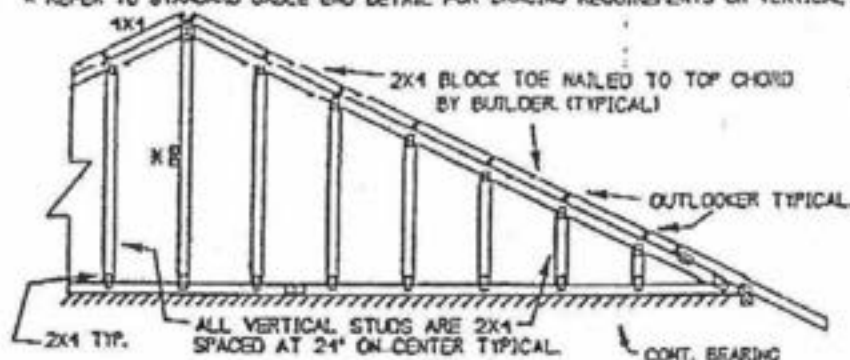


Model No.	Dimensions			Fasteners		Avg. Us.	Doug Fir-Larch/Southern Pine Allowable Loads						Spruce-Pine-Fir Allowable Loads					
	W	H	B	Carrying Member	Carried Member		Uplift* (13)	Uplift* (14)	Flwr (14)	Snow (11)	Coast (12)	Wind (13)	Uplift* (13)	Uplift* (14)	Flwr (14)	Snow (11)	Coast (12)	Wind (13)
HUS26-2	3 1/4"	5 1/2"	3	14-16d	8-16d	14667	1550	1550	2580	2965	3225	3440	1385	1550	2165	2490	2710	2890
HUS26-2	3 1/4"	6 5/8"	4	20-16d	8-16d	14667	2000	2000	3735	4320	4625	4835	1865	2000	3185	3660	3990	4245
HUS28-2	3 1/4"	6 7/8"	3	22-16d	8-16d	19650	2000	2000	3855	4465	4855	5180	1860	2000	3275	3765	4095	4370
HUS28-2	3 1/4"	7 1/4"	4	36-16d	10-16d	25180	2650	2650	6245	7185	7810	8000	2350	2650	5325	6070	6150	6220

STANDARD DROPPED GABLE END DETAIL.

PREPARED BY IGARD OFFICE

* REFER TO STANDARD GABLE END DETAIL FOR BRACING REQUIREMENTS ON VERTICAL STUDS.



MAXIMUM OUTLOOKERS EXTENSION.

LUMBER	WITH PLASTER		WITHOUT PLASTER	
	2x4	2x6	2x4	2x6
No.2ND 19 SP	2-8-3	3-10-11	2-10-13	1-2-9
No.2 19 SP	2-9-15	4-8-11	3-8-12	4-4-11
No.2D 19 SP	3-8-2	4-4-7	3-3-2	4-8-12
No.1ND 19 SP	3-8-2	4-5-6	3-3-2	4-9-11
No.1 19 SP	3-1-11	4-7-15	3-1-13	5-8-8
No.1D 19 SP	3-3-3	4-9-18	3-6-7	5-2-5
ND SS 19 SP	3-7-3	5-6-12	3-7-3	5-7-14
SS 19 SP	3-8-1	5-6-3	3-8-1	5-9-3
DSS 18 SP	3-8-13	5-10-7	3-8-13	5-10-7

NOTE 1

1. CONNECTION BETWEEN BOTTOM CHORD OF GABLE END TRUSS AND WALL TO BE PROVIDED BY PROJECT ENGINEER OR ARCHITECT.
2. FURNISH COPY OF THIS DRAWING TO CONTRACTOR FOR INSTALLATION OF OUTLOOKERS.
3. REFER TO STANDARD GABLE END DETAIL FOR BRACING REQUIREMENTS OF VERTICAL STUDS.

MINIMUM GRADE OF LUMBER

T.C. 2x4 No.2ND 19 SP
B.C. 2x4 No.2ND 19 SP
Webs 2x4 No.3 19 SP

TPJ-92 Cr11.
SSBC-91

LOADING (PSF)
L 0
TOP 30 15
BOTTOM 0 10
SPACING: 24 Inch O.C.

STR. INCR. 33 %
DRAWN BY: JB
CHECKED BY: JAI
REP. STRESS: YES



MiTek Industries Inc.

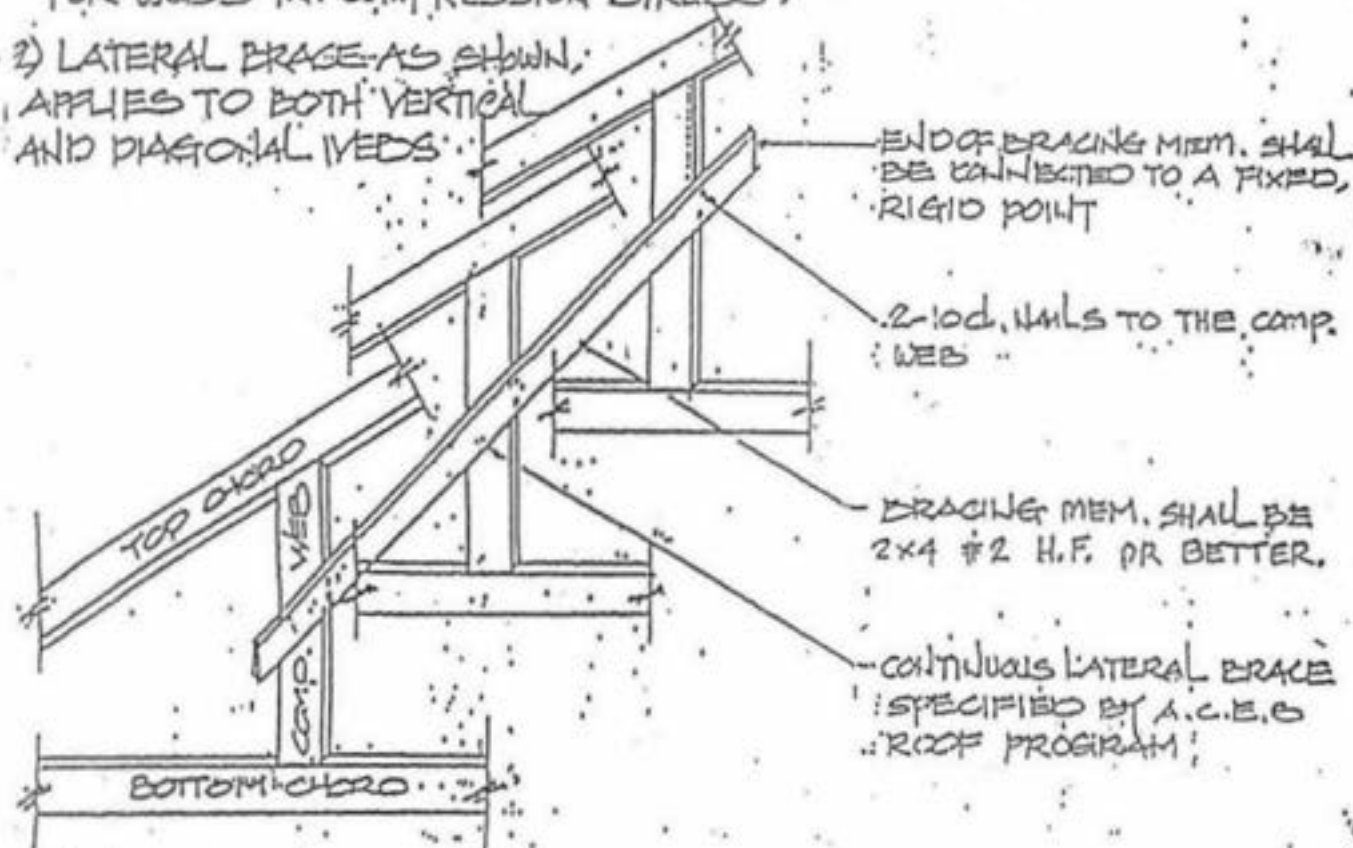


Approved: Mehmet Y. Tilter, Registered Engineer Inc.

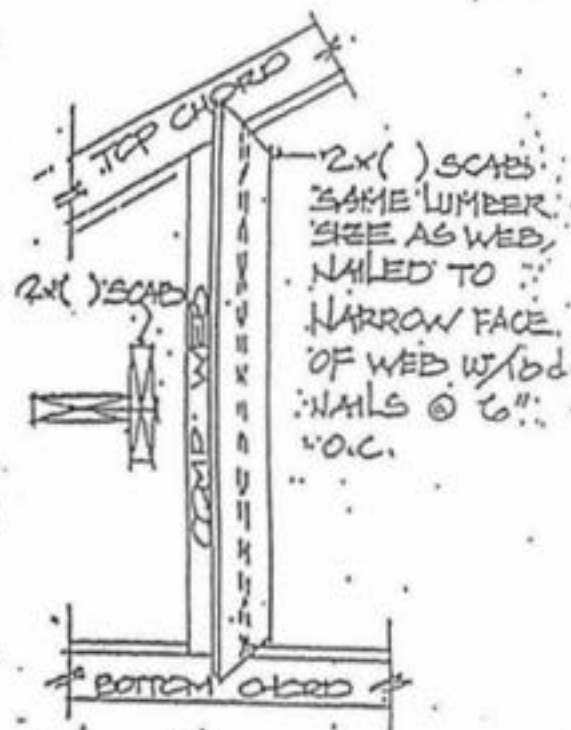
NOTE :

1) THIS DETAIL IS VALID ONLY FOR COMPUTER OUTPUT OF A.C.E.G. ROOF PROGRAM & SERVES AS AN INTERPRETATION OF CONTINUOUS LATERAL BRACING FOR WEBS IN COMPRESSION STRESS.

2) LATERAL BRACE AS SHOWN, APPLIES TO BOTH VERTICAL AND DIAGONAL WEBS.



MIN. LENGTH OF SCAB = $0.8 \times L$ (LENGTH OF WEB)



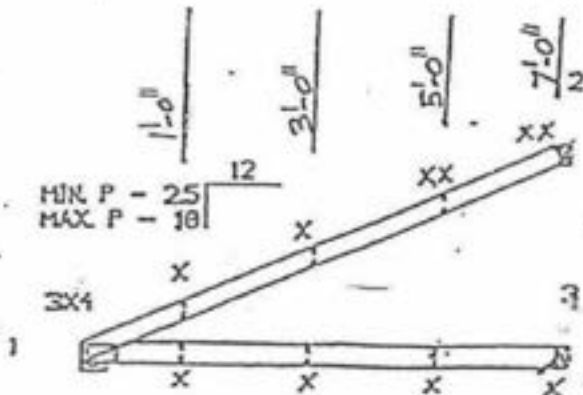
1/2 POINTS OR 1/2 POINTS BRACING

OPTIONAL T-BRACE IN CASE THAT CONTINUOUS LATERAL BRACING CAN NOT BE ACHIEVED

(1) (PSF) LIVE 1 — DEAD 1 — DEAD 1 —	SCALE SPACING — 1' TPI. OF DSGN CRIT. ALLOWABLE STRESS	BEHAX OF FLORIDA STANDARD LATERAL BRACE DETAIL FOR WEB	APPROVED FOR BEHAX FLA. STATE OF NEG. ENG. 13658	DESIGNED BY MY CHECKED BY DATE 7/22/83 DRG. # 13658

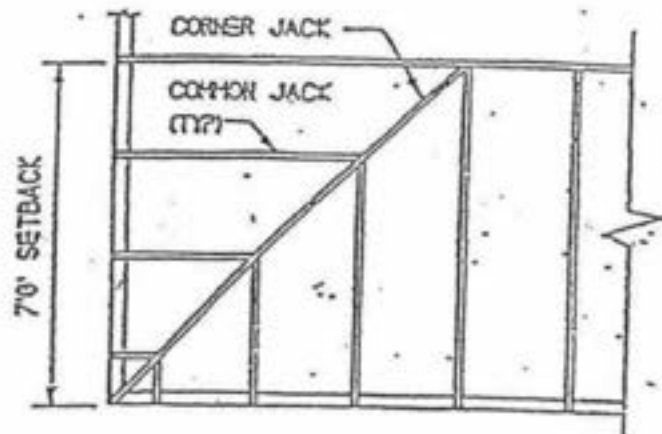
7'0" CORNER SET

OC. 20' x 10' 1/2"

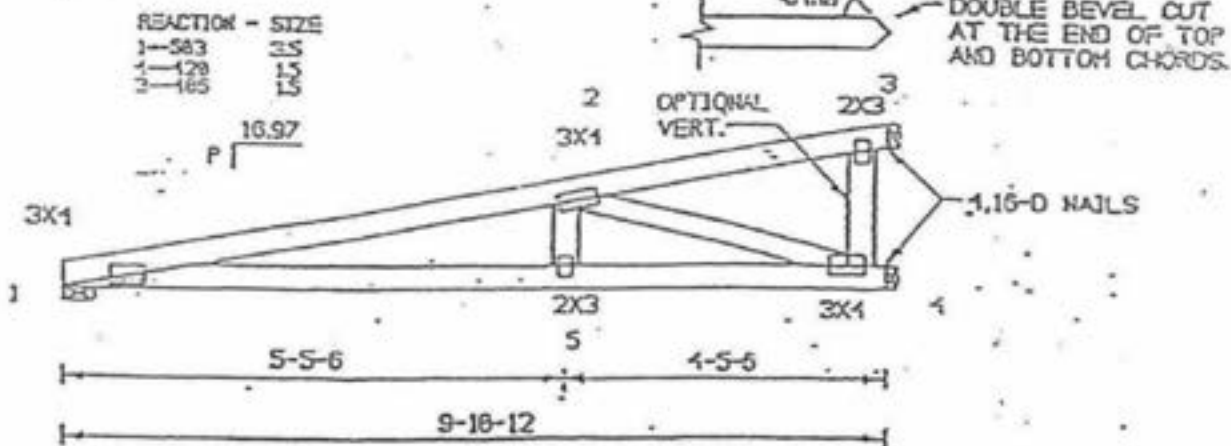


(X) 2-16d
(XX) 3-16d

COMMON JACKS



PARTIAL ROOF LAYOUT



CORNER JACK

NOTE: THIS DESIGN HAS BEEN CHECKED WITH 200 PSF WIND LOAD, WALL HEIGHT 10 FT MAX.
PROVIDE FOR UPLIFT AT SEARING WALL CONNECTIONS: 150 LBS AT COMMON JACKS & 250 LBS AT CORNER JACKS.

MINIMUM GRADE OF LUMBER

T.C. 2X4 10.2 19 SP
B.C. 2X4 10.2 19 SP
Joists 2X4 10.3 19 SP

TPI-92 C-11
SSBC-91



MITek Industries Inc.



Approved for use by MITek Industries Inc.

LOADING (PSF)
OP 32 15
OTOM 0 10
PACING 24' OC

STR. INCR: 33 X
DRAWN BY: TAI
CHECKED BY: JAI
REP. STRESS: NO

BACK DETAIL

12 191 Southern Pine

2x4 13 191 Southern Pine

Space vertical @ 5'0" o.c. max.

CONTINUOUS LATERAL BRACING ATTACHED TO TOP OR BOTTOM OF TOP CHORD OF TRUSS SUPPORTING PIGGYBACK AS SHOWN ON TRUSS ENGINEERING.
x4-12 HF, @ 24" & C w/ 2-16 ds

OVERALL SUPPORTING SPAN UP TO:

TYPE	42'0"	52'0"	TYPE	42'0"	52'0"	TYPE	42'0"	52'0"
A	3x4	3x5	A	24	26	A	3-4,5	3-5
B	3x6	3x7	B	46	48	B	5-6,3	5-7
C	2x4	2x4	C	24	24	C	2-4,5	2-4
D	5x6	5x6	D	46	47	D	5-6,3	5-7

Web Bracing Chart

Web Length	Bracing Required
To 7'9"	No Bracing
To 9'9"	1"x4" T-Bracing. Same grade and length as web; attach with 8d nails @ 8" p.o.
To 11'3"	2"x4" T-Bracing. Same grade and length as web; attach with 16d nails @ 8" o.c.

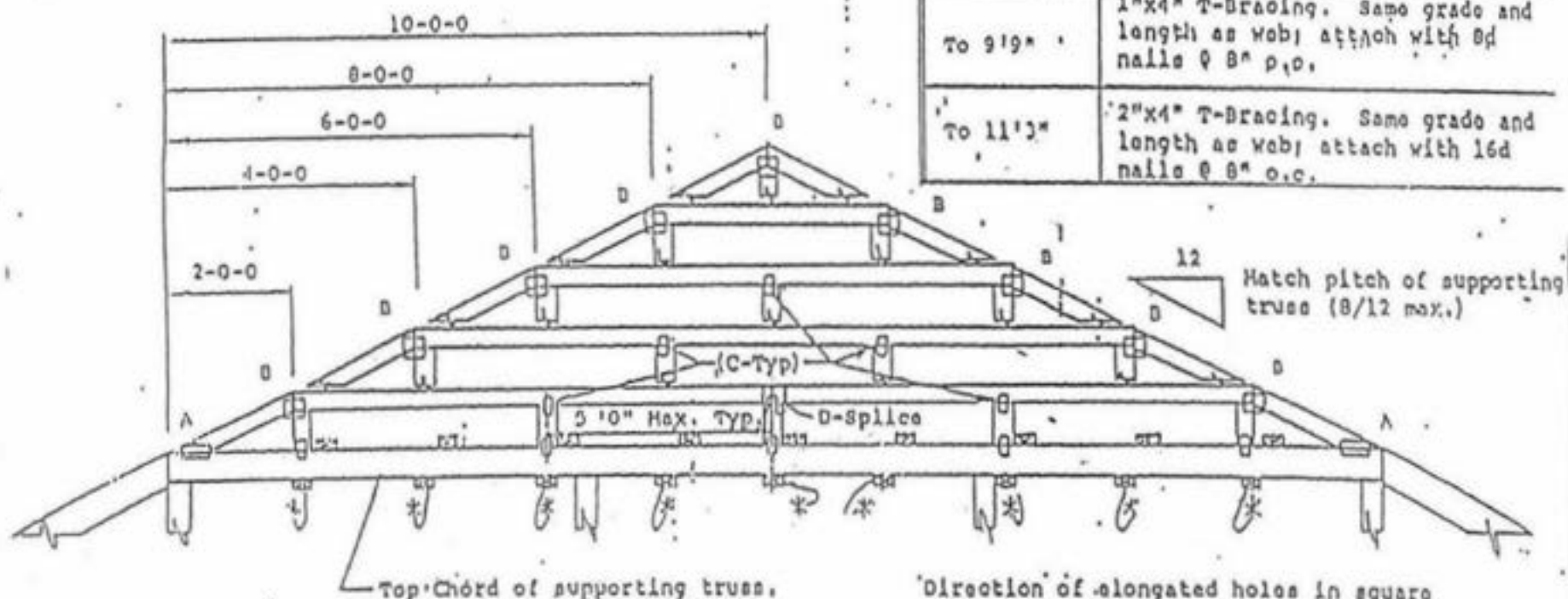


PLATE TYPE

Handwritten signature and date: 6-17-91

FURNISH A COPY OF THIS DESIGN TO ERECTION CONSTRUCTION

IMPORTANT! ALTHOUGH THESE PRODUCTS ARE NOT BE MANUFACTURED FOR ANY SPECIFIC APPLICATION, THEY ARE NOT TO BE USED IN ANY APPLICATION WHERE THE MANUFACTURER'S INSTRUCTIONS ARE NOT FOLLOWED. THE USER SHALL BE RESPONSIBLE FOR THE PROPER USE OF THESE PRODUCTS.

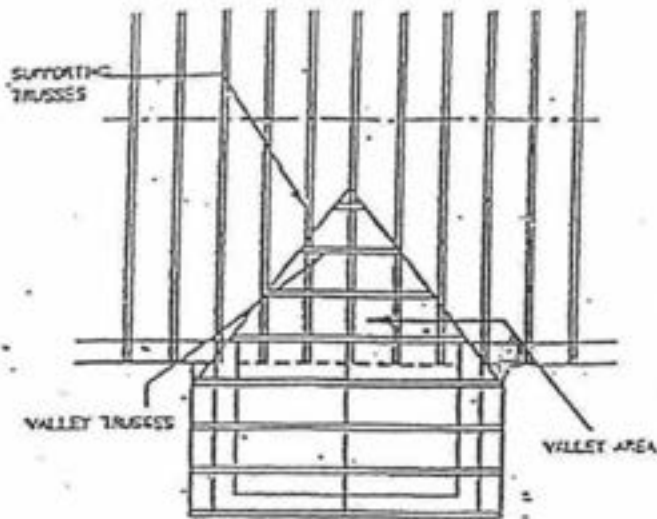
WARNING! THESE PRODUCTS ARE NOT TO BE USED IN ANY APPLICATION WHERE THE MANUFACTURER'S INSTRUCTIONS ARE NOT FOLLOWED. THE USER SHALL BE RESPONSIBLE FOR THE PROPER USE OF THESE PRODUCTS.

Handwritten signature: J. J. J.

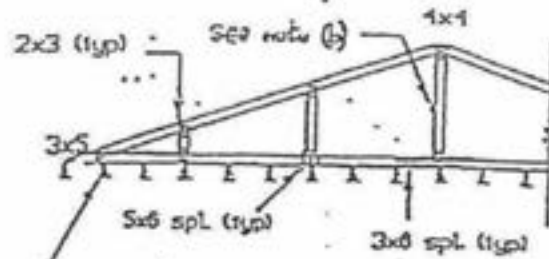
DESIGNER	DATE	REV.
TO LL	30	1st
TO OL	15	1st
PER	0439 EOK	
DATE	2/23/82	
QTY	660,182	

STANDARD ROOF VALLEY DETAIL

PREPARED BY PEARS OFFICE



PARTIAL ROOF LAYOUT

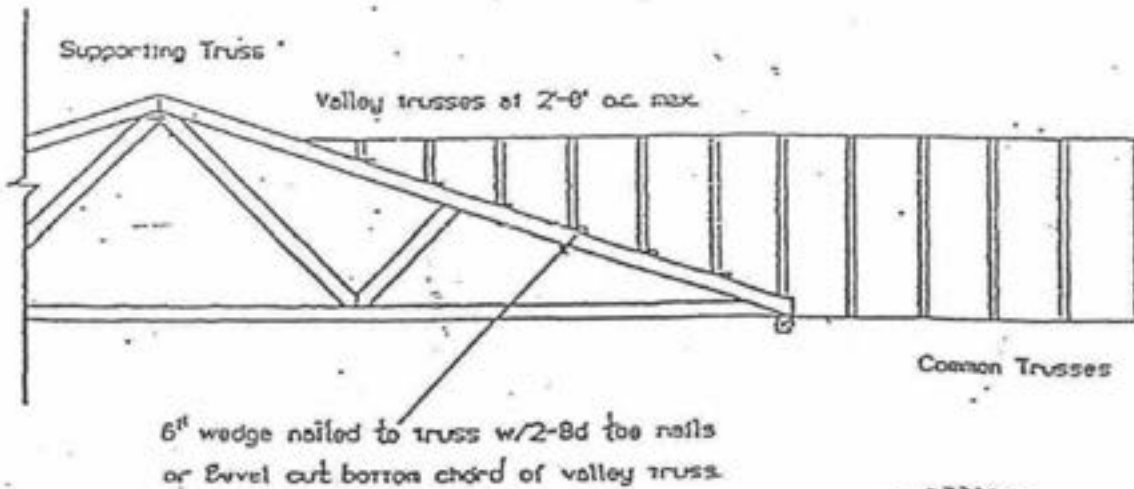


Supporting trusses at 2'-0" o.c. max

NOTES

- (a) Provide continuous bracing on verticals over 6'-3". Connect bracing to verticals w/2-8d nails and bracing must be tied to a fixed point at each end.
- (b) On spans over 24'-0", add verticals at 6'-0" o.c. max.

(c) STAYS @ 48" o/c. MAX.



MINIMUM GRADE OF LUMBER

2X4 No. 2/D 19 SP
2X4 No. 2/D 19 SP
2X4 No. 3 19 S

TP1-92 Cr11.
SS8C-91



Mitek Industries Inc.

WIND (PSF)
L D
30 15
OH 8 10
WIND 24 Inch O.C.

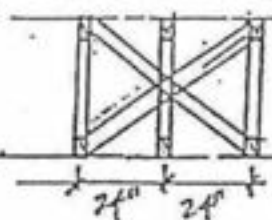
STR. INCR. : 33 %
DRAWN BY : P.G.
CHECKED BY : J.A.J.
REP. STRESS: YES



Approved for Mitek Industries Inc.

PREPARED BY NAME OFFICE

FOR CANTILEVERS UP TO 6'-0" LENGTH PROVIDE
BOTTOM CHORD BRACING AT 24" O/C MAX.
FOR 6'-0" AND GREATER CANTILEVERS, ADDITIONALLY
PROVIDE X-BRACING STARTING ABOVE THE TIE BEAM,
SPACED AT 6'-0" O/C MAX.

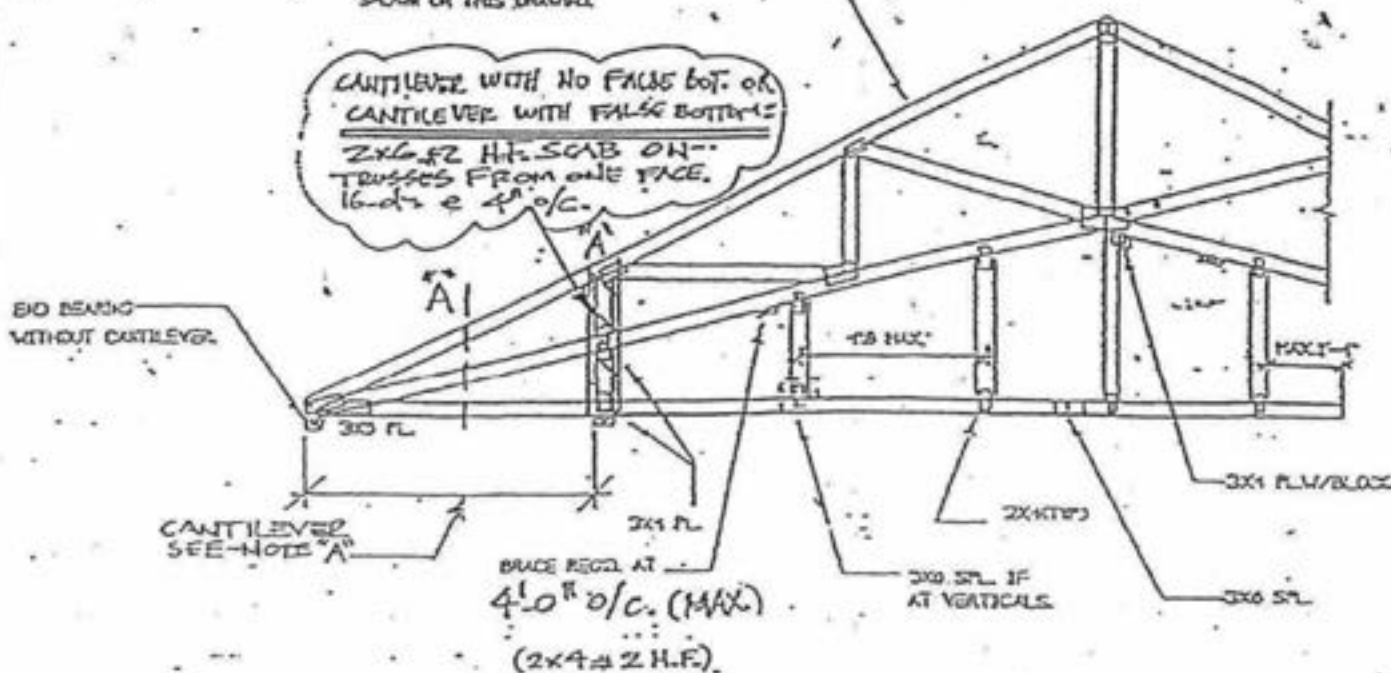


(2-16.d's MW.TYP.)

(X - DRAWING
OVER GANT. T. DRAW
AT 6'-0" O/C MAX.)

CONSULT TRUSS ENGINEERING FOR
SHAPE, LUMBER, PLATE SIZES AND
ALL OTHER INFORMATION NOT
SHOWN ON THIS DRAWING.

CANTILEVER WITH NO FALSE BOT. OR
CANTILEVER WITH FALSE BOTTOMS
2x6 @ 2 Ht. SCAB ON--
TRUSSES FROM ONE FACE.
16-d @ 24" o/c.



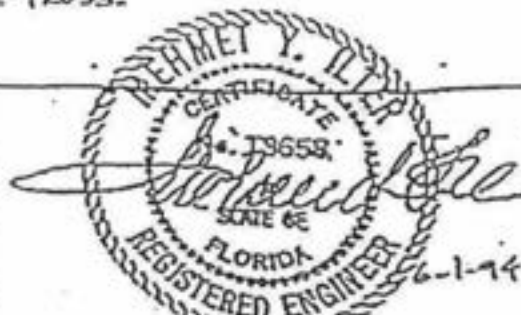
NOTE: TOP AND BOTTOM CHORD FILLERS ARE NOT A STRUCTURAL COMPONENT OF A TRUSS. PORTIONS OF, OR THE ENTIRE FILLER MAY BE REMOVED, OR ADDED TO, WITHOUT AFFECTING THE STRUCTURAL INTEGRITY OF THE TRUSS, PROVIDED THEY ARE NOT AFFECTING BEARING LOCATIONS OF THE TRUSS.

MINIMUM GRADE OF LUMBER					
T.C.	2X4	NO.2ND	19	SP	TP1-92 Cr11. SSAC-91
B.C.	2X4	NO.2ND	19	SP	
Webs	2X4	NO.3	19	SP	

LOADING	(PSF)	STR. INCR. : 33 %
TOP	0	DRAWN BY : B.V.
BOTTOM	30	CHECKED BY : JAI
SPACING	8	REP. STRESS YES
24 inch O.C.		



HiTek Industries Inc.



Approved for Mulek Industries Inc.

WARNING: This safety alert symbol is used to attract your attention. PERSONAL SAFETY IS INVOLVED! When you see this symbol - BECOME ALERT - HEED ITS MESSAGE.

CAUTION: A CAUTION identifies safe operating practices or indicates unsafe conditions that could result in personal injury or damage to structures.

HIB-91 Summary Sheet COMMENTARY and RECOMMENDATIONS for HANDLING, INSTALLING & BRACING METAL PLATE CONNECTED WOOD TRUSSES*

It is the responsibility of the builder, architect, building contractor, building engineer, owner or architect to ensure that the trusses are installed, handled, stored and braced in accordance with the design and specifications. The trusses must remain in the same high degree of safety throughout as with any other structural material. TPI does not intend these recommendations to be interpreted as a substitute for the professional judgment of an Engineer or Architect. The handling, installing and bracing of wood trusses is a particularized art. These recommendations are based upon the collective experience of leading truss fabricators.

CAUTION: The builder, building contractor, licensed contractor, erector or erection contractor is advised to obtain and read the entire booklet "Commentary and Recommendations for Handling, Installing & Bracing Metal Plate Connected Wood Trusses, HIB-91" from the Truss Plate Institute.

DANGER: A DANGER designates a condition where failure to follow instructions or heed warnings will most likely result in serious personal injury or death or damage to structures.

WARNING: A WARNING describes a condition where failure to follow instructions could result in severe personal injury or damage to structures.



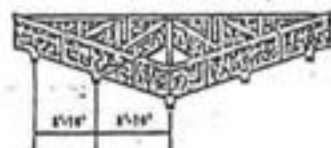
personnel in the wood truss industry, but must, due to the nature of the work, be provided, be presented as a guide to the use of a qualified building designer or architect. Thus, the Truss Plate Institute, Inc. accepts no responsibility for damage or injury from the use, application or reliance on the recommendations and information contained herein by building designers, architects, and others. Copyright © by Truss Plate Institute, Inc. All rights reserved. This document or any part thereof must not be reproduced in any form without written permission of the publisher, printed in the United States of America.

CAUTION: All temporary bracing should be no less than 2x4 grade marked lumber. All connections should be made with minimum of 2-16d nails. All trusses assumed 2" on-center or less. All multi-ply trusses should be connected together in accordance with design drawings prior to installation.

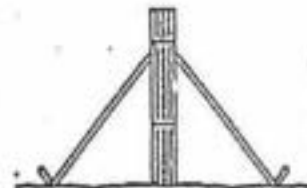
TRUSS STORAGE



CAUTION: Trusses should not be unloaded on rough terrain or uneven surfaces which could cause damage to the truss.



Trusses stored horizontally should be supported on blocking to prevent excessive lateral bending and lessen moisture gain.



Trusses stored vertically should be braced to prevent toppling or tipping.

WARNING: Do not break banding until installation begins or lift bundled trusses by the bands.

WARNING: Do not use damaged trusses.

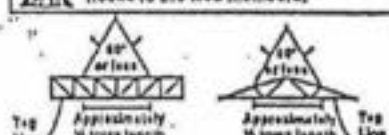
DANGER: Do not store bundles upright unless properly braced.

DANGER: Walking on trusses which are lying flat is extremely dangerous and should be strictly prohibited.

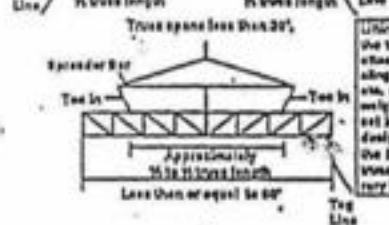
Frame 1

WARNING: Do not attach cables, chains, or hooks to the web members.

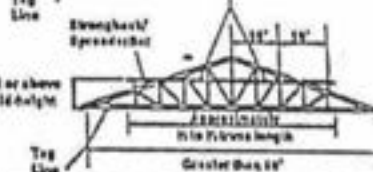
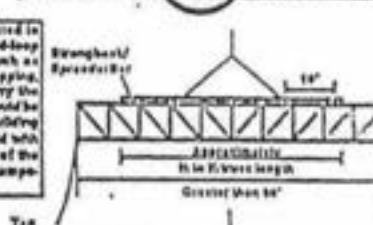
WARNING: Do not lift single trusses with spans greater than 36' by the peak.



"MECHANICAL" INSTALLATION



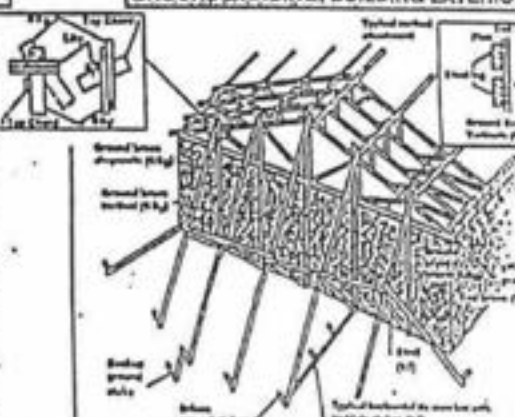
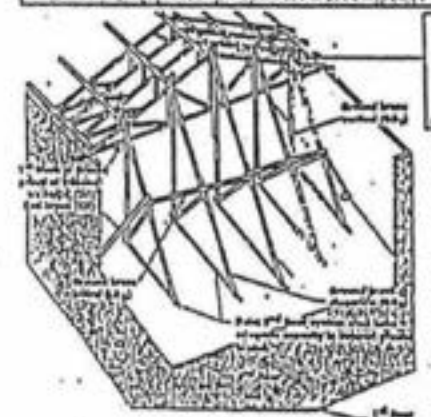
Lifting devices should be connected to the truss top chord with a spreader bar attachment utilizing materials such as slings, chains, cables, nylon slings, etc. of sufficient strength to carry the weight of the truss. Each truss should be set in proper position per the building designer's bracing plan and held with the string device until the ends of the truss are securely fastened and temporary bracing is installed.



CAUTION: Temporary bracing shown in this summary sheet is adequate for the installation of trusses with similar configurations. Consult a registered professional engineer if a different bracing arrangement is desired. The engineer may design bracing in accordance with TPI's Recommended Design Specification for Temporary Bracing of Metal Plate Connected Wood Trusses, DSD-89, and in some cases determine that a wider spacing is possible.

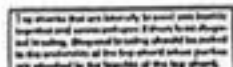
GROUND BRACING BUILDING INTERIOR

GROUND BRACING BUILDING EXTERIOR



CAUTION: Ground bracing required for all installations.

Frame 2

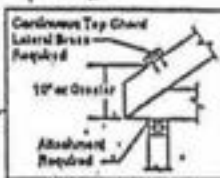


SPAN FEET	MINIMUM PITCH	TOP CHORD LATERAL BRACE SPACING (S)	TOP CHORD DIAGONAL BRACE SPACING (D)	
			SEMI	SPACING
Over 12'	4/12	3'	22	12
Over 20' + 4"	4/12	4"	10	7
Over 40' + 8"	4/12	3"	8	4

See a registered professional engineer

DP = Douglas fir-Larch SP = Southern Pine
HP = Hem-fir LP = Loblolly Pine

— All lateral braces lapped at least 2 times.



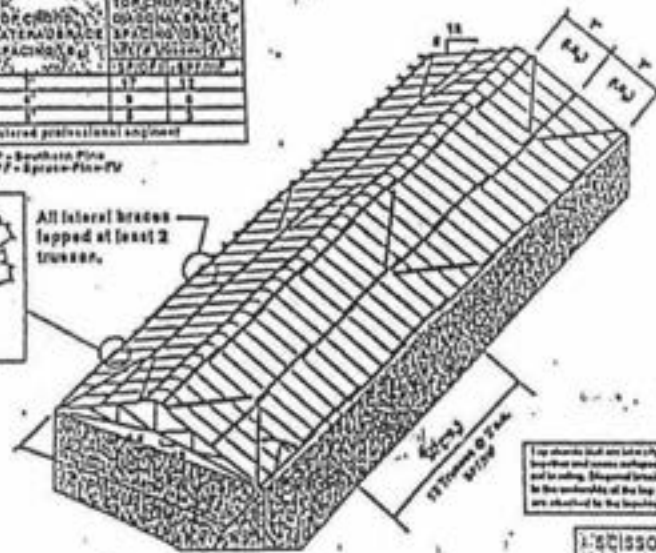
ИПЧЕОТРУСС

WARNING: Failure to follow these recommendations could result in severe personal injury or damage to trusses or buildings.

MINIMUM PITCH DIFFERENCE	TOP CHORD CATEGORY SPACING	TOP CHORD CATEGORY SPACING	TOP CHORD CATEGORY SPACING
Over 12"	3"	3"	3"
Over 10" - 12"	3"	3"	3"
Over 8" - 10"	3"	3"	3"
Over 6"	See a registered professional engineer		

GP = Douglas Fir-Larch
SP = Southern Pine
HP = Hem-Fir
LP = Loblolly-Pine-Fir

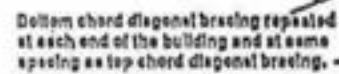
All lateral brace
lapped at least 1
truss.



Frame 3

Top chords that are left in place need not be moved. In fact, they are important and cannot be removed. There is no way to avoid this. The only way to avoid this is to remove the top chord. The only way to avoid this is to remove the top chord. The only way to avoid this is to remove the top chord.

SCISSORS TRUSS



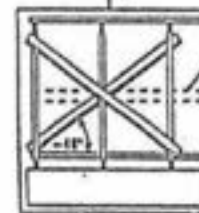
SPACING	MINIMUM FITTING	BOTTOM CHORD LATERAL SPACE SPACING (S _b)	BOTTOM CHORD DIAGONAL SPACING (S _d)
Weld 1/2"	4/13	18"	36"
Over 1/2" - 4/8"	4/13	18"	18"
Over 4/8" - 6/8"	4/13	18"	18"
Over 6/8"		See e) required greater lateral angles	

DP = Descriptive Pile-Less
 DP = Hum-fir
 DP = Southern Pine
 DP = Spruce-Pine

All lateral braces
lapped at least 2
braces.

BOTTOM CHORD PLATE

WARNING: Failure to follow these recommendations could result in severe personal injury or damage to trusses or buildings.



- Permanent continuous lateral bracing as specified by the truss engineering.

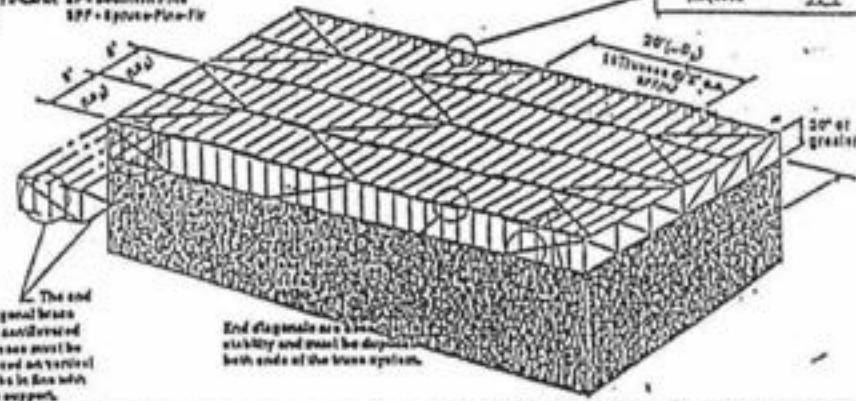
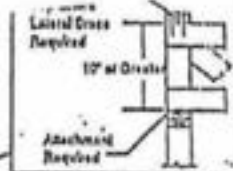
Cross bracing repeated at each end of the building and at 20' intervals.

Frame 4

SPACE	MINIMUM PITCH	TOP CHORD LATERAL BRACE SPACING (S _L)	TOP CHORD DIAGONAL BRACE SPACING (S _D) (# Trusses)
Up to 24"	3/12	8'	12'
Over 24" to 48"	5/12	7'	10'
Over 48" to 60"	5/12	6'	8'
Over 60"	See a registered professional engineer		

DP = Douglas Fir-Larch
SP = Southern Pine
SPP = Spruce-Pine-Fir

Top chords that are laterally braced and braced together and secure end joints if there is no diagonal bracing. Diagonal bracing should be added to the underside of the top chord where joints are attached to the trusses of the top chord.

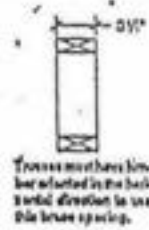
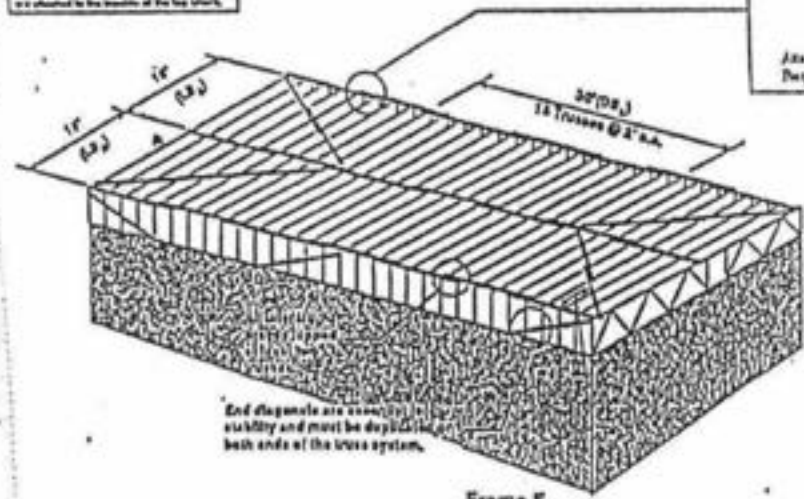
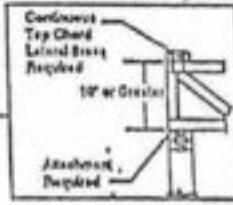


The end diagonal braces for end braced trusses must be placed on vertical webs in line with the supports.

WARNING: Failure to follow these recommendations could result in severe personal injury or damage to trusses or buildings.

4x2 PARALLEL CHORD TRUSS TOP CHORD

Top chords that are laterally braced and braced together and secure end joints if there is no diagonal bracing. Diagonal bracing should be added to the underside of the top chord where joints are attached to the trusses of the top chord.



Frame 5

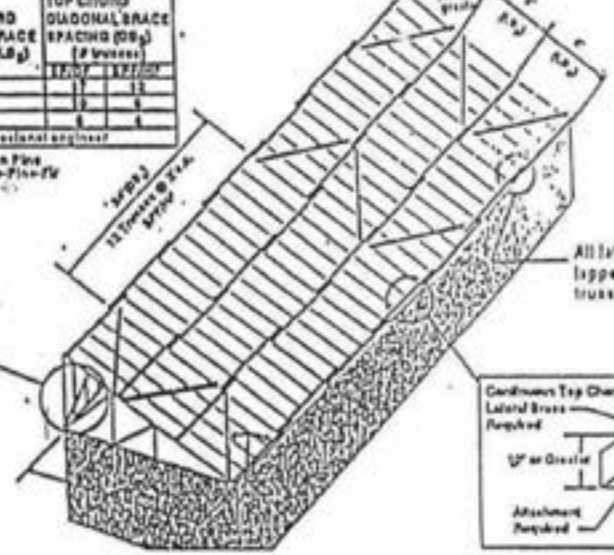
SPACE	MINIMUM PITCH	TOP CHORD LATERAL BRACE SPACING (S _L)	TOP CHORD DIAGONAL BRACE SPACING (S _D) (# Trusses)
Up to 24"	3/12	8'	12'
Over 24" to 48"	5/12	7'	10'
Over 48" to 60"	5/12	6'	8'
Over 60"	See a registered professional engineer		

DP = Douglas Fir-Larch
SP = Southern Pine
SPP = Spruce-Pine-Fir

Top chords that are laterally braced and braced together and secure end joints if there is no diagonal bracing. Diagonal bracing should be added to the underside of the top chord where joints are attached to the trusses of the top chord.

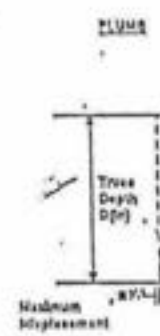
SPRUE TRUSS

Diagonal brace also required on end verticals.

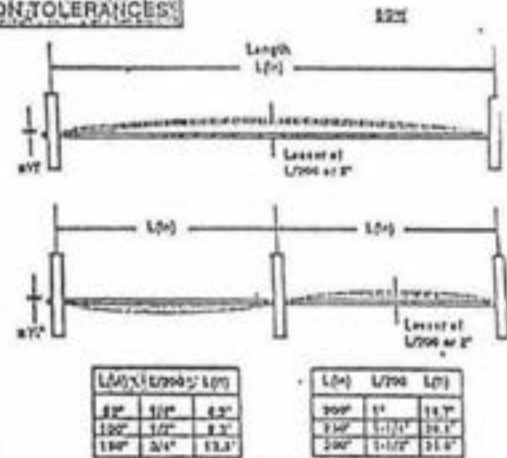


WARNING: Failure to follow these recommendations could result in severe personal injury or damage to trusses or buildings.

INSTALLATION TOLERANCES



Depth	D/100"	D/10"
12"	1/8"	1"
24"	1/4"	2"
36"	3/8"	3"
48"	1/2"	4"
60"	3/4"	5"
72"	1"	6"
84"	1 1/8"	7"
96"	1 1/4"	8"
108"	1 1/2"	9"



OUT-OF-PLUMB INSTALLATION TOLERANCES

WARNING: Do not cut trusses.

OUT-OF-PLANE INSTALLATION TOLERANCES

DANGER: Under no circumstances should construction loads of any description be placed on unbraced trusses.

Frame 6

====<<<ACES-32 Ver. 1.1>>>===== [001807] =====<<<FL.QUALITY-MMH>>>=====

Customer : FOGLIA Thu Mar 1 08:52:09 2001

Project #: 00469 Truss ID : CJ7C Family #: 204

Span : 9-10-13 B.Span : 7-0 Quant. : 2 Top Pitch : 4.243

==Bld(1/21/2000),v1.1=====

FORCES - LOAD CASE #1

1-2=-777 4-1= 725 2-4=-772

2-3= 0

3-4=-406

REACTIONS - SIZE

1=-693 8.00

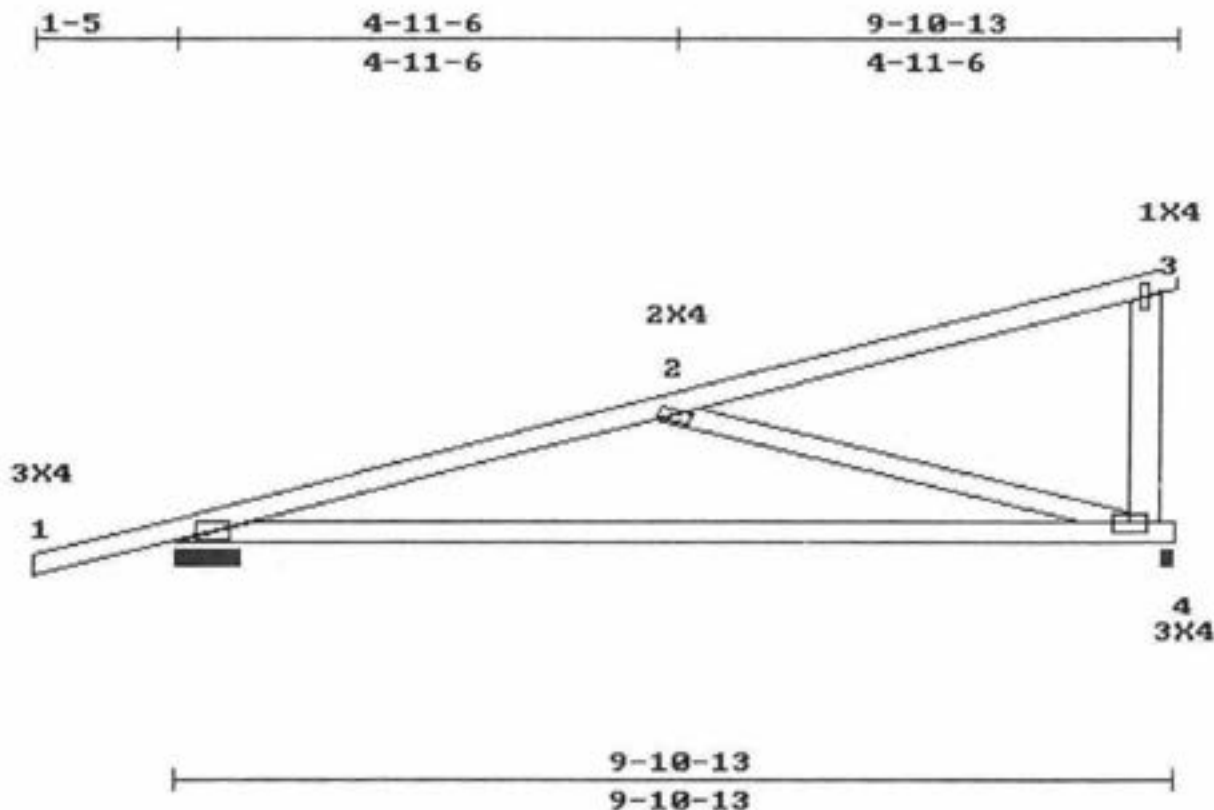
4=-791 1.50

(1)

PROVIDE FOR 996 LBS UPLIFT AT JOINT 1 (1.44)

PROVIDE FOR 1229 LBS UPLIFT AT JOINT 4 (1.55)

PROVIDE FOR 409 LBS HORIZ. REACTION AT JOINT 1



L. HL TO PK:10-6

LEFT HEIGHT:0-3-15 SPAN:9-10-13 RISE:3-9-15 RIGHT HEIGHT:3-9-15

LOADING (PSF)		MAX STRESSES		MINIMUM GRADE OF LUMBER	
L	D	TOP	BOTT	TOP CHORD	BOT CHORD
30	15	2-3=0.767	4-1=0.769	2X4 No.2ND 19 SP	2X4 No.1D 19 SP
0	10	LL.DEFL.@0=0.00 < L/360		WEBS	2X4 No.3 19 SP

REPETITIVE STRESSES NOT USED

SPACING : 24.0 in. o. c.

NO. OF MEMBERS = 1

LOADING	STRESS INCREASE	LOADING	PANEL(PLF) / JOINTS(LBS)
LUMBER	PLATE	TYPE	
1	1.33	1.33	UNIFORM 1- 2= 59 2- 3= 166 4- 1= 25

RIGHT END VERTICAL IS NOT EXPOSED.

TRUSS HAS BEEN CHECKED FOR 145 M.P.H. WIND LOAD, WALL HEIGHT 16 FT, ENCLOSED BLDG. TYPE, ASSUMING 10 PSF TOTAL D.L.(7+3) (SBC-97).

DEFLECTION(IN.) L.L= 0.00,D.L=0.00,T.L=0.00

PLATES ARE MITEK M20-249,200 MANUFACTURED FROM ASTM A 446 GRD A GALVANIZED STEEL(EXCEPT AS SHOWN)

PLATE MUST BE INSTALLED ON EA. FACE OF JOINT,SYMMETRICALLY(EXCEPT AS SHOWN)DESIGN CONFORMS W/NDS DESIGN SPECS, SSB,ANSI/TPI-95

THIS DESIGN IS FOR TRUSS FABRICATION ONLY.FOR PERMANENT AND TEMPORARY BRACING(WHICH IS ALWAYS REQD)CONSULT BLDG ARCHITECT OR ENGINEER.

MAR 01 2001

====<<<ACES-32 Ver. 1.1>>>=====[001808]====<<<FL.QUALITY-MMH>>>=====
 Customer : FOGLIA Thu Mar 1 08:52:29 2001
 Project # : 00469 Truss ID : C1 Family # : 205
 Span : 1-0 B.Span : 1-0 Quant. : 18 Top Pitch : 6/12
 ==Bld(1/21/2000),v1.1=====

FORCES - GRAVITY LOADS

1-2= 0
 2-3= 0

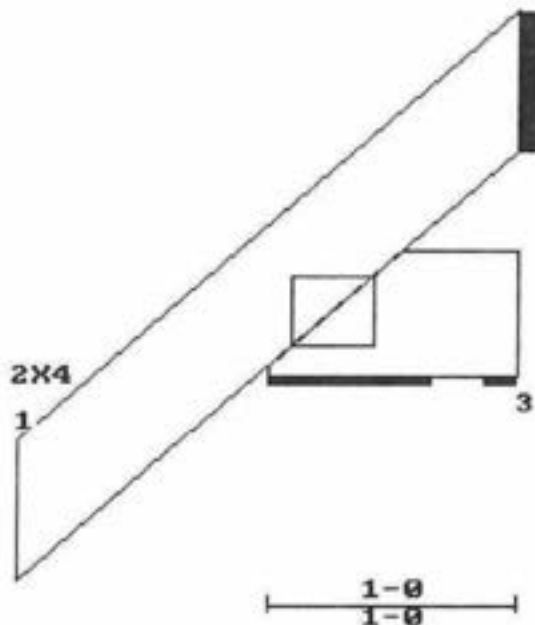
REACTIONS - SIZE

1=143 8.00
 3=6 1.50
 2=27 1.50

②

PROVIDE FOR 134 LBS UPLIFT AT JOINT 1 (0.94)
 PROVIDE FOR 52 LBS UPLIFT AT JOINT 2 (1.93)
 PROVIDE FOR 54 LBS HORIZ. REACTION AT JOINT 1

1-0 1-0
 1-0



L. HL TO PK:1-1-7
 LEFT HEIGHT:0-4-3

SPAN:1-0

RISE:0-10-3

RIGHT HEIGHT:0-10-3

LOADING (PSF)

TOP 30 15
 BOTT 0 10

MAX STRESSES

TOP 1-2=0.014
 BOTT 3-1=0.002
 LL.DEFL. < L/360

MINIMUM GRADE OF LUMBER

TOP CHORD:2X4 No.2ND 19 SP
 BOT CHORD:2X4 No.2ND 19 SP
 WEBS :2X4 No.3 19 SP

STR.INC.: LUMB = 1.33 PLATE = 1.33
 REPETITIVE STRESSES USED

SPACING : 24.0 in. o. c.
 NO. OF MEMBERS = 1

TRUSS HAS BEEN CHECKED FOR 145 M.P.H. WIND LOAD, WALL HEIGHT 16 FT, ENCLOSED BLDG. TYPE, ASSUMING 10 PSF TOTAL D.L.(7+3) (SBC-97).
 DEFLECTION(IN.) L.L= 0.00,D.L=0.00,T.L=0.00

PLATES ARE MITEK M20-249,200 MANUFACTURED FROM ASTM A 446 GRD A GALVANIZED STEEL(EXCEPT AS SHOWN)

PLATE MUST BE INSTALLED ON EA. FACE OF JOINT,SYMMETRICALLY(EXCEPT AS SHOWN)DESIGN CONFORMS W/ND S DESIGN SPECS, SSBC,ANSI/TP1-95

THIS DESIGN IS FOR TRUSS FABRICATION ONLY.FOR PERMANENT AND TEMPORARY BRACING(WHICH IS ALWAYS REQD)CONSULT BLDG ARCHITECT OR ENGINEER.

MAR 01 2001

====<<<<ACES-32 Ver. 1.1>>>>=====[001809]=====<<<<FL.QUALITY-MMH>>>>=====

Customer : FOGLIA
 Project #: 00469
 Span : 3-0

Truss ID : C3
 Quantity : 17

Thu Mar 1 08:52:51 2001
 Family # : 205
 Top Pitch : 6/12

==Bld(1/21/2000),v1.1=====

FORCES - GRAVITY LOADS

1-2= 0
 2-3= 0

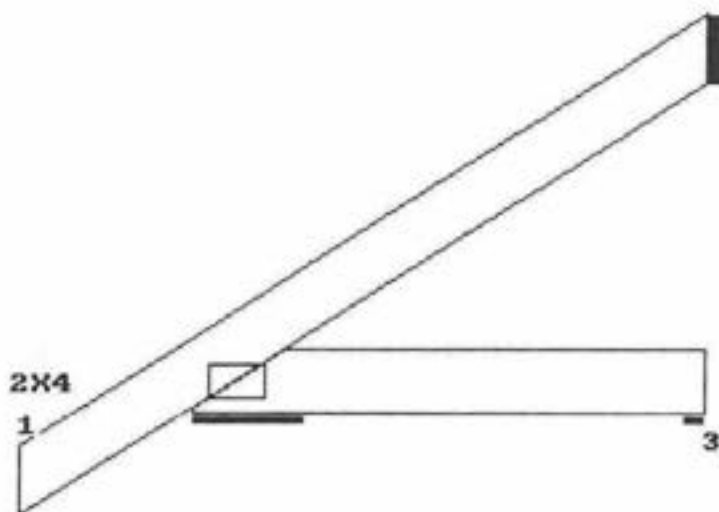
REACTIONS - SIZE

1=-253 8.00
 3=-26 1.50
 2=-117 1.50

②

PROVIDE FOR 205 LBS UPLIFT AT JOINT 1 (0.81)
 PROVIDE FOR 183 LBS UPLIFT AT JOINT 2 (1.56)
 PROVIDE FOR 172 LBS HORIZ. REACTION AT JOINT 1
 (SYMMETRIC PLATING TURNED ON)

1-0 3-0
 3-0



3-0
 3-0

L. HL TO PK:3-4-4
 LEFT HEIGHT:0-4-3

SPAN:3-0

RISE:1-10-3

RIGHT HEIGHT:1-10-3

LOADING (PSF)

TOP 30 15
 BOTT 0 10

MAX STRESSES

TOP 1-2=0.225
 BOTT 3-1=0.034
 LL.DEFL. < L/360

MINIMUM GRADE OF LUMBER

TOP CHORD:2X4 No.2ND 19 SP
 BOT CHORD:2X4 No.2ND 19 SP
 WEBS :2X4 No.3 19 SP

STR.INC.: LUMB = 1.33 PLATE = 1.33
 REPETITIVE STRESSES USED

SPACING : 24.0 in. o. c.
 NO. OF MEMBERS = 1

TRUSS HAS BEEN CHECKED FOR 145 M.P.H. WIND LOAD, WALL HEIGHT 16 FT, ENCLOSED BLDG. TYPE, ASSUMING 10 PSF TOTAL D.L.(7+3) (SBC-97).
 DEFLECTION(IN.) L.L.= 0.00,D.L.=0.00,T.L.=0.00

PLATES ARE MITEK M20-249,200 MANUFACTURED FROM ASTM A 446 GRD A GALVANIZED STEEL(EXCEPT AS SHOWN)

PLATE MUST BE INSTALLED ON EA. FACE OF JOINT,SYMMETRICALLY(EXCEPT AS SHOWN)DESIGN CONFORMS W/NDS DESIGN SPECS, SSB,ANSI/TPI-95

THIS DESIGN IS FOR TRUSS FABRICATION ONLY.FOR PERMANENT AND TEMPORARY BRACING(WHICH IS ALWAYS REQD)CONSULT BLDG ARCHITECT OR ENGINEER.

[Signature]

MAK 0 1 2001

====<<<<ACES-32 Ver. 1.1>>>>=====[001810]====<<<<FL.QUALITY-MMH>>>>=====
 Customer : FOGLIA Thu Mar 1 08:53:02 2001
 Project #: 00469 Truss ID : C5 Family #: 205
 Span : 5-0 Quantity : 11 Top Pitch : 6/12
 ==Bld(1/21/2000),v1.1=====

FORCES - GRAVITY LOADS

1-2= 0
 2-3= 0

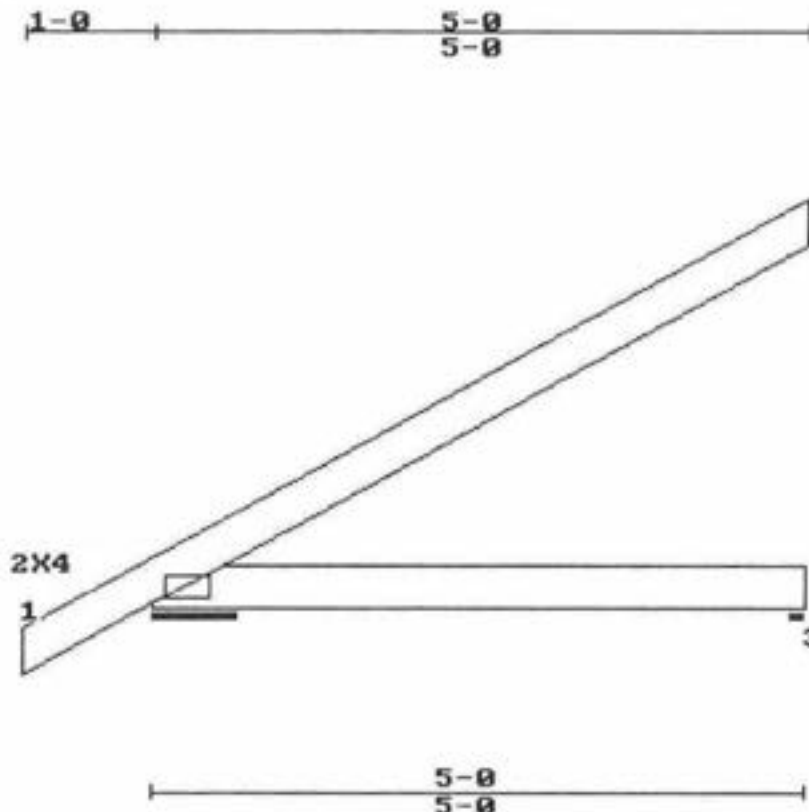
3-1= 0

REACTIONS - SIZE

1=-363 8.00
 3=-46 1.50
 2=-207 1.50

(4)

PROVIDE FOR 273 LBS UPLIFT AT JOINT 1 (0.75)
 PROVIDE FOR 316 LBS UPLIFT AT JOINT 2 (1.53)
 PROVIDE FOR 289 LBS HORIZ. REACTION AT JOINT 1
 (SYMMETRIC PLATING TURNED ON)



L. HL TO PK:5-7-1
 LEFT HEIGHT:0-4-3

SPAN:5-0

RISE:2-10-3

RIGHT HEIGHT:2-10-3

LOADING (PSF)

L D
 TOP 30 15
 BOTT 0 10

MAX STRESSES

TOP 1-2=0.702
 BOTT 3-1=0.103
 LL.DEFL. < L/360

MINIMUM GRADE OF LUMBER

TOP CHORD:2X4 No.2ND 19 SP
 BOT CHORD:2X4 No.2ND 19 SP
 WEBS :2X4 No.3 19 SP

STR.INC.: LUMB = 1.33 PLATE = 1.33
 REPETITIVE STRESSES USED

SPACING : 24.0 in. o. c.
 NO. OF MEMBERS = 1

TRUSS HAS BEEN CHECKED FOR 145 M.P.H. WIND LOAD, WALL HEIGHT 16 FT, ENCLOSED BLDG. TYPE, ASSUMING 10 PSF TOTAL D.L.(7+3) (SBC-97).
 DEFLECTION(IN.) L.L= 0.00,D.L=0.00,T.L=0.00

PLATES ARE MITEK M20-249,200 MANUFACTURED FROM ASTM A 446 GRD A GALVANIZED STEEL(EXCEPT AS SHOWN)

PLATE MUST BE INSTALLED ON EA. FACE OF JOINT,SYMMETRICALLY(EXCEPT AS SHOWN)DESIGN CONFORMS W/NDS DESIGN SPECS, SSB,ANSI/TP1-95

THIS DESIGN IS FOR TRUSS FABRICATION ONLY.FOR PERMANENT AND TEMPORARY BRACING(WHICH IS ALWAYS REQD)CONSULT BLDG ARCHITECT OR ENGINEER.

[Signature]
 MAR 01 2001

====<<<<ACES-32 Ver. 1.1>>>>=====[001811]====<<<<FL.QUALITY-MMH>>>>=====
 Customer : FOGLIA Truss ID : J7 Thu Mar 1 08:53:11 2001
 Project #: 00469 Quantity : 23 Family #: 205
 Span : 7-0 Top Pitch : 6/12
 ==Bld(1/21/2000),v1.1=====

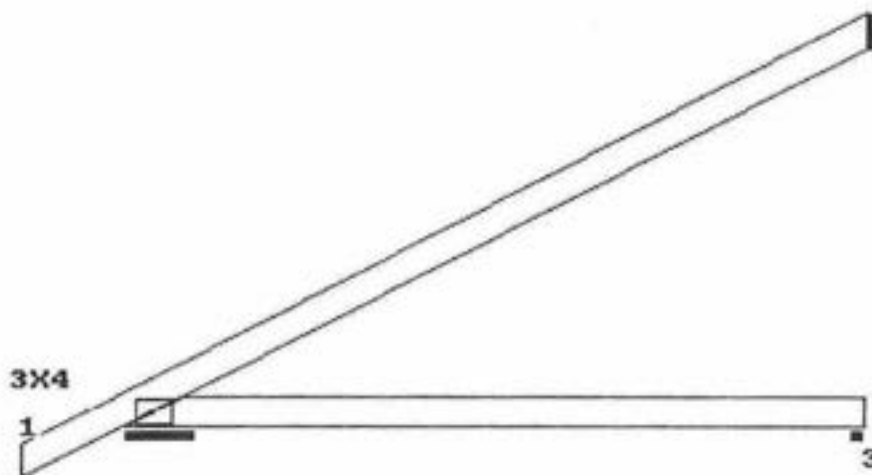
FORCES - GRAVITY LOADS
 1-2= 0 3-1= 0
 2-3= 0

REACTIONS - SIZE
 1=-473 8.00
 3=-66 1.50
 2=-297 1.50

5

PROVIDE FOR 341 LBS UPLIFT AT JOINT 1 (0.72)
 PROVIDE FOR 449 LBS UPLIFT AT JOINT 2 (1.51)
 PROVIDE FOR 407 LBS HORIZ. REACTION AT JOINT 1
 (SYMMETRIC PLATING TURNED ON)

1-0 7-0
 7-0



7-0
 7-0

L. HL TO PK:7-9-15
 LEFT HEIGHT:0-4-3

SPAN:7-0

RISE:3-10-3

RIGHT HEIGHT:3-10-3

LOADING (PSF)

TOP 30 15
 BOT 0 10

MAX STRESSES

TOP 1-2=0.993
 BOT 3-1=0.211
 LL.DEFL. < L/360

MINIMUM GRADE OF LUMBER

TOP CHORD:2X4 No.1D 19 SP
 BOT CHORD:2X4 No.2ND 19 SP
 WEBS :2X4 No.3 19 SP

STR.INC.: LUMB = 1.33 PLATE = 1.33
 REPETITIVE STRESSES USED

SPACING : 24.0 in. o. c.
 NO. OF MEMBERS = 1

TRUSS HAS BEEN CHECKED FOR 145 M.P.H. WIND LOAD, WALL HEIGHT 16 FT, ENCLOSED BLDG. TYPE, ASSUMING 10 PSF TOTAL D.L.(7*3) (SBC-97).
 DEFLECTION(IN.) L.L= 0.00,D.L=0.00,T.L=0.00

PLATES ARE MITEK M20-249,200 MANUFACTURED FROM ASTM A 446 GRD A GALVANIZED STEEL(EXCEPT AS SHOWN)

PLATE MUST BE INSTALLED ON EA. FACE OF JOINT,SYMMETRICALLY(EXCEPT AS SHOWN)DESIGN CONFORMS W/NDS DESIGN SPECS, SSSC,ANSI/TPI-95

THIS DESIGN IS FOR TRUSS FABRICATION ONLY.FOR PERMANENT AND TEMPORARY BRACING(WHICH IS ALWAYS REQD)CONSULT BLDG ARCHITECT OR ENGINEER.

MAR 01 2001

====<<<ACES-32 Ver. 1.1>>>===== [001812] =====<<<FL.QUALITY-MMH>>>=====

Customer : FOGLIA

Project #: 00469

Span : 24-0

Truss ID : H8

Quantity : 1

Thu Mar 1 08:53:27 2001

Family # : 314

Top Pitch : 6/12

==Bld(1/21/2000),v1.1=====

FORCES - LOAD CASE #1

REACTIONS - SIZE

1-2=5255	7-8=5255	8-9= 4576	2-13= 67	5-10=1080
2-3=5023		9-10= 4576	2-12=103	6-10= 1764
3-4=4484		10-11= 5228	3-12=1764	7-10=103
4-5=5279		11-12= 5228	4-12=1080	7-9= 67
5-6=4484		12-13= 4576	4-11= 124	
6-7=5023		13-1= 4576	5-11= 124	

1=2868 8.00
8=2868 8.00

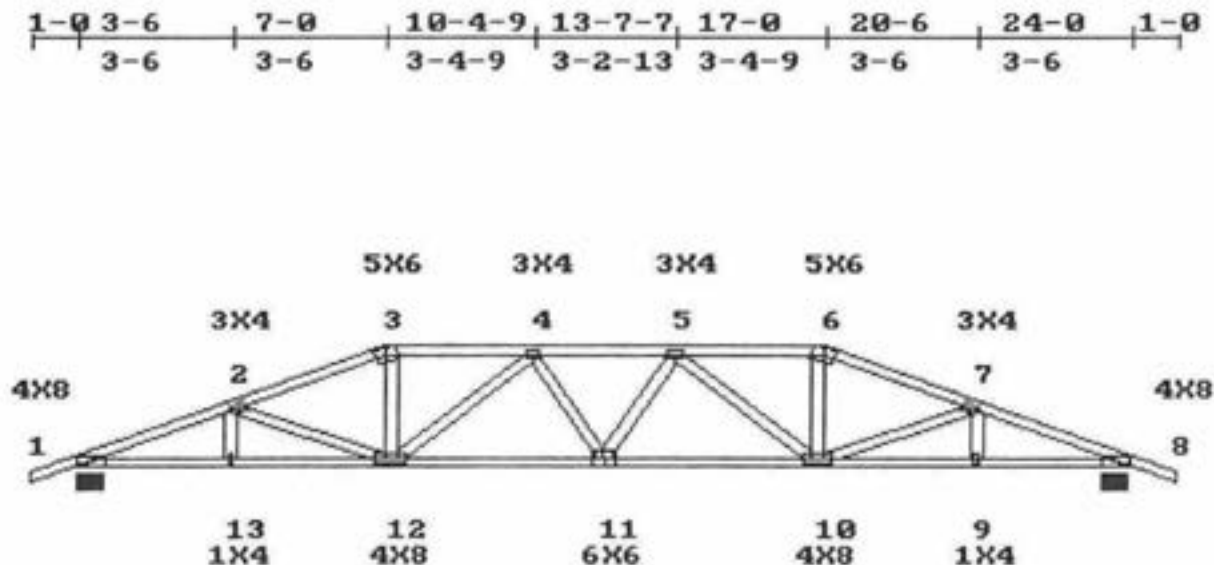
6

PROVIDE FOR 2744 LBS UPLIFT AT JOINT 1 (0.96)

PROVIDE FOR 2744 LBS UPLIFT AT JOINT 8 (0.96)

PROVIDE FOR 119 LBS HORIZ. REACTION AT JOINT 1

PLATE OFFSETS (X=LEFT,Y=TOP):[j3=2.5,2.5],[j6=3.5,2.5],[j11=3,2.5],(SYMMETRIC PLATING TURNED ON)



3-6	7-0	12-0	17-0	20-6	24-0
3-6	3-6	5-0	5-0	3-6	3-6

L. HL TO PK:7-9-15

LEFT HEIGHT:0-4-3

SPAN:24-0

RISE:3-10-3

R. HL TO PK:7-9-15

RIGHT HEIGHT:0-4-3

LOADING (PSF)

MAX STRESSES

MINIMUM GRADE OF LUMBER

TOP 30 15

TOP 7-8=0.908

TOP CHORD:2X4 No.1D 19 SP

BOTT 0 10

BOTT 10-11=0.997

BOT CHORD:2X4 No.1D 19 SP

LL.DEFL.@11=0.25 < L/360

WEBS :2X4 No.3 19 SP

REPETITIVE STRESSES NOT USED

SPACING : 24.0 in. o. c.
NO. OF MEMBERS = 1

LOADING STRESS INCREASE	LOADING LUMBER PLATE	LOADING TYPE	PANEL(PLF) / JOINTS(LBS)
1	1.33 1.33	UNIFORM	1- 3= 90 3- 6= 203 6- 8= 90 8-10= 20 10-12= 45 12- 1= 20
		CONCENTRATED	10= 787 12= 787

TRUSS HAS BEEN CHECKED FOR 145 M.P.H. WIND LOAD, WALL HEIGHT 16 FT, ENCLOSED BLDG. TYPE, ASSUMING 10 PSF TOTAL D.L.(7+3) (SBC-97).

DEFLECTION(IN.) L.L= 0.25,D.L=0.21,T.L=0.45

PLATES ARE MITEK M20-249,200 MANUFACTURED FROM ASTM A 446 GRD A GALVANIZED STEEL(EXCEPT AS SHOWN)

PLATE MUST BE INSTALLED ON EA. FACE OF JOINT,SYMMETRICALLY(EXCEPT AS SHOWN)DESIGN CONFORMS W/NDIS DESIGN SPECS, SSSC,ANSI/TPI-95

THIS DESIGN IS FOR TRUSS FABRICATION ONLY.FOR PERMANENT AND TEMPORARY BRACING(WHICH IS ALWAYS REQD)CONSULT BLDG ARCHITECT OR ENGINEER.

[Signature]
MAR 1 2001

====<<<<ACES-32 Ver. 1.1>>>>===== [001813] =====<<<<FL.QUALITY-MMH>>>>=====

Customer : FOGLIA Truss ID : H1 Thu Mar 1 08:53:39 2001

Project #: 00469 Quantity : 1 Family #: 314

Span : 24-0 Top Pitch : 6/12

==Bld(1/21/2000),v1.1=====

FORCES - GRAVITY LOADS

1-2=2190	6-7= 1920	2-10= 87	5-7= 87
2-3=1719	7-8= 1920	2-9=432	
3-4=1535	8-9= 1535	3-9= 301	
4-5=1719	9-10= 1920	3-8= 0	
5-6=2190	10-1= 1920	4-8= 301	
		5-8=432	

REACTIONS - SIZE

1=1393	8.00
6=1393	8.00

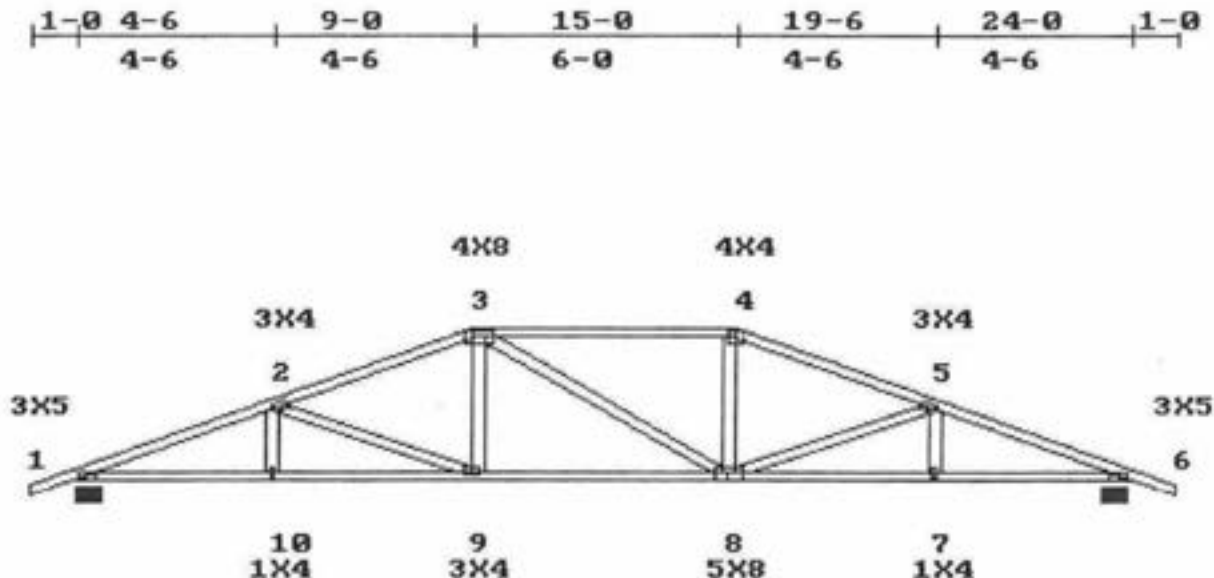
⑦

PROVIDE FOR 1196 LBS UPLIFT AT JOINT 1 (0.86)

PROVIDE FOR 1196 LBS UPLIFT AT JOINT 6 (0.86)

PROVIDE FOR 153 LBS HORIZ. REACTION AT JOINT 1

PLATE OFFSETS (X=LEFT,Y=TOP):[JB=4,2],(SYMMETRIC PLATING TURNED ON)



L. HL TO PK:10-0-12

LEFT HEIGHT:0-4-3

SPAN:24-0

RISE:4-10-3

R. HL TO PK:10-0-12

RIGHT HEIGHT:0-4-3

LOADING (PSF)

	L	D
TOP	30	15
BOTT	0	10

MAX STRESSES

TOP	3-4=0.966
BOTT	9-10=0.493
LL.DEFL.@9	=0.08 < L/360

MINIMUM GRADE OF LUMBER

TOP CHORD:	2X4	No.2ND 19 SP
BOT CHORD:	2X4	No.2ND 19 SP
WEBS	:2X4	No.3 19 SP

STR.INC.: LUMB = 1.33 PLATE = 1.33

REPETITIVE STRESSES USED

SPACING : 24.0 in. o. c.

NO. OF MEMBERS = 1

CHORD: 3-4 TO BE 2X4 No.1D 19 SP

TRUSS HAS BEEN CHECKED FOR 145 M.P.H. WIND LOAD, WALL HEIGHT 16 FT, ENCLOSED BLDG. TYPE, ASSUMING 10 PSF TOTAL D.L.(7+3) (SBC-97).

DEFLECTION(IN.) L.L= 0.08,D.L=0.07,T.L=0.15

PLATES ARE MITEK M20-249,200 MANUFACTURED FROM ASTM A 446 GRD A GALVANIZED STEEL(EXCEPT AS SHOWN)

PLATE MUST BE INSTALLED ON EA. FACE OF JOINT,SYMMETRICALLY(EXCEPT AS SHOWN)DESIGN CONFORMS W/NDS DESIGN SPECS, SSSC,ANSI/TP1-95

THIS DESIGN IS FOR TRUSS FABRICATION ONLY.FOR PERMANENT AND TEMPORARY BRACING(WHICH IS ALWAYS REQD)CONSULT BLDG ARCHITECT OR ENGINEER.

[Signature]

MAR 01 2001

====<<<<ACES-32 Ver. 1.1>>>>===== [002018] =====<<<<FL.QUALITY-MMH>>>>=====

Customer : FOGLIA Wed Mar 7 15:08:57 2001

Project #: 00469 Truss ID : H2 Family # : spec.

Span : 24-0 B.Span : 24-0 Quant. : 1 Top Pitch : 6/12

Bot pitch : 90/12

==Bld(1/21/2000),v1.1=====

FORCES - LOAD CASE #1										REACTIONS - SIZE	
1-2=3998	7-8=10061	10-11=3519	16-17=8648	2-20=57	5-16=2308	8-12=4402	1=2181	8.00			
2-3=3770	8-9=3770	11-12=3519	17-18=19	2-19=179	6-16=0	9-12=179	10=2181	8.00			
3-4=10061	9-10=3998	12-13=0	18-19=0	3-19=4402	6-15=2308	9-11=57					
4-5=5612		13-14=19	19-20=3519	3-17=5795	7-15=4046	14-12=5660					
5-6=5008		14-15=8648	20-1=3519	4-17=3527	7-14=3527	17-19=5660					
6-7=5612		15-16=5008		4-16=4046	8-14=5795						

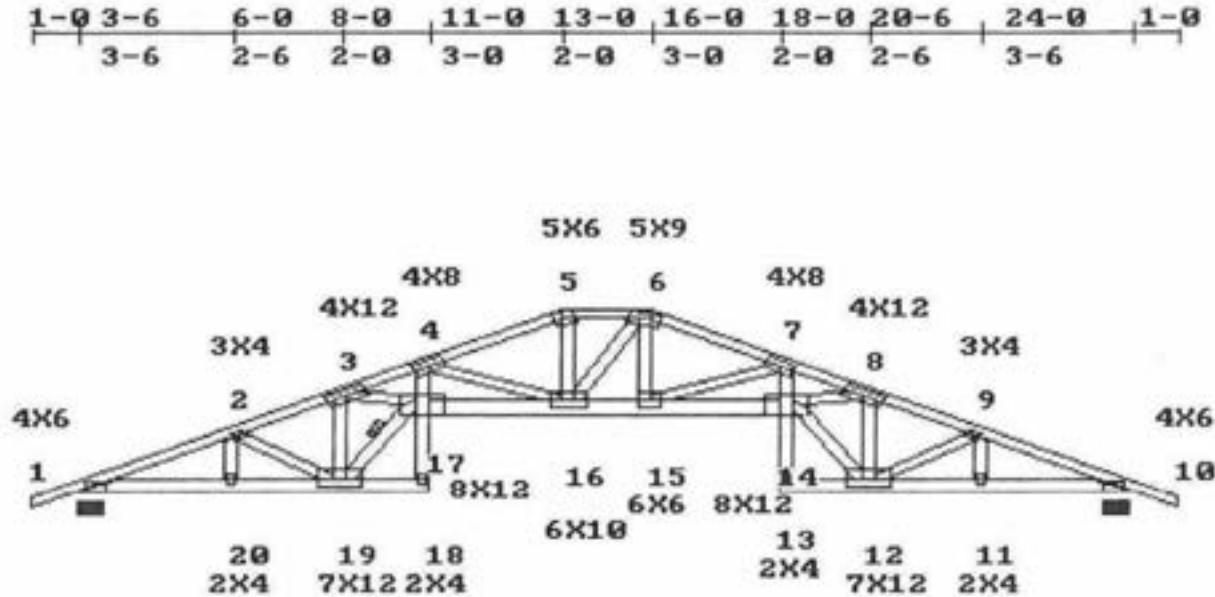
8

PROVIDE FOR 1870 LBS UPLIFT AT JOINT 1 (0.86)

PROVIDE FOR 1870 LBS UPLIFT AT JOINT 10 (0.86)

PROVIDE FOR 183 LBS HORIZ. REACTION AT JOINT 1

PLATE OFFSETS (X=LEFT,Y=TOP): [j5=2.5,2.5], [j14=6,2.5], [j17=6,2.5],



3-6	6-0	8-0	11-0	13-0	16-0	18-0	20-6	24-0
3-6	2-6	2-0	3-0	2-0	3-0	2-0	2-6	3-6

L. HL TO PK:12-3-9 INTERNAL RISE:2-6 R. HL TO PK:12-3-9

LEFT HEIGHT:0-4-3 SPAN:24-0 RISE:5-10-3 RIGHT HEIGHT:0-4-3

LOADING (PSF)		MAX STRESSES		MINIMUM GRADE OF LUMBER	
TOP	30	TOP	5-6=0.910	TOP CHORD:	2X4 SS 19 SP
BOTT	0	BOTT	16-17=0.652	BOT CHORD:	2X6 No.2D 19 SP
		LL.DEFL.@15=0.50	< L/360	WEBS	:2X4 No.3 19 SP

REPETITIVE STRESSES NOT USED

NOTE: PROVIDE FOR 0.69 IN. HORIZONTAL DISPLACEMENT AT ONE BEARING

LOADING	STRESS INCREASE	LOADING	PANEL(PLF) / JOINTS(LBS)
1	1.33	1.33	UNIFORM
			1-10= 90 10-13= 20 14-17= 210 18- 1= 20

CHORD: 5-6 TO BE 2X4 No.2ND 19 SP

CHORD: 13-14; 17-18 TO BE 2X4 No.3 19 SP

CHORD: 14-15; 15-16; 16-17 TO BE 2X6 SS 19 SP

WEB: 3-19; 4-17; 4-16; 7-15; 7-14; 8-12 TO BE 2X4 No.2ND 19 SP

WEB: 3-17; 8-14; 14-12; 17-19 TO BE 2X4 No.1D 19 SP

WEB 17-19 BRACED AT 1/2 POINTS AS SHOWN ABOVE

Note:Use 1x4 or 2x3 Cont.Bracing conn.w/2-8d nails,or T-brace of same size and grade as web conn. to narrow face w/10d nails 6 in. o.c

TRUSS HAS BEEN CHECKED FOR 145 M.P.H. WIND LOAD, WALL HEIGHT 16 FT, ENCLOSED BLDG. TYPE, ASSUMING 10 PSF TOTAL D.L.(7+3) (SBC-97).

DEFLECTION(IN.) L.L= 0.50,D.L=0.42,T.L=0.93

PLATES ARE MITEK M20-249,200 MANUFACTURED FROM ASTM A 446 GRD A GALVANIZED STEEL(EXCEPT AS SHOWN)

PLATE MUST BE INSTALLED ON EA. FACE OF JOINT,SYMMETRICALLY(EXCEPT AS SHOWN)DESIGN CONFORMS W/NDS DESIGN SPECS, SSSC,ANSI/TPI-95

MAR 07 2001

====<<<<ACES-32 Ver. 1.1>>>>===== [002017] =====<<<<FL.QUALITY-MMH>>>>=====

Customer : FOGLIA Wed Mar 7 15:07:38 2001

Project # : 00469 Truss ID : S2 Family # : spec.

Span : 24-0 B.Span : 24-0 Quant. : 1 Top Pitch : 6/12

Bot pitch : 90/12

==Bld(1/21/2000),v1.1=====

FORCES - LOAD CASE #1

1-2=-3998	7-8=-10082	11-12= 3519	17-18= 19	2-20= 57	5-16=-2128	9-13=-4402
2-3=-3770	8-9=-10061	12-13= 3519	18-19= 0	2-19=-179	6-16= 4172	10-13=-179
3-4=-10061	9-10=-3770	13-14= 0	19-20= 3519	3-19=-4402	7-16=-2128	10-12= 57
4-5=-10082	10-11=-3998	14-15= 19	20-1= 3519	3-17= 5795	7-15= 5050	15-13= 5660
5-6=-4866		15-16= 5696		4-17=-190	8-15=-190	17-19= 5660
6-7=-4866		16-17= 5696		5-17= 5050	9-15= 5795	

REACTIONS - SIZE

1=-2181	8.00
11=-2181	8.00

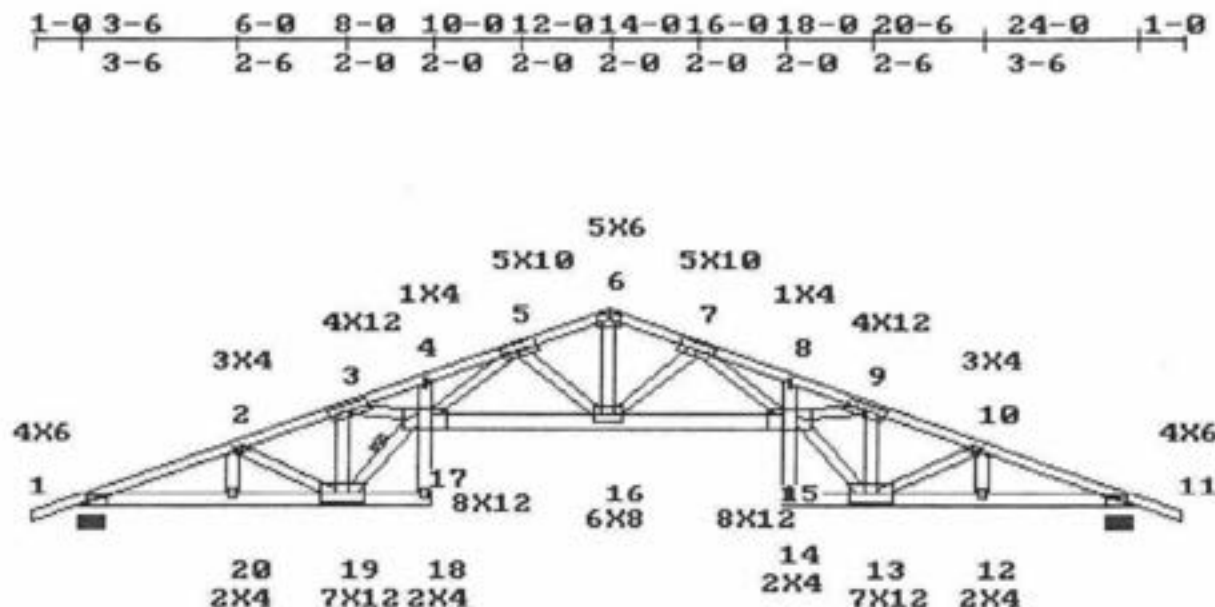
9

PROVIDE FOR 1850 LBS UPLIFT AT JOINT 1 (0.85)

PROVIDE FOR 1850 LBS UPLIFT AT JOINT 11 (0.85)

PROVIDE FOR 200 LBS HORIZ. REACTION AT JOINT 1

PLATE OFFSETS (X=LEFT,Y=TOP): (j6=3,2), (j15=6,2.5), (j17=6,2.5),



3-6	6-0	8-0	12-0	16-0	18-0	20-6	24-0
3-6	2-6	2-0	4-0	4-0	2-0	2-6	3-6

L. HL TO PK:13-5
LEFT HEIGHT:0-4-3

INTERNAL RISE:2-6
SPAN:24-0
RISE:6-4-3

R. HL TO PK:13-5
RIGHT HEIGHT:0-4-3

LOADING (PSF)

	L	D
TOP	30	15
BOTT	0	10

MAX STRESSES

TOP	3-4=0.969
BOTT	15-16=0.567
LL.DEFL.@16	=0.57 < L/360

MINIMUM GRADE OF LUMBER

TOP CHORD:2X4	SS 19 SP
BOT CHORD:2X6	No.2D 19 SP
WEBS	:2X4 No.3 19 SP

REPETITIVE STRESSES NOT USED

NOTE: PROVIDE FOR 0.75 IN. HORIZONTAL DISPLACEMENT AT ONE BEARING

SPACING : 24.0 in. o. c.
NO. OF MEMBERS = 1

LOADING	STRESS INCREASE	LOADING	PANEL(PLF) / JOINTS(LBS)
LUMBER	PLATE	TYPE	
1	1.33	1.33	UNIFORM
			1-11= 90 11-14= 20 15-17= 210 18- 1= 20

CHORD: 14-15; 17-18 TO BE 2X4 No.3 19 SP

CHORD: 15-16; 16-17 TO BE 2X6 SS 19 SP

WEB: 3-19; 5-17; 6-16; 7-15; 9-13 TO BE 2X4 No.2ND 19 SP

WEB: 3-17; 9-15; 15-13; 17-19 TO BE 2X4 No.1D 19 SP

WEB 17-19 BRACED AT 1/2 POINTS AS SHOWN ABOVE

Note:Use 1x4 or 2x3 Cont.Bracing conn.w/2-8d nails,or T-brace of same size and grade as web conn. to narrow face w/10d nails 6 in. o.c
TRUSS HAS BEEN CHECKED FOR 145 M.P.H. WIND LOAD, WALL HEIGHT 16 FT, ENCLOSED BLDG. TYPE, ASSUMING 10 PSF TOTAL D.L.(7+3) (SBC-97).

DEFLECTION(IN.) L.L= 0.57,D.L=0.47,T.L=1.04

PLATES ARE MITEK M20-249,200 MANUFACTURED FROM ASTM A 446 GRD A GALVANIZED STEEL(EXCEPT AS SHOWN)

PLATE MUST BE INSTALLED ON EA. FACE OF JOINT,SYMMETRICALLY(EXCEPT AS SHOWN)DESIGN CONFORMS W/NDS DESIGN SPECS, S88C,ANSI/TPI-95

THIS DESIGN IS FOR TRUSS FABRICATION ONLY.FOR PERMANENT AND TEMPORARY BRACING(WHICH IS ALWAYS REQD)CONSULT BLDG ARCHITECT OR ENGINEER.

MAR 07 2001

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The diagram shows a roof truss structure with various members labeled with numbers and sizes. The members are arranged in a truss configuration, with a central vertical member (7) and diagonal members (1, 2, 3, 4, 5, 6, 8, 9, 10, 11, 12). The sizes of the members are indicated by numbers and letters (e.g., 1X4, 2X4, 3X8, 4X12, 5X5, 6X8, 7X6, 8X12, 9X12, 10X12, 11X12, 12X12). The diagram also includes a section line at the top with various numbers and letters (e.g., 4-3-1, 6-1, 7-10-15, 11-10-15, 15-10-15, 19-6-13, 0-7-4).

$\begin{array}{ccccccc} & & & & 11-11 & & 17-8-14 & & 23-2-6 \\ 4-3-1 & 6-1 & 7-10-15 & & 15-10-15 & & 19-6-13 & & \\ 4-3-1 & 1-9-15 & & 4-0-1 & 3-11-15 & & 1-9-15 & 3-7-9 \end{array}$

LOADING (PSF)		MAX STRESSES	MINIMUM GRADE OF LUMBER	
	L D			
TOP	30 15	TOP 5-6=0.954	TOP CHORD:2X4	No.2ND 19 SP
BOTT	0 10	BOTT 18-19=0.560	BOT CHORD:2X6	No.2D 19 SP
		LL.DEFL.@20=0.54 < L/360	WEBS :2X4	No.3 19 SP

SPACING : 24.0 in. o. c.
NO. OF MEMBERS = 1

LOADING	STRESS	INCREASE	LOADING	PANEL(PLF) / JOINTS(LBS)				
	LUMBER	PLATE	TYPE					
1	1.33	1.33	UNIFORM	2-12= 90	13-16= 20	17-19= 210	20- 1= 20	

MAR 07 2001

Note: Use 1x4 or 2x3 Cont. Bracing conn. w/ 2-Bd nails, or T-brace of same size and grade as web conn. to narrow face w/ 10d nails 6 in. o.c.

53

TRUSS HAS BEEN CHECKED FOR 145 M.P.H. WIND LOAD, WALL HEIGHT 16 FT, ENCLOSED BLDG. TYPE, ASSUMING 10 PSF TOTAL D.L.(7+3) (SBC-97).
DEFLECTION(IN.) L.L= 0.54,D.L=0.45,T.L=0.99
PLATES ARE MITEK M20-249,200 MANUFACTURED FROM ASTM A 446 GRD A GALVANIZED STEEL(EXCEPT AS SHOWN)
PLATE MUST BE INSTALLED ON EA. FACE OF JOINT,SYMMETRICALLY(EXCEPT AS SHOWN)DESIGN CONFORMS W/NDS DESIGN SPECS, SSBC,ANSI/TPI-95
THIS DESIGN IS FOR TRUSS FABRICATION ONLY.FOR PERMANENT AND TEMPORARY BRACING(WHICH IS ALWAYS REQD)CONSULT BLDG ARCHITECT OR ENGINEER.



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MAR 07 2001

====<<<ACES-32 Ver. 1.1>>>===== [002015] =====<<<FL.QUALITY-MMH>>>=====

Customer : FOGLIA Wed Mar 7 15:03:46 2001

Project # : 00469 Truss ID : S4 Family # : spec.

Span : 24-6-6 B.Span : 24-6-6 Quant. : 1 Top Pitch : 6/12

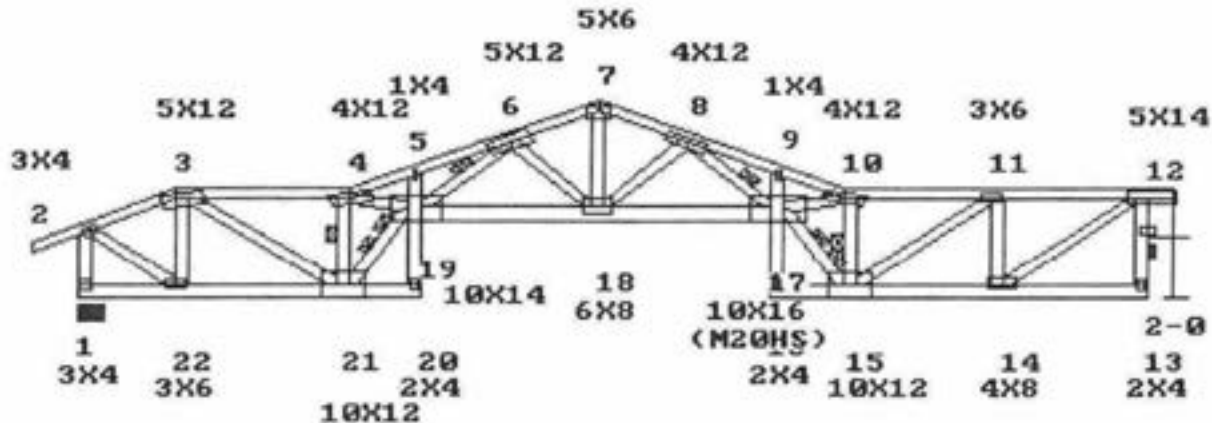
Bot pitch : 90/12

==Bld(1/21/2000),v1.1=====

FORCES - LOAD CASE #1

TOP CHORD: 1-2=-2256, 2-3=-1536, 3-4=-3541, 4-5=-10707, 5-6=-10784, 6-7=-5225, 7-8=-5217,
8-9=-11190, 9-10=-11113, 10-11=-3804, 11-12=-2104, 12-13= 35,
BOTTOM CHORD: 13-14= 0, 14-15= 2104, 15-16= 0, 16-17= 15, 17-18= 6198, 18-19= 6090, 19-20= 15,
20-21= 0, 21-22= 1371, 22-1= 0,
WEBS: 2-22= 1920, 3-22=-1285, 3-21= 2760, 4-21=-7552, 4-19= 6593, 5-19=-281, 6-19= 5384,
6-18=-2241, 7-18= 4490, 8-18=-2426, 8-17= 5801, 9-17=-283, 10-17= 6715, 10-15=-7752,
11-15= 2285, 11-14=-1841, 12-14= 2842, 17-15= 7337, 19-21= 6888,
REACTIONS,SIZE:1=-2277,8.00 12=-2168,3.03
PROVIDE FOR 1954 LBS UPLIFT AT JOINT 1 (0.86)
PROVIDE FOR 1869 LBS UPLIFT AT JOINT 12 (0.86)
PROVIDE FOR 221 LBS HORIZ. REACTION AT JOINT 1
PLATE OFFSETS (X=LEFT,Y=TOP): [J3=6,2], [J7=3,2], [J12=2.5,3], [J14=4,2.5], [J17=8,4], [J19=7,4.5],

1-1 2-3-1 6-3-1 7-10-14 11-11 15-10-14 21-0-13 24-6-6 0-7-4
2-3-1 4-0 1-7-13 2-0-1 1-11-15 3-6 3-5-9
2-0-1 1-11-15 1-7-15



2-3 6-3 7-10-13 11-10-15 15-10-14 17-6-13 21-0-13 24-6-6
2-3 4-0 1-7-13 3-11-15 3-6 3-5-9

L. HL TO PK:2-6-4 INTERNAL RISE:2-6
LEFT HEIGHT:2-4-11 SPAN:24-6-6 RISE:6-4-3 RIGHT HEIGHT:3-6-4

LOADING (PSF) MAX STRESSES MINIMUM GRADE OF LUMBER
L D TOP 10-11=0.937 TOP CHORD:2X4 No.2ND 19 SP
30 15 BOT 17-18=0.600 BOT CHORD:2X6 No.2D 19 SP
0 10 LL.DEFL.@16=0.71 < L/360 WEBS :2X4 No.3 19 SP

REPETITIVE STRESSES NOT USED

NOTE: PROVIDE FOR 0.86 IN. HORIZONTAL DISPLACEMENT AT ONE BEARING

SPACING : 24.0 in. o. c.
NO. OF MEMBERS = 1

LOADING	STRESS INCREASE	LOADING	PANEL(PLF) / JOINTS(LBS)
LUMBER	PLATE	TYPE	
1	1.33	1.33	UNIFORM 2-12= 90 13-16= 20 17-19= 210 20- 1= 20

CHORD: 3-4 TO BE 2X4 No.10 19 SP
CHORD: 4-5; 5-6; 6-7; 7-8; 8-9; 9-10 TO BE 2X4 SS 19 SP
CHORD: 16-17; 19-20 TO BE 2X4 No.3 19 SP
CHORD: 17-18; 18-19 TO BE 2X6 SS 19 SP
WEB: 4-21; 4-19; 8-17; 10-17; 10-15; 17-15; 19-21 TO BE 2X4 No.10 19 SP
WEB: 6-19; 7-18 TO BE 2X4 No.2ND 19 SP
BOTH END VERTICALS ARE NOT EXPOSED.
WEB 4-21; 6-19; 8-17; 10-15 BRACED at 1/2 POINTS AS SHOWN ABOVE
WEB 17-15; 19-21 BRACED at 1/3 POINTS AS SHOWN ABOVE
Note:Use 1x4 or 2x3 Cont.Bracing conn.w/2-8d nails,or T-brace of same size and grade as web conn. to narrow face w/10d nails 6 in. o.c

MAR 07 2001

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TRUSS HAS BEEN CHECKED FOR 145 M.P.H. WIND LOAD, WALL HEIGHT 16 FT, ENCLOSED BLDG. TYPE, ASSUMING 10 PSF TOTAL D.L.(7+3) (SBC-97).
DEFLECTION(IN.) L.L= 0.71,D.L=0.59,T.L=1.30
PLATES ARE MITEK M20-249,200 M20HS-187,150 MANUFACTURED FROM ASTM A 446 GRD A GALVANIZED STEEL(EXCEPT AS SHOWN)
PLATE MUST BE INSTALLED ON EA. FACE OF JOINT,SYMMETRICALLY(EXCEPT AS SHOWN)DESIGN CONFORMS W/ND S DESIGN SPECS, SBC,ANSI/TP1-95
THIS DESIGN IS FOR TRUSS FABRICATION ONLY.FOR PERMANENT AND TEMPORARY BRACING(WHICH IS ALWAYS REQD)CONSULT BLDG ARCHITECT OR ENGINEER.

 (W)

MAR 07 2001

====<<<ACES-32 Ver. 1.1>>>===== [002014] =====<<<FL.QUALITY-MMH>>>=====

Customer : FOGLIA Wed Mar 7 15:01:33 2001

Project #: 00469 Truss ID : S5 Family #: spec.

Span : 24-6-6 B.Span : 24-6-6 Quant. : 1 Top Pitch : 6/12

Bot pitch : 90/12

==Bld(1/21/2000),v1.1=====

FORCES - LOAD CASE #1						REACTIONS - SIZE	
1-2=-2100	7-8=-8104	11-12= 0	17-18= 6800	2-20= 1996	5-16=-3784	9-14= 6024	1=-2142 8.00
2-3=-1997	8-9=-7123	12-13= 0	18-19= 25	3-20=-2610	6-16= 3970	9-12=-3482	10=-2043 3.09
3-4=-6677	9-10=-1982	13-14= 27	19-20= 0	3-18= 5850	7-16=-4126	10-12= 2620	
4-5=-7792	10-11= 48	14-15= 7235	20-1= 0	4-18=-1391	7-15=-685	14-12= 2707	
5-6=-4843		15-16= 8104		4-17= 1561	8-15= 1375	18-20= 2519	
6-7=-4843		16-17= 7792		5-17=-825	8-14=-1258		

12

PROVIDE FOR 1842 LBS UPLIFT AT JOINT 1 (0.86)

PROVIDE FOR 1813 LBS UPLIFT AT JOINT 10 (0.89)

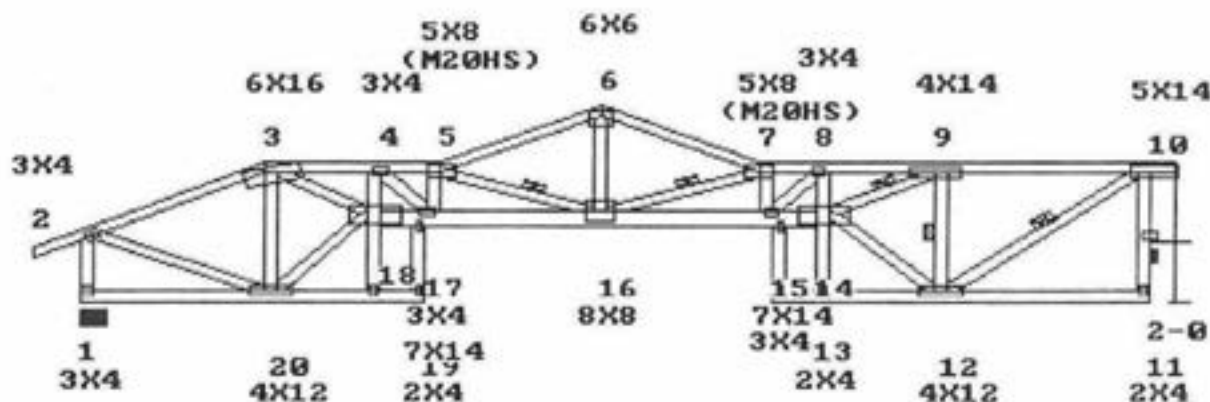
PROVIDE FOR 304 LBS HORIZ. REACTION AT JOINT 1

PLATE OFFSETS (X=LEFT,Y=TOP): [j3=8,2], [j6=3,2], [j10=2.5,3], [j14=7,2], [j18=7,2],

1-1 4-3-1 6-10-14 11-10-15 16-10-14 24-6-6 0-7-4

4-3-1 2-7-13 3-7-14 3-7-14 2-10 4-9-8

1-4-3 1-4-1



8-3-1 15-6-13 19-8-14

4-3-1 6-10-14 11-10-15 16-10-14 24-6-6

4-3-1 2-7-13 3-7-14 3-7-14 2-10 4-9-8

L. HL TO PK:4-9-1

LEFT HEIGHT:2-4-11

INTERNAL RISE:2-6

SPAN:24-6-6

RISE:6-4-2

RIGHT HEIGHT:4-6-3

LOADING (PSF)

MAX STRESSES

MINIMUM GRADE OF LUMBER

TOP 30 15

TOP 2-3=0.979

TOP CHORD:2X4 No.2ND 19 SP

BOTT 0 10

BOTT 15-16=0.684

BOT CHORD:2X6 No.2D 19 SP

LL.DEFL.@15=0.46 < L/360

WEBS :2X4 No.3 19 SP

SPACING : 24.0 in. o. c.

NO. OF MEMBERS = 1

REPETITIVE STRESSES NOT USED

NOTE: PROVIDE FOR 0.62 IN. HORIZONTAL DISPLACEMENT AT ONE BEARING

LOADING STRESS INCREASE

LOADING

PANEL(PLF) / JOINTS(LBS)

LUMBER PLATE TYPE

1 1.33 1.33

UNIFORM

2-10= 90

11-13= 20

14-15= 20

15-17= 200

17-18= 20

19- 1= 20

CHORD: 3-4; 4-5; 7-8; 8-9; 9-10 TO BE 2X4 SS 19 SP

CHORD: 5-6; 6-7 TO BE 2X4 No.1D 19 SP

CHORD: 13-14; 18-19 TO BE 2X4 No.3 19 SP

CHORD: 14-15; 15-16; 16-17; 17-18 TO BE 2X6 SS 19 SP

WEB: 3-18; 9-14 TO BE 2X4 No.1D 19 SP

WEB: 5-16; 6-16; 7-16 TO BE 2X4 No.2ND 19 SP

BOTH END VERTICALS ARE NOT EXPOSED.

WEB 5-16; 7-16; 9-14; 9-12; 10-12 BRACED AT 1/2 POINTS AS SHOWN ABOVE

Note:Use 1x4 or 2x3 Cont.Bracing conn.w/2-8d nails,or f-brace of same size and grade as web conn. to narrow face w/10d nails 6 in. o.c

TRUSS HAS BEEN CHECKED FOR 145 M.P.H. WIND LOAD, WALL HEIGHT 16 FT, ENCLOSED BLDG. TYPE, ASSUMING 10 PSF TOTAL D.L.(7+3) (SBC-97).

DEFLECTION(IN.) L.L= 0.46,D.L=0.38,T.L=0.84

MAR 07 2001

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PLATES ARE MITEK M20-249,200 M20HS-187,150 MANUFACTURED FROM ASTM A 446 GRD A GALVANIZED STEEL(EXCEPT AS SHOWN)
PLATE MUST BE INSTALLED ON EA. FACE OF JOINT,SYMMETRICALLY(EXCEPT AS SHOWN)DESIGN CONFORMS W/NDG DESIGN SPECS, SSBC,ANSI/TPI-95
THIS DESIGN IS FOR TRUSS FABRICATION ONLY.FOR PERMANENT AND TEMPORARY BRACING(WHICH IS ALWAYS REQD)CONSULT BLDG ARCHITECT OR ENGINEER.

12

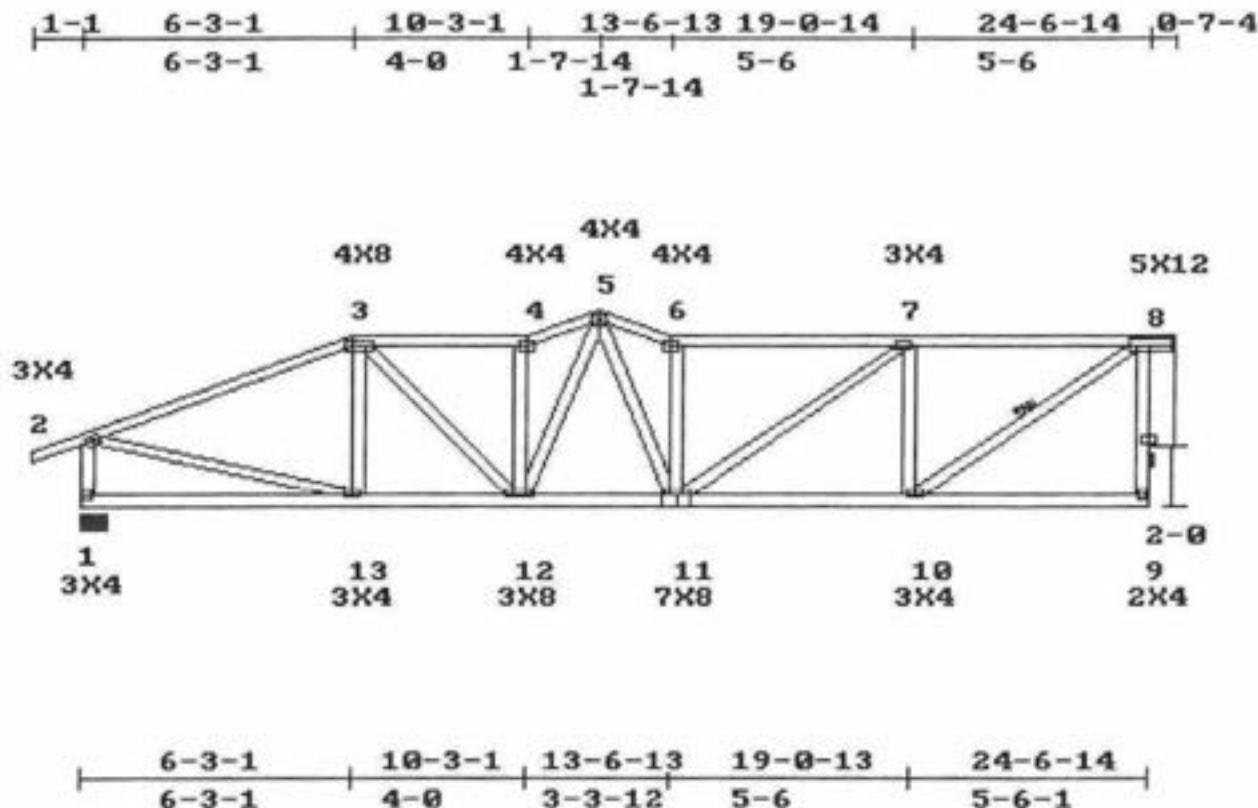
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MAR 07 2001

====<<<<ACES-32 Ver. 1.1>>>>=====[001819]====<<<<FL.QUALITY-MMH>>>>=====
 Customer : FOGLIA Thu Mar 1 09:01:59 2001
 Project #: 00469 Truss ID : S6 Family #: special
 Span : 24-6-14 Quantity : 1 Top Pitch : 6/12
 ==Bld(1/21/2000),v1.1=====

FORCES - GRAVITY LOADS			REACTIONS - SIZE	
1-2=1402	7-8=1115	9-10= 0	2-13= 1264	6-11=1096
2-3=1340	8-9= 55	10-11= 1114	3-13=304	7-11= 638
3-4=1549		11-12= 1372	3-12= 572	7-10=931
4-5=1724		12-13= 1197	4-12=1013	8-10= 1525
5-6=1760		13-1= 0	5-12= 659	
6-7=1580			5-11= 777	

(13)

PROVIDE FOR 1242 LBS UPLIFT AT JOINT 1 (0.85)
 PROVIDE FOR 1299 LBS UPLIFT AT JOINT 8 (0.92)
 PROVIDE FOR 388 LBS HORIZ. REACTION AT JOINT 1
 PLATE OFFSETS (X=LEFT,Y=TOP):{j8=2,3},{j11=4,2},{SYMMETRIC PLATING TURNED ON}



L. HL TO PK:6-11-15
 LEFT HEIGHT:2-4-11 SPAN:24-6-14 RISE:6-4-2 RIGHT HEIGHT:5-6-3

LOADING (PSF)		MAX STRESSES		MINIMUM GRADE OF LUMBER	
TOP	30	TOP	6-7=0.996	TOP CHORD:	2X4 No.1D 19 SP
BOTT	0	BOTT	11-12=0.205	BOT CHORD:	2X6 No.2D 19 SP
		LL.DEFL.@11=0.06	< L/360	WEBS	:2X4 No.3 19 SP

STR.INC.: LUMB = 1.33 PLATE = 1.33 SPACING : 24.0 in. o. c.
 REPETITIVE STRESSES USED NO. OF MEMBERS = 1

CHORD: 3-4; 4-5; 5-6 TO BE 2X4 No.2ND 19 SP
 BOTH END VERTICALS ARE NOT EXPOSED.
 WEB 8-10 BRACED at 1/2 POINTS AS SHOWN ABOVE
 Note:Use 1x4 or 2x3 Cont.Bracing conn.w/2-8d nails,or T-brace of same size and grade as web conn. to narrow face w/10d nails 6 in. o.c
 TRUSS HAS BEEN CHECKED FOR 145 M.P.H. WIND LOAD, WALL HEIGHT 16 FT, ENCLOSED BLDG. TYPE, ASSUMING 10 PSF TOTAL D.L.(7+3) (SBC-97).
 DEFLECTION(IN.) L.L= 0.06,D.L=0.05,T.L=0.12
 PLATES ARE MITEK M20-249,200 MANUFACTURED FROM ASTM A 446 GRD A GALVANIZED STEEL(EXCEPT AS SHOWN)
 PLATE MUST BE INSTALLED ON EA. FACE OF JOINT,SYMMETRICALLY(EXCEPT AS SHOWN)DESIGN CONFORMS W/NDOS DESIGN SPECS, SSBC,ANSI/TPI-95
 THIS DESIGN IS FOR TRUSS FABRICATION ONLY.FOR PERMANENT AND TEMPORARY BRACING(WHICH IS ALWAYS REQD)CONSULT BLDG ARCHITECT OR ENGINEER.

MAR 01 2001

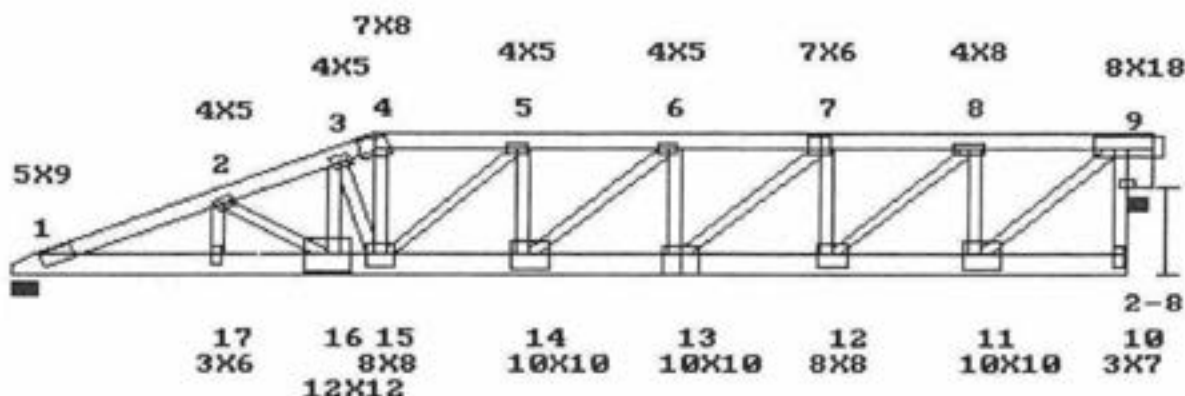
====<<<<ACES-32 Ver. 1.1>>>>=====[001820]====<<<<FL.QUALITY-MMH>>>>=====
 Customer : FOGLIA Thu Mar 1 09:02:58 2001
 Project # : 00469 Truss ID : GT25 Family # : 313
 Span : 24-7-6 B.Span : 25-3-14 Quant. : 3 Top Pitch : 6/12
 ==Bld(1/21/2000),v1.1=====

FORCES - LOAD CASE #1						REACTIONS - SIZE	
1-2=-22696	7-8=-16634	10-11= 0	16-17= 20386	2-17= 55	5-14= 2570	8-11=-7947	1=-10210 8.00
2-3=-22348	8-9=-9843	11-12= 9843	17-1= 20386	2-16=-510	6-14= 1042	9-11= 14988	9=-13122 6.00
3-4=-21157	9-10= 1678	12-13= 16634		3-16= 4983	6-13=-935		
4-5=-18687		13-14= 20371		3-15=-5154	7-13= 5691		
5-6=-21055		14-15= 21055		4-15= 9827	7-12=-4441		
6-7=-20371		15-16= 19989		5-15=-3607	8-12= 10340		

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PROVIDE FOR 10339 LBS UPLIFT AT JOINT 1 (1.01)
 PROVIDE FOR 13047 LBS UPLIFT AT JOINT 9 (0.99)
 PROVIDE FOR 448 LBS HORIZ. REACTION AT JOINT 1
 PLATE OFFSETS (X=LEFT,Y=TOP):[j4=4,4],[j7=3,5],[j9=5,5],[j13=5,3],

4-6-12 7-1-8 11-3-14 14-7-12 17-11-10 24-7-6 0-7-4
 4-6-12 2-6-12 3-3-14 3-3-14 3-3-14 3-3-14 3-3-14
 0-10-8



4-6-12 7-1-8 11-3-14 14-7-12 17-11-10 21-3-8 24-7-6
 4-6-12 2-6-12 3-3-14 3-3-14 3-3-14 3-3-14 3-3-14

L. HL TO PK:8-11-5
 LEFT HEIGHT:0-4-3 SPAN:24-7-6 RISE:4-4-3 RIGHT HEIGHT:4-4-3

LOADING (PSF)			MAX STRESSES		MINIMUM GRADE OF LUMBER	
TOP	30	15	TOP 1-2=0.906		TOP CHORD:2X6	No.2D 19 SP
BOTT	0	10	BOTT 14-15=0.512		BOT CHORD:2X8	SS 19 SP
			LL.DEFL.@14=0.18 < L/360		WEBS :2X4	No.3 19 SP

REPETITIVE STRESSES NOT USED

SPACING : 24.0 in. o. c.
 NO. OF MEMBERS = 3

LOADING	STRESS INCREASE	LOADING	PANEL(PLF) / JOINTS(LBS)
LUMBER	1.33	PLATE	1- 2= 90 2- 9= 45 10-16= 1010 16-17= 10 17- 1= 20
		TYPE	
		UNIFORM	
		CONCENTRATED	16= 4208

WEB: 8-12; 9-11 TO BE 2X4 No.2ND 19 SP
 RIGHT END VERTICAL IS NOT EXPOSED.
 3 MEMBERS NAILED TOG. W/1 ROW(S) OF .131x3 in. NAILS 12 in. o.c.(TOP CHS.),AND 3 ROW(S) OF .131x3 in. NAILS 5 in. o.c.(BOTT. CHS.)
 For Webs use 1 ROW of NAILS 12 in. o.c.
 TRUSS HAS BEEN CHECKED FOR 145 M.P.H. WIND LOAD, WALL HEIGHT 16 FT, ENCLOSED BLDG. TYPE, ASSUMING 10 PSF TOTAL D.L.(7+3) (SBC-97).
 DEFLECTION(IN.) L.L= 0.18,D.L=0.15,T.L=0.33
 PLATES ARE MITEK M20-249,200 MANUFACTURED FROM ASTM A 446 GRD A GALVANIZED STEEL(EXCEPT AS SHOWN)
 PLATE MUST BE INSTALLED ON EA. FACE OF JOINT,SYMMETRICALLY(EXCEPT AS SHOWN)DESIGN CONFORMS W/NDSD DESIGN SPECS, SSBC,ANSI/TPI-95
 THIS DESIGN IS FOR TRUSS FABRICATION ONLY.FOR PERMANENT AND TEMPORARY BRACING(WHICH IS ALWAYS REQD)CONSULT BLDG ARCHITECT OR ENGINEER.

MAR 01 2001

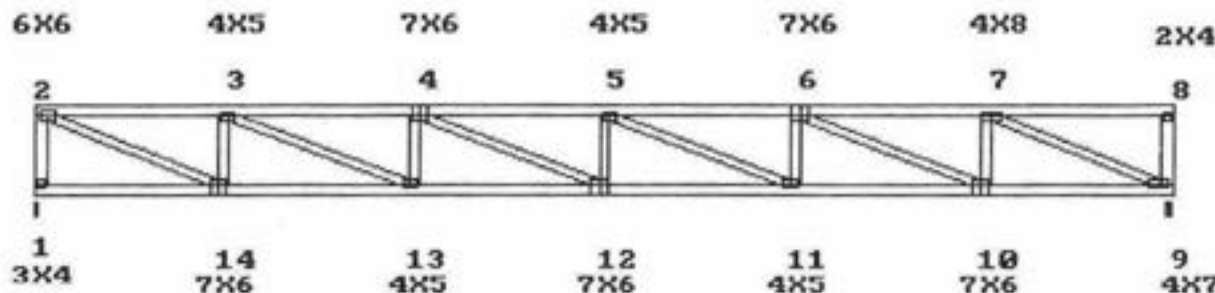
====<<<<ACES-32 Ver. 1.1>>>>=====[001821]====<<<<FL.QUALITY-MMH>>>>=====
 Customer : FOGLIA Thu Mar 1 09:03:20 2001
 Project #: 00469 Truss ID : FG3 Family # : 301
 Span : 35-2 B.Span : 35-3-8 Quant. : 2 Top Pitch : /12
 ==Bld(1/21/2000),v1.1=====

FORCES - LOAD CASE #1				REACTIONS - SIZE	
1-2=-4064	7-8= 0	9-10= 5940	2-14= 6897	5-11=-1405	1=-4193 2.77
2-3=-5940	8-9=-561	10-11= 9588	3-14=-3242	6-11= 967	9=-4193 2.77
3-4=-9588		11-12= 10804	3-13= 4214	6-10=-4214	
4-5=-10804		12-13= 9588	4-13=-1846	7-10= 2371	
5-6=-9588		13-14= 5940	4-12= 1405	7-9=-6897	
6-7=-5940		14-1= 0	5-12=-440		

(15)

PROVIDE FOR 5479 LBS UPLIFT AT JOINT 1 (1.31)
 PROVIDE FOR 5479 LBS UPLIFT AT JOINT 9 (1.31)
 PLATE OFFSETS (X=LEFT,Y=TOP):[j4=3,5],[j6=3,5],[j10=3,2],[j12=3,2],[j14=3,2],

5-10-5	11-8-11	17-7	23-5-5	29-3-11	35-2
5-10-5	5-10-5	5-10-5	5-10-5	5-10-5	5-10-5



5-10-5	11-8-11	17-7	23-5-5	29-3-11	35-2
5-10-5	5-10-5	5-10-5	5-10-5	5-10-5	5-10-5

LEFT HEIGHT:3-10-3 SPAN:35-2 RISE:3-10-3 RIGHT HEIGHT:3-10-3

LOADING (PSF)		MAX STRESSES		MINIMUM GRADE OF LUMBER	
TOP	30 15	TOP	4-5=0.908	TOP CHORD:	2X6 No.2D 19 SP
BOTT	0 10	BOTT	11-12=0.715	BOT CHORD:	2X6 No.2D 19 SP
		LL.DEFL.@12=0.36	< L/360	WEBS:	2X4 No.3 19 SP

REPETITIVE STRESSES NOT USED

SPACING : 24.0 in. o. c.
 NO. OF MEMBERS = 2

LOADING	STRESS INCREASE	LOADING	PANEL(PLF) / JOINTS(LBS)
LUMBER	PLATE	TYPE	
1	1.33	1.33	UNIFORM 2- 8= 195 9- 1= 45

WEB: 1-2; 8-9 TO BE 2X6 No.2D 19 SP
 WEB: 2-14; 7-9 TO BE 2X4 No.2D 19 SP
 BOTH END VERTICALS ARE NOT EXPOSED.
 2 MEMBERS NAILED TOG. W/1 ROW(S) OF .131x3 in. NAILS 12 in. o.c.(TOP CHS.),AND 1 ROW(S) OF .131x3 in. NAILS 12 in. o.c.(BOTT. CHS.)
 For Webs use 1 ROW of NAILS 12 in. o.c.
 TRUSS HAS BEEN CHECKED FOR 145 M.P.H. WIND LOAD, WALL HEIGHT 16 FT, ENCLOSED BLDG. TYPE, ASSUMING 10 PSF TOTAL D.L.(7+3) (SBC-97).
 DEFLECTION(IN.) L.L= 0.36,D.L=0.30,T.L=0.66
 PLATES ARE MITEK M20-249,200 MANUFACTURED FROM ASTM A 446 GRD A GALVANIZED STEEL(EXCEPT AS SHOWN)
 PLATE MUST BE INSTALLED ON EA. FACE OF JOINT,SYMMETRICALLY(EXCEPT AS SHOWN)DESIGN CONFORMS W/NDG DESIGN SPECS, SBC,ANSI/TPI-95
 THIS DESIGN IS FOR TRUSS FABRICATION ONLY.FOR PERMANENT AND TEMPORARY BRACING(WHICH IS ALWAYS REQD)CONSULT BLDG ARCHITECT OR ENGINEER.

[Signature]
 MAR 01 2001

====<<<<ACES-32 Ver. 1.1>>>>===== [001991] =====<<<<FL.QUALITY-MMH>>>>=====

Customer : FOGLIA Mon Mar 5 12:07:19 2001

Project #: 00469 Truss ID : GT36 Family #: spec.

Span : 36-10 B.Span : 36-10 Quant. : 3 Top Pitch : 6/12

Bot pitch : 90/12

==Bld(1/21/2000),v1.1=====

FORCES - LOAD CASE #1

TOP CHORD: 1-2=-6857, 2-3=-6492, 3-4=-1548, 4-5= 4349, 5-6=-5721, 6-7=-5118, 7-8=-10654,

8-9=-14038, 9-10=-15314, 10-11=-8638, 11-12=-13114,

BOTTOM CHORD: 12-13= 0, 13-14= 8638, 14-15= 15314, 15-16= 12556, 16-17= 10654, 17-18=-2920, 18-19=-21353,

19-20= 0, 20-21= 5807, 21-22= 6094, 22-1= 6094,

WEBS: 2-22= 68, 2-21=-317, 3-21= 6198, 3-20=-6118, 4-20= 6598, 4-18=-6695, 5-18=-14098,

5-17= 12365, 6-17= 2445, 7-17=-9306, 7-16= 7200, 8-16=-3199, 8-15= 8686, 9-15=-5732,

9-14=-1390, 10-14= 11654, 10-13=-9701, 11-13= 15643, 18-20= 1779,

REACTIONS, SIZE: 1=-3430, 8.00 12=-14739, 8.00 19=-23020, 12.74

PROVIDE FOR 4011 LBS UPLIFT AT JOINT 1 (1.17)

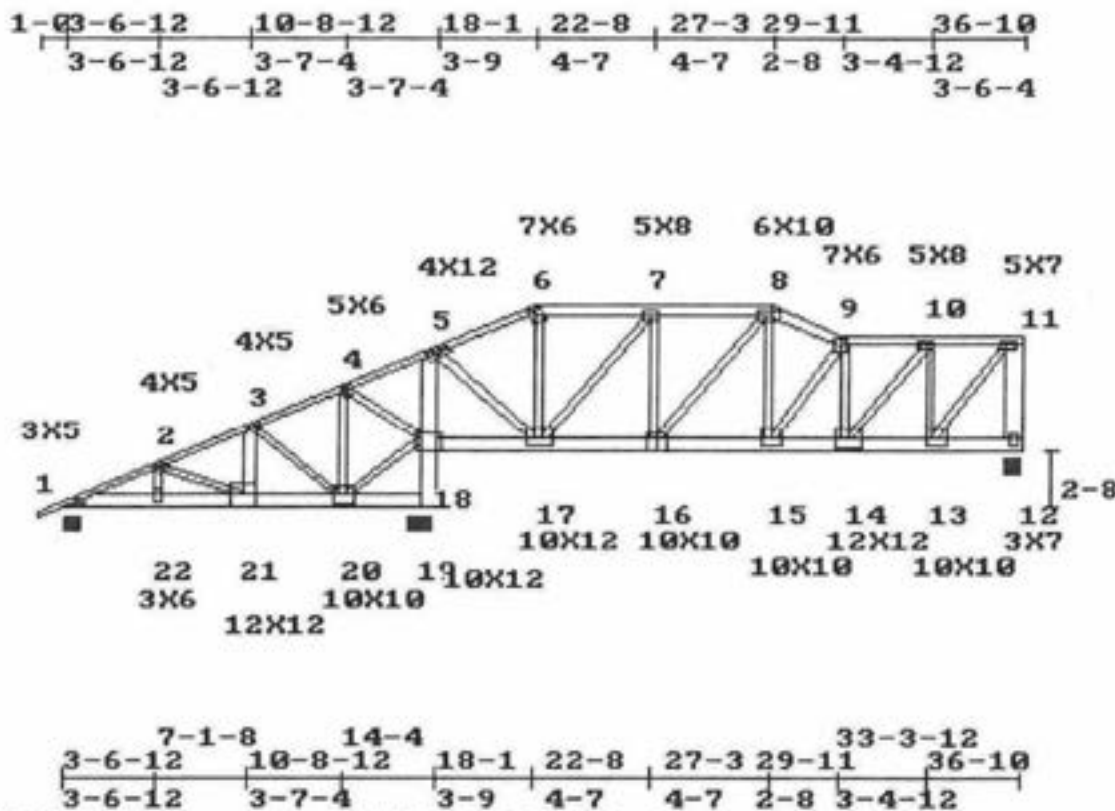
PROVIDE FOR 16480 LBS UPLIFT AT JOINT 12 (1.12)

PROVIDE FOR 26321 LBS UPLIFT AT JOINT 19 (1.14)

PROVIDE FOR 1129 LBS HORIZ. REACTION AT JOINT 1

PLATE OFFSETS (X=LEFT, Y=TOP): [j4=3,3], [j6=3,2.5], [j9=3,4], [j10=4.5,2.5], [j11=2.5,2.5], [j13=3.5,5], [j16=5,3], [j18=6,3],

*** WARNING: NO PLATE AT JOINT(S): j19, REVIEW REQUIRED ***



L. HL TO PK: 20-2-10 INTERNAL RISE: 2-8

LEFT HEIGHT: 0-4-3 SPAN: 36-10 RISE: 9-4-11 RIGHT HEIGHT: 5-4-11

LOADING (PSF) MAX STRESSES MINIMUM GRADE OF LUMBER

TOP 30 15 TOP 9-10=0.711 TOP CHORD: 2X4 No.2ND 19 SP

BOTT 0 10 BOTT 15-16=0.487 BOT CHORD: 2X8 SS 19 SP

LL.DEFL.@15=0.14 < L/360 WEBS : 2X4 No.3 19 SP

SPACING : 24.0 in. o. c.

NO. OF MEMBERS = 3

REPETITIVE STRESSES NOT USED

LOADING	STRESS INCREASE	LOADING	PANEL(PLF) / JOINTS(LBS)
LUMBER	PLATE	TYPE	
1	1.33	1.33	UNIFORM 1- 3= 90 3-11= 45 12-18= 1010 19-21= 1010 21- 1= 20
			CONCENTRATED 14= 5101 21= 4208

CHORD: 6-7; 7-8; 8-9; 9-10; 10-11 TO BE 2X6 No.2D 19 SP

WEB: 11-12 TO BE 2X8 SS 19 SP

WEB: 3-21 TO BE 2X6 No.2D 19 SP

WEB: 4-20; 7-17; 10-14; 10-13 TO BE 2X4 No.2ND 19 SP

WEB: 5-18; 5-17; 11-13 TO BE 2X4 No.1D 19 SP

RIGHT END VERTICAL IS NOT EXPOSED.

1 MEMBERS NAILED TOG. W/1 ROW(S) OF .131x3 in. NAILS 12 in. o.c.(TOP CHS.), AND 3 ROW(S) OF .131x3 in. NAILS 5 in. o.c.(BOTT. CHS.)

For Webs use 1 ROW of NAILS 12 in. o.c.

MAR 06 2001

****NOTE:Special hanger is reqd. to dist. bottom chord concentrated load(s) to all plies.****

TRUSS HAS BEEN CHECKED FOR 145 M.P.H. WIND LOAD, WALL HEIGHT 16 FT, ENCLOSED BLDG. TYPE, ASSUMING 10 PSF TOTAL D.L.(7+3) (SBC-97).
DEFLECTION(IN.) L.L= 0.14,D.L=0.12,T.L=0.26

PLATES ARE MITEK M20-249,200 M16-174,122 MANUFACTURED FROM ASTM A 446 GRD A GALVANIZED STEEL(EXCEPT AS SHOWN)

PLATE MUST BE INSTALLED ON EA. FACE OF JOINT,SYMMETRICALLY(EXCEPT AS SHOWN)DESIGN CONFORMS W/NDG DESIGN SPECS, SBC,ANSI/TP1-95

THIS DESIGN IS FOR TRUSS FABRICATION ONLY.FOR PERMANENT AND TEMPORARY BRACING(WHICH IS ALWAYS REQD)CONSULT BLDG ARCHITECT OR ENGINEER.

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MAR 06 2001

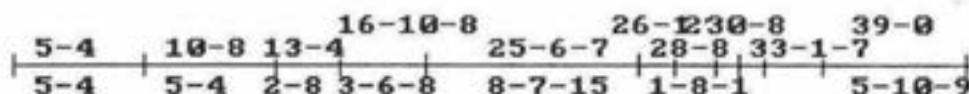
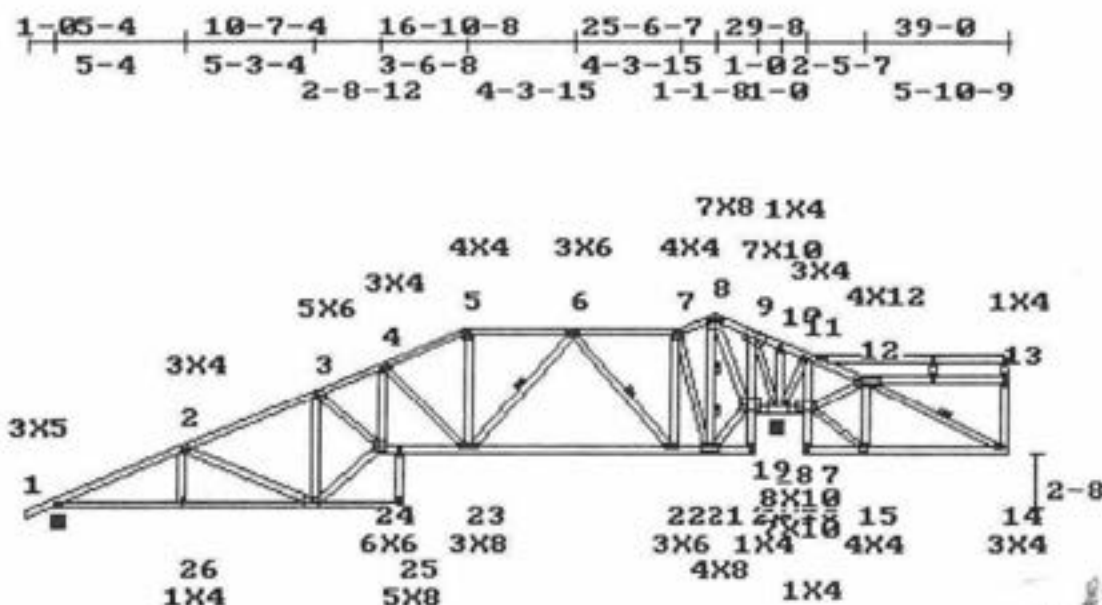
====<<<<ACES-32 Ver. 1.1>>>>=====[001822]====<<<<FL.QUALITY-MMH>>>>=====
 Customer : FOGLIA Thu Mar 1 09:12:57 2001
 Project #: 00469 Truss ID : S26 Family #: special
 Span : 39-0 Quantity : 1 Top Pitch : 6/12
 Bot pitch : 90/12

24-25 12/12,

==Bld(1/21/2000),v1.1=====

FORCES - GRAVITY LOADS

TOP CHORD: 1-2=-2479, 2-3=-1931, 3-4=-2703, 4-5=-1734, 5-6=-1548, 6-7=-298, 7-8= 51 ,
 8-9= 798, 9-10= 1620, 10-11= 1620, 11-12= 1513, 12-13= 0, 13-14=-258,
 BOTTOM CHORD: 14-15=-518, 15-16= 0, 16-17= 23, 17-18=-1364, 18-19=-790, 19-20= 15, 20-21= 0,
 21-22= 298, 22-23= 1068, 23-24= 2418, 24-25= 2420, 25-26= 2180, 26-1= 2180,
 WEBS: 2-26= 79, 2-25=-505, 3-25=-1382, 3-24= 978, 4-24= 1138, 4-23=-1320, 5-23= 427,
 6-23= 805, 6-22=-1290, 7-22= 1137, 7-21=-1417, 8-21= 1465, 8-19=-2095, 9-19= 1953,
 9-18=-2527, 10-18=-90, 11-18=-245, 11-17= 130, 12-17=-1000, 12-15= 529, 12-14= 606,
 17-15=-685, 19-21=-76,
 REACTIONS, SIZE: 1=-1565, 8.00 18=-2782, 8.00
 PROVIDE FOR 1474 LBS UPLIFT AT JOINT 1 (0.94)
 PROVIDE FOR 4565 LBS UPLIFT AT JOINT 18 (1.64)
 PROVIDE FOR 698 LBS HORIZ. REACTION AT JOINT 1
 PLATE OFFSETS (X=LEFT, Y=TOP): [j3=3,3], [j8=4,1.5], [j19=5,5], [j25=4,2], (SYMMETRIC PLATING TURNED ON)



MAR 01 2001

L. HL TO PK: 18-10-6
 LEFT HEIGHT: 0-4-3

INTERNAL RISE: 4-8
 SPAN: 39-0

RISE: 9-6-3

RIGHT HEIGHT: 3-9-7

LOADING (PSF) MAX STRESSES MINIMUM GRADE OF LUMBER
 TOP 30 15 TOP 1-2=0.959 TOP CHORD: 2X4 No.1D 19 SP
 BOTT 0 10 BOTT 14-15=0.756 BOT CHORD: 2X4 No.1D 19 SP
 LL.DEFL.@14=0.26 < L/360 WEBS : 2X4 No.3 19 SP
 STR.INC.: LUMB = 1.33 PLATE = 1.33 SPACING : 24.0 in. o. c.
 REPETITIVE STRESSES USED NO. OF MEMBERS = 1

CHORD: 5-6; 6-7; 7-8; 8-9; 9-10; 10-11; 11-12; 17-18; 18-19; 20-21; 21-22; 22-23; 23-24; 24-25; 25-26; 26-1 TO BE 2X4 No.2ND 19 SP
 CHORD: 16-17; 19-20 TO BE 2X4 No.3 19 SP
 WEB: 8-19; 9-18 TO BE 2X4 No.2ND 19 SP
 RIGHT END VERTICAL IS NOT EXPOSED.
 WEB 6-23; 6-22; 12-14 BRACED at 1/2 POINTS AS SHOWN ABOVE
 WEB 8-21 BRACED at 1/3 POINTS AS SHOWN ABOVE
 Note: Use 1x4 or 2x3 Cont.Bracing conn.w/2-8d nails, or T-brace of same size and grade as web conn. to narrow face w/10d nails 6 in. o.c.
 TRUSS HAS BEEN CHECKED FOR 145 M.P.H. WIND LOAD, WALL HEIGHT 16 FT, ENCLOSED BLDG. TYPE, ASSUMING 10 PSF TOTAL D.L.(7+3) (SBC-97).
 REFLECTION(IN.) L.L.= 0.26, D.L.=0.22, T.L.=0.48
 *LATES ARE MITEK M20-249, 200 MANUFACTURED FROM ASTM A 446 GRD A GALVANIZED STEEL(EXCEPT AS SHOWN)
 *LATE MUST BE INSTALLED ON EA. FACE OF JOINT, SYMMETRICALLY(EXCEPT AS SHOWN) DESIGN CONFORMS W/NDOS DESIGN SPECS, S5BC, ANSI/TP1-95
 THIS DESIGN IS FOR TRUSS FABRICATION ONLY.FOR PERMANENT AND TEMPORARY BRACING(WHICH IS ALWAYS REGO)CONSULT BLDG ARCHITECT OR ENGINEER.

====<<<ACES-32 Ver. 1.1>>>===== [001823] =====<<<FL.QUALITY-MMH>>>=====

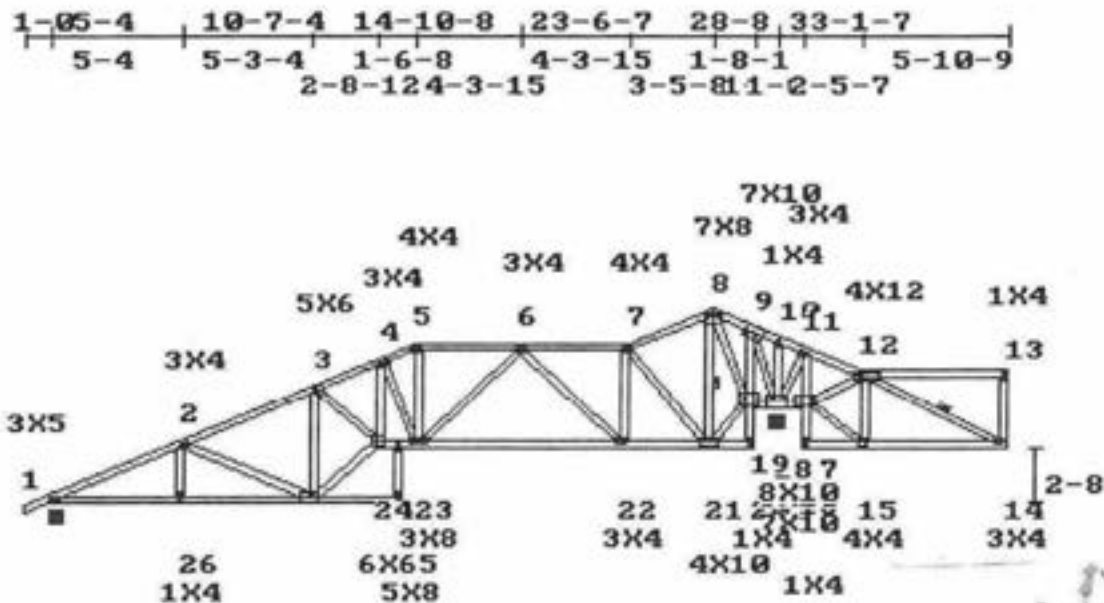
Customer : FOGLIA Truss ID : S25 Thu Mar 1 09:13:17 2001
 Project #: 00469 Quantity : 1 Family #: special
 Span : 39-0 Top Pitch : 6/12
 Bot pitch : 90/12

24-25 12/12,

==8ld(1/21/2000),v1.1=====

FORCES - GRAVITY LOADS

TOP CHORD: 1-2=-2479, 2-3=-1931, 3-4=-2703, 4-5=-2216, 5-6=-1974, 6-7=-860, 7-8= 51 ,
 8-9= 798, 9-10= 1620, 10-11= 1620, 11-12= 1513, 12-13= 0, 13-14=-258,
 BOTTOM CHORD: 14-15=-518, 15-16= 0, 16-17= 23, 17-18=-1364, 18-19=-790, 19-20= 15, 20-21= 0,
 21-22= 860, 22-23= 1591, 23-24= 2419, 24-25= 2420, 25-26= 2180, 26-1= 2180,
 WEBS: 2-26= 79, 2-25=-505, 3-25=-1382, 3-24= 978, 4-24= 1171, 4-23=-1245, 5-23= 745,
 6-23= 572, 6-22=-1096, 7-22= 937, 7-21=-1556, 8-21= 1375, 8-19=-2095, 9-19= 1953,
 9-18=-2527, 10-18=-90, 11-18=-245, 11-17= 130, 12-17=-1000, 12-15= 529, 12-14= 606,
 17-15=-685, 19-21=-76,
 REACTIONS, SIZE: 1=-1565, 8.00 18=-2782, 8.00
 PROVIDE FOR 1474 LBS UPLIFT AT JOINT 1 (0.94)
 PROVIDE FOR 4565 LBS UPLIFT AT JOINT 18 (1.64)
 PROVIDE FOR 698 LBS HORIZ. REACTION AT JOINT 1
 PLATE OFFSETS (X=LEFT, Y=TOP): [j3=3,3], [j8=4,1.5], [j19=5,5], [j25=4,2], (SYMMETRIC PLATING TURNED ON)



====<<<<ACES-32 Ver. 1.1>>>>=====[001824]=====<<<<FL.QUALITY-MMH>>>>=====

Customer : FOGLIA Thu Mar 1 09:13:49 2001

Project #: 00469 Truss ID : S24 Family # : spec.

Span : 39-0 B.Span : 39-0 Quant. : 1 Top Pitch : 6/12

Bot pitch : 90/12

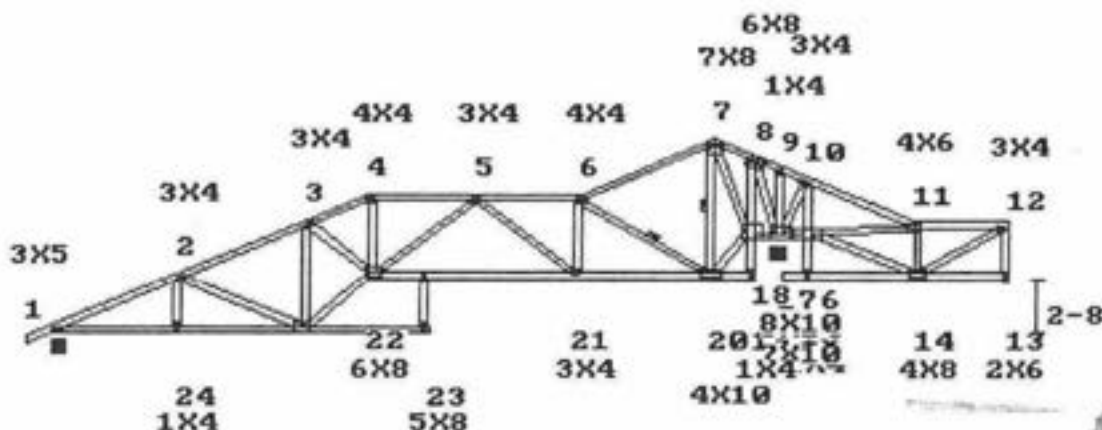
22-23 12/12,

==8ld(1/21/2000),v1.1=====

FORCES - GRAVITY LOADS

TOP CHORD: 1-2=2509, 2-3=1985, 3-4=2844, 4-5=2538, 5-6=1592, 6-7= 51, 7-8= 787,
8-9= 1597, 9-10= 1597, 10-11= 1494, 11-12= 301, 12-13= 37,
BOTTOM CHORD: 13-14= 0, 14-15= 0, 15-16= 43, 16-17=1341, 17-18=779, 18-19= 15, 19-20= 0,
20-21= 1592, 21-22= 2281, 22-23= 2486, 23-24= 2204, 24-1= 2204,
WEBS: 2-24= 74, 2-23=477, 3-23=1444, 3-22= 1044, 4-22= 967, 5-22= 342, 5-21=924,
6-21= 741, 6-20=2006, 7-20= 1289, 7-18=2090, 8-18= 1952, 8-17=2518, 9-17=90,
10-17=256, 10-16= 44, 11-16=1046, 11-14= 425, 12-14=364, 16-14=331, 18-20=75,
REACTIONS, SIZE: 1=1570, 8.00 17=2786, 8.00
PROVIDE FOR 1511 LBS UPLIFT AT JOINT 1 (0.96)
PROVIDE FOR 4456 LBS UPLIFT AT JOINT 17 (1.60)
PROVIDE FOR 602 LBS HORIZ. REACTION AT JOINT 1
PLATE OFFSETS (X=LEFT, Y=TOP): [J7=4, 1.5], [J18=5, 5], [J23=4, 2],

1-2-1-4, 10-2-12, 17-3-8, 27-128-9, 35-2-9
5-1-4, 5-1-8, 4-3-15, 5-5-9, 1-0, 4-5-9
2-8-12, 4-3-15, 1-81-0, 3-10-7



5-0-4, 10-2-8, 12-10-8, 21-6-7, 27-028-8, 230-8, 39-0
5-0-4, 5-2-4, 8-7-15, 5-5-9, 1-0, 4-5-9

MAR 01 2001

L. HL TO PK:14-5-14
LEFT HEIGHT:0-4-3

INTERNAL RISE:4-8
SPAN:39-0

RISE:9-6-12

RIGHT HEIGHT:2-9-15

LOADING (PSF)

TOP 30 15
BOTT 0 10

MAX STRESSES

TOP 1-2=0.942
BOTT 16-17=0.757
LL.DEFL.@13=0.25 < L/360

MINIMUM GRADE OF LUMBER

TOP CHORD:2X4 No.1D 19 SP
BOT CHORD:2X4 No.2ND 19 SP
WEBS :2X4 No.3 19 SP

STR.INC.: LUMB = 1.33 PLATE = 1.33
REPETITIVE STRESSES USED

SPACING : 24.0 in. o. c.
NO. OF MEMBERS = 1

CHORD: 7-8; 8-9; 9-10; 10-11; 11-12 TO BE 2X4 No.2ND 19 SP

CHORD: 15-16; 18-19 TO BE 2X4 No.3 19 SP

WEB: 7-18; 8-17 TO BE 2X4 No.2ND 19 SP

RIGHT END VERTICAL IS NOT EXPOSED.

WEB 6-20; 7-20 BRACED AT 1/2 POINTS AS SHOWN ABOVE

Note:Use 1x4 or 2x3 Cont.Bracing conn.w/2-8d nails,or T-brace of same size and grade as web conn. to narrow face w/10d nails 6 in. o.c

TRUSS HAS BEEN CHECKED FOR 145 M.P.H. WIND LOAD, WALL HEIGHT 16 FT, ENCLOSED BLDG. TYPE, ASSUMING 10 PSF TOTAL D.L.(7+3) (SBC-97).

DEFLECTION(IN.) L.L= 0.25,D.L=0.21,T.L=0.46

PLATES ARE MITEK M20-249,200 MANUFACTURED FROM ASTM A 446 GRD A GALVANIZED STEEL(EXCEPT AS SHOWN)

PLATE MUST BE INSTALLED ON EA. FACE OF JOINT,SYMMETRICALLY(EXCEPT AS SHOWN)DESIGN CONFORMS W/NDIS DESIGN SPECS, SSBC,ANSI/TP1-95

THIS DESIGN IS FOR TRUSS FABRICATION ONLY.FOR PERMANENT AND TEMPORARY BRACING(WHICH IS ALWAYS REQD)CONSULT BLDG ARCHITECT OR ENGINEER.

====<<<ACES-32 Ver. 1.1>>>===== [001825] =====<<<FL.QUALITY-MMH>>>=====

Customer : FOGLIA Thu Mar 1 09:14:53 2001

Project #: 00469 Truss ID : S23 Family # : spec.

Span : 39-0 B.Span : 39-0 Quant. : 1 Top Pitch : 6/12

Bot pitch : 90/12

==8ld(1/21/2000),v1.1=====

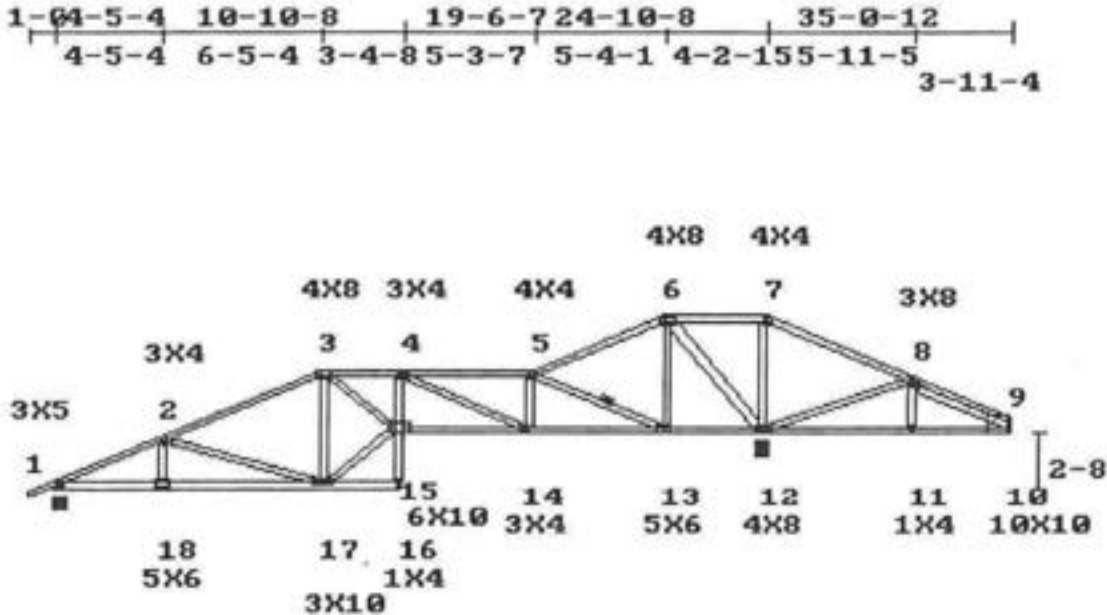
FORCES - GRAVITY LOADS						REACTIONS - SIZE	
1-2=-2440	7-8= 1061	10-11=-315	16-17= 0	2-18= 105	5-14= 526	8-11= 97	1=-1512
2-3=-1766	8-9= 0	11-12=-315	17-18= 2138	2-17=-593	5-13=-2394	8-10= 378	12=-2835
3-4=-3132	9-10=-171	12-13= 234	18-1= 2138	3-17=-1010	6-13= 1217	15-17= 2046	
4-5=-2350		13-14= 2350		3-15= 2066	6-12=-1934		
5-6=-262		14-15= 3131		4-15= 28	7-12=-935		
6-7= 948		15-16= 32		4-14=-886	8-12=-687		

PROVIDE FOR 1467 LBS UPLIFT AT JOINT 1 (0.97)

PROVIDE FOR 4641 LBS UPLIFT AT JOINT 12 (1.64)

PROVIDE FOR 667 LBS HORIZ. REACTION AT JOINT 1

PLATE OFFSETS (X=LEFT,Y=TOP):[j13=3,2],[j18=3,2],



4-5-4 10-10-8 14-3 19-6-7 24-10-8 29-1-7 35-0-11 39-0

4-5-4 6-5-4 3-4-8 5-3-7 5-4-1 4-2-15 5-11-4

L. HL TO PK:12-1-14 INTERNAL RISE:2-8 R. HL TO PK :11-0-9

LEFT HEIGHT:0-4-3 SPAN:39-0 RISE:8-5-8 RIGHT HEIGHT:0-10-3

LOADING (PSF)		MAX STRESSES		MINIMUM GRADE OF LUMBER	
L	D	TOP	BOTT	TOP CHORD	BOTT CHORD
30	15	7-8=0.983	10-11=0.788	2X4 SS 19 SP	2X4 No.1D 19 SP
0	10	LL.DEFL.@16=0.18 < L/360		WEBS :2X4 No.3 19 SP	

STR.INC.: LUMB = 1.33 PLATE = 1.33

REPETITIVE STRESSES USED

SPACING : 24.0 in. o. c.

NO. OF MEMBERS = 1

CHORD: 3-4; 4-5; 5-6; 7-8; 8-9 TO BE 2X4 No.1D 19 SP

CHORD: 6-7; 15-16; 16-17; 18-1 TO BE 2X4 No.2ND 19 SP

CHORD: 11-12; 12-13 TO BE 2X4 SS 19 SP

WEB: 9-10 TO BE 2X4 No.2ND 19 SP

WEB: 6-12 TO BE 2X6 No.2D 19 SP

RIGHT END VERTICAL IS NOT EXPOSED.

WEB 5-13 BRACED AT 1/2 POINTS AS SHOWN ABOVE

Note:Use 1x4 or 2x3 Cont.Bracing conn.w/2-8d nails,or T-brace of same size and grade as web conn. to narrow face w/10d nails 6 in. o.c

TRUSS HAS BEEN CHECKED FOR 145 M.P.H. WIND LOAD, WALL HEIGHT 16 FT, ENCLOSED BLDG. TYPE, ASSUMING 10 PSF TOTAL D.L.(7+3) (SBC-97).

DEFLECTION(IN.) L.L= 0.18,D.L=0.15,T.L=0.34

PLATES ARE MITEK M20-249,200 MANUFACTURED FROM ASTM A 446 GRD A GALVANIZED STEEL(EXCEPT AS SHOWN)

PLATE MUST BE INSTALLED ON EA. FACE OF JOINT,SYMMETRICALLY(EXCEPT AS SHOWN)DESIGN CONFORMS W/ND5 DESIGN SPECS, SSBC,ANSI/TPI-95

THIS DESIGN IS FOR TRUSS FABRICATION ONLY.FOR PERMANENT AND TEMPORARY BRACING(WHICH IS ALWAYS REQD)CONSULT BLDG ARCHITECT OR ENGINEER.

MAR 01 2001

====<<<ACES-32 Ver. 1.1>>>===== [001826] =====<<<FL.QUALITY-MMH>>>=====

Customer : FOGLIA Thu Mar 1 09:15:25 2001

Project #: 00469 Truss ID : S22 Family #: spec.

Span : 39-0 B.Span : 39-0 Quant. : 1 Top Pitch : 6/12

Bot pitch : 90/12

==Bld(1/21/2000),v1.1=====

FORCES - GRAVITY LOADS

FORCES - GRAVITY LOADS				REACTIONS - SIZE				
1-2=2458	7-8= 1032	11-12=-375	17-18= 0	2-18=-315	5-14=-3921	9-12=-423	1=-1565	8.00
2-3=-2097	8-9= 819	12-13=-732	18-1= 2153	3-18=-674	6-14= 1392	9-11= 450	13=-2782	8.00
3-4=-5237	9-10= 0	13-14= 1079		3-16= 3564	6-13=-2532	16-18= 2102		
4-5=-4789	10-11=-171	14-15= 4789		4-16=-135	7-13=-371			
5-6=-1206		15-16= 5247		4-15=-525	8-13=-978			
6-7= 1032		16-17= 52		5-15= 343	8-12= 319			

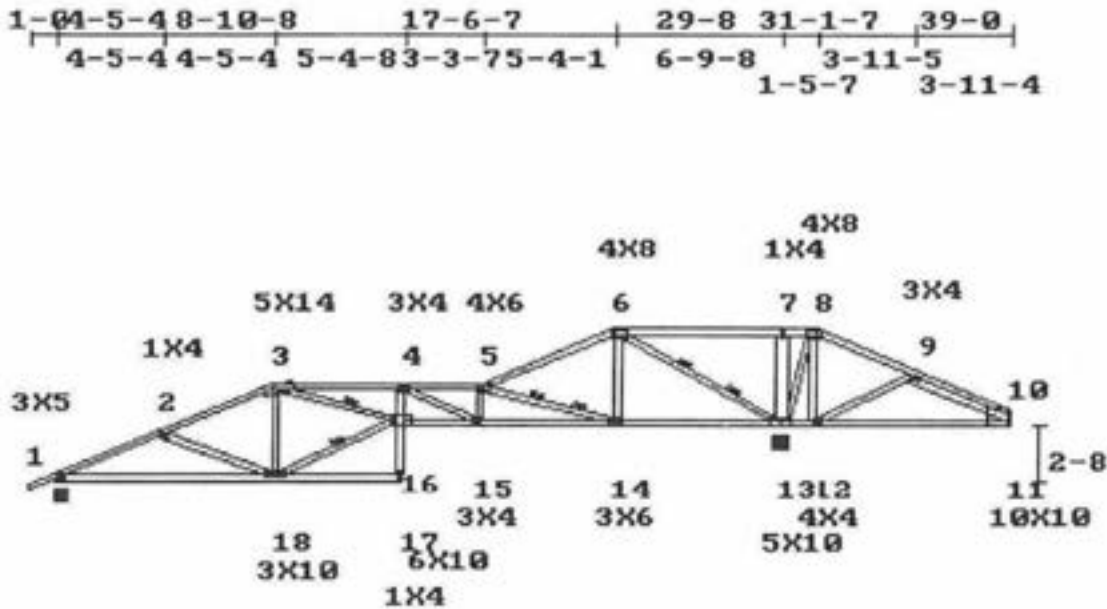
(20)

PROVIDE FOR 1238 LBS UPLIFT AT JOINT 1 (0.79)

PROVIDE FOR 3636 LBS UPLIFT AT JOINT 13 (1.31)

PROVIDE FOR 516 LBS HORIZ. REACTION AT JOINT 1

PLATE OFFSETS (X=LEFT,Y=TOP):(J3=5.5,2),(J13=5,2),



8-10-8		14-3		17-6-7		22-10-8		29-8		31-1-7		39-0	
8-10-8		5-4-83		3-3-75		4-1		6-9-8		7-10-9			

L. HL TO PK:9-11-1 INTERNAL RISE:2-8 R. HL TO PK :8-9-12

LEFT HEIGHT:0-4-3 SPAN:39-0 RISE:7-5-8 RIGHT HEIGHT:0-10-3

LOADING (PSF)		MAX STRESSES		MINIMUM GRADE OF LUMBER	
TOP	30	15	TOP 6-7=0.988	TOP CHORD:2X4	No.2ND 19 SP
BOTT	0	10	BOTT 18-1=0.952	BOT CHORD:2X4	SS 19 SP
		LL.DEFL.@17=0.40 < L/360		WEBS	:2X4 No.3 19 SP

STR.INC.: LUMB = 1.33 PLATE = 1.33 SPACING : 24.0 in. o. c.

REPETITIVE STRESSES USED NO. OF MEMBERS = 1

CHORD: 3-4; 4-5; 6-7; 7-8; 13-14; 14-15; 15-16 TO BE 2X4 No.1D 19 SP

CHORD: 16-17 TO BE 2X4 No.3 19 SP

CHORD: 17-18; 18-1 TO BE 2X4 No.2ND 19 SP

WEB: 3-16 TO BE 2X4 No.2ND 19 SP

WEB: 7-13 TO BE 2X6 No.2D 19 SP

RIGHT END VERTICAL IS NOT EXPOSED.

WEB 3-16; 16-18 BRACED at 1/2 POINTS AS SHOWN ABOVE

WEB 5-14; 6-13 BRACED at 1/3 POINTS AS SHOWN ABOVE

Note:Use 1x4 or 2x3 Cont.Bracing conn.w/2-8d nails,or T-brace of same size and grade as web conn. to narrow face w/10d nails 6 in. o.c

TRUSS HAS BEEN CHECKED FOR 145 M.P.H. WIND LOAD, WALL HEIGHT 16 FT, ENCLOSED BLDG. TYPE, ASSUMING 10 PSF TOTAL D.L.(7+3) (SBC-97).

DEFLECTION(IN.) L.L= 0.40,D.L=0.34,T.L=0.74

PLATES ARE MITEK M20-249,200 MANUFACTURED FROM ASTM A 446 GRD A GALVANIZED STEEL(EXCEPT AS SHOWN)

PLATE MUST BE INSTALLED ON EA. FACE OF JOINT,SYMMETRICALLY(EXCEPT AS SHOWN)DESIGN CONFORMS W/NDG DESIGN SPECS, SSBG,ANSI/TPI-95

THIS DESIGN IS FOR TRUSS FABRICATION ONLY,FOR PERMANENT AND TEMPORARY BRACING(WHICH IS ALWAYS REQD)CONSULT BLDG ARCHITECT OR ENGINEER.

MAR 01 2001

====<<<<ACES-32 Ver. 1.1>>>>===== [001994] =====<<<<FL.QUALITY-MMH>>>>=====

Customer : FOGLIA

Mon Mar 5 12:16:00 2001

Project #: 00469

Truss ID : HH2

Family # : 313

Span : 14-0

B.Span : 14-4

Quant. : 2

Top Pitch : 6/12

==Bld(1/21/2000),v1.1=====

FORCES - LOAD CASE #1

REACTIONS - SIZE

1-2=2176 5-6=1381 2-7=903
2-3=1381 6-7=1919 2-6=754
3-4=0 7-1=1919 3-6=683
4-5=348 3-5=1957

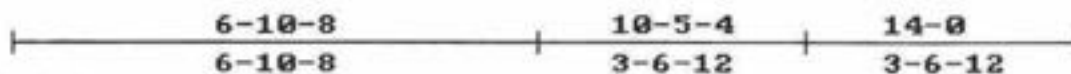
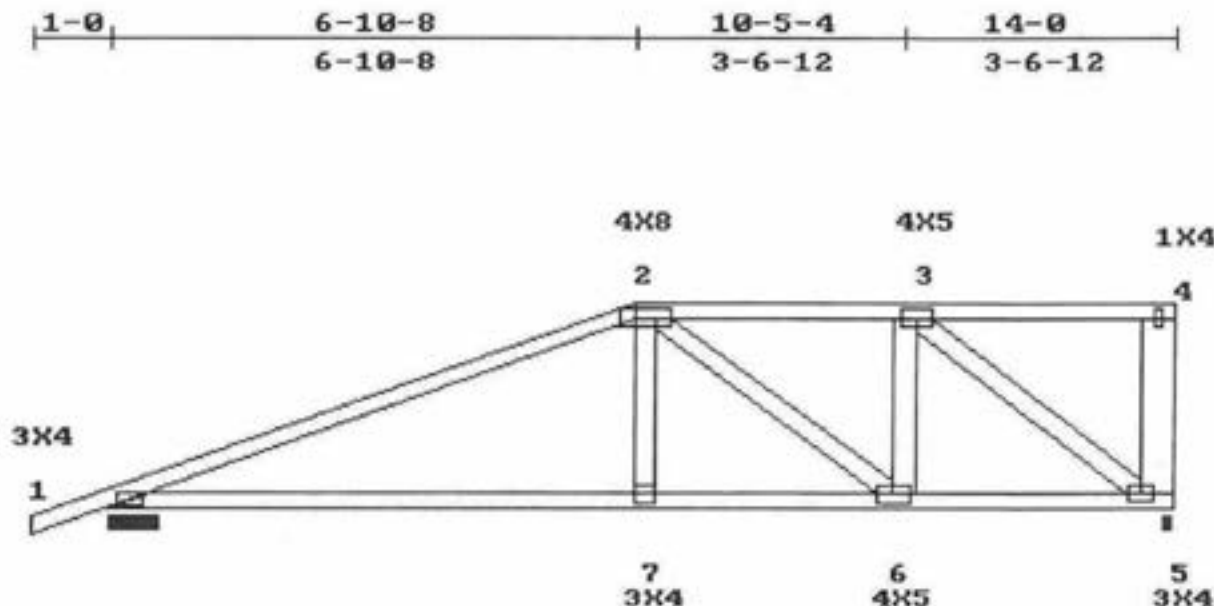
1=1385 8.00
5=1811 2.03

22

PROVIDE FOR 1982 LBS UPLIFT AT JOINT 1 (1.43)

PROVIDE FOR 2346 LBS UPLIFT AT JOINT 5 (1.30)

PROVIDE FOR 410 LBS HORIZ. REACTION AT JOINT 1



L. HL TO PK:7-8-4

LEFT HEIGHT:0-4-3

SPAN:14-0

RISE:3-9-7

RIGHT HEIGHT:3-9-7

LOADING (PSF)

MAX STRESSES

MINIMUM GRADE OF LUMBER

TOP 30 15

TOP 1-2=0.676

TOP CHORD:2X4 No.2ND 19 SP

BOTT 0 10

BOTT 6-7=0.385

BOT CHORD:2X4 No.2ND 19 SP

LL.DEFL.@7=0.03 < L/360

WEBS :2X4 No.3 19 SP

REPETITIVE STRESSES NOT USED

SPACING : 24.0 in. o. c.

NO. OF MEMBERS = 2

LOADING STRESS INCREASE

LOADING

PANEL(PLF) / JOINTS(LBS)

LUMBER PLATE

TYPE

1 1.33 1.33

UNIFORM

1- 2= 90

2- 4= 200

5- 7= 44

7- 1= 20

CONCENTRATED

7= 759

WEB: 4-5 TO BE 2X6 No.2D 19 SP

RIGHT END VERTICAL IS NOT EXPOSED.

2 MEMBERS NAILED TOG. W/1 ROW(S) OF .131x3 in. NAILS 12 in. o.c.(TOP CHS.),AND 1 ROW(S) OF .131x3 in. NAILS 12 in. o.c.(BOTT. CHS.)

For Webs use 1 ROW OF NAILS 12 in. o.c.

TRUSS HAS BEEN CHECKED FOR 145 M.P.H. WIND LOAD, WALL HEIGHT 16 FT, ENCLOSED BLDG. TYPE, ASSUMING 10 PSF TOTAL D.L.(7+3) (SBC-97).

DEFLECTION(IN.) L.L= 0.03,D.L=0.02,T.L=0.05

PLATES ARE MITEK M20-249,200 MANUFACTURED FROM ASTM A 446 GRD A GALVANIZED STEEL(EXCEPT AS SHOWN)

PLATE MUST BE INSTALLED ON EA. FACE OF JOINT,SYMMETRICALLY(EXCEPT AS SHOWN)DESIGN CONFORMS W/NDS DESIGN SPECS, SSB,ANSI/TPI-95

THIS DESIGN IS FOR TRUSS FABRICATION ONLY.FOR PERMANENT AND TEMPORARY BRACING(WHICH IS ALWAYS REQD)CONSULT BLDG ARCHITECT OR ENGINEER.

MAR 06 2001

====<<<ACES-32 Ver. 1.1>>>===== [001828] =====<<<FL.QUALITY-MMH>>>=====

Customer : FOGLIA

Project # : 00469

Span : 9-8-11

Truss ID : CJ9

Quantity : 3

Thu Mar 1 09:17:12 2001

Family # : 204

Top Pitch : 4.243/12

==Bld(1/21/2000),v1.1=====

FORCES - LOAD CASE #1

REACTIONS - SIZE

1-2=749 4-1= 699 2-4=744

1=674 8.00

2-3= 0

4=763 1.66

3-4=392

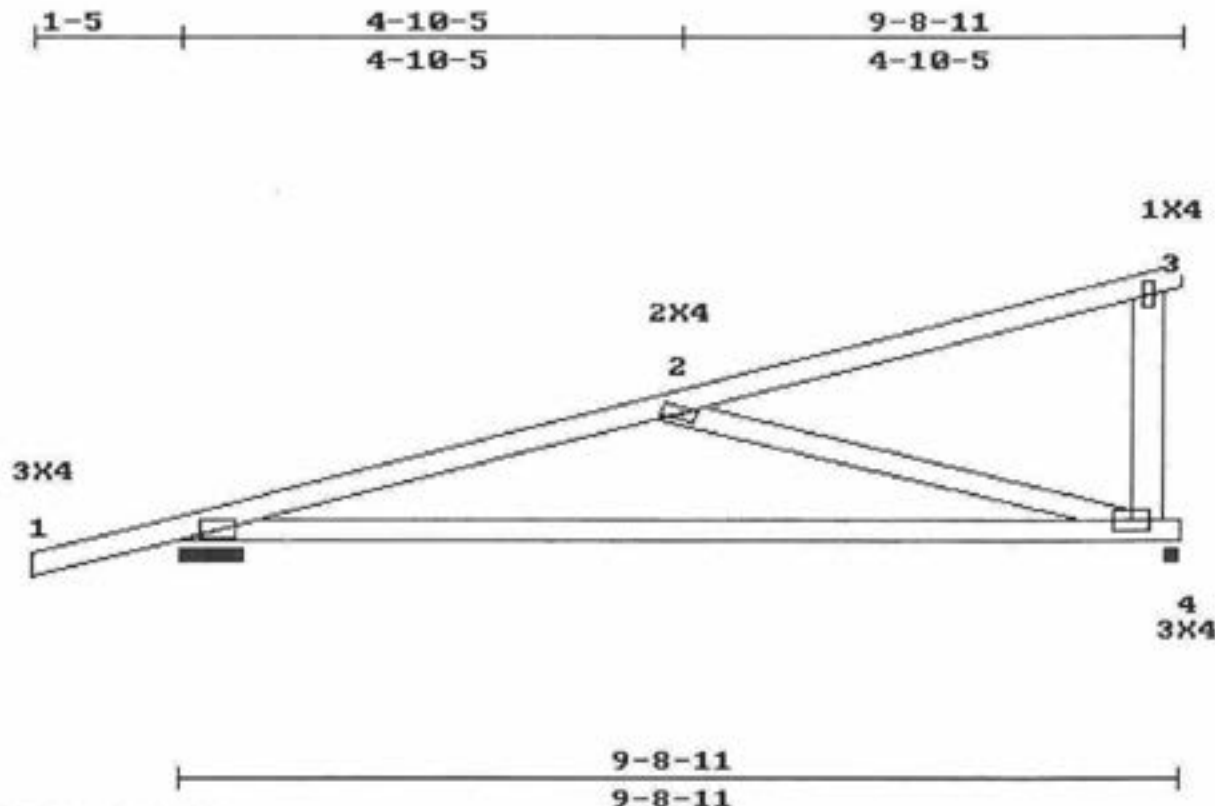
23

PROVIDE FOR 974 LBS UPLIFT AT JOINT 1 (1.45)

PROVIDE FOR 1195 LBS UPLIFT AT JOINT 4 (1.57)

PROVIDE FOR 401 LBS HORIZ. REACTION AT JOINT 1

(SYMMETRIC PLATING TURNED ON)



L. HL TO PK:10-3-12

LEFT HEIGHT:0-3-15

SPAN:9-8-11

RISE:3-9-3

RIGHT HEIGHT:3-9-3

LOADING (PSF)

MAX STRESSES

MINIMUM GRADE OF LUMBER

TOP 30 15

TOP 2-3=0.724

TOP CHORD:2X4 No.2ND 19 SP

BOTT 0 10

BOTT 4-1=0.878

BOT CHORD:2X4 No.2ND 19 SP

LL.DEFL.@0=0.00 < L/360

WEBS :2X4 No.3 19 SP

REPETITIVE STRESSES NOT USED

SPACING : 24.0 in. o. c.

NO. OF MEMBERS = 1

LOADING STRESS INCREASE

LOADING

PANEL(PLF) / JOINTS(LBS)

LUMBER PLATE

TYPE

1 1.33 1.33 UNIFORM 1- 2= 58 2- 3= 163 4- 1= 25

RIGHT END VERTICAL IS NOT EXPOSED.

TRUSS HAS BEEN CHECKED FOR 145 M.P.H. WIND LOAD, WALL HEIGHT 16 FT, ENCLOSED BLDG. TYPE, ASSUMING 10 PSF TOTAL D.L.(7+3) (SBC-97).

DEFLECTION(IN.) L.L= 0.00,D.L=0.00,T.L=0.00

PLATES ARE MITEK M20-249,200 MANUFACTURED FROM ASTM A 446 GRD A GALVANIZED STEEL(EXCEPT AS SHOWN)

PLATE MUST BE INSTALLED ON EA. FACE OF JOINT,SYMMETRICALLY(EXCEPT AS SHOWN)DESIGN CONFORMS W/NDIS DESIGN SPECS, SSSC,ANSI/TP1-95

THIS DESIGN IS FOR TRUSS FABRICATION ONLY.FOR PERMANENT AND TEMPORARY BRACING(WHICH IS ALWAYS REQD)CONSULT BLDG ARCHITECT OR ENGINEER.

[Signature]
MAR 01 2001

====<<<ACES-32 Ver. 1.1>>>===== [001829] =====<<<FL.QUALITY-MMH>>>=====

Customer : FOGLIA	Truss ID : J6	Thu Mar 1 09:17:22 2001
Project #: 00469	Quantity : 11	Family # : 205
Span : 6-10-8		Top Pitch : 6/12

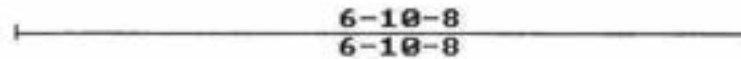
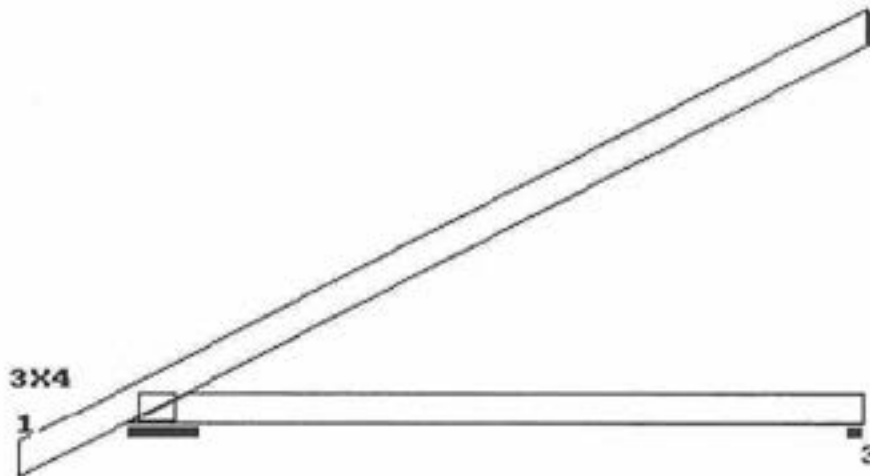
==Bld(1/21/2000),v1.1=====

FORCES - GRAVITY LOADS
 1-2= 0 3-1= 0
 2-3= 0

REACTIONS - SIZE
 1--466 8.00
 3--65 1.50
 2--292 1.50

(24)

PROVIDE FOR 337 LBS UPLIFT AT JOINT 1 (0.72)
 PROVIDE FOR 441 LBS UPLIFT AT JOINT 2 (1.51)
 PROVIDE FOR 400 LBS HORIZ. REACTION AT JOINT 1
 (SYMMETRIC PLATING TURNED ON)



L. HL TO PK:7-8-4 SPAN:6-10-8 RISE:3-9-7 RIGHT HEIGHT:3-9-7
 LEFT HEIGHT:0-4-3

LOADING (PSF)		MAX STRESSES	MINIMUM GRADE OF LUMBER
L	D	TOP 1-2=0.961	TOP CHORD:2X4 No.1D 19 SP
30	15	BOTT 3-1=0.203	BOT CHORD:2X4 No.2ND 19 SP
BOTT 0	10	LL.DEFL. < L/360	WEBS :2X4 No.3 19 SP

STR.INC.: LUMB = 1.33 PLATE = 1.33 SPACING : 24.0 in. o. c.
 REPETITIVE STRESSES USED NO. OF MEMBERS = 1

TRUSS HAS BEEN CHECKED FOR 145 M.P.H. WIND LOAD, WALL HEIGHT 16 FT, ENCLOSED BLDG. TYPE, ASSUMING 10 PSF TOTAL D.L.(7+3) (SBC-97).
 DEFLECTION(IN.) L.L= 0.00,D.L=0.00,T.L=0.00
 PLATES ARE MITEK M20-249,200 MANUFACTURED FROM ASTM A 446 GRD A GALVANIZED STEEL(EXCEPT AS SHOWN)
 PLATE MUST BE INSTALLED ON EA. FACE OF JOINT,SYMMETRICALLY(EXCEPT AS SHOWN)DESIGN CONFORMS W/NDG DESIGN SPECS, SBC,ANSI/TPI-95
 THIS DESIGN IS FOR TRUSS FABRICATION ONLY.FOR PERMANENT AND TEMPORARY BRACING(WHICH IS ALWAYS REQD)CONSULT BLDG ARCHITECT OR ENGINEER.

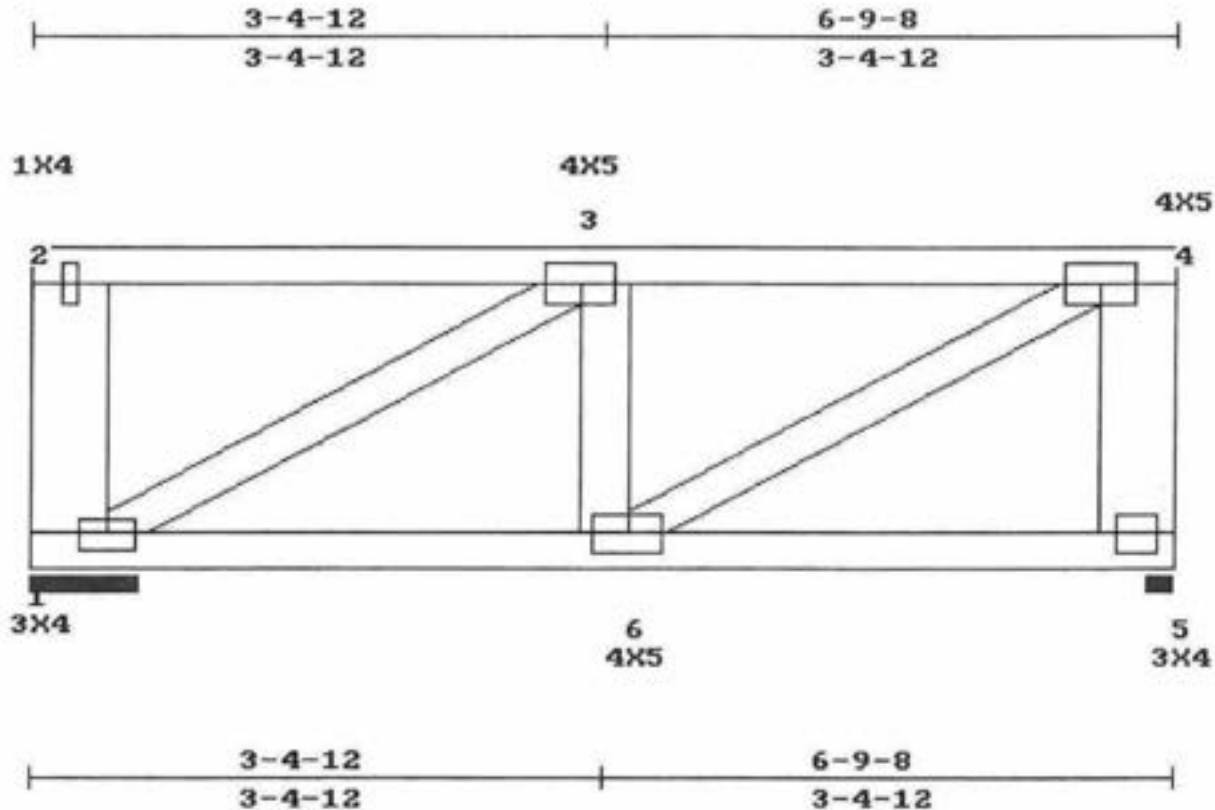
[Signature]
 MAR 01 2001

====<<<<ACES-32 Ver. 1.1>>>>=====[001996]====<<<<FL.QUALITY-MMH>>>>=====
 Customer : FOGLIA Mon Mar 5 12:20:24 2001
 Project #: 00469 Truss ID : FG2 Family #: 301
 Span : 6-9-8 B.Span : 6-9-8 Quant. : 2 Top Pitch : /12
 ==Bld(1/21/2000),v1.1=====

FORCES - LOAD CASE #1			REACTIONS - SIZE	
1-2=-459	5-6= 0	3-1=-850	1=-998	8.00
2-3= 0	6-1= 680	3-6=-429	5=-998	2.03
3-4=-680		4-6= 837		
4-5=-968				

(25)

PROVIDE FOR 1214 LBS UPLIFT AT JOINT 1 (1.22)
 PROVIDE FOR 1214 LBS UPLIFT AT JOINT 5 (1.22)



LEFT HEIGHT:2-8	SPAN:6-9-8	RISE:2-8	RIGHT HEIGHT:2-8

LOADING (PSF)	MAX STRESSES	MINIMUM GRADE OF LUMBER
TOP 30 15	TOP 3-4=0.376	TOP CHORD:2X4 No.2ND 19 SP
BOTT 0 10	BOTT 6-1=0.086	BOT CHORD:2X4 No.2ND 19 SP
	LL.DEFL.@6=0.00 < L/360	WEBS :2X4 No.3 19 SP

REPETITIVE STRESSES NOT USED

SPACING : 24.0 in. o. c.
 NO. OF MEMBERS = 2

LOADING STRESS INCREASE	LOADING	PANEL(PLF) / JOINTS(LBS)
LUMBER PLATE TYPE		
1 1.33 1.33 UNIFORM		2- 4= 290 5- 1= 18

WEB: 1-2; 4-5 TO BE 2X6 No.2D 19 SP
 BOTH END VERTICALS ARE NOT EXPOSED.
 2 MEMBERS NAILED TOG. W/1 ROW(S) OF .131x3 in. NAILS 9 in. o.c.(TOP CHS.),AND 1 ROW(S) OF .131x3 in. NAILS 12 in. o.c.(BOTT. CHS.)
 For Webs use 1 ROW of NAILS 12 in. o.c.
 TRUSS HAS BEEN CHECKED FOR 145 M.P.H. WIND LOAD, WALL HEIGHT 16 FT, ENCLOSED BLDG. TYPE, ASSUMING 10 PSF TOTAL D.L.(7+3) (SBC-97).
 DEFLECTION(IN.) L.L= 0.00,D.L=0.00,T.L=0.01
 PLATES ARE MITEK M20-249,200 MANUFACTURED FROM ASTM A 446 GRD A GALVANIZED STEEL(EXCEPT AS SHOWN)
 PLATE MUST BE INSTALLED ON EA. FACE OF JOINT,SYMMETRICALLY(EXCEPT AS SHOWN)DESIGN CONFORMS W/NDG DESIGN SPECS, SSBC,ANSI/TPI-95
 THIS DESIGN IS FOR TRUSS FABRICATION ONLY.FOR PERMANENT AND TEMPORARY BRACING(WHICH IS ALWAYS REQD)CONSULT BLDG ARCHITECT OR ENGINEER.

MAR 06 2001

====<<<ACES-32 Ver. 1.1>>>===== [001831] =====<<<FL.QUALITY-MMH>>>=====

Customer : FOGLIA Thu Mar 1 09:17:55 2001

Project #: 00469 Truss ID : C4 Family #: 205

Span : 5-0 Quantity : 1 Top Pitch : 6/12

==Bld(1/21/2000),v1.1=====

FORCES - GRAVITY LOADS

1-2= 0
2-3= 0

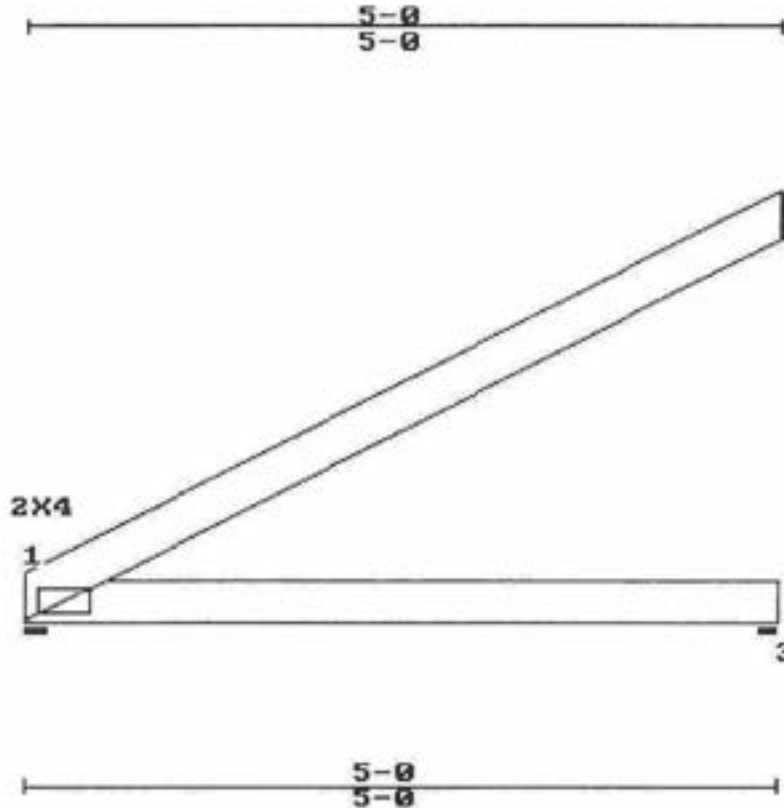
3-1= 0

REACTIONS - SIZE

1=-267 2.00
3=-49 1.50
2=-218 1.50

(24)

PROVIDE FOR 163 LBS UPLIFT AT JOINT 1 (0.61)
PROVIDE FOR 325 LBS UPLIFT AT JOINT 2 (1.49)
PROVIDE FOR 289 LBS HORIZ. REACTION AT JOINT 1
(SYMMETRIC PLATING TURNED ON)



L. HL TO PK:5-7-1
LEFT HEIGHT:0-4-3

SPAN:5-0

RISE:2-10-3

RIGHT HEIGHT:2-10-3

LOADING (PSF)

	L	D
TOP	30	15
BOTT	0	10

MAX STRESSES

TOP	1-2=0.702
BOTT	3-1=0.103
LL.DEFL.	< L/360

MINIMUM GRADE OF LUMBER

TOP CHORD:2X4	No.2ND 19 SP
BOT CHORD:2X4	No.2ND 19 SP
WEBS	:2X4 No.3 19 SP

STR.INC.: LUMB = 1.33 PLATE = 1.33
REPETITIVE STRESSES USED

SPACING : 24.0 in. o. c.
NO. OF MEMBERS = 1

TRUSS HAS BEEN CHECKED FOR 145 M.P.H. WIND LOAD, WALL HEIGHT 16 FT, ENCLOSED BLDG. TYPE, ASSUMING 10 PSF TOTAL D.L.(7+3) (SBC-97).
DEFLECTION(IN.) L.L= 0.00,D.L=0.00,T.L=0.00

PLATES ARE MITEK M20-249,200 MANUFACTURED FROM ASTM A 446 GRD A GALVANIZED STEEL(EXCEPT AS SHOWN)

PLATE MUST BE INSTALLED ON EA. FACE OF JOINT,SYMMETRICALLY(EXCEPT AS SHOWN)DESIGN CONFORMS W/ND S DESIGN SPECS, SSSC,ANSI/TP1-95

THIS DESIGN IS FOR TRUSS FABRICATION ONLY.FOR PERMANENT AND TEMPORARY BRACING(WHICH IS ALWAYS REQD)CONSULT BLDG ARCHITECT OR ENGINEER.

MAR 01 2001

====<<<ACES-32 Ver. 1.1>>>===== [001832] =====<<<FL.QUALITY-MMH>>>=====

Customer : FOGLIA Thu Mar 1 09:18:20 2001

Project #: 00469 Truss ID : NG2 Family # : 301

Span : 9-7 Quantity : 2 Top Pitch : /12

==Bld(1/21/2000),v1.1=====

FORCES - LOAD CASE #1				REACTIONS - SIZE	
1-2=-229	7-8=-1158	8-9= 0	3-1=-1950	6-10= 943	1=-1168 2.00
2-3= 0		9-10= 1714	3-12= 480	6-9=-911	8=-1168 2.00
3-4=-1714		10-11= 2536	4-12=-943	7-9= 1950	
4-5=-2536		11-12= 2536	4-11= 19		
5-6=-2536		12-1= 1714	5-11= 0		
6-7=-1714			5-10=-442		

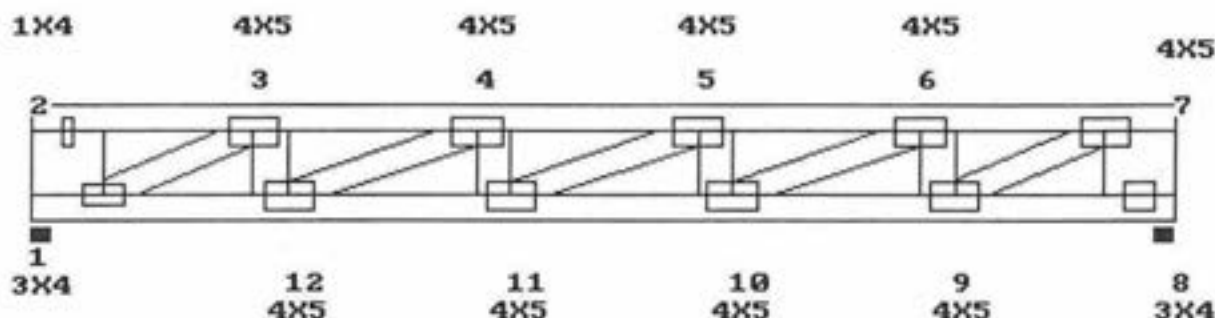
(27)

PROVIDE FOR 1510 LBS UPLIFT AT JOINT 1 (1.29)

PROVIDE FOR 1510 LBS UPLIFT AT JOINT 8 (1.29)

(SYMMETRIC PLATING TURNED ON)

2-0-1	3-10-6	5-8-10	7-6-15	9-7
2-0-1	1-10-5	1-10-5	1-10-5	2-0-1



2-0-1	3-10-6	5-8-10	7-6-15	9-7
2-0-1	1-10-5	1-10-5	1-10-5	2-0-1

LEFT HEIGHT:1-4 SPAN:9-7 RISE:1-4 RIGHT HEIGHT:1-4

LOADING (PSF)		MAX STRESSES		MINIMUM GRADE OF LUMBER	
TOP	L 30 D 15	TOP	5-6=0.345	TOP CHORD:2X4	No.2ND 19 SP
BOTT	0 10	BOTT	11-12=0.239	BOT CHORD:2X4	No.2ND 19 SP
		LL.DEFL.@11=0.03 < L/360	WEBS	:2X4	No.3 19 SP

REPETITIVE STRESSES NOT USED

SPACING : 24.0 in. o. c.
NO. OF MEMBERS = 2

LOADING	STRESS INCREASE	LOADING	PANEL(PLF) / JOINTS(LBS)
LUMBER	PLATE	TYPE	
1	1.33	1.33	2- 7= 238 8- 1= 10

WEB: 1-2; 7-8 TO BE 2X8 SS 19 SP

BOTH END VERTICALS ARE NOT EXPOSED.

2 MEMBERS NAILED TOG. W/1 ROW(S) OF .131x3 in. NAILS 11 in. o.c.(TOP CHS.),AND 1 ROW(S) OF .131x3 in. NAILS 12 in. o.c.(BOTT. CHS.)

For Webs use 1 ROW of NAILS 12 in. o.c.

TRUSS HAS BEEN CHECKED FOR 145 M.P.H. WIND LOAD, WALL HEIGHT 16 FT, ENCLOSED BLDG. TYPE, ASSUMING 10 PSF TOTAL D.L.(7+3) (SBC-97).

DEFLECTION(IN.) L.L= 0.03,D.L=0.03,T.L=0.06

PLATES ARE MITEK M20-249,200 MANUFACTURED FROM ASTM A 446 GRD A GALVANIZED STEEL(EXCEPT AS SHOWN)

PLATE MUST BE INSTALLED ON EA. FACE OF JOINT,SYMMETRICALLY(EXCEPT AS SHOWN)DESIGN CONFORMS W/ND5 DESIGN SPECS, 558C,ANSI/TPI-95

THIS DESIGN IS FOR TRUSS FABRICATION ONLY.FOR PERMANENT AND TEMPORARY BRACING(WHICH IS ALWAYS REQD)CONSULT BLDG ARCHITECT OR ENGINEER.

MAR 01 2001

[Signature]

====<<<<ACES-32 Ver. 1.1>>>>===== [001833] =====<<<<FL.QUALITY-MMH>>>>=====

Customer : FOGLIA

Project # : 00469

Span : 1-0

Truss ID : C1A

Quantity : 2

Thu Mar 1 09:18:32 2001

Family # : 205

Top Pitch : 6/12

==Bld(1/21/2000),v1.1=====

FORCES - GRAVITY LOADS

REACTIONS - SIZE

1-2= 271 3-1= 0

3-10 1.50

2-3= 0

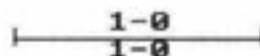
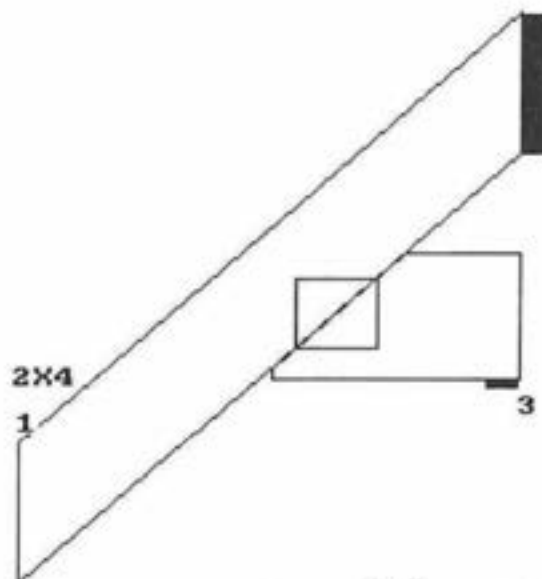
2-159 1.50

PROVIDE FOR 60 LBS UPLIFT AT JOINT 3 (6.00)

PROVIDE FOR 281 LBS UPLIFT AT JOINT 2 (1.76)

PROVIDE FOR 146 LBS HORIZ. REACTION AT JOINT 3

(SYMMETRIC PLATING TURNED ON)



L. HL TO PK:1-1-7

LEFT HEIGHT:0-4-3

SPAN:1-0

RISE:0-10-3

RIGHT HEIGHT:0-10-3

LOADING (PSF)

L D

MAX STRESSES

TOP 1-2=0.055

BOTT 3-1=0.040

LL.DEFL. < L/360

MINIMUM GRADE OF LUMBER

TOP CHORD:2X4 No.2ND 19 SP

BOT CHORD:2X4 No.2ND 19 SP

WEBS :2X4 No.3 19 SP

STR.INC.: LUMB = 1.33 PLATE = 1.33

REPETITIVE STRESSES USED

SPACING : 24.0 in. o. c.

NO. OF MEMBERS = 1

TRUSS HAS BEEN CHECKED FOR 145 M.P.H. WIND LOAD, WALL HEIGHT 16 FT, ENCLOSED BLDG. TYPE, ASSUMING 10 PSF TOTAL D.L.(7+3) (SBC-97).

DEFLECTION(IN.) L.L= 0.00,D.L=0.00,T.L=0.00

PLATES ARE MITEK M20-249,200 MANUFACTURED FROM ASTM A 446 GRD A GALVANIZED STEEL(EXCEPT AS SHOWN)

PLATE MUST BE INSTALLED ON EA. FACE OF JOINT,SYMMETRICALLY(EXCEPT AS SHOWN)DESIGN CONFORMS W/NOS DESIGN SPECS, SSBC,ANSI/TP1-95

THIS DESIGN IS FOR TRUSS FABRICATION ONLY.FOR PERMANENT AND TEMPORARY BRACING(WHICH IS ALWAYS REQD)CONSULT BLDG ARCHITECT OR ENGINEER.

MAR 01 2001

====<<<ACES-32 Ver. 1.1>>>===== [001834] =====<<<FL.QUALITY-MMH>>>=====

Customer : FOGLIA

Project # : 00469

Span : 3-0

Truss ID : C3A

Quantity : 2

Thu Mar 1 09:18:43 2001

Family # : 205

Top Pitch : 6/12

==Bld(1/21/2000),v1.1=====

FORCES - GRAVITY LOADS

1-2= 358 3-1=148

2-3= 0

REACTIONS - SIZE

3=30 1.50

2=292 1.50

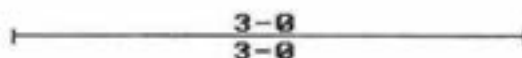
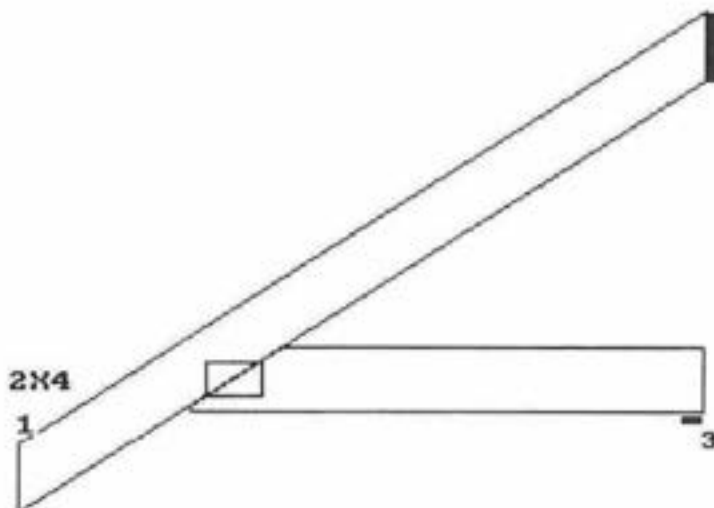
(2a)

PROVIDE FOR 180 LBS UPLIFT AT JOINT 3 (6.00)

PROVIDE FOR 470 LBS UPLIFT AT JOINT 2 (1.61)

PROVIDE FOR 593 LBS HORIZ. REACTION AT JOINT 3

(SYMMETRIC PLATING TURNED ON)



L. HL TO PK:3-4-4

LEFT HEIGHT:0-4-3

SPAN:3-0

RISE:1-10-3

RIGHT HEIGHT:1-10-3

LOADING (PSF)

L D

TOP 30 15

BOTT 0 10

MAX STRESSES

TOP 1-2=0.237

BOTT 3-1=0.312

LL.DEFL. < L/360

MINIMUM GRADE OF LUMBER

TOP CHORD:2X4 No.2ND 19 SP

BOT CHORD:2X4 No.2ND 19 SP

WEBS :2X4 No.3 19 SP

STR.INC.: LUMB = 1.33 PLATE = 1.33

REPETITIVE STRESSES USED

SPACING : 24.0 in. o. c.

NO. OF MEMBERS = 1

TRUSS HAS BEEN CHECKED FOR 145 M.P.H. WIND LOAD, WALL HEIGHT 16 FT, ENCLOSED BLDG. TYPE, ASSUMING 10 PSF TOTAL D.L.(7+3) (SBC-97).

DEFLECTION(IN.) L.L= 0.00,D.L=0.00,T.L=0.00

PLATES ARE MITEK M20-249,200 MANUFACTURED FROM ASTM A 446 GRD A GALVANIZED STEEL(EXCEPT AS SHOWN)

PLATE MUST BE INSTALLED ON EA. FACE OF JOINT,SYMMETRICALLY(EXCEPT AS SHOWN)DESIGN CONFORMS W/NDIS DESIGN SPECS, SSSC,ANSI/TPI-95

THIS DESIGN IS FOR TRUSS FABRICATION ONLY.FOR PERMANENT AND TEMPORARY BRACING(WHICH IS ALWAYS REQD)CONSULT BLDG ARCHITECT OR ENGINEER.

[Signature]

MAR 01 2001

====<<<ACES-32 Ver. 1.1>>>=====[001835]=====<<<FL.QUALITY-MMH>>>=====

Customer : FOGLIA

Project # : 00469

Span : 5-0

Truss ID : C5A

Quantity : 2

Thu Mar 1 09:18:53 2001

Family # : 205

Top Pitch : 6/12

==Bld(1/21/2000),v1.1=====

FORCES - GRAVITY LOADS

1-2=1717 3-1=1190

2-3= 0

REACTIONS - SIZE

3=50 1.50

2=984 1.50

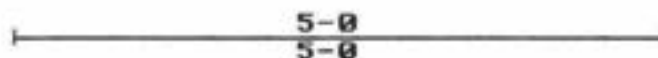
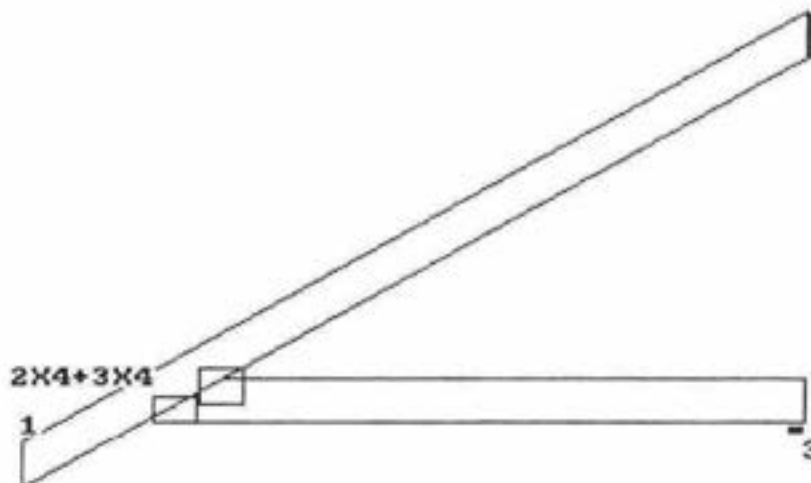
(20)

PROVIDE FOR 300 LBS UPLIFT AT JOINT 3 (6.00)

PROVIDE FOR 934 LBS UPLIFT AT JOINT 2 (0.95)

PROVIDE FOR 1189 LBS HORIZ. REACTION AT JOINT 3

(SYMMETRIC PLATING TURNED ON)



L. HL TO PK:5-7-1

LEFT HEIGHT:0-4-3

SPAN:5-0

RISE:2-10-3

RIGHT HEIGHT:2-10-3

LOADING (PSF)

L D

TOP 30 15

BOTT 0 10

MAX STRESSES

TOP 1-2=0.742

BOTT 3-1=0.620

LL.DEFL. < L/360

MINIMUM GRADE OF LUMBER

TOP CHORD:2X4 No.2ND 19 SP

BOT CHORD:2X4 No.2ND 19 SP

WEBS :2X4 No.3 19 SP

STR.INC.: LUMB = 1.33 PLATE = 1.33

REPETITIVE STRESSES USED

SPACING : 24.0 in. o. c.

NO. OF MEMBERS = 1

TRUSS HAS BEEN CHECKED FOR 145 M.P.H. WIND LOAD, WALL HEIGHT 16 FT, ENCLOSED BLDG. TYPE, ASSUMING 10 PSF TOTAL D.L.(7*3) (SBC-97).

DEFLECTION(IN.) L.L.= 0.00,D.L.=0.00,T.L.=0.00

PLATES ARE MITEK M20-249,200 MANUFACTURED FROM ASTM A 446 GRD A GALVANIZED STEEL(EXCEPT AS SHOWN)

PLATE MUST BE INSTALLED ON EA. FACE OF JOINT,SYMMETRICALLY(EXCEPT AS SHOWN)DESIGN CONFORMS W/NDS DESIGN SPECS, SSBC,ANSI/TPI-95

THIS DESIGN IS FOR TRUSS FABRICATION ONLY.FOR PERMANENT AND TEMPORARY BRACING(WHICH IS ALWAYS REQD)CONSULT BLDG ARCHITECT OR ENGINEER.

[Signature]
MAR 01 2001

====<<<<ACES-32 Ver. 1.1>>>>===== [001993] =====<<<<FL.QUALITY-MMH>>>>=====

Customer : FOGLIA Mon Mar 5 12:14:23 2001

Project #: 00469 Truss ID : H4 Family #: 314

Span : 25-11-8 B.Span : 25-11-8 Quant. : 2 Top Pitch : 6/12

==Bld(1/21/2000),v1.1=====

FORCES - LOAD CASE #1

1-2=1592	7-8=652	2-11=188	5-8=1050
2-3=1416	8-9=961	3-11=2313	6-8=346
3-4=592	9-10=2643	3-10=2074	
4-5=2643	10-11=592	4-10=3065	
5-6=1076	11-1=1416	4-9=2888	
6-7=733		5-9=2603	

REACTIONS - SIZE

1=1102	2.00
9=5008	5.66

31

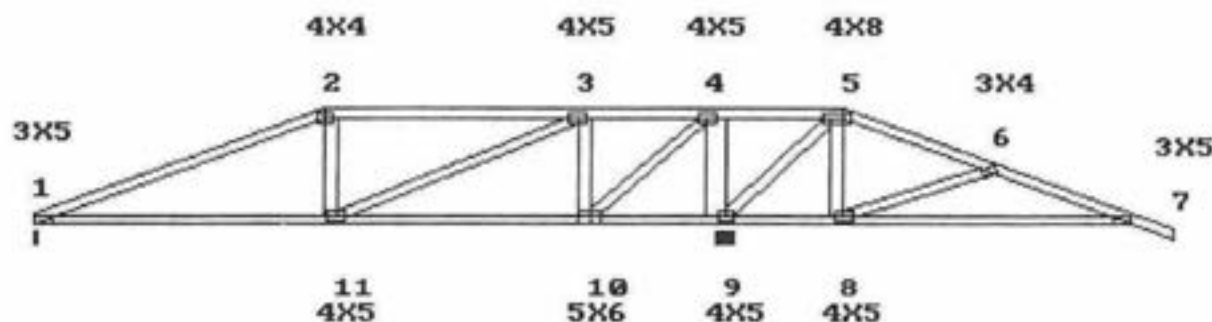
PROVIDE FOR 1163 LBS UPLIFT AT JOINT 1 (1.06)

PROVIDE FOR 8145 LBS UPLIFT AT JOINT 9 (1.63)

PROVIDE FOR 117 LBS HORIZ. REACTION AT JOINT 1

PLATE OFFSETS (X=LEFT,Y=TOP):(1)10=3,2),

6-10-8	12-11-12	16-1-8	19-1	22-6-4	25-11-8	1-0
6-10-8	6-1-4	3-1-12	2-11-8	3-5-4	3-5-4	



6-10-8	12-11-12	16-1-8	19-1	25-11-8
6-10-8	6-1-4	3-1-12	2-11-8	6-10-8

L. HL TO PK:7-8-4 LEFT HEIGHT:0-4-3 SPAN:25-11-8 RISE:3-9-7 R. HL TO PK:7-8-4 RIGHT HEIGHT:0-4-3

LOADING (PSF)	MAX STRESSES	MINIMUM GRADE OF LUMBER
L D	TOP 3-4=0.878	TOP CHORD:2X4 No.2ND 19 SP
TOP 30 15	BOTT 7-8=0.714	BOT CHORD:2X4 No.2ND 19 SP
BOTT 0 10	LL.DEFL.@7=0.14 < L/360	WEBS :2X4 No.3 19 SP

REPETITIVE STRESSES NOT USED

SPACING : 24.0 in. o. c.

NO. OF MEMBERS = 2

LOADING	STRESS INCREASE	LOADING	PANEL(PLF) / JOINTS(LBS)
LUMBER	PLATE	TYPE	
1	1.33	1.33	UNIFORM 1- 2= 90 2- 5= 200 5- 7= 90 7- 8= 20 8-11= 44 11- 1= 20
			CONCENTRATED 8= 759 11= 759

WEB: 4-9 TO BE 2X6 No.20 19 SP

2 MEMBERS NAILED TOG. W/1 ROW(S) OF .131x3 in. NAILS 12 in. o.c.(TOP CHS.),AND 1 ROW(S) OF .131x3 in. NAILS 12 in. o.c(BOTT. CHS.)

For Webs use 1 ROW of NAILS 12 in. o.c.

TRUSS HAS BEEN CHECKED FOR 145 M.P.H. WIND LOAD, WALL HEIGHT 16 FT, ENCLOSED BLDG. TYPE, ASSUMING 10 PSF TOTAL D.L.(7+3) (SBC-97).

DEFLECTION(IN.) L.L= 0.14,D.L=0.11,T.L=0.25

PLATES ARE MITEK M20-249,200 MANUFACTURED FROM ASTM A 446 GRD A GALVANIZED STEEL(EXCEPT AS SHOWN)

PLATE MUST BE INSTALLED ON EA. FACE OF JOINT,SYMMETRICALLY(EXCEPT AS SHOWN)DESIGN CONFORMS W/ND S DESIGN SPECS, SSBC,ANSI/TPI-95

THIS DESIGN IS FOR TRUSS FABRICATION ONLY.FOR PERMANENT AND TEMPORARY BRACING(WHICH IS ALWAYS REQD)CONSULT BLDG ARCHITECT OR ENGINEER.

MAR 06 2001

====<<<ACES-32 Ver. 1.1>>>=====[001837]====<<<FL.QUALITY-MMH>>>=====
 Customer : FOGLIA Thu Mar 1 09:19:58 2001
 Project #: 00469 Family #: 301
 Span : 7-0 B.Span : 7-0 Quant. : 2 Top Pitch : /12
 ==Bld(1/21/2000),v1.1=====

FORCES - LOAD CASE #1

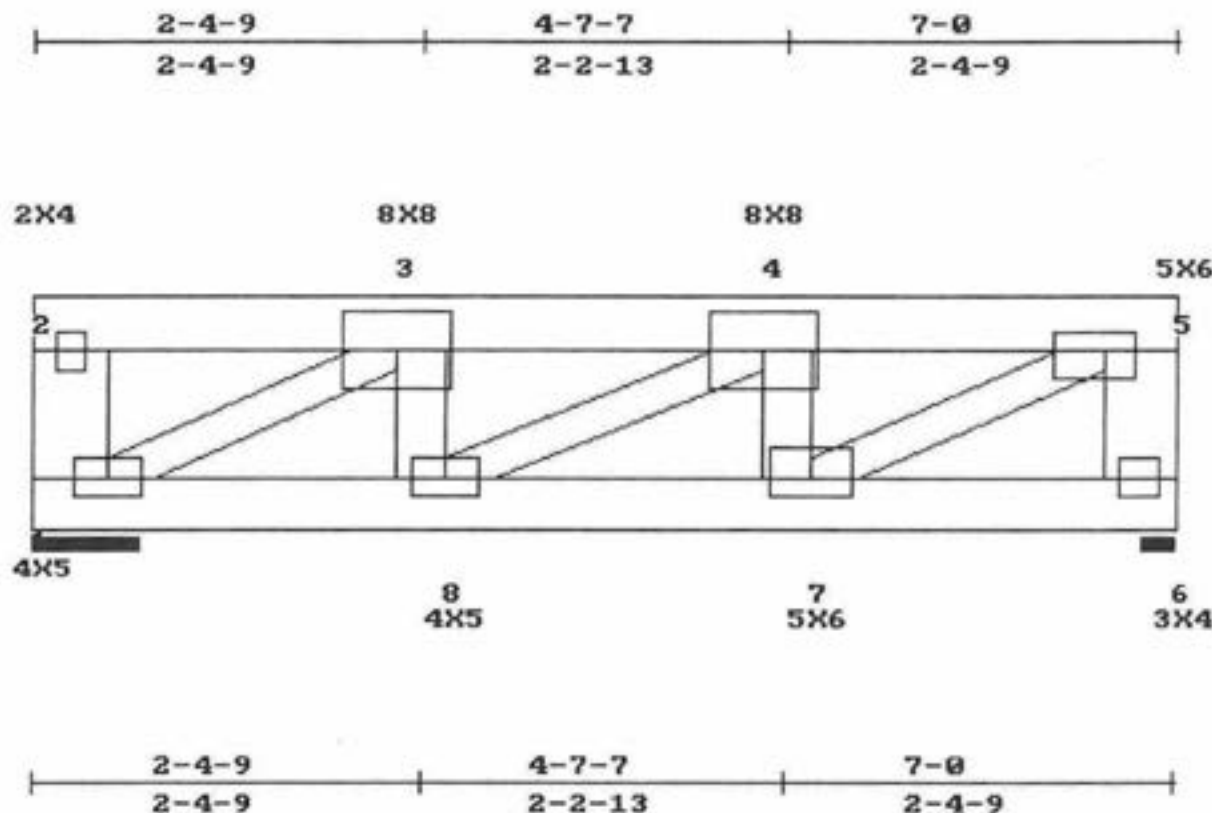
1-2=-1625 6-7= 0 3-1=-5930
 2-3= 0 7-8= 4956 3-8= 139
 3-4=-4821 8-1= 4821 4-8=-164
 4-5=-4956 4-7=-3312
 5-6=-5077 5-7= 5988

REACTIONS - SIZE

1=-5101 8.00
 6=-5101 2.59

32

PROVIDE FOR 4916 LBS UPLIFT AT JOINT 1 (0.96)
 PROVIDE FOR 4916 LBS UPLIFT AT JOINT 6 (0.96)
 PLATE OFFSETS (X=LEFT,Y=TOP):[J5=3.5,2],[J7=3,3],



LEFT HEIGHT:2-0 SPAN:7-0 RISE:2-0 RIGHT HEIGHT:2-0

LOADING (PSF)		MAX STRESSES		MINIMUM GRADE OF LUMBER	
L	D	TOP	BOTT	TOP CHORD	BOT CHORD
30	15	4-5=0.617	7-8=0.296	2X6 No.2D 19 SP	2X6 No.2D 19 SP
0	10	LL.DEFL.@8=0.02 < L/360		WEBS :2X4 No.3 19 SP	

REPETITIVE STRESSES NOT USED

SPACING : 24.0 in. o. c.
 NO. OF MEMBERS = 2

LOADING STRESS INCREASE	LOADING LUMBER PLATE	LOADING TYPE	PANEL(PLF) / JOINTS(LBS)
1	1.33 1.33	UNIFORM	2- 5= 1510 6- 1= 21

WEB: 1-2; 5-6 TO BE 2X6 No.2D 19 SP
 BOTH END VERTICALS ARE NOT EXPOSED.
 2 MEMBERS NAILED TOG. W/2 ROW(S) OF .131x3 in. NAILS 3 in. o.c.(TOP CHS.),AND 1 ROW(S) OF .131x3 in. NAILS 12 in. o.c(BOTT. CHS.)
 For Webs use 1 ROW OF NAILS 12 in. o.c.
 TRUSS HAS BEEN CHECKED FOR 145 M.P.H. WIND LOAD, WALL HEIGHT 16 FT, ENCLOSED BLDG. TYPE, ASSUMING 10 PSF TOTAL D.L.(7+3) (SBC-97).
 DEFLECTION(IN.) L.L= 0.02,D.L=0.02,T.L=0.04
 PLATES ARE MITEK M20-249,200 MANUFACTURED FROM ASTM A 446 GRD A GALVANIZED STEEL(EXCEPT AS SHOWN)
 PLATE MUST BE INSTALLED ON EA. FACE OF JOINT,SYMMETRICALLY(EXCEPT AS SHOWN)DESIGN CONFORMS W/ND5 DESIGN SPECS, SSSC,ANSI/TPI-95
 THIS DESIGN IS FOR TRUSS FABRICATION ONLY.FOR PERMANENT AND TEMPORARY BRACING(WHICH IS ALWAYS REQD)CONSULT BLDG ARCHITECT OR ENGINEER.

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 MAR 01 2001

====<<<<ACES-32 Ver. 1.1>>>>===== [001999] =====<<<<FL.QUALITY-MMH>>>>=====

Customer : FOGLIA Mon Mar 5 12:28:54 2001

Project #: 00469 Truss ID : T2 Family #: 313

Span : 35-1-8 B.Span : 35-1-8 Quant. : 1 Top Pitch : 6/12

==Bld(1/21/2000),v1.1=====

FORCES - GRAVITY LOADS

1-2=-28	7-8=-2321	9-10=0	3-1=-1833	6-11=-655
2-3=0	8-9=-1851	10-11=2321	3-13=2691	7-11=1424
3-4=-2505		11-12=3505	4-13=-620	7-10=-1406
4-5=-2505		12-13=3566	5-13=-1276	8-10=2787
5-6=-3566		13-1=252	5-12=95	
6-7=-3505			6-12=74	

REACTIONS - SIZE

1=-1920	2.68
9=-1920	2.68

33

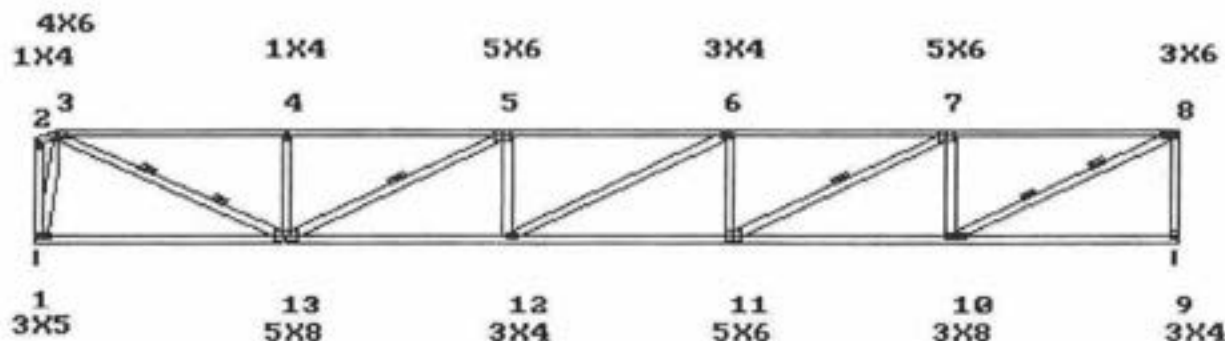
PROVIDE FOR 1699 LBS UPLIFT AT JOINT 1 (0.88)

PROVIDE FOR 1708 LBS UPLIFT AT JOINT 9 (0.89)

PROVIDE FOR 37 LBS HORIZ. REACTION AT JOINT 1

PLATE OFFSETS (X=LEFT,Y=TOP): [j5=3,3], [j7=3,3], [j8=4,1.5], [j11=3,2], [j13=4,2],

0-8-157-8-8	14-6-5	21-4-2	28-1-15	35-1-8
0-8-156-11-9	6-9-13	6-9-13	6-9-13	6-11-9



7-8-8	14-6-5	21-4-2	28-1-15	35-1-8
7-8-8	6-9-13	6-9-13	6-9-13	6-11-9

L. HL TO PK:0-10

LEFT HEIGHT:4-5-11

SPAN:35-1-8

RISE:4-10-3

RIGHT HEIGHT:4-10-3

LOADING (PSF)

	L	D
TOP	30	15
BOTT	0	10

MAX STRESSES

TOP	6-7=0.871
BOTT	12-13=0.898
LL.DEFL.@12	=0.25 < L/360

MINIMUM GRADE OF LUMBER

TOP CHORD:2X4	No.2ND 19 SP
BOT CHORD:2X4	No.1D 19 SP
WEBS	:2X4 No.3 19 SP

STR.INC.: LUMB = 1.33 PLATE = 1.33

REPETITIVE STRESSES USED

SPACING : 24.0 in. o. c.

NO. OF MEMBERS = 1

CHORD: 3-4; 4-5; 5-6; 6-7; 7-8 TO BE 2X4 SS 19 SP

WEB: 1-2 TO BE 2X4 No.2ND 19 SP

BOTH END VERTICALS ARE NOT EXPOSED.

WEB 5-13; 7-11 BRACED at 1/2 POINTS AS SHOWN ABOVE

WEB 3-13; 8-10 BRACED at 1/3 POINTS AS SHOWN ABOVE

Note:Use 1x4 or 2x3 Cont.Bracing conn.w/2-8d nails,or T-brace of same size and grade as web conn. to narrow face w/10d nails 6 in. o.c

TRUSS HAS BEEN CHECKED FOR 145 M.P.H. WIND LOAD, WALL HEIGHT 16 FT, ENCLOSED BLDG. TYPE, ASSUMING 10 PSF TOTAL D.L.(7+3) (SBC-97).

DEFLECTION(IN.) L.L= 0.25,D.L=0.21,T.L=0.45

PLATES ARE MITEK M20-249,200 MANUFACTURED FROM ASTM A 446 GRD A GALVANIZED STEEL(EXCEPT AS SHOWN)

PLATE MUST BE INSTALLED ON EA. FACE OF JOINT,SYMMETRICALLY(EXCEPT AS SHOWN)DESIGN CONFORMS W/NDS DESIGN SPECS, SIBC,ANSI/TPI-95

THIS DESIGN IS FOR TRUSS FABRICATION ONLY.FOR PERMANENT AND TEMPORARY BRACING(WHICH IS ALWAYS REQD)CONSULT BLDG ARCHITECT OR ENGINEER.

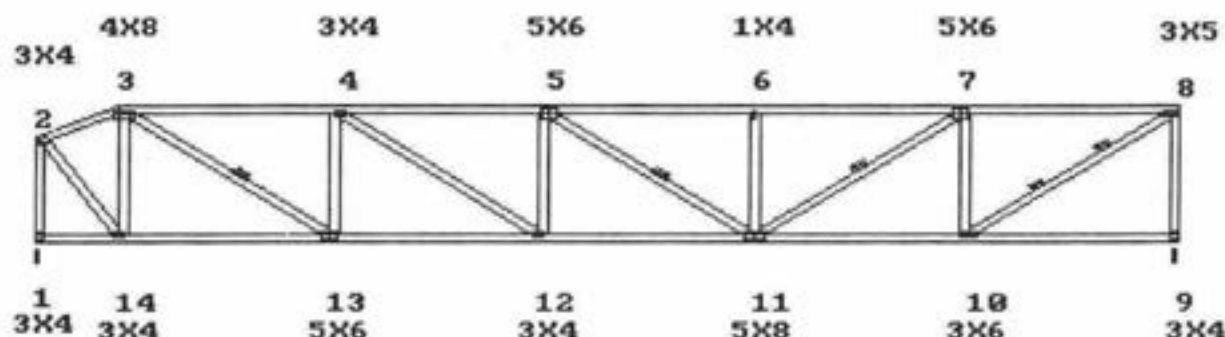
MAR 06 2001

====<<<<ACES-32 Ver. 1.1>>>>=====[001992]====<<<<FL.QUALITY-MMH>>>>=====
 Customer : FOGLIA Mon Mar 5 12:13:36 2001
 Project #: 00469 Truss ID : T3 Family #: 313
 Span : 35-1-8 B.Span : 35-1-8 Quant. : 1 Top Pitch : 6/12
 ==Bld(1/21/2000),v1.1=====

FORCES - GRAVITY LOADS				REACTIONS - SIZE	
1-2=1895	7-8=1853	9-10= 0	2-14= 1599	5-11=219	1=1920 2.68
2-3=899	8-9=1855	10-11= 1853	3-14=1291	6-11=581	9=1920 2.68
3-4=2354		11-12= 3024	3-13= 2014	7-11= 1316	
4-5=3024		12-13= 2354	4-13=1155	7-10=1434	
5-6=2856		13-14= 804	4-12= 878	8-10=2424	
6-7=2856		14-1= 0	5-12=439		

PROVIDE FOR 1683 LBS UPLIFT AT JOINT 1 (0.88)
 PROVIDE FOR 1723 LBS UPLIFT AT JOINT 9 (0.90)
 PROVIDE FOR 145 LBS HORIZ. REACTION AT JOINT 1
 PLATE OFFSETS (X=LEFT,Y=TOP): [j5=3,3], [j7=3,3], [j8=3,1.5], [j11=4,2], [j13=3,2],

2-6-15	9-2-2	15-7-8	22-0-15	28-6-5	35-1-8
2-6-15	6-7-3	6-5-7	6-5-7	6-5-7	6-7-3



2-6-15	9-2-2	15-7-8	22-0-15	28-6-5	35-1-8
2-6-15	6-7-3	6-5-7	6-5-7	6-5-7	6-7-3

L. HL TO PK:2-10-9
 LEFT HEIGHT:4-5-11 SPAN:35-1-8 RISE:5-9-3 RIGHT HEIGHT:5-9-3

LOADING (PSF)		MAX STRESSES		MINIMUM GRADE OF LUMBER	
TOP	30	D	TOP 4-5=0.838	TOP CHORD:2X4	No.2ND 19 SP
BOTT	0	10	BOTT 11-12=0.756	BOT CHORD:2X4	No.2ND 19 SP
			LL.DEFL.@12=0.19 < L/360	WEBS :2X4	No.3 19 SP

STR.INC.: LUMB = 1.33 PLATE = 1.33 SPACING : 24.0 in. o. c.
 REPETITIVE STRESSES USED NO. OF MEMBERS = 1

CHORD: 3-4; 4-5; 5-6; 6-7; 7-8 TO BE 2X4 SS 19 SP
 BOTH END VERTICALS ARE NOT EXPOSED.
 WEB 3-13; 5-11; 7-11 BRACED AT 1/2 POINTS AS SHOWN ABOVE
 WEB 8-10 BRACED AT 1/3 POINTS AS SHOWN ABOVE
 Note:Use 1x4 or 2x3 Cont.Bracing conn.w/2-8d nails,or T-brace of same size and grade as web conn. to narrow face w/10d nails 6 in. o.c
 TRUSS HAS BEEN CHECKED FOR 145 M.P.H. WIND LOAD, WALL HEIGHT 16 FT, ENCLOSED BLDG. TYPE, ASSUMING 10 PSF TOTAL D.L.(7+3) (SBC-97).
 DEFLECTION(IN.) L.L= 0.19,D.L=0.16,T.L=0.34
 PLATES ARE MITEK M20-249,200 MANUFACTURED FROM ASTM A 446 GRD A GALVANIZED STEEL(EXCEPT AS SHOWN)
 PLATE MUST BE INSTALLED ON EA. FACE OF JOINT,SYMMETRICALLY(EXCEPT AS SHOWN)DESIGN CONFORMS W/NDOS DESIGN SPECS, SSBC,ANSI/TP1-95
 THIS DESIGN IS FOR TRUSS FABRICATION ONLY.FOR PERMANENT AND TEMPORARY BRACING(WHICH IS ALWAYS REQD)CONSULT BLDG ARCHITECT OR ENGINEER.

MAR 06 2001

====<<<ACES-32 Ver. 1.1>>>===== [001840] =====<<<FL.QUALITY-MMH>>>=====

Customer : FOGLIA Truss ID : S27 Thu Mar 1 09:30:29 2001

Project #: 00469 Quantity : 1 Family #: special

Span : 35-1-8 Top Pitch : 6/12

====Bldg(1/21/2000),v1.1=====

FORCES - GRAVITY LOADS

1-2=-270	7-8=-1274	9-10= 0	3-1=-2154	6-12= 574
2-3= 0	8-9=-1875	10-11= 1140	3-14= 1045	6-11=-970
3-4=-1453		11-12= 2157	4-14=-1262	7-11= 1493
4-5=-2314		12-13= 2548	4-13= 374	7-10=-998
5-6=-2548		13-14= 2314	5-13=-344	8-10= 1587
6-7=-2157		14-1= 1453	5-12=-298	

REACTIONS - SIZE

1=-1920 2.68

9=-1920 2.68

35

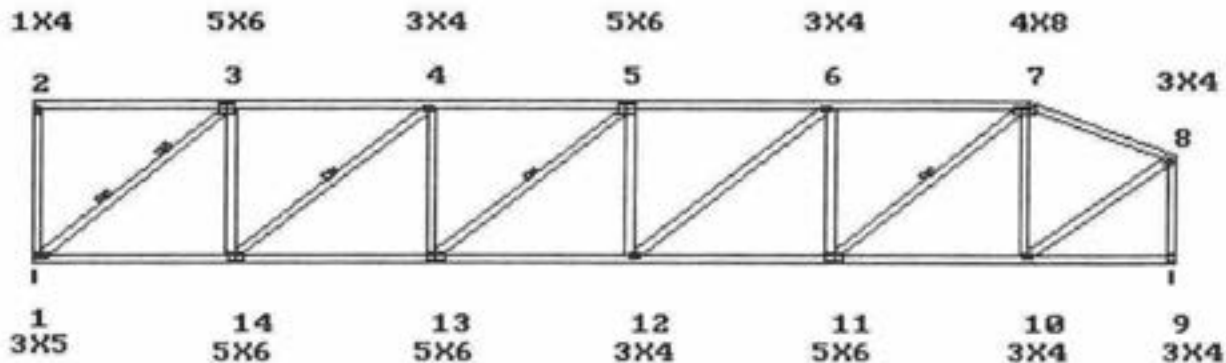
PROVIDE FOR 1744 LBS UPLIFT AT JOINT 1 (0.91)

PROVIDE FOR 1662 LBS UPLIFT AT JOINT 9 (0.87)

PROVIDE FOR 262 LBS HORIZ. REACTION AT JOINT 1

PLATE OFFSETS (X=LEFT,Y=TOP): [j3=3,3], [j5=3,3], [j11=3,2], [j13=3,2], [j14=3,2], (SYMMETRIC PLATING TURNED ON)

6-1-5	12-2-10	18-3-15	24-5-4	30-6-9	35-1-8
6-1-5	6-1-5	6-1-5	6-1-5	6-1-5	4-6-15



6-1-5	12-2-10	18-3-15	24-5-4	30-6-9	35-1-8
6-1-5	6-1-5	6-1-5	6-1-5	6-1-5	4-6-15

LEFT HEIGHT:6-10-3 SPAN:35-1-8 RISE:6-10-4 R. HL TO PK :5-1-7

RIGHT HEIGHT:4-6-12

LOADING (PSF)	MAX STRESSES	MINIMUM GRADE OF LUMBER
L D	TOP 5-6=0.994	TOP CHORD:2X4 No.2ND 19 SP
TOP 30 15	BOTT 12-13=0.648	BOT CHORD:2X4 No.2ND 19 SP
BOTT 0 10	LL.DEFL.@12=0.14 < L/360	WEBS :2X4 No.3 19 SP

STR.INC.: LUMB = 1.33 PLATE = 1.33 SPACING : 24.0 in. o. c.

REPETITIVE STRESSES USED NO. OF MEMBERS = 1

CHORD: 3-4; 4-5; 5-6; 6-7 TO BE 2X4 No.1D 19 SP

BOTH END VERTICALS ARE NOT EXPOSED.

WEB 4-14; 5-13; 7-11 BRACED at 1/2 POINTS AS SHOWN ABOVE

WEB 3-1 BRACED at 1/3 POINTS AS SHOWN ABOVE

Note:Use 1x4 or 2x3 Cont.Bracing conn.w/2-8d nails,or T-brace of same size and grade as web conn. to narrow face w/10d nails 6 in. o.c

TRUSS HAS BEEN CHECKED FOR 145 M.P.H. WIND LOAD, WALL HEIGHT 16 FT, ENCLOSED BLDG. TYPE, ASSUMING 10 PSF TOTAL D.L.(7+3) (SBC-97).

DEFLECTION(IN.) L.L= 0.14,D.L=0.11,T.L=0.25

PLATES ARE MITEK M20-249,200 MANUFACTURED FROM ASTM A 446 GRD A GALVANIZED STEEL(EXCEPT AS SHOWN)

PLATE MUST BE INSTALLED ON EA. FACE OF JOINT,SYMMETRICALLY(EXCEPT AS SHOWN)DESIGN CONFORMS W/NDS DESIGN SPECS, SBC,ANSI/TP1-95

THIS DESIGN IS FOR TRUSS FABRICATION ONLY.FOR PERMANENT AND TEMPORARY BRACING(WHICH IS ALWAYS REQD)CONSULT BLDG ARCHITECT OR ENGINEER.

[Signature]

MAR 01 2001

====<<<<ACES-32 Ver. 1.1>>>>===== [001841] =====<<<<FL.QUALITY-MMH>>>>=====

Customer : FOGLIA Truss ID : S28 Thu Mar 1 09:30:50 2001

Project #: 00469 Quantity : 1 Family #: special

Span : 35-2 Bot pitch : 6/12

Bot pitch : 90/12

==81d(1/21/2000),v1.1=====

FORCES - GRAVITY LOADS

1-2=-177	7-8=-1506	9-10= 0	15-1= 1370	3-1=-2187	5-11=-251
2-3= 0	8-9=-1857	10-11= 2052	3-15= 1252	6-11= 318	
3-4=-1408		11-12= 2210	4-15=-535	6-10=-1119	
4-5=-2210		12-13= 1822	4-13=-691	7-10= 105	
5-6=-2052		13-14= 0	4-12= 616	8-10= 1615	
6-7=-1347		14-15= 60	5-12=-356	15-13= 1995	

REACTIONS - SIZE

1=-1922	2.68
9=-1922	2.68

(36)

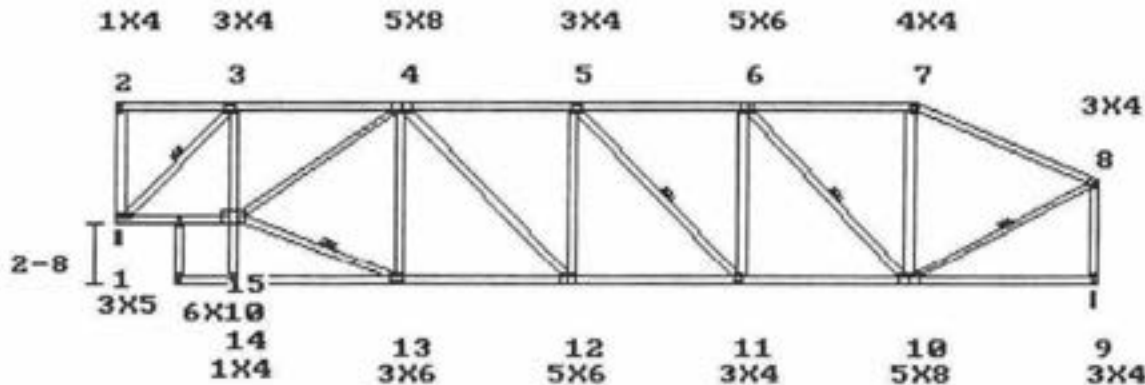
PROVIDE FOR 1741 LBS UPLIFT AT JOINT 1 (0.91)

PROVIDE FOR 1670 LBS UPLIFT AT JOINT 9 (0.87)

PROVIDE FOR 382 LBS HORIZ. REACTION AT JOINT 1

PLATE OFFSETS (X=LEFT,Y=TOP): [j4=4,3], [j6=3,3], [j10=4,2], [j12=3,2], (SYMMETRIC PLATING TURNED ON)

4-0-8	10-2	16-3-9	22-5-1	28-6-9	35-2
4-0-8	6-1-8	6-1-8	6-1-8	6-1-8	6-7-7



4-0-8	10-2	16-3-9	22-5-1	28-6-9	35-2
4-0-8	6-1-8	6-1-8	6-1-8	6-1-8	6-7-7

LEFT HEIGHT: 5-2-3 SPAN: 35-2 RISE: 7-10-3 R. HL TO PK : 7-4-13

RIGHT HEIGHT: 4-6-7

LOADING (PSF)	MAX STRESSES	MINIMUM GRADE OF LUMBER
TOP L D	TOP 4-5=0.914	TOP CHORD: 2X4 No.1D 19 SP
BOTT 0 10	BOTT 11-12=0.587	BOT CHORD: 2X4 No.2ND 19 SP
	LL.DEFL.@12=0.11 < L/360	WEBS : 2X4 No.3 19 SP

STR.INC.: LUMB = 1.33 PLATE = 1.33

REPETITIVE STRESSES USED

SPACING : 24.0 in. o. c.

NO. OF MEMBERS = 1

CHORD: 7-8 TO BE 2X4 SS 19 SP

CHORD: 14-15 TO BE 2X4 No.3 19 SP

BOTH END VERTICALS ARE NOT EXPOSED.

WEB 3-1; 5-11; 6-10; 8-10; 15-13 BRACED AT 1/2 POINTS AS SHOWN ABOVE

Note: Use 1x4 or 2x3 Cont.Bracing conn.w/2-8d nails, or T-brace of same size and grade as web conn. to narrow face w/10d nails 6 in. o.c

TRUSS HAS BEEN CHECKED FOR 145 M.P.H. WIND LOAD, WALL HEIGHT 16 FT, ENCLOSED BLDG. TYPE, ASSUMING 10 PSF TOTAL D.L.(7+3) (SBC-97).

DEFLECTION(IN.) L.L= 0.11, D.L=0.09, T.L=0.20

PLATES ARE MITEK M20-249,200 MANUFACTURED FROM ASTM A 446 GRD A GALVANIZED STEEL(EXCEPT AS SHOWN)

PLATE MUST BE INSTALLED ON EA. FACE OF JOINT, SYMMETRICALLY(EXCEPT AS SHOWN) DESIGN CONFORMS W/NDS DESIGN SPECS, S58C, ANSI/TPI-95

THIS DESIGN IS FOR TRUSS FABRICATION ONLY.FOR PERMANENT AND TEMPORARY BRACING(WHICH IS ALWAYS REQD)CONSULT BLDG ARCHITECT OR ENGINEER.

MAR 01 2001

====<<<ACES-32 Ver. 1.1>>>===== [001842] =====<<<FL.QUALITY-MMH>>>=====

Customer : FOGLIA Thu Mar 1 09:31:24 2001

Project #: 00469 Truss ID : T4 Family # : spec.

Span : 35-1-8 B.Span : 35-3-8 Quant. : 1 Top Pitch : 6/12

Bot pitch : 90/12

==8ld(1/21/2000),v1.1=====

FORCES - GRAVITY LOADS

1-2=-269	7-8=-1611	10-11= 1064	3-1=-2241	6-12=-1928
2-3= 0	8-9= 0	11-12= 1439	3-15= 1148	7-12= 845
3-4=-2142	9-10=-184	12-13= 0	4-15=-941	7-11=-419
4-5=-2885		13-14= 40	4-14= 417	8-11= 661
5-6=-2874		14-15= 2580	5-14=-460	8-10=-1964
6-7=-1806		15-1= 1609	6-14= 1887	14-12= 2195

REACTIONS - SIZE

1=-1920	3.06
10=-1920	2.89

(37)

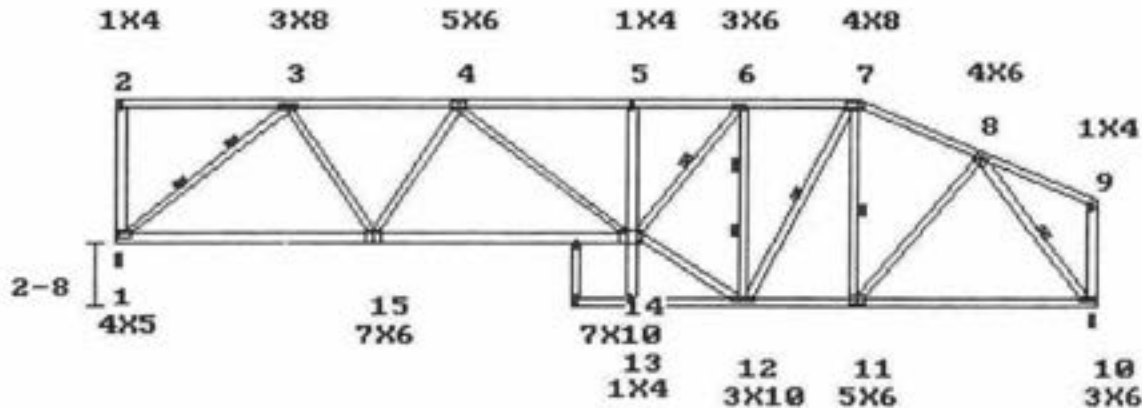
PROVIDE FOR 2206 LBS UPLIFT AT JOINT 1 (1.15)

PROVIDE FOR 2078 LBS UPLIFT AT JOINT 10 (1.08)

PROVIDE FOR 604 LBS HORIZ. REACTION AT JOINT 1

PLATE OFFSETS (X=LEFT,Y=TOP): [J4=3,3], [J11=3,2], [J14=5,2], [J15=3,2],

6-1-3	12-2-5	18-3-8	22-5	26-6-9	35-1-8
6-1-3	6-1-3	6-1-3	4-1-8	4-1-8	4-4-8
					4-2-8



9-1-12	18-3-8	22-5-1	26-6-9	35-1-8
9-1-12	9-1-12	4-1-8	4-1-8	8-6-15

LEFT HEIGHT:6-2-3 SPAN:35-1-8 RISE:8-10-3 R. HL TO PK :9-7-1

RIGHT HEIGHT:4-6-11

LOADING (PSF)	MAX STRESSES	MINIMUM GRADE OF LUMBER
L D	TOP 4-5=0.844	TOP CHORD:2X4 SS 19 SP
TOP 30 15	BOTT 11-12=0.527	BOT CHORD:2X4 No.2ND 19 SP
BOTT 0 10	LL.DEFL.@14=0.17 < L/360	WEBS :2X4 No.3 19 SP

STR.INC.: LUMB = 1.33 PLATE = 1.33 SPACING : 24.0 in. o. c.

REPETITIVE STRESSES USED NO. OF MEMBERS = 1

CHORD: 7-8; 8-9 TO BE 2X4 No.10 19 SP

CHORD: 13-14 TO BE 2X4 No.3 19 SP

CHORD: 14-15; 15-1 TO BE 2X6 No.2D 19 SP

BOTH END VERTICALS ARE NOT EXPOSED.

WEB 6-14; 7-12; 7-11; 8-10 BRACED at 1/2 POINTS AS SHOWN ABOVE

WEB 3-1; 6-12 BRACED at 1/3 POINTS AS SHOWN ABOVE

Note:Use 1x4 or 2x3 Cont.Bracing conn.w/2-8d nails,or T-brace of same size and grade as web conn. to narrow face w/10d nails 6 in. o.c

TRUSS HAS BEEN CHECKED FOR 145 M.P.H. WIND LOAD, WALL HEIGHT 16 FT, ENCLOSED BLDG. TYPE, ASSUMING 10 PSF TOTAL D.L.(7*3) (SBC-97).

DEFLECTION(IN.) L.L= 0.17,D.L=0.14,T.L=0.30

PLATES ARE MITEK M20-249,200 MANUFACTURED FROM ASTM A 446 GRD A GALVANIZED STEEL(EXCEPT AS SHOWN)

PLATE MUST BE INSTALLED ON EA. FACE OF JOINT,SYMMETRICALLY(EXCEPT AS SHOWN)DESIGN CONFORMS W/MDS DESIGN SPECS, SSB,ANSI/TPI-95

THIS DESIGN IS FOR TRUSS FABRICATION ONLY.FOR PERMANENT AND TEMPORARY BRACING(WHICH IS ALWAYS REQD)CONSULT BLDG ARCHITECT OR ENGINEER.

[Signature]
MAR 01 2001

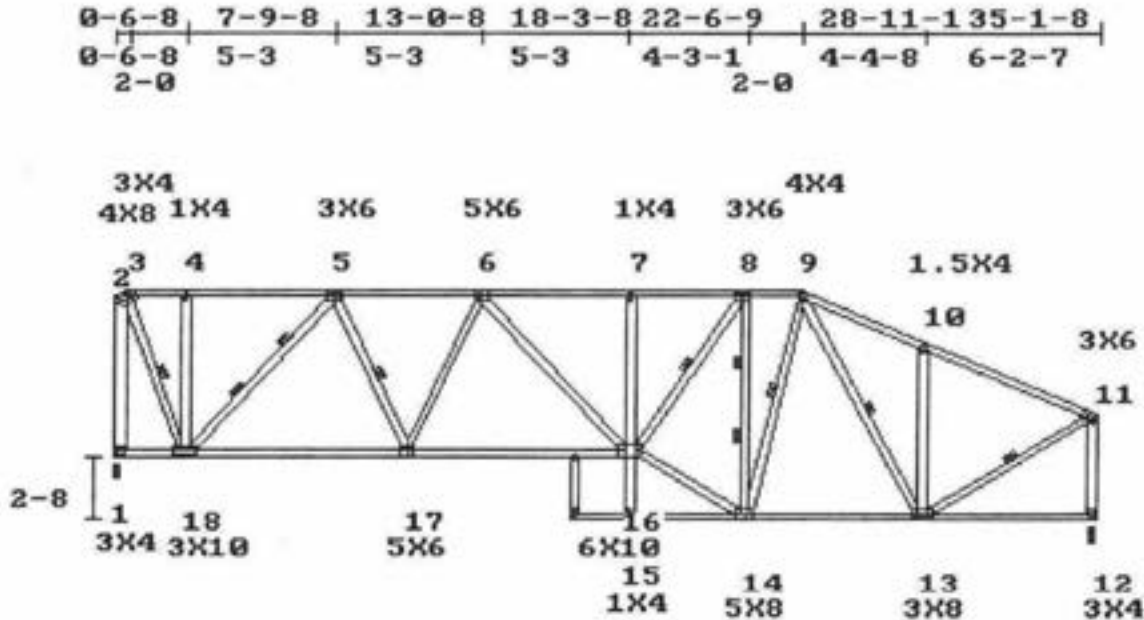
====<<<<ACES-32 Ver. 1.1>>>>=====[001843]====<<<<FL.QUALITY-MMH>>>>=====
 Customer : FOGLIA Thu Mar 1 09:31:53 2001
 Project #: 00469 Truss ID : H5 Family # : spec.
 Span : 35-1-8 B.Span : 35-3-8 Quant. : 1 Top Pitch : 6/12
 Bot pitch : 90/12

==Bld(1/21/2000),v1.1==

REACTIONS - SIZE
 1=-1920 3.07
 12=-1920 2.85

(38)

PROVIDE FOR 2212 LBS UPLIFT AT JOINT 1 (1.15)
 PROVIDE FOR 2055 LBS UPLIFT AT JOINT 12 (1.07)
 PROVIDE FOR 725 LBS HORIZ. REACTION AT JOINT 1
 PLATE OFFSETS (X=LEFT,Y=TOP): [j6=3,3], [j14=4,2], [j17=3,2],



2-6-8 10-5 18-3-8 22-6-9 28-11-1 35-1-8
 2-6-8 7-10-8 7-10-8 4-3-1 6-4-8 6-2-7

L. HL TO PK:0-7-4 LEFT HEIGHT:6-10-14 SPAN:35-1-8 RISE:9-10-3 R. HL TO PK:11-9-15
 RIGHT HEIGHT:4-6-11

LOADING (PSF)		MAX STRESSES		MINIMUM GRADE OF LUMBER	
L	D	TOP	BOTT	TOP CHORD	BOT CHORD
30	15	5-6=0.916	16-17=0.701	2X4 No.2ND 19 SP	2X4 No.2ND 19 SP
0	10	LL.DEFL.@16=0.22 < L/360		WEBS :2X4 No.3 19 SP	

STR.INC.: LUMB = 1.33 PLATE = 1.33 SPACING : 24.0 in. o. c.
 REPETITIVE STRESSES USED NO. OF MEMBERS = 1

CHORD: 3-4; 4-5; 5-6; 6-7; 7-8; 8-9 TO BE 2X4 No.1D 19 SP

CHORD: 9-10; 10-11 TO BE 2X4 SS 19 SP

CHORD: 15-16 TO BE 2X4 No.3 19 SP

WEB: 1-2 TO BE 2X6 No.2D 19 SP

BOTH END VERTICALS ARE NOT EXPOSED.

WEB 2-18; 5-17; 8-16; 9-14; 9-13; 11-13 BRACED at 1/2 POINTS AS SHOWN ABOVE

WEB 5-18; 8-14 BRACED at 1/3 POINTS AS SHOWN ABOVE

Note:Use 1x4 or 2x3 Cont.Bracng conn.w/2-8d nails,or T-brace of same size and grade as web conn. to narrow face w/10d nails 6 in. o.c

TRUSS HAS BEEN CHECKED FOR 145 M.P.H. WIND LOAD, WALL HEIGHT 16 FT, ENCLOSED BLDG. TYPE, ASSUMING 10 PSF TOTAL D.L.(7+3) (SBC-97).

DEFLECTION(IN.) L.L= 0.22,D.L=0.18,T.L=0.40

PLATES ARE MITEK M20-249,200 MANUFACTURED FROM ASTM A 446 GRD A GALVANIZED STEEL(EXCEPT AS SHOWN)

PLATE MUST BE INSTALLED ON EA. FACE OF JOINT,SYMMETRICALLY(EXCEPT AS SHOWN)DESIGN CONFORMS W/NDS DESIGN SPECS, SSBC,NDS,ANSI/TPI-95

THIS DESIGN IS FOR TRUSS FABRICATION ONLY.FOR PERMANENT AND TEMPORARY BRACING(WHICH IS ALWAYS REQD)CONSULT BLDG ARCHITECT OR ENGINEER.

MAR 01 2001

====<<<<ACES-32 Ver. 1.1>>>>=====[001844]=====<<<<FL.QUALITY-MMH>>>>=====

Customer : FOGLIA Truss ID : H6 Thu Mar 1 09:32:28 2001

Project #: 00469 Quantity : 1 Family #: special

Span : 35-1-8 Bot pitch : 6/12

Bot pitch : 90/12

==Bld(1/21/2000),v1.1=====

FORCES - GRAVITY LOADS

1-2=-1895	7-8=-1631	10-11= 0	16-1= 0	2-16= 1608	6-14=-428	14-12= 1737	REACTIONS - SIZE
2-3=-615	8-9=-1467	11-12= 1313	3-16=-70	7-14= 1422			1=-1920 2.98
3-4=-550	9-10=-1859	12-13= 0	4-16=-1606	7-12=-1007			10=-1920 2.84
4-5=-1728		13-14= 41	4-15= 911	8-12= 220			
5-6=-2123		14-15= 1963	5-15=-744	8-11=-807			
6-7=-2115		15-16= 1440	5-14= 283	9-11= 1610			

(34)

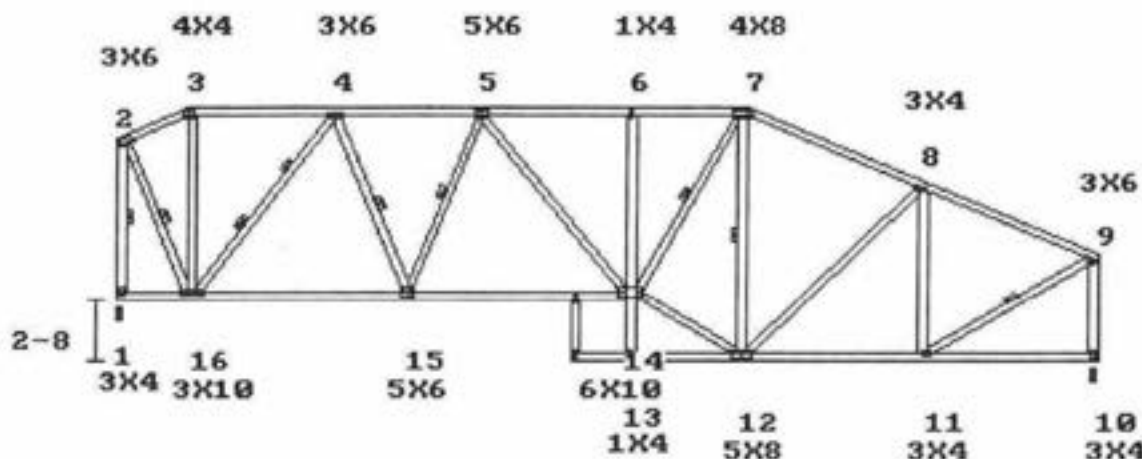
PROVIDE FOR 2143 LBS UPLIFT AT JOINT 1 (1.12)

PROVIDE FOR 2043 LBS UPLIFT AT JOINT 10 (1.06)

PROVIDE FOR 766 LBS HORIZ. REACTION AT JOINT 1

PLATE OFFSETS (X=LEFT,Y=TOP): [j5=3,3], [j12=4,2], [j15=3,2], (SYMMETRIC PLATING TURNED ON)

2-6-8 7-9-8 13-0-8 18-3-8 22-6-9 28-11-1 35-1-8
2-6-8 5-3 5-3 4-3-1 6-4-8 6-2-7



2-6-8 10-5 18-3-8 22-6-9 28-11-1 35-1-8
2-6-8 7-10-8 7-10-8 4-3-1 6-4-8 6-2-7

L. HL TO PK:2-10-2

LEFT HEIGHT:6-10-14

SPAN:35-1-8

RISE:10-10-3

R. HL TO PK :14-0-12

RIGHT HEIGHT:4-6-11

LOADING (PSF)

MAX STRESSES

MINIMUM GRADE OF LUMBER

TOP 30 15

TOP 7-8=0.961

TOP CHORD:2X4 No.2ND 19 SP

BOTT 0 10

BOTT 14-15=0.657

BOT CHORD:2X4 No.2ND 19 SP

LL.DEFL.@14=0.12 < L/360

WEBS :2X4 No.3 19 SP

STR.INC.: LUMB = 1.33 PLATE = 1.33

SPACING : 24.0 in. o. c.

REPETITIVE STRESSES USED

NO. OF MEMBERS = 1

CHORD: 3-4; 4-5; 5-6; 6-7 TO BE 2X4 No.10 19 SP

CHORD: 7-8; 8-9 TO BE 2X4 SS 19 SP

CHORD: 13-14 TO BE 2X4 No.3 19 SP

BOTH END VERTICALS ARE NOT EXPOSED.

WEB 1-2; 2-16; 4-15; 5-15; 7-14; 7-12; 9-11 BRACED at 1/2 POINTS AS SHOWN ABOVE

WEB 4-16 BRACED at 1/3 POINTS AS SHOWN ABOVE

Note:Use 1x4 or 2x3 Cont.Bracing conn.w/2-8d nails,or T-brace of same size and grade as web conn. to narrow face w/10d nails 6 in. o.c

TRUSS HAS BEEN CHECKED FOR 145 M.P.H. WIND LOAD, WALL HEIGHT 16 FT, ENCLOSED BLDG. TYPE, ASSUMING 10 PSF TOTAL D.L.(7+3) (SBC-97).

DEFLECTION(IN.) L.L= 0.12,D.L=0.10,T.L=0.22

PLATES ARE MITEK M20-249,200 MANUFACTURED FROM ASTM A 446 GRD A GALVANIZED STEEL(EXCEPT AS SHOWN)

PLATE MUST BE INSTALLED ON EA. FACE OF JOINT,SYMMETRICALLY(EXCEPT AS SHOWN)DESIGN CONFORMS W/ND S DESIGN SPECS, SSBC,ANSI/TPI-95

THIS DESIGN IS FOR TRUSS FABRICATION ONLY.FOR PERMANENT AND TEMPORARY BRACING(WHICH IS ALWAYS REQD)CONSULT BLDG ARCHITECT OR ENGINEER.

====<<<<ACES-32 Ver. 1.1>>>>===== [001845] =====<<<<FL.QUALITY-MMH>>>>=====

Customer : FOGLIA Thu Mar 1 09:36:58 2001

Project #: 00469 Truss ID : T5 Family #: spec.

Span : 35-1-8 B.Span : 35-3-8 Quant. : 1 Top Pitch : 6/12

==Bld(1/21/2000),v1.1=====

FORCES - GRAVITY LOADS

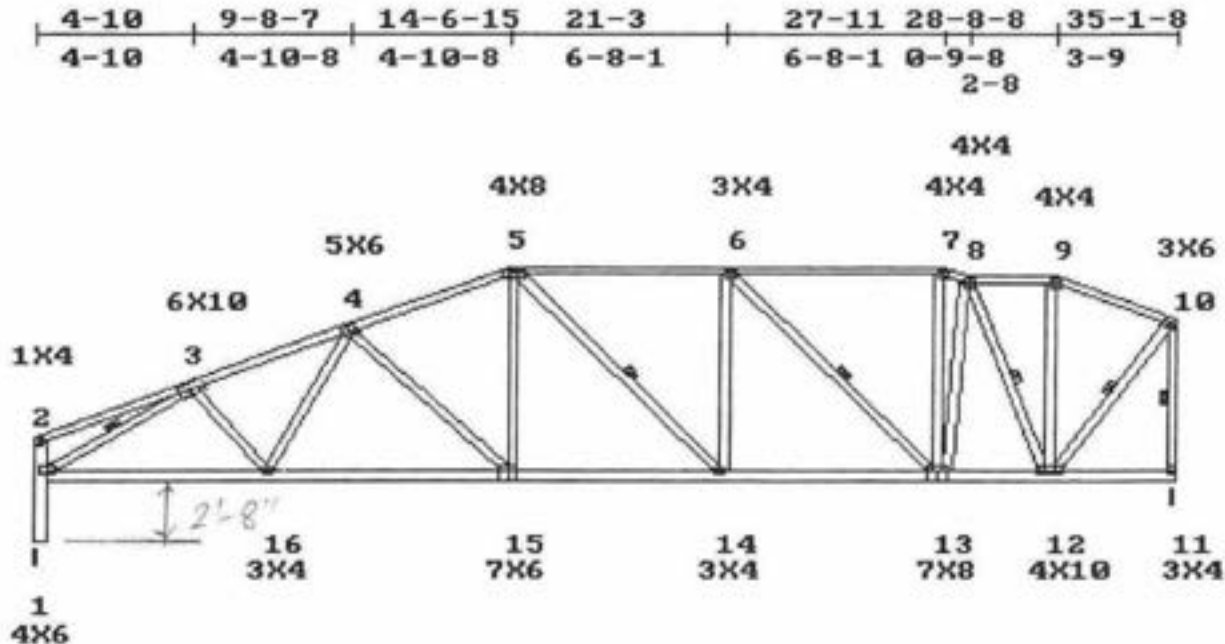
FORCES - GRAVITY LOADS			REACTIONS - SIZE	
1-2=-214	-7-8=-1385	11-12= 0	3-1=-2574	6-14= 172
2-3= 0	8-9=-750	12-13= 1183	3-16= 190	6-13=-969
3-4=-2330	9-10=-837	13-14= 1823	4-16=-19	7-13= 283
4-5=-2074	10-11=-1886	14-15= 1852	4-15=-392	8-13= 593
5-6=-1823	15-16= 2091	5-15= 452	8-12=-1432	9-12= 87
6-7=-1238	16-1= 1986	5-14=-48		

PROVIDE FOR 2029 LBS UPLIFT AT JOINT 1 (1.06)

PROVIDE FOR 2082 LBS UPLIFT AT JOINT 11 (1.08)

PROVIDE FOR 801 LBS HORIZ. REACTION AT JOINT 1

PLATE OFFSETS (X=LEFT,Y=TOP):[j4=3,3],[j13=4,2],[j15=3,2],



7-2-8		14-6-15		21-3		27-11		31-4-8		35-1-8	
7-2-8	7-4-7	6-8-1	6-8	3-5-8	3-9						

L. HL TO PK:16-3-9 LEFT HEIGHT:1-10-11 SPAN:35-1-8 RISE:9-2-3 R. HL TO PK:4-2-5 RIGHT HEIGHT:6-10-15

LOADING (PSF)		MAX STRESSES		MINIMUM GRADE OF LUMBER	
TOP	30	TOP	3-4=0.983	TOP CHORD:	2X4 No.1D 19 SP
BOTT	0	BOTT	15-16=0.337	BOT CHORD:	2X6 No.2D 19 SP
		LL.DEFL.@15=0.10	< L/360	WEBS	:2X4 No.3 19 SP

STR.INC.: LUMB = 1.33 PLATE = 1.33 SPACING : 24.0 in. o. c.

REPETITIVE STRESSES USED NO. OF MEMBERS = 1

CHORD: 5-6; 6-7 TO BE 2X4 SS 19 SP

CHORD: 7-8; 8-9; 9-10 TO BE 2X4 No.2ND 19 SP

WEB: 1-2 TO BE 2X6 No.2D 19 SP

BOTH END VERTICALS ARE NOT EXPOSED.

WEB 10-11; 3-1; 5-14; 6-13; 8-12; 10-12 BRACED at 1/2 POINTS AS SHOWN ABOVE

Note:Use 1x4 or 2x3 Cont.Bracing conn.w/2-8d nails,or T-brace of same size and grade as web conn. to narrow face w/10d nails 6 in. o.c

TRUSS HAS BEEN CHECKED FOR 145 M.P.H. WIND LOAD, WALL HEIGHT 16 FT, ENCLOSED BLDG. TYPE, ASSUMING 10 PSF TOTAL D.L.(7+3) (SBC-97).

DEFLECTION(IN.) L.L= 0.10,D.L=0.09,T.L=0.19

PLATES ARE MITEK M20-249,200 MANUFACTURED FROM ASTM A 446 GRD A GALVANIZED STEEL(EXCEPT AS SHOWN)

PLATE MUST BE INSTALLED ON EA. FACE OF JOINT,SYMMETRICALLY(EXCEPT AS SHOWN)DESIGN CONFORMS W/NDS DESIGN SPECS, SSBC,ANSI/TP1-95

THIS DESIGN IS FOR TRUSS FABRICATION ONLY.FOR PERMANENT AND TEMPORARY BRACING(WHICH IS ALWAYS REQD)CONSULT BLDG ARCHITECT OR ENGINEER.

NOV-01-2001

[Signature]

====<<<<ACES-32 Ver. 1.1>>>>=====[001846]=====<<<<FL.QUALITY-MMH>>>>=====

Customer : FOGLIA Thu Mar 1 09:46:18 2001

Project #: 00469 Truss ID : T6 Family # : spec.

Span : 35-1-8 B.Span : 35-3-8 Quant. : 1 Top Pitch : 6/12

==81d(1/21/2000),v1.1=====

FORCES - GRAVITY LOADS

1-2=207	7-8=1470	11-12= 404	3-1=2514	6-13=673
2-3= 0	8-9=981	12-13= 981	3-15= 183	7-13= 232
3-4=2275	9-10= 0	13-14= 1605	4-15= 26	8-13= 616
4-5=1896	10-11=75	14-15= 2026	4-14=511	8-12=1510
5-6=1694		15-1= 1916	5-14= 377	9-12= 1702
6-7=1315			6-14= 207	9-11=1841

REACTIONS - SIZE

1=1915	8.00
11=1915	2.05

(4)

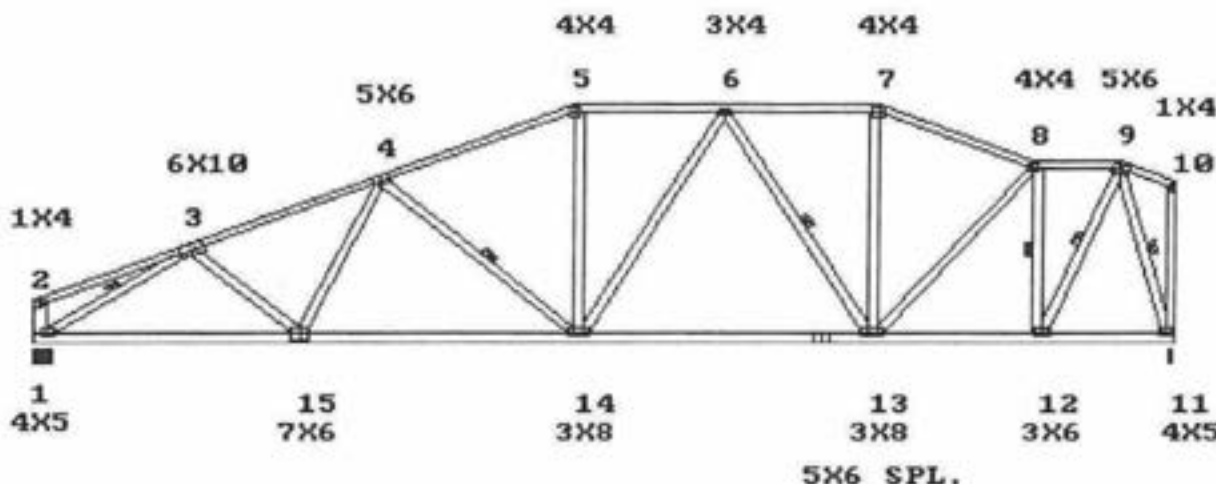
PROVIDE FOR 1983 LBS UPLIFT AT JOINT 1 (1.04)

PROVIDE FOR 2028 LBS UPLIFT AT JOINT 11 (1.06)

PROVIDE FOR 841 LBS HORIZ. REACTION AT JOINT 1

PLATE OFFSETS (X=LEFT,Y=TOP):[j4=3,3],[j9=3,2],[j15=3,2],

4-10	10-8-7	16-6-15	21-3	25-11	30-8-8	35-1-8
4-10	5-10-8	5-10-8	4-8-1	4-8-1	4-9-8	2-8 1-9



8-2-8	16-6-15	25-11	30-8-8	35-1-8
8-2-8	8-4-7	9-4-1	4-9-8	4-5

L. HL TO PK:18-6-7 R. HL TO PK:1-11-8

LEFT HEIGHT:1-10-11 SPAN:35-1-8 RISE:10-2-2 RIGHT HEIGHT:6-10-14

LOADING (PSF)	MAX STRESSES	MINIMUM GRADE OF LUMBER
TOP 30 15	TOP 7-8=0.954	TOP CHORD:2X4 SS 19 SP
BOTT 0 10	BOTT 14-15=0.379	BOT CHORD:2X6 No.2D 19 SP
	LL.DEFL.@14=0.10 < L/360	WEBS :2X4 No.3 19 SP

STR.INC.: LUMB = 1.33 PLATE = 1.33

REPETITIVE STRESSES USED

SPACING : 24.0 in. o. c.

NO. OF MEMBERS = 1

CHORD: 5-6; 6-7; 7-8 TO BE 2X4 No.1D 19 SP

CHORD: 8-9; 9-10 TO BE 2X4 No.2ND 19 SP

WEB: 1-2 TO BE 2X6 No.2D 19 SP

BOTH END VERTICALS ARE NOT EXPOSED.

WEB 3-1; 4-14; 6-13; 8-12; 9-12; 9-11 BRACED at 1/2 POINTS AS SHOWN ABOVE

Note:Use 1x4 or 2x3 Cont.Bracing conn.w/2-Bd nails,or T-brace of same size and grade as web conn. to narrow face w/10d nails 6 in. o.c

TRUSS HAS BEEN CHECKED FOR 145 M.P.H. WIND LOAD, WALL HEIGHT 16 FT, ENCLOSED BLDG. TYPE, ASSUMING 10 PSF TOTAL D.L.(7+3) (SBC-97).

DEFLECTION(IN.) L.L= 0.10,D.L=0.08,T.L=0.18

PLATES ARE MITEK M20-249,200 MANUFACTURED FROM ASTM A 446 GRD A GALVANIZED STEEL(EXCEPT AS SHOWN)

PLATE MUST BE INSTALLED ON EA. FACE OF JOINT,SYMMETRICALLY(EXCEPT AS SHOWN)DESIGN CONFORMS W/NDS DESIGN SPECS, SSBC,ANSI/TP1-95

THIS DESIGN IS FOR TRUSS FABRICATION ONLY.FOR PERMANENT AND TEMPORARY BRACING(WHICH IS ALWAYS REQD)CONSULT BLDG ARCHITECT OR ENGINEER.

MAR 01 2001

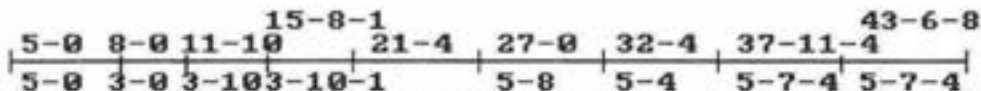
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42

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PROVIDE FOR 2630 LBS UPLIFT AT JOINT 18 (1.04)
PROVIDE FOR 1939 LBS UPLIFT AT JOINT 11 (1.00)
PROVIDE FOR 533 LBS UPLIFT AT JOINT 1 (1.24)
PROVIDE FOR 1358 LBS HORIZ. REACTION AT JOINT 18
PLATE OFFSETS (X=LEFT,Y=TOP): [[4=3.3],[6=3.3],[13=3.2],[15=3.2],[18=5.2]

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LOADING (PSF)      MAX STRESSES      MINIMUM GRADE OF LUMBER
      L      D      TOP      5-6=0.876      TOP CHORD:2X4      SS 19 SP
TOP      30      15      BOTTL 15-16=0.315      BOT CHORD:2X6      No.2D 19 SP
BOTTL      0      10      LL.DEFL.@15=0.11 < L/360      WEBS      :2X4      No.3 19 SP
=====
STR.INC.: LUMB = 1.33 PLATE = 1.33      SPACING : 24.0 in. o. c.
REPETITIVE STRESSES USED      NO. OF MEMBERS = 1

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MAR 06 2001

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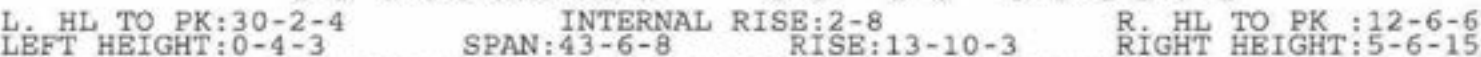
5 12:24:51 2001
Family # : spec.
Top Pitch : 67/12
Bot pitch : 90/12

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REACTIONS - SIZE

43

PLATE OFFSETS (X=LEFT,Y=TOP): ([4=3,3],[6=3,3],[13=3,2],[15=3,2],[18=5,2])



THIS DESIGN IS FOR TRUSS FABRICATION ONLY. FOR PERMANENT AND TEMPORARY BRACING (WHICH IS ALWAYS REQUIRED) CONSULT BLDG ARCHITECT OR ENGINEER.

MAR 06 2005

====<<<<ACES-32 Ver. 1.1>>>>=====[001849]=====<<<<FL.QUALITY-MMH>>>>=====

Customer : FOGLIA Thu Mar 1 09:48:14 2001

Project #: 00469 Truss ID : S32 Family #: spec.

Span : 43-6-8 B.Span : 43-6-8 Quant. : 1 Top Pitch : 6/12

Bot pitch : 90/12

====81d(1/21/2000),v1.1=====

FORCES - GRAVITY LOADS				REACTIONS - SIZE			
1-2=-100	7-8=-1502	10-11= 0	16-17= 29	2-18=-6	5-14=-66	8-11=-807	16=-2528 8.00
2-3= 830	8-9=-1386	11-12= 1242	17-18= 0	2-16=-835	5-13=-459	9-11= 1560	10=-1933 2.00
3-4=-2139	9-10=-1865	12-13= 1833	18-1= 90	3-16=-2452	6-13= 418	16-18= 123	1=-429 4.00
4-5=-2449		13-14= 2191		3-15= 2511	6-12=-903		
5-6=-2049		14-15= 1914		4-15=-713	7-12= 769		
6-7=-1494		15-16=-467		4-14= 333	8-12= 149		

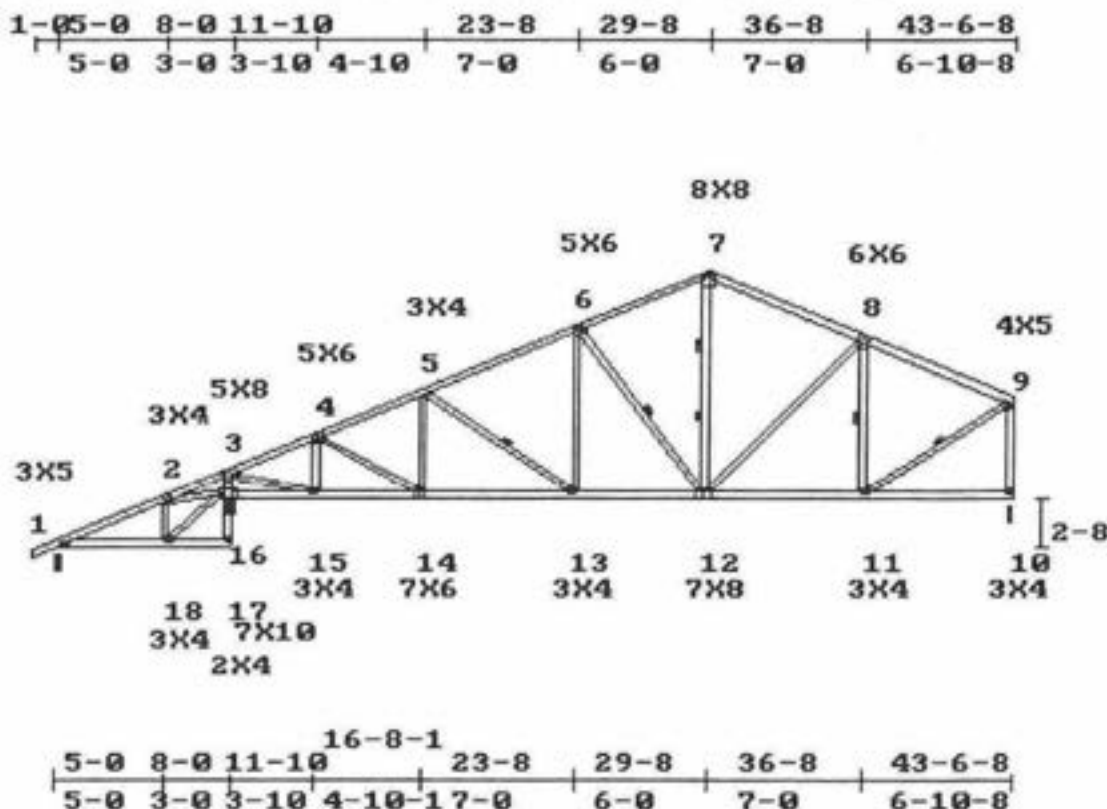
PROVIDE FOR 2575 LBS UPLIFT AT JOINT 16 (1.02)

PROVIDE FOR 1859 LBS UPLIFT AT JOINT 10 (0.96)

PROVIDE FOR 534 LBS UPLIFT AT JOINT 1 (1.24)

PROVIDE FOR 1402 LBS HORIZ. REACTION AT JOINT 16

PLATE OFFSETS (X=LEFT,Y=TOP): [j4=3,3], [j6=3,3], [j7=4,1.5], [j8=3,4], [j12=4,2], [j14=3,2], [j16=5,2],



L. HL TO PK:33-2 INTERNAL RISE:2-8 R. HL TO PK:15-6-2

LEFT HEIGHT:0-4-3 SPAN:43-6-8 RISE:15-2-3 RIGHT HEIGHT:5-6-15

LOADING (PSF)		MAX STRESSES		MINIMUM GRADE OF LUMBER	
TOP	30	TOP	5-6=0.983	TOP CHORD:	2X4 SS 19 SP
BOTT	0	BOTT	13-14=0.333	BOT CHORD:	2X6 No.2D 19 SP
		LL.DEFL.@13=0.11	< L/360	WEBS:	2X4 No.3 19 SP

STR.INC.: LUMB = 1.33 PLATE = 1.33 SPACING : 24.0 in. o. c.

REPETITIVE STRESSES USED NO. OF MEMBERS = 1

CHORD: 7-8; 8-9 TO BE 2X6 No.2D 19 SP

CHORD: 16-17 TO BE 2X4 No.3 19 SP

RIGHT END VERTICAL IS NOT EXPOSED.

WEB 5-13; 6-12; 8-11; 9-11 BRACED AT 1/2 POINTS AS SHOWN ABOVE

WEB 7-12 BRACED AT 1/3 POINTS AS SHOWN ABOVE

Note:Use 1x4 or 2x3 Cont.Bracing conn.w/2-8d nails,or T-brace of same size and grade as web conn. to narrow face w/10d nails 6 in. o.c

TRUSS HAS BEEN CHECKED FOR 145 M.P.H. WIND LOAD, WALL HEIGHT 16 FT, ENCLOSED BLDG. TYPE, ASSUMING 10 PSF TOTAL D.L.(7*3) (SBC-97).

DEFLECTION(IN.) L.L= 0.11,D.L=0.09,T.L=0.19

PLATES ARE MITEK M20-249,200 MANUFACTURED FROM ASTM A 446 GRD A GALVANIZED STEEL(EXCEPT AS SHOWN)

PLATE MUST BE INSTALLED ON EA. FACE OF JOINT,SYMMETRICALLY(EXCEPT AS SHOWN)DESIGN CONFORMS W/ND S DESIGN SPECS, SSBC,ANSI/TPI-95

THIS DESIGN IS FOR TRUSS FABRICATION ONLY.FOR PERMANENT AND TEMPORARY BRACING(WHICH IS ALWAYS REGO)CONSULT BLDG ARCHITECT OR ENGINEER.

====<<<ACES-32 Ver. 1.1>>>===== [001850] =====<<<FL.QUALITY-MMH>>>=====

Customer : FOGLIA Thu Mar 1 09:49:12 2001

Project # : 00469 Truss ID : T7 Family # : spec.

Span : 34-5-14 B.Span : 34-6-14 Quant. : 1 Top Pitch : 6/12

==Bld(1/21/2000),v1.1=====

FORCES - GRAVITY LOADS

1-2=-1820	7-8=-1822	8-9= 0	2-12= 1515	6-10=-210
2-3=-1345		9-10= 1985	3-12=-779	6-9=-410
3-4=-1447		10-11= 1814	3-11= 140	7-9= 2058
4-5=-1447		11-12= 1206	4-11= 665	
5-6=-2028		12-1= 0	5-11=-819	
6-7=-2215			5-10= 263	

REACTIONS - SIZE

1=-1888	2.00
8=-1888	2.00

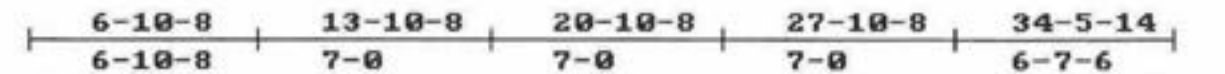
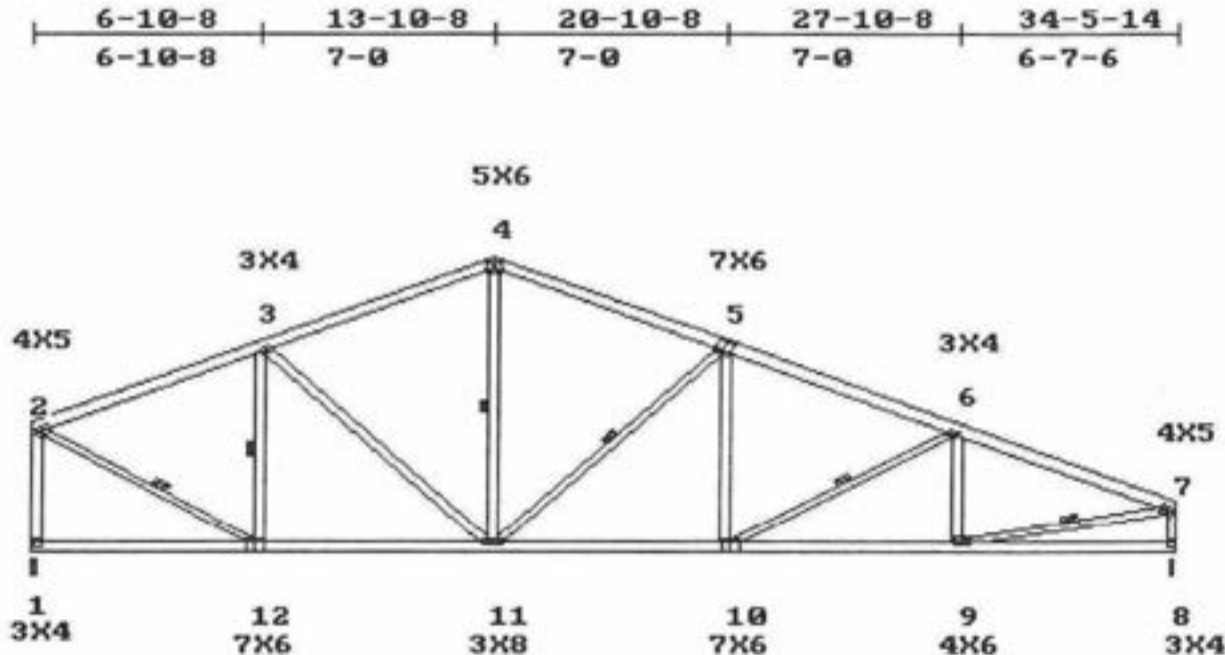
KS

PROVIDE FOR 1822 LBS UPLIFT AT JOINT 1 (0.96)

PROVIDE FOR 1875 LBS UPLIFT AT JOINT 8 (0.99)

PROVIDE FOR 761 LBS HORIZ. REACTION AT JOINT 1

PLATE OFFSETS (X=LEFT,Y=TOP): [j5=3,5], [j9=3,2.5], [j10=3,2], [j12=3,2],



L. HL TO PK:15-6-2 LEFT HEIGHT:5-6-15 SPAN:34-5-14 RISE:12-6-3 R. HL TO PK :23-0-9 RIGHT HEIGHT:2-2-8

LOADING (PSF)		MAX STRESSES	MINIMUM GRADE OF LUMBER
TOP	30	5-6=0.982	TOP CHORD:2X6 No.2D 19 SP
BOTT	0	9-10=0.321	BOT CHORD:2X6 No.2D 19 SP
		LL.DEFL.@10=0.07 < L/360	WEBS :2X4 No.3 19 SP

STR.INC.: LUMB = 1.33 PLATE = 1.33 SPACING : 24.0 in. o. c.

REPETITIVE STRESSES USED NO. OF MEMBERS = 1

BOTH END VERTICALS ARE NOT EXPOSED.

WEB 2-12; 3-12; 4-11; 5-11; 6-10; 7-9 BRACED at 1/2 POINTS AS SHOWN ABOVE

Note:Use 1x4 or 2x3 Cont.Bracing conn.w/2-8d nails,or T-brace of same size and grade as web conn. to narrow face w/10d nails 6 in. o.c

TRUSS HAS BEEN CHECKED FOR 145 M.P.H. WIND LOAD, WALL HEIGHT 16 FT, ENCLOSED BLDG. TYPE, ASSUMING 10 PSF TOTAL D.L.(7+3) (SBC-97).

DEFLECTION(IN.) L.L= 0.07,D.L=0.06,T.L=0.13

PLATES ARE MITEK M20-249,200 MANUFACTURED FROM ASTM A 446 GRD A GALVANIZED STEEL(EXCEPT AS SHOWN)

PLATE MUST BE INSTALLED ON EA. FACE OF JOINT,SYMMETRICALLY(EXCEPT AS SHOWN)DESIGN CONFORMS W/WDS DESIGN SPECS, SIBC,ANSI/TPI-95

THIS DESIGN IS FOR TRUSS FABRICATION ONLY.FOR PERMANENT AND TEMPORARY BRACING(WHICH IS ALWAYS REGD)CONSULT BLDG ARCHITECT OR ENGINEER.

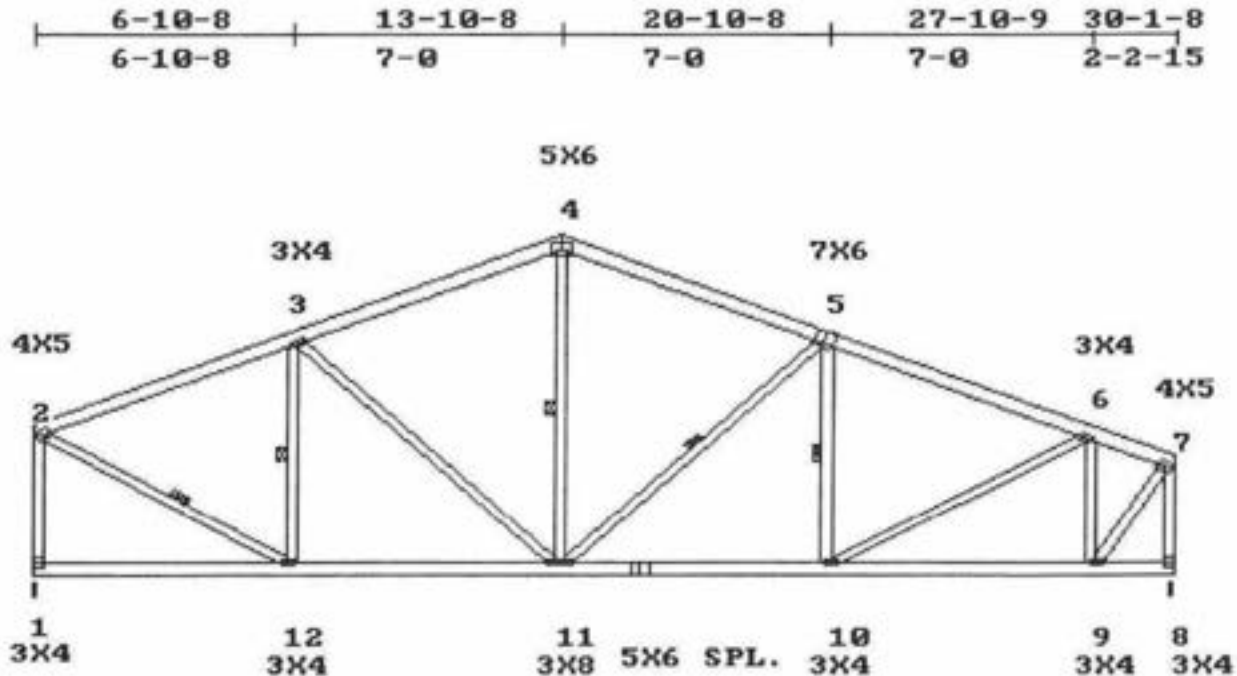
MAR 01 2001

====<<<ACES-32 Ver. 1.1>>>=====[001851]====<<<FL.QUALITY-MMH>>>=====
 Customer : FOGLIA Thu Mar 1 09:49:44 2001
 Project #: 00469 Truss ID : T7A Family #: spec.
 Span : 30-1-8 B.Span : 30-2-8 Quant. : 1 Top Pitch : 6/12
 ==8ld(1/21/2000),v1.1=====

FORCES - GRAVITY LOADS			REACTIONS - SIZE	
1-2=-1580	7-8=-1626	8-9= 0	2-12= 1275	6-10= 703
2-3=-1132		9-10= 658	3-12=-634	6-9=-1118
3-4=-1140		10-11= 1229	3-11= 8	7-9= 1377
4-5=-1140		11-12= 1014	4-11= 389	
5-6=-1374		12-1= 0	5-11=-330	
6-7=-731			5-10=-270	

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PROVIDE FOR 1600 LBS UPLIFT AT JOINT 1 (0.97)
 PROVIDE FOR 1604 LBS UPLIFT AT JOINT 8 (0.97)
 PROVIDE FOR 449 LBS HORIZ. REACTION AT JOINT 1
 PLATE OFFSETS (X=LEFT,Y=TOP):[J5=3,5],



6-10-8	13-10-8	20-10-8	27-10-9	30-1-8
6-10-8	7-0	7-0	7-0	2-2-15

L. HL TO PK:15-6-2 LEFT HEIGHT:5-6-15 SPAN:30-1-8 RISE:12-6-3 R. HL TO PK:18-2 RIGHT HEIGHT:4-4-11

LOADING (PSF)		MAX STRESSES		MINIMUM GRADE OF LUMBER	
TOP	30	BOTT	10-11=0.232	TOP CHORD:	2X6 No.2D 19 SP
BOTT	0	LL.DEFL.	@10=0.04 < L/360	BOT CHORD:	2X6 No.2D 19 SP
				WEBS	:2X4 No.3 19 SP

STR.INC.: LUMB = 1.33 PLATE = 1.33 SPACING : 24.0 in. o. c.
 REPETITIVE STRESSES USED NO. OF MEMBERS = 1

BOTH END VERTICALS ARE NOT EXPOSED.
 WEB 2-12; 3-12; 4-11; 5-11; 5-10 BRACED AT 1/2 POINTS AS SHOWN ABOVE
 Note:Use 1x4 or 2x3 Cont.Bracing conn.w/2-8d nails,or T-brace of same size and grade as web conn. to narrow face w/10d nails 6 in. o.c
 TRUSS HAS BEEN CHECKED FOR 145 M.P.H. WIND LOAD, WALL HEIGHT 16 FT, ENCLOSED BLDG. TYPE, ASSUMING 10 PSF TOTAL D.L.(7+3) (SBC-97).
 DEFLECTION(IN.) L.L= 0.04,D.L=0.03,T.L=0.07
 PLATES ARE MITEK M20-249,200 MANUFACTURED FROM ASTM A 446 GRD A GALVANIZED STEEL(EXCEPT AS SHOWN)
 PLATE MUST BE INSTALLED ON EA. FACE OF JOINT,SYMMETRICALLY(EXCEPT AS SHOWN)DESIGN CONFORMS W/NDI DESIGN SPECS, SIBC,ANSI/TPI-95
 THIS DESIGN IS FOR TRUSS FABRICATION ONLY.FOR PERMANENT AND TEMPORARY BRACING(WHICH IS ALWAYS REQD)CONSULT BLDG ARCHITECT OR ENGINEER.

[Signature]
 MAR 01 2001

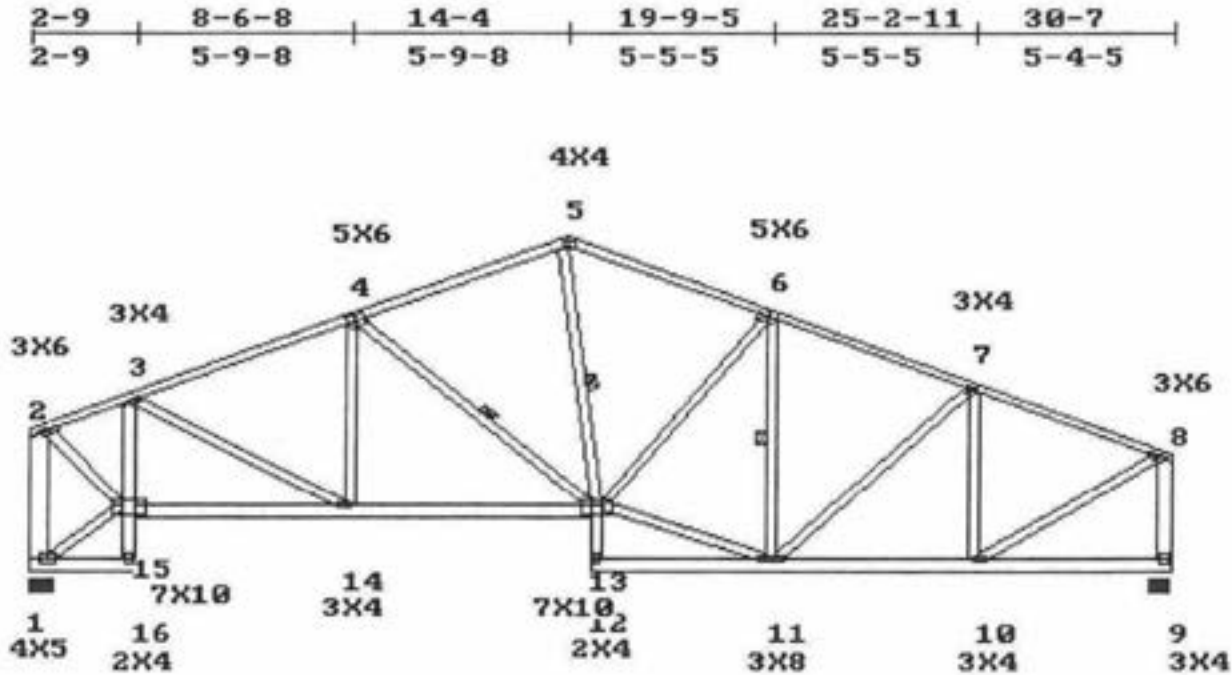
====<<<<ACES-32 Ver. 1.1>>>>=====[001852]====<<<<FL.QUALITY-MMH>>>>=====
 Customer : FOGLIA Thu Mar 1 09:50:03 2001
 Project #: 00469 Truss ID : S33 Family #: spec.
 Span : 30-7 B.Span : 40-8 Quant. : 1 Top Pitch : 6/12
 Bot pitch : 90/12

==Bld(1/21/2000),v1.1==

FORCES - GRAVITY LOADS						REACTIONS - SIZE	
1-2=1633	7-8=1181	9-10=0	15-16=24	2-15=1385	6-13=108	15-1=0	1=1657 8.00
2-3=948	8-9=1606	10-11=1056	16-1=0	3-15=1013	6-11=610		9=1657 8.00
3-4=1543		11-12=0		3-14=622	7-11=243		
4-5=1350		12-13=46		4-14=247	7-10=741		
5-6=1416		13-14=1380		4-13=255	8-10=1353		
6-7=1352		14-15=882		5-13=734	13-11=1317		

(47)

PROVIDE FOR 1609 LBS UPLIFT AT JOINT 1 (0.97)
 PROVIDE FOR 1612 LBS UPLIFT AT JOINT 9 (0.97)
 PROVIDE FOR 424 LBS HORIZ. REACTION AT JOINT 1
 PLATE OFFSETS (X=LEFT,Y=TOP): [j4=3,3], [j6=3,3], [j13=5,2], [j15=5,2],



2-9 | 8-6-8 | 15-0 | 19-9-5 | 25-2-11 | 30-7
 2-9 | 5-9-8 | 6-5-8 | 4-9-5 | 5-5-6 | 5-4-5

L. HL TO PK:16-0-5 INTERNAL RISE:2-0 R. HL TO PK:18-2
 LEFT HEIGHT:5-4-3 SPAN:30-7 RISE:12-6-3 RIGHT HEIGHT:4-4-11

LOADING (PSF)		MAX STRESSES		MINIMUM GRADE OF LUMBER	
TOP	30	TOP	6-7=0.989	TOP CHORD:2X4	SS 19 SP
BOTT	0	BOTT	13-14=0.229	BOT CHORD:2X6	No.2D 19 SP
		LL.DEFL.@13=0.06	< L/360	WEBS	:2X4 No.3 19 SP

STR.INC.: LUMB = 1.33 PLATE = 1.33 SPACING : 24.0 in. o. c.
 REPETITIVE STRESSES USED NO. OF MEMBERS = 1

CHORD: 6-7; 7-8 TO BE 2X4 No.1D 19 SP
 CHORD: 12-13; 15-16 TO BE 2X4 No.3 19 SP
 WEB: 1-2; 8-9 TO BE 2X6 No.2D 19 SP
 BOTH END VERTICALS ARE NOT EXPOSED.
 WEB 4-13; 5-13; 6-11 BRACED AT 1/2 POINTS AS SHOWN ABOVE
 Note:Use 1x4 or 2x3 Cont.Bracing conn.w/2-8d nails,or T-brace of same size and grade as web conn. to narrow face w/10d nails 6 in. o.c
 TRUSS HAS BEEN CHECKED FOR 145 M.P.H. WIND LOAD, WALL HEIGHT 16 FT, ENCLOSED BLDG. TYPE, ASSUMING 10 PSF TOTAL D.L.(7+3) (SBC-97).
 DEFLECTION(IN.) L.L= 0.06,D.L=0.05,T.L=0.10
 PLATES ARE MITEK M20-249,200 MANUFACTURED FROM ASTM A 446 GRD A GALVANIZED STEEL(EXCEPT AS SHOWN)
 PLATE MUST BE INSTALLED ON EA. FACE OF JOINT,SYMMETRICALLY(EXCEPT AS SHOWN)DESIGN CONFORMS W/NDS DESIGN SPECS, SBC,ANSI/TP1-95
 THIS DESIGN IS FOR TRUSS FABRICATION ONLY.FOR PERMANENT AND TEMPORARY BRACING(WHICH IS ALWAYS REQD)CONSULT BLDG ARCHITECT OR ENGINEER.

[Signature]
 MAR 01 2001

====<<<<ACES-32 Ver. 1.1>>>>=====[001853]=====<<<<FL.QUALITY-MMH>>>>=====

Customer : FOGLIA Thu Mar 1 09:50:37 2001

Project #: 00469 Truss ID : S34 Family #: spec.

Span : 40-7 B.Span : 40-8 Quant. : 1 Top Pitch : 6/12

Bot pitch : 90/12

==8ld(1/21/2000),v1.1=====

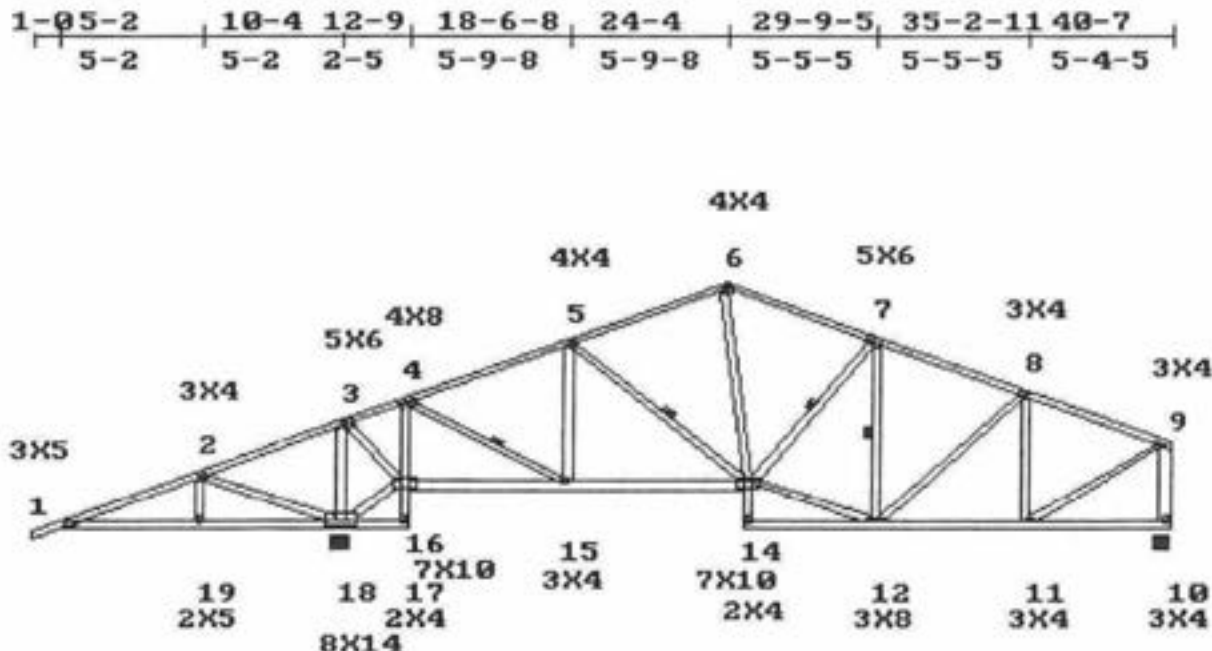
FORCES - GRAVITY LOADS								REACTIONS - SIZE	
1-2= 893	7-8=-1051	10-11= 0	16-17= 23	2-19= 103	5-15=-847	8-11=-594	18=-3131	8.00	
2-3= 1530	8-9=-975	11-12= 872	17-18= 0	2-18=-631	5-14= 303	9-11= 1118	10=-1418	8.00	
3-4= 795	9-10=-1366	12-13= 0	18-19=-802	3-18=-1574	6-14= 373	14-12= 1024			
4-5=-735		13-14= 46	19-1=-802	3-16= 1117	7-14=-91	16-18=-1823			
5-6=-964		14-15= 658		4-16=-2028	7-12=-389				
6-7=-998		15-16=-643		4-15= 1624	8-12= 108				

PROVIDE FOR 5086 LBS UPLIFT AT JOINT 18 (1.62)

PROVIDE FOR 1180 LBS UPLIFT AT JOINT 10 (0.83)

PROVIDE FOR 914 LBS HORIZ. REACTION AT JOINT 18

PLATE OFFSETS (X=LEFT,Y=TOP):[j3=3,3],[j7=3,3],[j14=5,2],[j16=5,2],[j19=3,1],



5-2	10-4	12-9	18-6-8	25-0	29-9-5	35-2-11	40-7
5-2	5-2	2-5	5-9-8	6-5-8	4-9-5	5-5-6	5-4-5

L. HL TO PK:27-2-7 INTERNAL RISE:2-0 R. HL TO PK:18-2

LEFT HEIGHT:0-4-3 SPAN:40-7 RISE:12-6-3 RIGHT HEIGHT:4-4-11

LOADING (PSF)		MAX STRESSES		MINIMUM GRADE OF LUMBER	
TOP	30	TOP	5-6=0.997	TOP CHORD:2X4	No.2ND 19 SP
BOTT	0	BOTT	18-19=0.557	BOT CHORD:2X6	No.2D 19 SP
		LL.DEFL.@19=0.08	< L/360	WEBS	:2X4 No.3 19 SP

STR.INC.: LUMB = 1.33 PLATE = 1.33 SPACING : 24.0 in. o. c.

REPETITIVE STRESSES USED NO. OF MEMBERS = 1

CHORD: 3-4; 4-5; 5-6; 6-7; 7-8; 8-9 TO BE 2X4 No.10 19 SP

CHORD: 13-14; 16-17 TO BE 2X4 No.3 19 SP

WEB: 9-10; 3-18 TO BE 2X6 No.2D 19 SP

WEB: 16-18 TO BE 2X4 No.2ND 19 SP

RIGHT END VERTICAL IS NOT EXPOSED.

WEB 4-15; 5-14; 7-14; 7-12 BRACED AT 1/2 POINTS AS SHOWN ABOVE

Note:Use 1x4 or 2x3 Cont.Bracing conn.w/2-8d nails,or T-brace of same size and grade as web conn. to narrow face w/10d nails 6 in. o.c

TRUSS HAS BEEN CHECKED FOR 145 M.P.H. WIND LOAD, WALL HEIGHT 16 FT, ENCLOSED BLDG. TYPE, ASSUMING 10 PSF TOTAL D.L.(7+3) (SBC-97).

DEFLECTION(IN.) L.L= 0.08,D.L=0.06,T.L=0.14

PLATES ARE MITEK M20-249,200 MANUFACTURED FROM ASTM A 446 GRD A GALVANIZED STEEL(EXCEPT AS SHOWN)

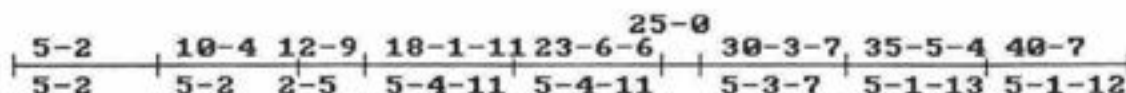
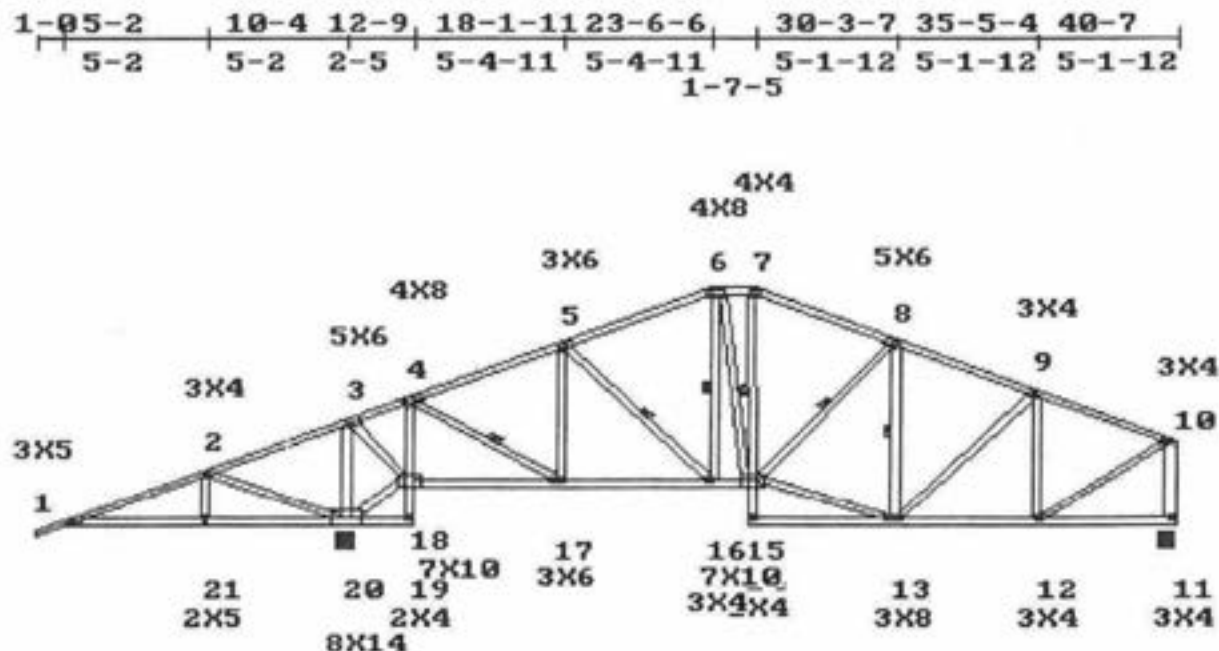
PLATE MUST BE INSTALLED ON EA. FACE OF JOINT,SYMMETRICALLY(EXCEPT AS SHOWN)DESIGN CONFORMS W/NDS DESIGN SPECS, S58C,ANSI/TPI-95

THIS DESIGN IS FOR TRUSS FABRICATION ONLY.FOR PERMANENT AND TEMPORARY BRACING(WHICH IS ALWAYS REQD)CONSULT BLDG ARCHITECT OR ENGINEER.

MAR 01 2001

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PROVIDE FOR 5110 LBS UPLIFT AT JOINT 20 (1.63)
 PROVIDE FOR 1201 LBS UPLIFT AT JOINT 11 (0.85)
 PROVIDE FOR 899 LBS HORIZ. REACTION AT JOINT 20
 PLATE OFFSETS (X=LEFT,Y=TOP): [3=3,3], [8=3,3], [15=5,2], [18=5,2], [21=3,1]



LOADING (PSF)		MAX STRESSES		MINIMUM GRADE OF LUMBER	
L	D	TOP	BOTT	TOP CHORD	NO. 2ND
30	15	5-6=0.951	20-21=0.557	2X4	19 SP
0	10	LL.DEFL.@21=0.08 < L/360		2X6	NO.2D 19 SP
				WEBS	2X4 NO.3 19 SP
STR.INC.: LUMB = 1.33 PLATE = 1.33				SPACING : 24.0 in. o. c.	
REPETITIVE STRESSES USED				NO. OF MEMBERS = 1	

CHORD: 3-4; 4-5; 5-6; 7-8; 8-9; 9-10 TO BE 2X4 No.10 19 SP
CHORD: 14-15; 18-19 TO BE 2X4 No.3 19 SP
WEB: 10-11; 3-20 TO BE 2X6 No.20 19 SP
WEB: 18-20 TO BE 2X4 No.2ND 19 SP
RIGHT END VERTICAL IS NOT EXPOSED.
WEB 4-17; 5-16; 6-16; 6-15; 8-15; 8-13 BRACED AT 1/2 POINTS AS SHOWN ABOVE
Note: Use 1x4 or 2x3 Cont. Bracing conn. w/2-8d nails, or T-brace of same size and grade as web conn. to narrow face w/10d nails 6 in. o.c.
TRUSS HAS BEEN CHECKED FOR 145 M.P.H. WIND LOAD, WALL HEIGHT 16 FT, ENCLOSED BLDG. TYPE, ASSUMING 10 PSF TOTAL D.L.(7+3) (SBC-97).
DEFLECTION(IN.) L.L= 0.08, D.L=0.07, T.L=0.14
PLATES ARE MITEK M20-249,200 MANUFACTURED FROM ASTM A 446 GRD A GALVANIZED STEEL(EXCEPT AS SHOWN)
PLATE MUST BE INSTALLED ON EA. FACE OF JOINT, SYMMETRICALLY(EXCEPT AS SHOWN) DESIGN CONFORMS W/WDG DESIGN SPECS, SSBC, ANSI/TPI-95
THIS DESIGN IS FOR TRUSS FABRICATION ONLY, FOR PERMANENT AND TEMPORARY BRACING(WHICH IS ALWAYS REQD) CONSULT BLDG ARCHITECT OR ENGINEER.

====<<<<ACES-32 Ver. 1.1>>>>===== [001855] =====<<<<FL.QUALITY-MMH>>>>=====

Customer : FOGLIA Thu Mar 1 09:51:21 2001

Project #: 00469 Truss ID : S41 Family #: spec.

Span : 40-7 B.Span : 40-8 Quant. : 1 Top Pitch : 6/12

Bot pitch : 90/12

==81d(1/21/2000),v1.1=====

FORCES - GRAVITY LOADS						REACTIONS - SIZE	
1-2= 893	7-8= 997	11-12= 0	17-18= 27	2-20= 103	5-16= 606	9-13= 126	19= 3131 8.00
2-3= 1529	8-9= 951	12-13= 935	18-19= 0	2-19= 631	6-16= 88	9-12= 461	11= 1418 8.00
3-4= 600	9-10= 1045	13-14= 0	19-20= 801	3-19= 1749	7-16= 511	10-12= 1107	
4-5= 588	10-11= 1353	14-15= 24	20-1= 801	3-17= 1280	7-15= 230	15-13= 1108	
5-6= 922		15-16= 993		4-17= 321	8-15= 557	17-19= 1709	
6-7= 823		16-17= 500		5-17= 1873	8-13= 527		

50

PROVIDE FOR 5163 LBS UPLIFT AT JOINT 19 (1.65)

PROVIDE FOR 1249 LBS UPLIFT AT JOINT 11 (0.88)

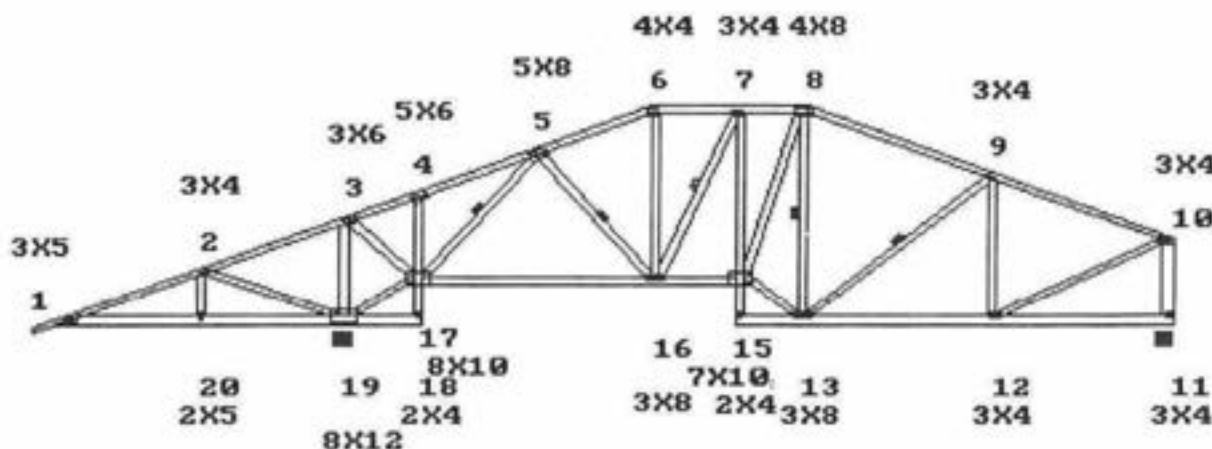
PROVIDE FOR 858 LBS HORIZ. REACTION AT JOINT 19

PLATE OFFSETS (X=LEFT,Y=TOP): [j4=3,3], [j15=5,2], [j17=5,3], [j20=3,1],

1-05-2 | 10-4 | 13-1-12 | 21-6-6 | 27-1-11 | 40-7

5-2 | 5-2 | 2-9-12 | 4-2-5 | 2-6-7 | 6-8-11 | 6-8-11

4-2-5 | 3-0-14



5-2 | 10-4 | 13-1-12 | 21-6-6 | 27-1-11 | 33-10-6 | 40-7

5-2 | 5-2 | 2-9-12 | 8-4-10 | 3-0-14 | 6-8-11 | 6-8-11

L. HL TO PK:24-0-14

LEFT HEIGHT:0-4-3

INTERNAL RISE:1-11-15

SPAN:40-7

RISE:11-1-7

R. HL TO PK:15-0-6

RIGHT HEIGHT:4-4-12

LOADING (PSF)

L D

TOP 30 15

BOTT 0 10

MAX STRESSES

TOP 8-9=0.944

BOTT 19-20=0.557

LL.DEFL.@20=0.08 < L/360

MINIMUM GRADE OF LUMBER

TOP CHORD:2X4 No.2ND 19 SP

BOT CHORD:2X6 No.2D 19 SP

WEBS :2X4 No.3 19 SP

STR.INC.: LUMB = 1.33 PLATE = 1.33

REPETITIVE STRESSES USED

SPACING : 24.0 in. o. c.

NO. OF MEMBERS = 1

CHORD: 8-9; 9-10 TO BE 2X4 SS 19 SP

CHORD: 14-15; 17-18 TO BE 2X4 No.3 19 SP

WEB: 10-11; 3-19 TO BE 2X6 No.2D 19 SP

WEB: 17-19 TO BE 2X4 No.2ND 19 SP

RIGHT END VERTICAL IS NOT EXPOSED.

WEB 5-17; 5-16; 7-16; 8-13; 9-13 BRACED AT 1/2 POINTS AS SHOWN ABOVE

Note:Use 1x4 or 2x3 Cont.Bracing conn.w/2-8d nails,or T-brace of same size and grade as web conn. to narrow face w/10d nails 6 in. o.c

TRUSS HAS BEEN CHECKED FOR 145 M.P.H. WIND LOAD, WALL HEIGHT 16 FT, ENCLOSED BLDG. TYPE, ASSUMING 10 PSF TOTAL D.L.(7+3) (SBC-97).

DEFLECTION(IN.) L.L= 0.08,D.L=0.06,T.L=0.14

PLATES ARE MITEK M20-249,200 MANUFACTURED FROM ASTM A 446 GRD A GALVANIZED STEEL(EXCEPT AS SHOWN)

PLATE MUST BE INSTALLED ON EA. FACE OF JOINT,SYMMETRICALLY(EXCEPT AS SHOWN)DESIGN CONFORMS W/NDIS DESIGN SPECS, SSSC,ANSI/TP1-95

THIS DESIGN IS FOR TRUSS FABRICATION ONLY.FOR PERMANENT AND TEMPORARY BRACING(WHICH IS ALWAYS REQD)CONSULT BLDG ARCHITECT OR ENGINEER.

MAR 01 2001

==<<<<ACES-32 Ver. 1.1>>>>===== [001856] =====<<<<FL.QUALITY-MMH>>>>=====

Customer : FOGLIA Thu Mar 1 09:51:42 2001

Project #: 00469 Truss ID : H40A Family #: spec.

Span : 44-5-2 B.Span : 44-6-2 Quant. : 1 Top Pitch : 6/12

==Bld(1/21/2000),v1.1=====

FORCES - GRAVITY LOADS				REACTIONS - SIZE			
1-2=893	7-8=1432	11-12=0	17-1=-801	2-17=103	6-15=-831	9-12=-565	16=-3316 8.00
2-3=1529	8-9=1720	12-13=1454		2-16=-631	6-14=349	10-12=1599	11=-1656 8.00
3-4=1529	9-10=-1626	13-14=1538		3-16=-439	7-14=197		
4-5=-847	10-11=-1607	14-15=1124		4-16=-2887	8-14=-446		
5-6=-757		15-16=153		4-15=1147	8-13=27		
6-7=-1279		16-17=-801		5-15=-42	9-13=113		

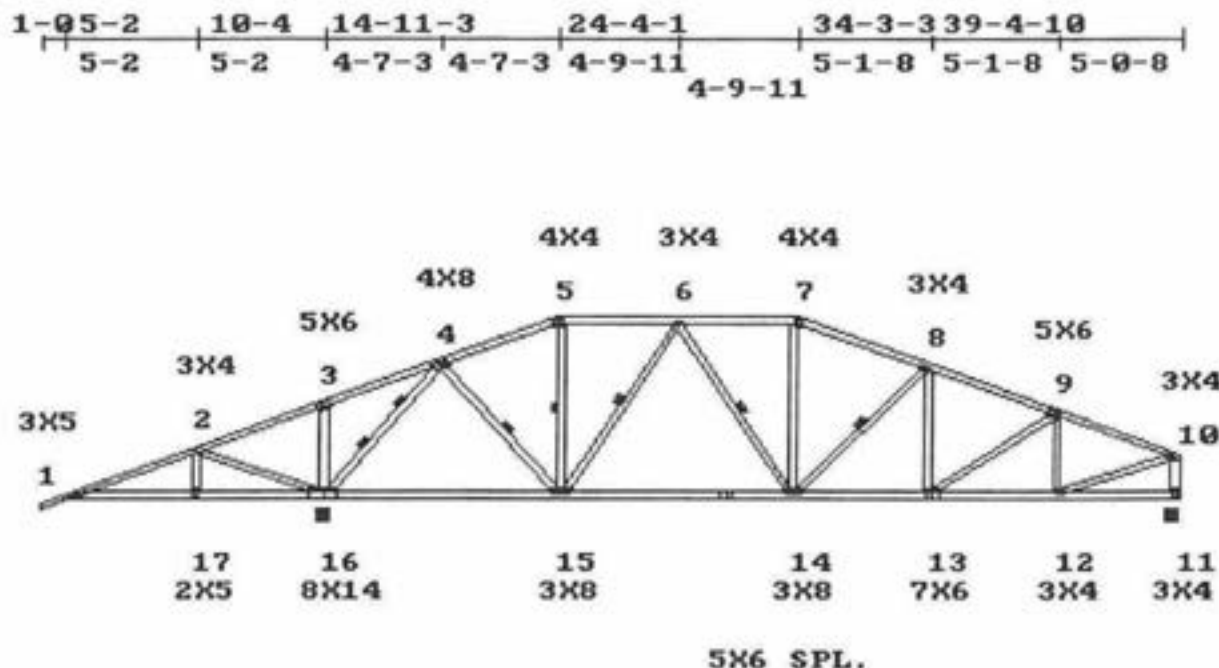
(51)

PROVIDE FOR 5363 LBS UPLIFT AT JOINT 16 (1.62)

PROVIDE FOR 1564 LBS UPLIFT AT JOINT 11 (0.94)

PROVIDE FOR 621 LBS HORIZ. REACTION AT JOINT 16

PLATE OFFSETS (X=LEFT,Y=TOP): [J3=3,3], [J9=3,3], [J13=3,2], [J16=7,3], [J17=3,1],



5-2 10-4 19-6-6 29-1-11 34-3-3 39-4-10 44-5-2

5-2 5-2 9-2-6 9-7-5 5-1-8 5-1-8 5-0-8

L. HL TO PK:21-10-1 R. HL TO PK:17-1-1

LEFT HEIGHT:0-4-3 SPAN:44-5-2 RISE:10-1-7 RIGHT HEIGHT:2-5-11

LOADING (PSF)		MAX STRESSES		MINIMUM GRADE OF LUMBER	
TOP	30	TOP	8-9=0.971	TOP CHORD:	2X4 No.2ND 19 SP
BOTT	0	BOTT	16-17=0.639	BOT CHORD:	2X6 No.2D 19 SP
		LL.DEFL.@17=0.07	< L/360	WEBS:	2X4 No.3 19 SP

STR.INC.: LUMB = 1.33 PLATE = 1.33

REPETITIVE STRESSES USED

SPACING : 24.0 in. o. c.

NO. OF MEMBERS = 1

CHORD: 5-6; 6-7; 7-8; 8-9; 9-10 TO BE 2X4 No.1D 19 SP

CHORD: 16-17; 17-1 TO BE 2X6 SS 19 SP

WEB: 10-11 TO BE 2X6 No.2D 19 SP

WEB: 3-16 TO BE 2X6 SS 19 SP

WEB: 4-16 TO BE 2X4 No.2ND 19 SP

RIGHT END VERTICAL IS NOT EXPOSED.

WEB 4-15; 5-15; 6-15; 6-14; 8-14 BRACED at 1/2 POINTS AS SHOWN ABOVE

WEB 4-16 BRACED at 1/3 POINTS AS SHOWN ABOVE

Note:Use 1x4 or 2x3 Cont.Bracing conn.w/2-8d nails,or T-brace of same size and grade as web conn. to narrow face w/10d nails 6 in. o.c

TRUSS HAS BEEN CHECKED FOR 145 M.P.H. WIND LOAD, WALL HEIGHT 16 FT, ENCLOSED BLDG. TYPE, ASSUMING 10 PSF TOTAL D.L.(7+3) (SBC-97).

DEFLECTION(IN.) L.L= 0.07,D.L=0.06,T.L=0.13

PLATES ARE MITEK M20-249,200 MANUFACTURED FROM ASTM A 446 GRD A GALVANIZED STEEL(EXCEPT AS SHOWN)

PLATE MUST BE INSTALLED ON EA. FACE OF JOINT,SYMMETRICALLY(EXCEPT AS SHOWN)DESIGN CONFORMS W/NDS DESIGN SPECS, SSB,ANSI/TPI-95

THIS DESIGN IS FOR TRUSS FABRICATION ONLY.FOR PERMANENT AND TEMPORARY BRACING(WHICH IS ALWAYS REQD)CONSULT BLDG ARCHITECT OR ENGINEER.

MAR 01 2001

====<<<<ACES-32 Ver. 1.1>>>>===== [001857] =====<<<<FL.QUALITY-MMH>>>>=====

Customer : FOGLIA Thu Mar 1 09:52:17 2001

Project #: 00469 Truss ID : S40 Family #: spec.

Span : 45-8-4 B.Span : 45-9-4 Quant. : 1 Top Pitch : 6/12

==81d(1/21/2000),v1.1=====

FORCES - LOAD CASE #1								REACTIONS - SIZE	
1-2=.644	7-8=-2851	11-12= 0	17-18=-577	2-18= 103	5-15= 1616	9-13= 272	17=-4678	10.36	
2-3= 1281	8-9=-3097	12-13= 2556	18-1=-577	2-17=-633	6-15=-1132	9-12=-723	11=-2368	8.00	
3-4=-2141	9-10=-2858	13-14= 2770		3-17=-4275	7-15= 451	10-12= 2685			
4-5=-2141	10-11=-2320	14-15= 2550		3-16= 3763	7-14= 403				
5-6=-2823		15-16= 2017		4-16=-357	8-14=-358				
6-7=-2823		16-17=-1146		5-16=-1205	8-13=-66				

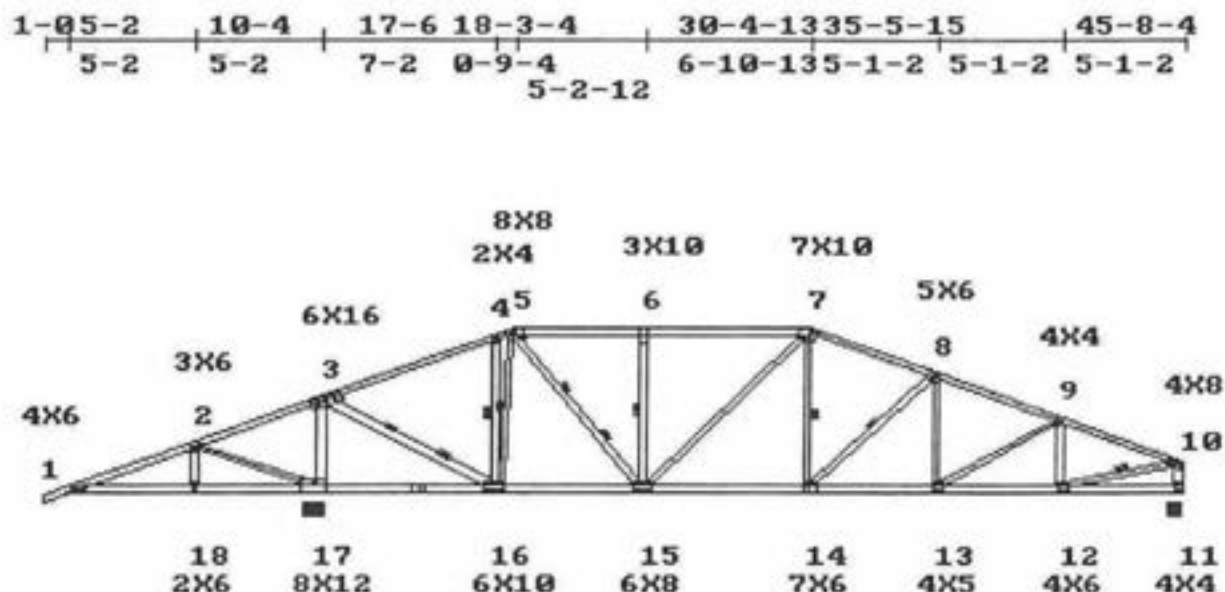
(52)

PROVIDE FOR 10255 LBS UPLIFT AT JOINT 17 (2.19)

PROVIDE FOR 4112 LBS UPLIFT AT JOINT 11 (1.74)

PROVIDE FOR 530 LBS HORIZ. REACTION AT JOINT 17

PLATE OFFSETS (X=LEFT,Y=TOP): [J5=4,2.5], [J7=5,4], [J8=3,3], [J10=3.5,2], [J14=3,2], [J17=6.5,4],



5X6 SPL.

5-2 10-4 17-6 23-6 30-4-1335-5-15 40-7-2 45-8-4

5-2 5-2 7-2 6-0 6-10-135-1-2 5-1-2 5-1-2

L. HL TO PK:20-5-2 R. HL TO PK:17-1-1

LEFT HEIGHT:0-4-3 SPAN:45-8-4 RISE:9-5-14 RIGHT HEIGHT:1-10-2

LOADING (PSF)		MAX STRESSES		MINIMUM GRADE OF LUMBER	
TOP	30	15	TOP 2-3=0.925	TOP CHORD:2X4	SS 19 SP
BOTT	0	10	BOTT 17-18=0.890	BOT CHORD:2X6	No.2D 19 SP
			LL.DEFL.@14=0.18 < L/360	WEBS	:2X4 No.3 19 SP

REPETITIVE STRESSES NOT USED

SPACING : 24.0 in. o. c.

NO. OF MEMBERS = 1

LOADING	STRESS INCREASE	LOADING	PANEL(PLF) / JOINTS(LBS)
LUMBER	1.33	1.33	UNIFORM
PLATE			CONCENTRATED
			1- 5= 90 5- 6= 290 6-10= 90 11- 1= 20
			15= 500 16= 500

CHORD: 5-6; 6-7 TO BE 2X6 SS 19 SP

WEB: 10-11; 3-16 TO BE 2X6 No.20 19 SP

WEB: 3-17 TO BE 2X6 SS 19 SP

WEB: 5-15 TO BE 2X4 No.10 19 SP

RIGHT END VERTICAL IS NOT EXPOSED.

WEB 4-16; 5-16; 6-15; 7-14; 8-14; 10-12 BRACED at 1/2 POINTS AS SHOWN ABOVE

WEB 3-16; 5-15 BRACED at 1/3 POINTS AS SHOWN ABOVE

Note:Use 1x4 or 2x3 Cont.Bracing conn.w/2-8d nails,or T-brace of same size and grade as web conn. to narrow face w/10d nails 6 in. o.c.

TRUSS HAS BEEN CHECKED FOR 145 M.P.H. WIND LOAD, WALL HEIGHT 16 FT, ENCLOSED BLDG. TYPE, ASSUMING 10 PSF TOTAL D.L.(7+3) (SBC-97).

DEFLECTION(IN.) L.L= 0.18,D.L=0.15,T.L=0.33

PLATES ARE MITEK M20-249,200 MANUFACTURED FROM ASTM A 446 GRD A GALVANIZED STEEL(EXCEPT AS SHOWN)

PLATE MUST BE INSTALLED ON EA. FACE OF JOINT,SYMMETRICALLY(EXCEPT AS SHOWN)DESIGN CONFORMS W/NDS DESIGN SPECS, SSSC,ANSI/TP1-95

[Signature]

MAR 01 2001

THIS DESIGN IS FOR TRUSS FABRICATION ONLY, FOR PERMANENT AND TEMPORARY BRACING (WHICH IS ALWAYS REQD) CONSULT BLDG ARCHITECT OR ENGINEER.

52

V. K. Kishore

2001. 11. 1

====<<<<ACES-32 Ver. 1.1>>>>===== [001858] =====<<<<FL.QUALITY-MMH>>>>=====

Customer : FOGLIA Thu Mar 1 09:52:41 2001

Project #: 00469 Truss ID : G1 Family #: spec.

Span : 1-10-8 B.Span : 1-10-8 Quant. : 2 Top Pitch : 6/12

==Bld(1/21/2000),v1.1=====

REACTIONS - SIZE

1=-483 8.00

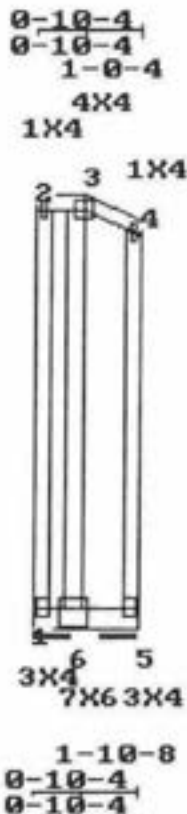
5=-483 8.00

(53)

PROVIDE FOR 629 LBS UPLIFT AT JOINT 1 (1.30)

PROVIDE FOR 555 LBS UPLIFT AT JOINT 5 (1.15)

PLATE OFFSETS (X=LEFT,Y=TOP):[J6=3,2],



LEFT HEIGHT:9-0-11 SPAN:1-10-8 RISE:9-0-11 R. HL TO PK :1-1-11

RIGHT HEIGHT:8-6-9

LOADING (PSF)		MAX STRESSES		MINIMUM GRADE OF LUMBER	
L	D	TOP	BOTT	TOP CHORD	No.2ND 19 SP
30	15	2-3=0.052	6-1=0.106	BOT CHORD <td>No.2D 19 SP</td>	No.2D 19 SP
0	10	LL.DEFL.@5=0.00 < L/360		WEBS	:2X4 No.3 19 SP

REPETITIVE STRESSES NOT USED

SPACING : 24.0 in. o. c.

NO. OF MEMBERS = 2

LOADING	STRESS INCREASE	LOADING	PANEL(PLF) / JOINTS(LBS)
LUMBER	PLATE	TYPE	
1	1.33	1.33	UNIFORM 2- 4= 340 5- 1= 270

BOTH END VERTICALS ARE NOT EXPOSED.

2 MEMBERS NAILED TOG. W/1 ROW(S) OF .131x3 in. NAILS 7 in. o.c.(TOP CHS.),AND 1 ROW(S) OF .131x3 in. NAILS 9 in. o.c.(BOTT. CHS.)

For Webs use 1 ROW of NAILS 12 in. o.c.

TRUSS HAS BEEN CHECKED FOR 145 M.P.H. WIND LOAD, WALL HEIGHT 16 FT, ENCLOSED BLDG. TYPE, ASSUMING 10 PSF TOTAL D.L.(7+3) (SBC-97).

DEFLECTION(IN.) L.L= 0.00,D.L=0.00,T.L=0.00

PLATES ARE MITEK M20-249,200 MANUFACTURED FROM ASTM A 446 GRD A GALVANIZED STEEL(EXCEPT AS SHOWN)

PLATE MUST BE INSTALLED ON EA. FACE OF JOINT,SYMMETRICALLY(EXCEPT AS SHOWN)DESIGN CONFORMS W/NDS DESIGN SPECS, S58C,NDS,ANSI/TP1-95

THIS DESIGN IS FOR TRUSS FABRICATION ONLY.FOR PERMANENT AND TEMPORARY BRACING(WHICH IS ALWAYS REQD)CONSULT BLDG ARCHITECT OR ENGINEER.

[Signature]

MAR 01 2001

====<<<ACES-32 Ver. 1.1>>>===== [001859] =====<<<FL.QUALITY-MMH>>>=====

Customer : FOGLIA Thu Mar 1 09:53:01 2001

Project #: 00469 Truss ID : G1A Family #: 201

Span : 1-10-8 B.Span : 1-10-8 Quant. : 2 Top Pitch : 6/12

==Bld(1/21/2000),v1.1=====

FORCES - LOAD CASE #1

1-2=-290 4-1= 0 2-4= 0

2-3= 0

3-4=-290

REACTIONS - SIZE

1=-500 2.00

4=-500 2.00

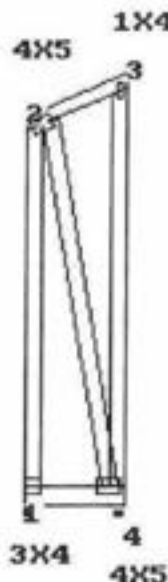
PROVIDE FOR 177 LBS UPLIFT AT JOINT 1 (0.35)

PROVIDE FOR 1190 LBS UPLIFT AT JOINT 4 (2.38)

PROVIDE FOR 113 LBS HORIZ. REACTION AT JOINT 1

1-10-8

1-10-8



1-10-8

1-10-8

L. HL TO PK:2-1-2

LEFT HEIGHT:8-6-9

SPAN:1-10-8

RISE:9-5-13

RIGHT HEIGHT:9-5-13

LOADING (PSF)

	L	D
TOP	30	15
BOTT	0	10

MAX STRESSES

TOP 2-3=0.147

BOTT 4-1=0.024

LL.DEFL.@0=0.00 < L/360

MINIMUM GRADE OF LUMBER

TOP CHORD:2X4 No.2ND 19 SP

BOT CHORD:2X6 No.2D 19 SP

WEBS :2X4 No.3 19 SP

REPETITIVE STRESSES NOT USED

SPACING : 24.0 in. o. c.

NO. OF MEMBERS = 2

LOADING STRESS INCREASE	LOADING LUMBER PLATE	LOADING TYPE	PANEL(PLF) / JOINTS(LBS)
1	1.33	1.33	UNIFORM 2- 3= 340 4- 1= 245

BOTH END VERTICALS ARE NOT EXPOSED.

2 MEMBERS NAILED TOG. W/1 ROW(S) OF .131x3 in. NAILS 7 in. o.c.(TOP CHS.),AND 1 ROW(S) OF .131x3 in. NAILS 11 in. o.c(BOTT. CHS.)

For Webs use 1 ROW of NAILS 12 in. o.c.

TRUSS HAS BEEN CHECKED FOR 145 M.P.H. WIND LOAD, WALL HEIGHT 16 FT, ENCLOSED BLDG. TYPE, ASSUMING 10 PSF TOTAL D.L.(7+3) (SBC-97).

DEFLECTION(IN.) L.L= 0.00,D.L=0.00,T.L=0.00

PLATES ARE MITEK M20-249,200 MANUFACTURED FROM ASTM A 446 GRD A GALVANIZED STEEL(EXCEPT AS SHOWN)

PLATE MUST BE INSTALLED ON EA. FACE OF JOINT,SYMMETRICALLY(EXCEPT AS SHOWN)DESIGN CONFORMS W/NDIS DESIGN SPECS, SSSC,ANSI/TPI-95

THIS DESIGN IS FOR TRUSS FABRICATION ONLY.FOR PERMANENT AND TEMPORARY BRACING(WHICH IS ALWAYS REQD)CONSULT BLDG ARCHITECT OR ENGINEER.

MAR 01 2001

==<<<<ACES-32 Ver. 1.1>>>>===== [001860] =====<<<<FL.QUALITY-MMH>>>>=====

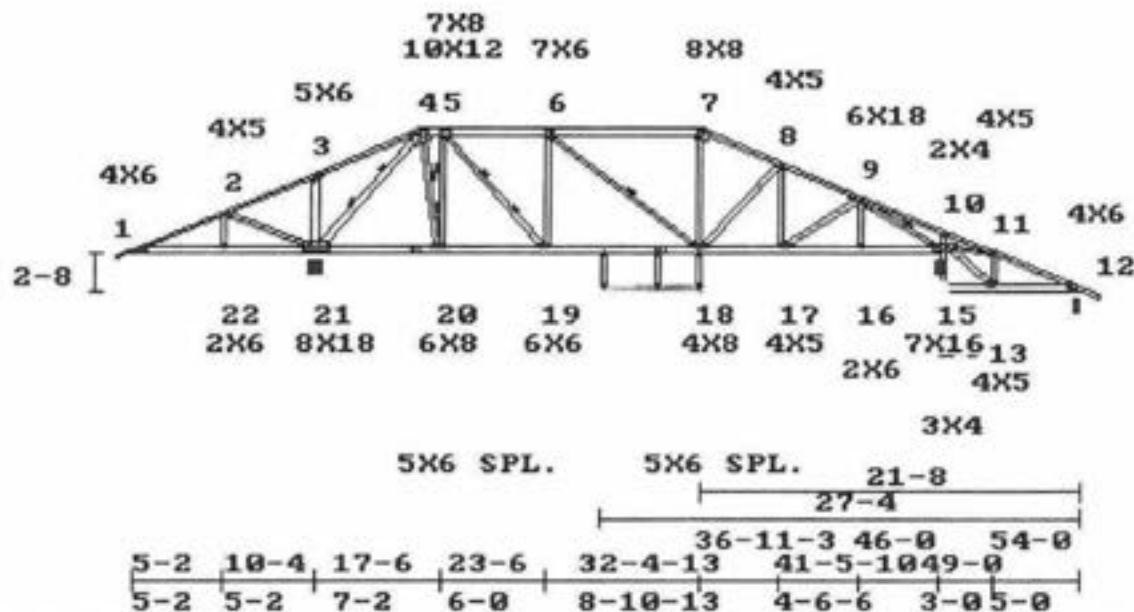
Customer : FOGLIA	Truss ID : S39	Thu Mar 1 09:53:40 2001
Project # : 00469	Quant. : 1	Family # : spec.
Span : 54-0	B.Span : 54-0	Top Pitch : 6/12
		Bot pitch : 90/12

==B(d(1/21/2000),v1.1)=====

FORCES - LOAD CASE #1

TOP CHORD: 1-2= 645 , 2-3= 1283, 3-4= 1283, 4-5=2042, 5-6=3106, 6-7=2704, 7-8=3023,
 8-9=3259, 9-10=67, 10-11=21, 11-12=368,
 BOTTOM CHORD: 12-13= 331, 13-14= 0, 14-15= 29, 15-16= 2758, 16-17= 2758, 17-18= 2915, 18-19= 3106,
 19-20= 2039, 20-21= 1591, 21-22=579, 22-1=579,
 WEBS: 2-22= 103, 2-21=633, 3-21=500, 4-21=4628, 4-20= 2978, 5-20=2312, 5-19= 1782,
 6-19=778, 6-18=542, 7-18= 768, 8-18=343, 8-17=32, 9-17= 199, 9-16= 92,
 9-15=3389, 10-15=362, 11-15=312, 11-13=230, 15-13= 452,
 REACTIONS, SIZE: 21=4632, 10.38 15=2771, 8.00 12=437, 4.00
 PROVIDE FOR 10276 LBS UPLIFT AT JOINT 21 (2.22)
 PROVIDE FOR 5039 LBS UPLIFT AT JOINT 15 (1.82)
 PROVIDE FOR 1132 LBS UPLIFT AT JOINT 12 (2.59)
 PROVIDE FOR 710 LBS HORIZ. REACTION AT JOINT 21
 PLATE OFFSETS (X=LEFT, Y=TOP): [j3=3,3], [j4=6.5,2], [j5=3.5,4], [j6=3,5], [j7=4,2.5], [j15=8,2], [j20=4.5,3],

1-5-2 10-4 16-3-423-6 32-4-13 41-5-1049-0 1-0
 5-2 5-2 5-11-46-0 8-10-13 4-6-6 3-05-0
 1-2-12 4-6-6 4-6-6



L. HL TO PK:18-2-5 LEFT HEIGHT:0-4-3 SPAN:54-0 RISE:11-1-12 R. HL TO PK:24-1-12 RIGHT HEIGHT:0-4-3

LOADING (PSF)		MAX STRESSES		MINIMUM GRADE OF LUMBER	
TOP	30 15	TOP	8-9=0.864	TOP CHORD:	2X4 No.1D 19 SP
BOTT	0 10	BOTT	21-22=0.891	BOT CHORD:	2X6 No.2D 19 SP
		LL.DEFL.@18=0.22 < L/360		WEBS:	2X4 No.3 19 SP

REPETITIVE STRESSES NOT USED

SPACING : 24.0 in. o. c.
 NO. OF MEMBERS = 1

LOADING	STRESS INCREASE	LOADING	PANEL(PLF) / JOINTS(LBS)
LUMBER	PLATE	TYPE	
1	1.33	1.33	UNIFORM 1- 5= 90 5- 6= 240 6-12= 90 12-14= 20 15- 1= 20
			CONCENTRATED 19= 500 20= 500

CHORD: 4-5; 5-6; 6-7 TO BE 2X6 SS 19 SP
 CHORD: 7-8; 8-9 TO BE 2X4 SS 19 SP
 CHORD: 9-10; 10-11; 11-12 TO BE 2X4 No.2ND 19 SP
 CHORD: 14-15 TO BE 2X4 No.3 19 SP
 WEB: 3-21 TO BE 2X6 No.2D 19 SP
 WEB: 4-21 TO BE 2X6 SS 19 SP
 WEB: 4-20; 5-19; 9-15 TO BE 2X4 No.1D 19 SP
 WEB: 5-20 TO BE 2X4 No.2ND 19 SP
 WEB 6-18; 9-15 BRACED at 1/2 POINTS AS SHOWN ABOVE

MAR 01 2001

WEB 4-21; 4-20; 5-20; 5-19 BRACED at 1/3 POINTS AS SHOWN ABOVE

PROVIDE 2*4 CONTINUOUS BRACING AT BOTTOM CHORD NOT EXCEEDING 4' O.C. OVER DROPPED BOTTOM CHORD AREAS WHERE CEILING IS NOT APPLIED.

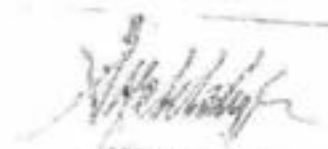
Note: Use 1x4 or 2x3 Cont. Bracing conn. w/2-8d nails, or T-brace of same size and grade as web conn. to narrow face w/10d nails 6 in. o.c.
TRUSS HAS BEEN CHECKED FOR 145 M.P.H. WIND LOAD, WALL HEIGHT 16 FT, ENCLOSED BLDG. TYPE, ASSUMING 10 PSF TOTAL D.L. (7+3) (SBC-97).
DEFLECTION (IN.) L.L.= 0.22, D.L.=0.18, T.L.=0.40

PLATES ARE MITEK M20-249,200 MANUFACTURED FROM ASTM A 446 GRD A GALVANIZED STEEL (EXCEPT AS SHOWN)

PLATE MUST BE INSTALLED ON EA. FACE OF JOINT, SYMMETRICALLY (EXCEPT AS SHOWN) DESIGN CONFORMS W/NDS DESIGN SPECS, SSB, ANSI/TPI-95

THIS DESIGN IS FOR TRUSS FABRICATION ONLY. FOR PERMANENT AND TEMPORARY BRACING (WHICH IS ALWAYS REQD) CONSULT BLDG ARCHITECT OR ENGINEER.

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MAR 6 1 2004

====<<<<ACES-32 Ver. 1.1>>>>===== [001861] =====<<<<FL.QUALITY-MMH>>>>=====

Customer : FOGLIA Thu Mar 1 09:54:04 2001
 Project #: 00469 Truss ID : S38A Family #: spec.
 Span : 54-0 B.Span : 54-0 Quant. : 1 Top Pitch : 6/12
 Bot pitch : 90/12

23-24 12/12,

==8ld(1/21/2000),v1.1=====

FORCES - GRAVITY LOADS

TOP CHORD: 1-2= 920, 2-3= 1611, 3-4= 298, 4-5= -129, 5-6= -1223, 6-7= -1704, 7-8= -1710,
 8-9= -2116, 9-10= -2113, 10-11= 480, 11-12= 517, 12-13= -199,

BOTTOM CHORD: 13-14= 179, 14-15= 0, 15-16= 29, 16-17= 1637, 17-18= 1637, 18-19= 1667, 19-20= 55,
 20-21= 0, 21-22= 813, 22-23= 129, 23-24= -2038, 24-25= -826, 25-1= -826,

WEBS: 2-25= 122, 2-24= -680, 3-24= -4521, 3-23= 3673, 4-23= -2025, 4-22= 1388, 5-22= -1435,
 5-21= 859, 6-21= -1182, 6-19= 830, 7-19= -293, 8-19= 322, 8-18= 459, 9-18= -436,
 10-18= 289, 10-17= 87, 10-16= -2720, 11-16= -301, 12-16= -643, 12-14= -88, 16-14= 245,
 19-21= 1360,

REACTIONS, SIZE: 24= -3437, 8.00 16= -2250, 8.00 13= -473, 4.00

PROVIDE FOR 5640 LBS UPLIFT AT JOINT 24 (1.64)

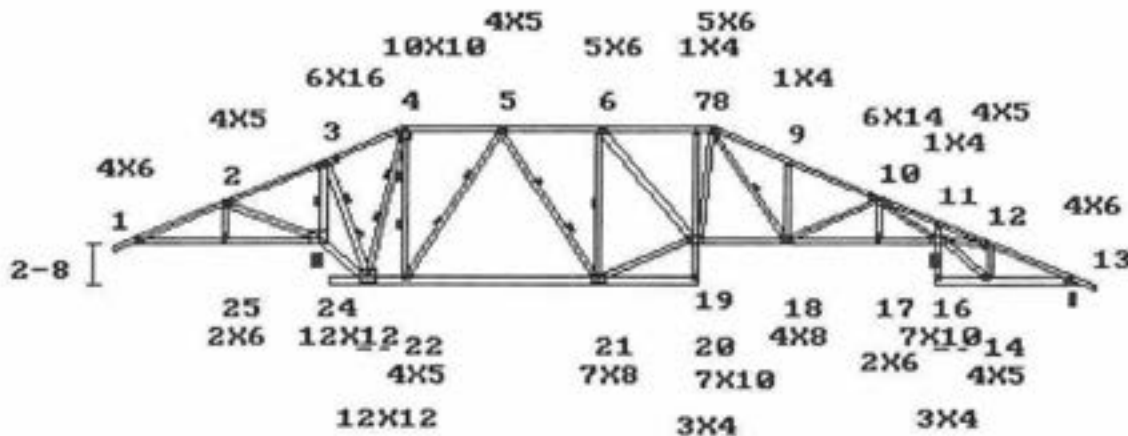
PROVIDE FOR 2229 LBS UPLIFT AT JOINT 16 (0.99)

PROVIDE FOR 607 LBS UPLIFT AT JOINT 13 (1.28)

PROVIDE FOR 680 LBS HORIZ. REACTION AT JOINT 24

PLATE OFFSETS (X=LEFT, Y=TOP): [j4=5,2], [j6=3,3], [j16=5,2], [j19=5,2], [j21=4,2], [j23=6,7], [j24=6,7],

1-05-4-8 15-5-8 26-8-8 33-2-9 42-8-14 54-01-0
 5-4-8 4-8-8 5-7-8 0-10-9 5-3-2 3-05-0
 5-4-8 5-7-8 5-7-8 4-3-2 3-3-2



10-9 15-5-8 42-8-1 49-0
 5-4-8 13-5 26-8-8 32-4 37-5-11 46-0 54-0
 5-4-8 2-8 11-3 5-7-8 5-3-3 3-05-0

L. HL TO PK: 17-3-6

LEFT HEIGHT: 0-4-3

SPAN: 54-0

RISE: 10-8-14

R. HL TO PK: 23-2-14

RIGHT HEIGHT: 0-4-3

LOADING (PSF)

L D

TOP 30 15

BOTT 0 10

MAX STRESSES

TOP 8-9=0.973

BOTT 23-24=0.649

LL.DEFL.@25=0.15 < L/360

MINIMUM GRADE OF LUMBER

TOP CHORD: 2X4 No.2ND 19 SP

BOT CHORD: 2X6 No.2D 19 SP

WEBS : 2X4 No.3 19 SP

STR.INC.: LUMB = 1.33 PLATE = 1.33

REPETITIVE STRESSES USED

SPACING : 24.0 in. o. c.

NO. OF MEMBERS = 1

CHORD: 4-5; 5-6; 6-7; 7-8; 8-9; 9-10 TO BE 2X4 No.1D 19 SP

CHORD: 15-16; 19-20 TO BE 2X4 No.3 19 SP

WEB: 3-24; 3-23 TO BE 2X6 No.2D 19 SP

WEB 3-24; 6-21; 8-18 BRACED at 1/2 POINTS AS SHOWN ABOVE

WEB 3-23; 4-23; 4-22; 5-22; 5-21 BRACED at 1/3 POINTS AS SHOWN ABOVE

Note: Use 1x4 or 2x3 Cent. Bracing conn. w/ 2-8d nails, or T-brace of same size and grade as web conn. to narrow face w/ 10d nails o.c.

TRUSS HAS BEEN CHECKED FOR 145 M.P.H. WIND LOAD, WALL HEIGHT 16 FT, ENCLOSED BLDG. TYPE, ASSUMING 10 PSF TOTAL D.L. (7+3) (SBC-97).

DEFLECTION (IN.) L.L.= 0.15, D.L.= 0.13, T.L.= 0.28

PLATES ARE MITEK M20-249, 200 MANUFACTURED FROM ASTM A 446 GRD A GALVANIZED STEEL (EXCEPT AS SHOWN)

PLATE MUST BE INSTALLED ON EA. FACE OF JOINT, SYMMETRICALLY (EXCEPT AS SHOWN) DESIGN CONFORMS W/ NDS DESIGN SPECS, SSSC, ANSI/TPI-95

THIS DESIGN IS FOR TRUSS FABRICATION ONLY. FOR PERMANENT AND TEMPORARY BRACING (WHICH IS ALWAYS REQD) CONSULT BLDG ARCHITECT OR ENGINEER.

MAR 07 2001

====<<<<ACES-32 Ver. 1.1>>>>===== [001862] =====<<<<FL.QUALITY-MMH>>>>=====

Customer : FOGLIA

Project # : 00469

Span : 54-0

B.Span : 54-0

Truss ID : S38

Quant. : 1

Thu Mar 1 09:54:24 2001

Family # : spec.

Top Pitch : 6/12

Bot pitch : 90/12

24-26 12/12,

==Bld(1/21/2000),v1.1=====

FORCES - GRAVITY LOADS

TOP CHORD: 1-2= 964, 2-3= 1582, 3-4= 1598, 4-5= 495, 5-6= 715, 6-7= 2219, 7-8= 2232,
8-9= 2004, 9-10= 1972, 10-11= 854, 11-12= 885, 12-13= 1,
BOTTOM CHORD: 13-14= 1, 14-15= 0, 15-16= 24, 16-17= 1370, 17-18= 1370, 18-19= 1764, 19-20= 1789,
20-21= 68, 21-22= 0, 22-23= 1525, 23-24= 715, 24-25= 1332, 25-26= 1354, 26-27= 865,
27-1= 865,
WEBS: 2-27= 108, 2-26= 631, 3-26= 223, 4-26= 3836, 4-25= 50, 4-24= 2706, 5-24= 2047,
5-23= 1024, 6-23= 1214, 6-22= 310, 6-20= 910, 7-20= 533, 8-20= 676, 8-19= 43,
9-19= 44, 9-18= 322, 10-18= 548, 10-17= 60, 10-16= 2904, 11-16= 259, 12-16= 794,
12-14= 77, 16-14= 2, 20-22= 1589,

REACTIONS, SIZE: 26= 3424, 8.00 16= 2323, 3.93 13= 413, 2.00

PROVIDE FOR 5698 LBS UPLIFT AT JOINT 26 (1.66)

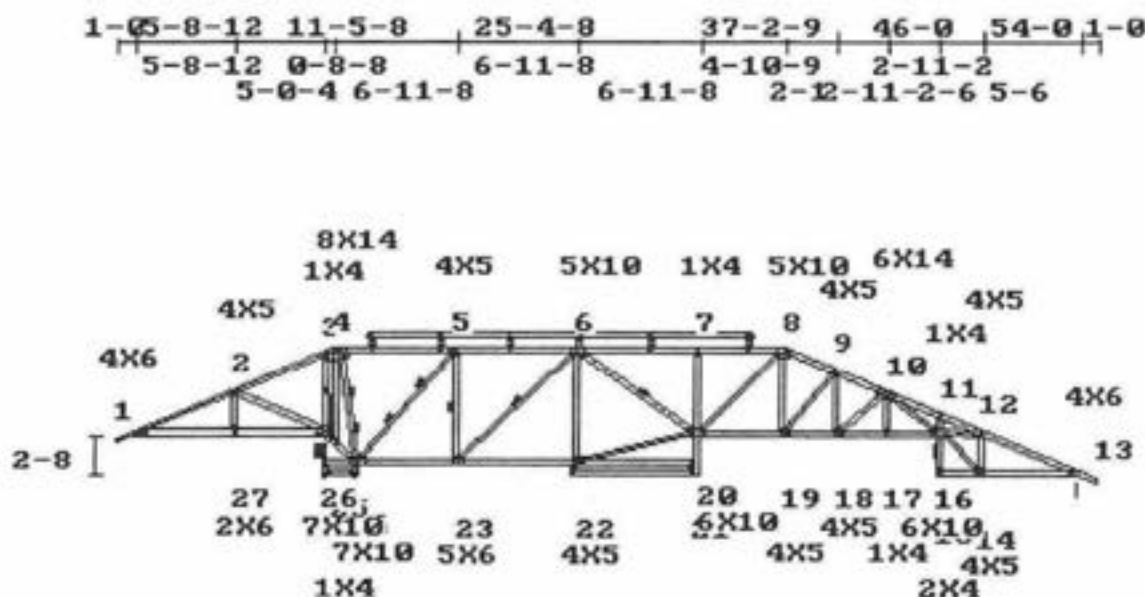
PROVIDE FOR 2296 LBS UPLIFT AT JOINT 16 (0.99)

PROVIDE FOR 574 LBS UPLIFT AT JOINT 13 (1.39)

PROVIDE FOR 625 LBS HORIZ. REACTION AT JOINT 26

PLATE OFFSETS (X=LEFT, Y=TOP): [j4=7,2], [j6=5,3], [j8=5,2], [j23=3,2], [j24=5,4], [j26=5,4],

*** WARNING: NO PLATE AT JOINT(S): j21, REVIEW REQUIRED ***



L. HL TO PK: 12-9-12

LEFT HEIGHT: 0-4-3

SPAN: 54-0

RISE: 8-8-15

R. HL TO PK: 18-9-3

RIGHT HEIGHT: 0-4-3

LOADING (PSF)

L D

TOP 30 15

BOTT 0 10

MAX STRESSES

TOP 6-7=0.915

BOTT 25-26=0.772

LL.DEFL.@27=0.12 < L/360

MINIMUM GRADE OF LUMBER

TOP CHORD: 2X4 No.2ND 19 SP

BOT CHORD: 2X4 No.2ND 19 SP

WEBS : 2X4 No.3 19 SP

STR.INC.: LUMB = 1.33 PLATE = 1.33

REPETITIVE STRESSES USED

SPACING : 24.0 in. o. c.

NO. OF MEMBERS = 1

CHORD: 4-5; 5-6; 6-7; 7-8 TO BE 2X4 SS 19 SP

CHORD: 15-16; 20-21 TO BE 2X4 No.3 19 SP

CHORD: 26-27; 27-1 TO BE 2X6 No.2D 19 SP

WEB: 4-26; 4-24 TO BE 2X4 No.1D 19 SP

WEB 4-26; 5-23; 6-23; 6-20 BRACED at 1/2 POINTS AS SHOWN ABOVE

WEB 4-24; 5-24 BRACED at 1/3 POINTS AS SHOWN ABOVE

PROVIDE 2"X4 CONTINUOUS BRACING AT BOTTOM CHORD NOT EXCEEDING 4' O.C. OVER DROPPED BOTTOM CHORD AREAS WHERE CEILING IS NOT APPLIED.

Note: Use 1x4 or 2x3 Cont. Bracing conn. w/ 2-8d nails, or T-brace of same size and grade as web conn. to narrow face w/ 10d nails 6 in. o.c.

TRUSS HAS BEEN CHECKED FOR 145 M.P.H. WIND LOAD, WALL HEIGHT 16 FT, ENCLOSED BLDG. TYPE, ASSUMING 10 PSF TOTAL D.L. (7+3) (SBC-97).

DEFLECTION (IN.) L.L.= 0.12, D.L.= 0.10, T.L.= 0.23

MAR 01 2001

PLATES ARE MITEK M20-249,200 MANUFACTURED FROM ASTM A 446 GRD A GALVANIZED STEEL(EXCEPT AS SHOWN)
PLATE MUST BE INSTALLED ON EA. FACE OF JOINT, SYMMETRICALLY(EXCEPT AS SHOWN)DESIGN CONFORMS W/NDS DESIGN SPECS, SSBC,ANSI/TPI-95
THIS DESIGN IS FOR TRUSS FABRICATION ONLY.FOR PERMANENT AND TEMPORARY BRACING(WHICH IS ALWAYS REQD)CONSULT BLDG ARCHITECT OR ENGINEER.

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[Handwritten signature]
MAR 22 2003

Bot pitch : 90712

PLATES ARE MITEK M20-249,200 MANUFACTURED FROM ASTM A 446 GRD A GALVANIZED STEEL(EXCEPT AS SHOWN)
PLATE MUST BE INSTALLED ON EA. FACE OF JOINT, SYMMETRICALLY(EXCEPT AS SHOWN)DESIGN CONFORMS W/ND S DESIGN SPECS, S58C,ANSI/TPI-95
THIS DESIGN IS FOR TRUSS FABRICATION ONLY.FOR PERMANENT AND TEMPORARY BRACING(WHICH IS ALWAYS REQD)CONSULT BLDG ARCHITECT OR ENGINEER.

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MAR 01 2001

====<<<<ACES-32 Ver. 1.1>>>>===== [001864] =====<<<<FL.QUALITY-MMH>>>>=====

Customer : FOGLIA

Project # : 00469

Span : 45-6-8

B.Span : 45-6-8

Truss ID : S36

Quant. : 1

Thu Mar 1 09:55:40 2001

Family # : spec.

Top Pitch : 67/12

Bot pitch : 90/12

==Bld(1/21/2000),v1.1=====

FORCES - GRAVITY LOADS

FORCES - GRAVITY LOADS				REACTIONS - SIZE			
1-2= 817	7-8=-2618	11-12= 0	17-18= 0	2-21= 95	5-17= 1249	8-13=-553	19=-3432 8.00
2-3= 1395	8-9=-2635	12-13= 1737	18-19= 53	2-20=-580	6-17=-1597	9-13= 1086	11=-1676 2.33
3-4= 1613	9-10=-1941	13-14= 53	19-20=-1246	3-20= 324	6-16= 108	9-12=-335	
4-5= 1623	10-11=-1614	14-15= 0	20-21=-729	3-19=-1277	6-15= 838	10-12= 1799	
5-6=-281		15-16= 1275	21-1=-729	4-19=-301	7-15=-1234	13-15= 1923	
6-7=-1797		16-17= 1275		5-19=-2575	7-13= 1111	19-17= 301	

PROVIDE FOR 4639 LBS UPLIFT AT JOINT 19 (1.35)

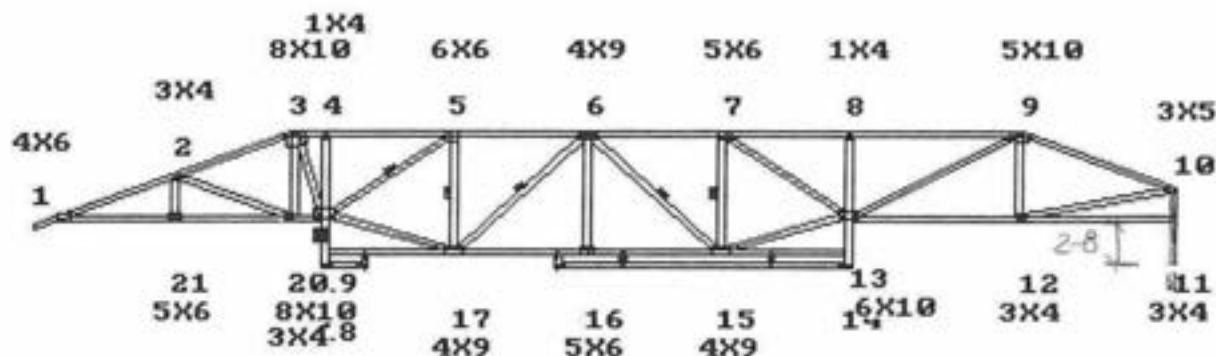
PROVIDE FOR 1312 LBS UPLIFT AT JOINT 11 (0.78)

PROVIDE FOR 301 LBS HORIZ. REACTION AT JOINT 19

PLATE OFFSETS (X=LEFT,Y=TOP):{j3=5,2},{j7=3,3},{j9=5,2},{j16=3,2},{j19=5,5},{j21=3,2},

*** WARNING: NO PLATE AT JOINT(S):j14,j18, REVIEW REQUIRED ***

1-04-8-12 10-9 21-6-8 26-11-4 39-2-9 45-6-8
4-8-12 1-3-8 5-4-12 5-4-12 5-4-12 6-10-9 6-3-15
4-8-12 5-4-12



9-5-8 16-1-12 32-4
4-8-12 10-9 21-6-8 26-11-4 39-2-9 45-6-8
4-8-12 1-3-8 5-4-12 5-4-12 5-4-12 6-10-9 6-3-15

L. HL TO PK:10-6-14

LEFT HEIGHT:0-4-3

SPAN:45-6-8

RISE:7-8-15

R. HL TO PK :7-0-14

RIGHT HEIGHT:1-10-15

LOADING (PSF)

L D

TOP 30 15

BOTT 0 10

MAX STRESSES

TOP 8-9=0.910

BOTT 19-20=0.927

LL.DEFL.@14=0.14 < L/360

MINIMUM GRADE OF LUMBER

TOP CHORD:2X4 No.2ND 19 SP

BOT CHORD:2X4 No.2ND 19 SP

WEBS :2X4 No.3 19 SP

STR.INC.: LUMB = 1.33 PLATE = 1.33

REPETITIVE STRESSES USED

SPACING : 24.0 in. o. c.

NO. OF MEMBERS = 1

CHORD: 3-4; 4-5; 5-6; 6-7; 7-8; 8-9; 19-20; 20-21; 21-1 TO BE 2X4 No.1D 19 SP

CHORD: 9-10 TO BE 2X4 SS 19 SP

CHORD: 13-14; 18-19 TO BE 2X4 No.3 19 SP

RIGHT END VERTICAL IS NOT EXPOSED.

WEB 5-19; 5-17; 6-17; 6-15; 7-15 BRACED AT 1/2 POINTS AS SHOWN ABOVE

Note:Use 1x4 or 2x3 Cont.Bracing conn.w/2-8d nails,or T-brace of same size and grade as web conn. to narrow face w/10d nails 6 in. o.c

TRUSS HAS BEEN CHECKED FOR 145 M.P.H. WIND LOAD, WALL HEIGHT 16 FT, ENCLOSED BLDG. TYPE, ASSUMING 10 PSF TOTAL D.L.(7+3) (SBC-97).

DEFLECTION(IN.) L.L= 0.14,D.L=0.12,T.L=0.26

PLATES ARE MITEK M20-249,200 MANUFACTURED FROM ASTM A 446 GRD A GALVANIZED STEEL(EXCEPT AS SHOWN)

PLATE MUST BE INSTALLED ON EA. FACE OF JOINT,SYMMETRICALLY(EXCEPT AS SHOWN)DESIGN CONFORMS W/ND S DESIGN SPECS, SSBC,ANSI/TPI-95

THIS DESIGN IS FOR TRUSS FABRICATION ONLY.FOR PERMANENT AND TEMPORARY BRACING(WHICH IS ALWAYS REQD)CONSULT BLDG ARCHITECT OR ENGINEER.

MAR 01 2001

====<<<<ACES-32 Ver. 1.1>>>>===== [001865] =====<<<<FL.QUALITY-MMH>>>>=====

Customer : FOGLIA Thu Mar 1 09:55:58 2001

Project #: 00469 Truss ID : S35 Family # : spec.

Span : 45-6 B.Span : 45-6-8 Quant. : 1 Top Pitch : 6/12

Bot pitch : 90/12

==Bld(1/21/2000),v1.1=====

FORCES - GRAVITY LOADS

FORCES - GRAVITY LOADS				REACTIONS - SIZE				
1-2= 804	7-8=3262	11-12= 0	17-18= 0	2-20=363	6-17=1633	8-12=1829	19=3329	8.00
2-3= 1174	8-9=1708	12-13= 3245	18-19= 63	3-20= 257	6-16= 128	9-12= 382	11=1775	2.47
3-4= 1730	9-10=1901	13-14= 63	19-20=1048	3-19=1256	6-15= 582	10-12= 1849		
4-5= 1746	10-11=1733	14-15= 0	20-1=723	4-19=390	7-15=1053	13-15= 2597		
5-6=830		15-16= 2042		5-19=2991	7-13= 921	19-17= 874		
6-7=2474		16-17= 2042		5-17= 945	8-13= 437			

69

PROVIDE FOR 4385 LBS UPLIFT AT JOINT 19 (1.32)

PROVIDE FOR 1436 LBS UPLIFT AT JOINT 11 (0.81)

PROVIDE FOR 263 LBS HORIZ. REACTION AT JOINT 19

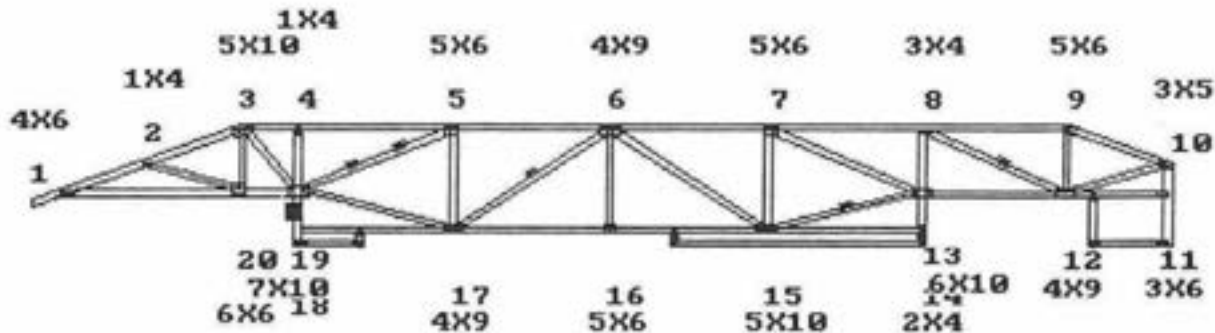
PLATE OFFSETS (X=LEFT,Y=TOP):[j3=5,2],[j5=3,3],[j7=3,3],[j16=3,2],[j19=5,2],

*** WARNING: NO PLATE AT JOINT(S):j18, REVIEW REQUIRED ***

1-03-8-12 9-8-716-1-5 22-6-4 28-11-2 35-4 41-2-9

3-8-12 2-2-15 6-4-14 6-4-14 6-4-14 5-10-9

3-8-12 6-4-14 4-3-7



7-5-8 9-8-716-1-5 22-6-3 28-11-2 35-4 41-2-9

7-5-8 2-2-15 6-4-14 6-4-14 6-4-14 5-10-9

10-2

20-6

45-6

L. HL TO PK:8-4-1 LEFT HEIGHT:0-4-3 SPAN:45-6 RISE:6-8-15 R. HL TO PK:4-9-8 RIGHT HEIGHT:1-11-3

LOADING (PSF)		MAX STRESSES		MINIMUM GRADE OF LUMBER	
L	D	TOP	BOTT	TOP CHORD	BOT CHORD
30	15	6-7=0.941	12-13=0.733	2X4 No.2ND 19 SP	2X4 No.2ND 19 SP
0	10	LL.DEFL.@15=0.17	LL.DEFL.@15=0.17	2X4 No.3 19 SP	2X4 No.3 19 SP

STR.INC.: LUMB = 1.33 PLATE = 1.33 SPACING : 24.0 in. o. c.

REPETITIVE STRESSES USED NO. OF MEMBERS = 1

CHORD: 3-4; 4-5; 5-6; 6-7 TO BE 2X4 No.1D 19 SP

CHORD: 7-8; 8-9 TO BE 2X4 SS 19 SP

CHORD: 13-14; 18-19 TO BE 2X4 No.3 19 SP

CHORD: 19-20; 20-1 TO BE 2X6 No.2D 19 SP

WEB: 10-11 TO BE 2X6 No.2D 19 SP

WEB: 5-19 TO BE 2X4 No.2ND 19 SP

RIGHT END VERTICAL IS NOT EXPOSED.

WEB 6-17; 8-12; 13-15 BRACED at 1/2 POINTS AS SHOWN ABOVE

WEB 5-19 BRACED at 1/3 POINTS AS SHOWN ABOVE

PROVIDE 2*4 CONTINUOUS BRACING AT BOTTOM CHORD NOT EXCEEDING 4' O.C. OVER DROPPED BOTTOM CHORD AREAS WHERE CEILING IS NOT APPLIED.

Note:Use 1x4 or 2x3 Cont.Bracing conn.w/2-8d nails,or T-brace of same size and grade as web conn. to narrow face w/10d nails 6 in. o.c

TRUSS HAS BEEN CHECKED FOR 145 M.P.H. WIND LOAD, WALL HEIGHT 16 FT, ENCLOSED BLDG. TYPE, ASSUMING 10 PSF TOTAL D.L.(7*3) (SBC-97).

DEFLECTION(IN.) L.L= 0.17,D.L=0.14,T.L=0.30

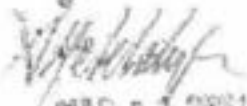
PLATES ARE MITEK M20-249,200 MANUFACTURED FROM ASTM A 446 GRD A GALVANIZED STEEL(EXCEPT AS SHOWN)

PLATE MUST BE INSTALLED ON EA. FACE OF JOINT,SYMMETRICALLY(EXCEPT AS SHOWN)DESIGN CONFORMS W/NOS DESIGN SPECS, SSBC,ANSI/TPI-95

MAR 01 2001

THIS DESIGN IS FOR TRUSS FABRICATION ONLY.FOR PERMANENT AND TEMPORARY BRACING(WHICH IS ALWAYS REQD)CONSULT BLDG ARCHITECT OR ENGINEER.

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MAR 2 1 2001

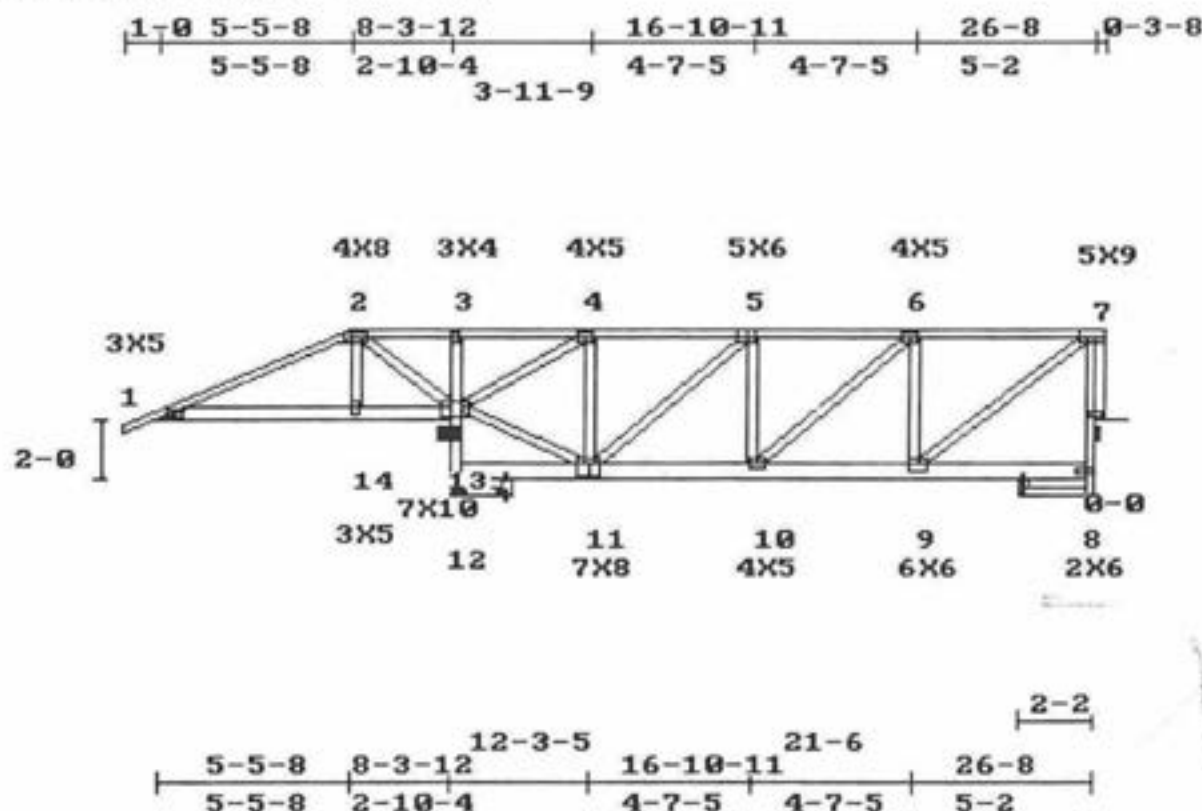
====<<<ACES-32 Ver. 1.1>>>=====[002010]====<<<FL.QUALITY-MMH>>>=====
 Customer : FOGLIA Wed Mar 7 07:20:30 2001
 Project #: 00469 Truss ID : S14 Family # : spec.
 Span : 26-8 B.Span : 27-0 Quant. : 2 Top Pitch : 6/12
 Bot pitch : 90/12

==81d(1/21/2000),v1.1==

FORCES - LOAD CASE #1				REACTIONS - SIZE	
1-2= 924	7-8= 801	8-9= 0	14-1=-828	2-14= 662	5-10= 870
2-3= 2457	9-10= 2135	2-13=-2192	6-10=-702	13=-4240	8.00
3-4= 2476	10-11= 1644	3-13=-359	6-9= 143	7=-2893	2.00
4-5=-316	11-12= 0	4-13=-3421	7-9= 2887		
5-6=-1644	12-13= 153	4-11= 1526	13-11= 357		
6-7=-2135	13-14=-828	5-11=-1896			

(61)

PROVIDE FOR 7051 LBS UPLIFT AT JOINT 13 (1.66)
 PROVIDE FOR 3123 LBS UPLIFT AT JOINT 7 (1.08)
 PROVIDE FOR 317 LBS HORIZ. REACTION AT JOINT 13
 PLATE OFFSETS (X=LEFT,Y=TOP):[J5=3,3],[J7=2.5,3],[J11=4,2],[J13=5,2],[J14=3,1.5],
 *** WARNING: NO PLATE AT JOINT(S):J12, REVIEW REQUIRED ***



L. HL TO PK:6-1-4
 LEFT HEIGHT:0-4-3

SPAN:26-8

RISE:5-8-15

RIGHT HEIGHT:5-0-15

LOADING (PSF)			MAX STRESSES		MINIMUM GRADE OF LUMBER	
	L	D	TOP	BOTT	TOP CHORD	BOT CHORD
TOP	30	15	6-7=0.501	8-9=0.355	2X4 No.2ND 19 SP	2X6 No.2D 19 SP
BOTT	0	10	LL.DEFL.@14=0.04 < L/360		WEBS :2X4	No.3 19 SP

REPETITIVE STRESSES NOT USED

SPACING : 24.0 in. o. c.
 NO. OF MEMBERS = 2

LOADING	STRESS INCREASE	LOADING TYPE	PANEL(PLF) / JOINTS(LBS)
1	1.33	1.33	1- 2= 90 2- 6= 105 6- 7= 45 8- 9= 310 9-12= 80 13-14= 80 14- 1= 20
		UNIFORM	9= 1101 14= 487
		CONCENTRATED	

CHORD: 12-13 TO BE 2X4 No.3 19 SP

RIGHT END VERTICAL IS NOT EXPOSED.

PROVIDE 2"4 CONTINUOUS BRACING AT BOTTOM CHORD NOT EXCEEDING 4' O.C. OVER DROPPED BOTTOM CHORD AREAS WHERE CEILING IS NOT APPLIED.
 2 MEMBERS NAILED TOG. W/1 ROW(S) OF .131x3 in. NAILS 12 in. o.c.(TOP CHS.),AND 1 ROW(S) OF .131x3 in. NAILS 8 in. o.c.(BOTT. CHS.)
 For Webs use 1 ROW OF NAILS 12 in. o.c.

TRUSS HAS BEEN CHECKED FOR 145 M.P.H. WIND LOAD, WALL HEIGHT 16 FT, ENCLOSED BLDG. TYPE, ASSUMING 10 PSF TOTAL D.L.(7+3) (SBC-97).

DEFLECTION(IN.) L.L= 0.04,O.L=0.03,T.L=0.07

PLATES ARE MITEK M20-249,200 MANUFACTURED FROM ASTM A 446 GRD A GALVANIZED STEEL(EXCEPT AS SHOWN)

PLATE MUST BE INSTALLED ON EA. FACE OF JOINT,SYMMETRICALLY(EXCEPT AS SHOWN)DESIGN CONFORMS W/NDS DESIGN SPECS, SBC,ANSI/TPI-95

THIS DESIGN IS FOR TRUSS FABRICATION ONLY.FOR PERMANENT AND TEMPORARY BRACING(WHICH IS ALWAYS REQD)CONSULT BLDG ARCHITECT OR ENGINEER.

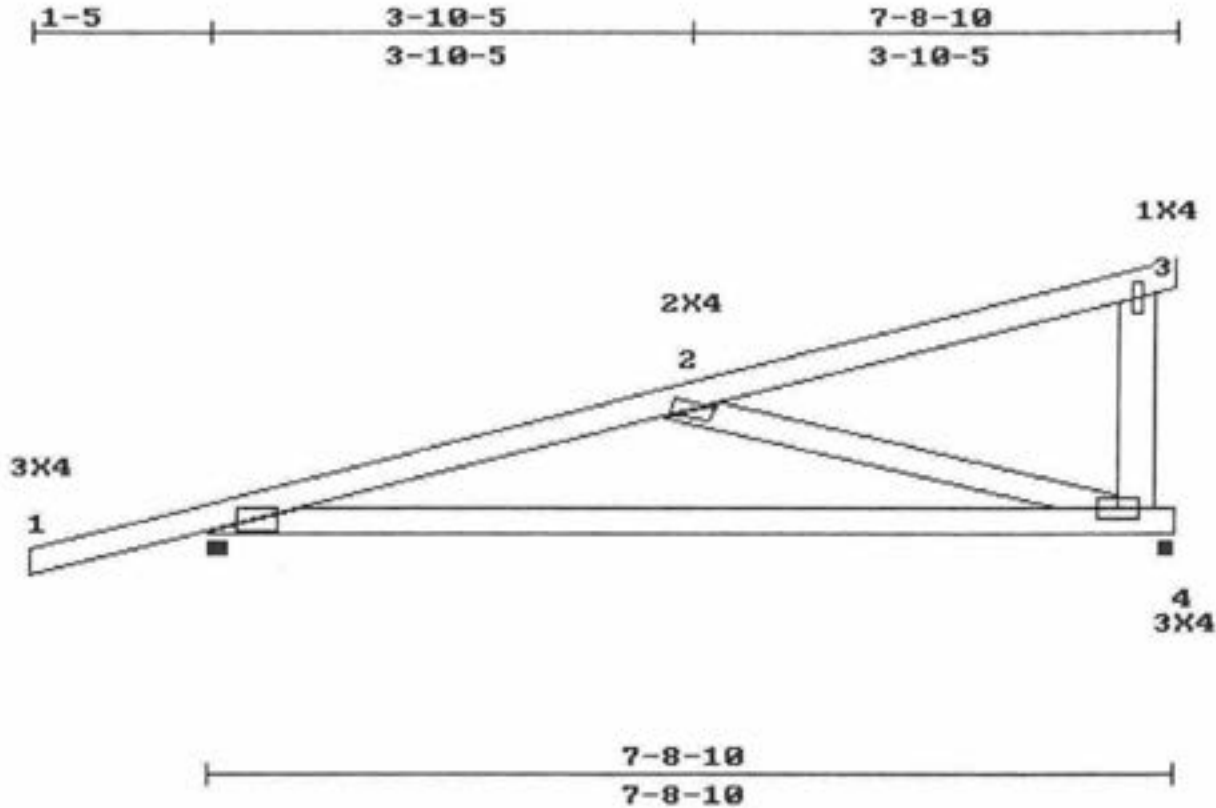
====<<<ACES-32 Ver. 1.1>>>=====[001867]====<<<FL.QUALITY-MMH>>>=====
 Customer : FOGLIA Thu Mar 1 09:57:29 2001
 Project #: 00469 Truss ID : CJ7A Family # : 204
 Span : 7-8-10 B.Span : 5-5-8 Quant. : 1 Top Pitch : 4.243
 ==Bld(1/21/2000),v1.1=====

FORCES - LOAD CASE #1
 1-2=486 4-1=456 2-4=485
 2-3=0
 3-4=247

REACTIONS - SIZE
 1=333 2.00
 4=490 1.50

(62)

PROVIDE FOR 790 LBS UPLIFT AT JOINT 1 (2.38)
 PROVIDE FOR 867 LBS UPLIFT AT JOINT 4 (1.77)
 PROVIDE FOR 318 LBS HORIZ. REACTION AT JOINT 1



L. HL TO PK:8-2-4
 LEFT HEIGHT:0-3-15 SPAN:7-8-10 RISE:3-0-11 RIGHT HEIGHT:3-0-11

LOADING (PSF)		MAX STRESSES		MINIMUM GRADE OF LUMBER	
TOP	30	TOP	1-2=0.498	TOP CHORD:2X4	No.2ND 19 SP
BOTT	0	BOTT	4-1=0.382	BOT CHORD:2X4	No.2ND 19 SP
		LL.DEFL.@0=0.00	< L/360	WEBS	:2X4 No.3 19 SP

REPETITIVE STRESSES NOT USED

SPACING : 24.0 in. o. c.
 NO. OF MEMBERS = 1

LOADING	STRESS INCREASE	LOADING	PANEL(PLF) / JOINTS(LBS)
LUMBER	1.33	PLATE	1- 2= 47 2- 3= 130 4- 1= 20
1	1.33	UNIFORM	

RIGHT END VERTICAL IS NOT EXPOSED.
 TRUSS HAS BEEN CHECKED FOR 145 M.P.H. WIND LOAD, WALL HEIGHT 16 FT, ENCLOSED BLDG. TYPE, ASSUMING 10 PSF TOTAL D.L.(7+3) (SBC-97).
 DEFLECTION(IN.) L.L= 0.00,D.L=0.00,T.L=0.00
 PLATES ARE MITEK M20-249,200 MANUFACTURED FROM ASTM A 446 GRD A GALVANIZED STEEL(EXCEPT AS SHOWN)
 PLATE MUST BE INSTALLED ON EA. FACE OF JOINT,SYMMETRICALLY(EXCEPT AS SHOWN)DESIGN CONFORMS W/NDS DESIGN SPECS, SBC,ANSI/TPI-95
 THIS DESIGN IS FOR TRUSS FABRICATION ONLY.FOR PERMANENT AND TEMPORARY BRACING(WHICH IS ALWAYS REQD)CONSULT BLDG ARCHITECT OR ENGINEER.

MAR 01 2001

====<<<<ACES-32 Ver. 1.1>>>>=====[001868]====<<<<FL.QUALITY-MMH>>>>=====

Customer : FOGLIA

Thu Mar 1 09:57:50 2001

Project #: 00469

Truss ID : NG1

Family # : 301

Span : 9-0

B.Span : 9-0

Quant. : 2

Top Pitch : /12

==Bld(1/21/2000),v1.1=====

FORCES - LOAD CASE #1

REACTIONS - SIZE

1-2=246	7-8= 0	3-1=1954	6-8= 1954
2-3= 0	8-9= 1772	3-10= 338	
3-4=1772	9-10= 2341	4-10=631	
4-5=2341	10-1= 1772	4-9=207	
5-6=1772		5-9= 631	
6-7=1071		5-8=758	

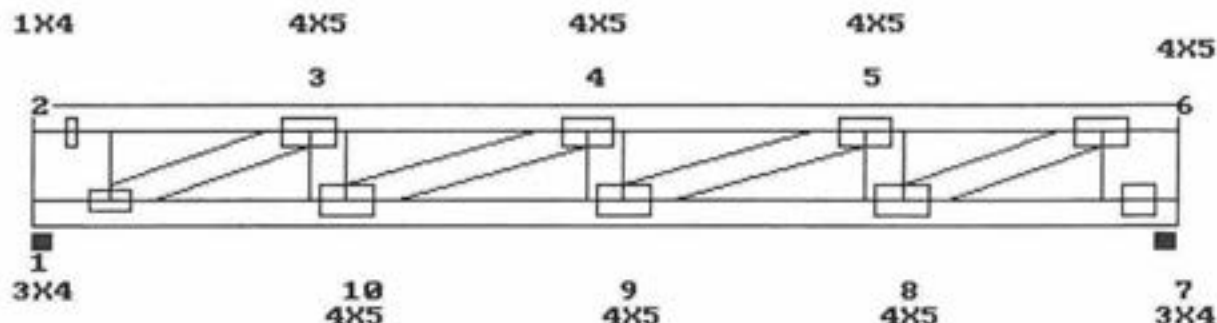
1=1104	2.00
7=1104	2.00

(3)

PROVIDE FOR 1425 LBS UPLIFT AT JOINT 1 (1.29)

PROVIDE FOR 1425 LBS UPLIFT AT JOINT 7 (1.29)

2-3-14	4-6	6-8-2	9-0
2-3-14	2-2-2	2-2-2	2-3-14



2-3-14	4-6	6-8-2	9-0
2-3-14	2-2-2	2-2-2	2-3-14

LEFT HEIGHT:1-4

SPAN:9-0

RISE:1-4

RIGHT HEIGHT:1-4

LOADING (PSF)

MAX STRESSES

MINIMUM GRADE OF LUMBER

L	D
30	15

TOP	4-5=0.325
BOTT	9-10=0.236

TOP CHORD:2X4	No.2ND 19 SP
BOT CHORD:2X4	No.2ND 19 SP

BOTT	0	10

LL.DEFL.@9=0.03 < L/360

WEBS	:2X4	No.3 19 SP

REPETITIVE STRESSES NOT USED

SPACING : 24.0 in. o. c.
NO. OF MEMBERS = 2

LOADING	STRESS INCREASE	LOADING	PANEL(PLF) / JOINTS(LBS)
LUMBER	PLATE	TYPE	
1	1.33	1.33	UNIFORM 2- 6= 220 7- 1= 30

WEB: 1-2; 6-7 TO BE 2X8 SS 19 SP

BOTH END VERTICALS ARE NOT EXPOSED.

2 MEMBERS NAILED TOG. W/1 ROW(S) OF .131x3 in. NAILS 12 in. o.c.(TOP CHS.),AND 1 ROW(S) OF .131x3 in. NAILS 12 in. o.c(BOTT. CHS.)

For Webs use 1 ROW of NAILS 12 in. o.c.

TRUSS HAS BEEN CHECKED FOR 145 M.P.H. WIND LOAD, WALL HEIGHT 16 FT, ENCLOSED BLDG. TYPE, ASSUMING 10 PSF TOTAL D.L.(7+3) (SBC-97).

DEFLECTION(IN.) L.L= 0.03,D.L=0.03,T.L=0.06

PLATES ARE MITEK M20-249,200 MANUFACTURED FROM ASTM A 446 GRD A GALVANIZED STEEL(EXCEPT AS SHOWN)

PLATE MUST BE INSTALLED ON EA. FACE OF JOINT,SYMMETRICALLY(EXCEPT AS SHOWN)DESIGN CONFORMS W/NDIS DESIGN SPECS, SSB,ANSI/TP1-95

THIS DESIGN IS FOR TRUSS FABRICATION ONLY.FOR PERMANENT AND TEMPORARY BRACING(WHICH IS ALWAYS REQD)CONSULT BLDG ARCHITECT OR ENGINEER.

[Signature]

MAR 01 2001

====<<<<ACES-32 Ver. 1.1>>>>=====[001869]====<<<<FL.QUALITY-MMH>>>>=====

Customer : FOGLIA

Project #: 00469

Span : 5-5-8

Truss ID : CE5

Quantity : 1

Thu Mar 1 09:58:08 2001

Family #: 205

Top Pitch : 6/12

==Bld(1/21/2000),v1.1=====

FORCES - GRAVITY LOADS

1-2= 0 3-1= 0
2-3= 0

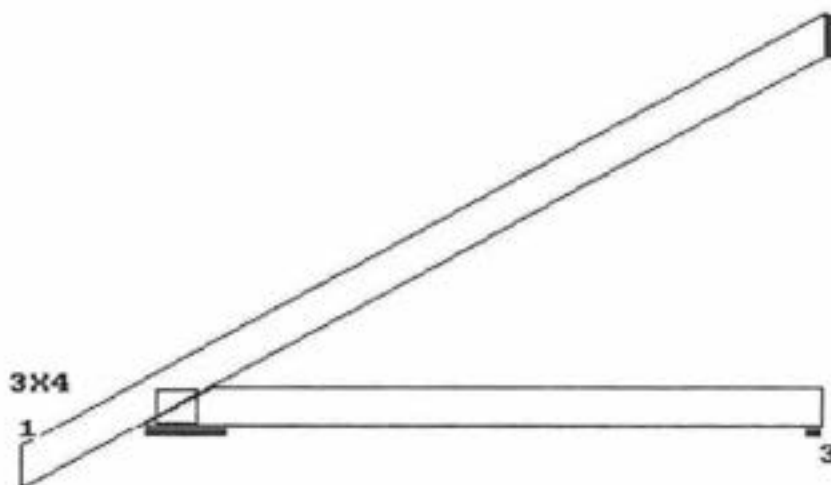
REACTIONS - SIZE

1--388 8.00
3--51 1.50
2--228 1.50

(64)

PROVIDE FOR 289 LBS UPLIFT AT JOINT 1 (0.74)
PROVIDE FOR 347 LBS UPLIFT AT JOINT 2 (1.52)
PROVIDE FOR 316 LBS HORIZ. REACTION AT JOINT 1
(SYMMETRIC PLATING TURNED ON)

1-0 5-5-8
5-5-8



5-5-8
5-5-8

L. HL TO PK:6-1-4
LEFT HEIGHT:0-4-3

SPAN:5-5-8

RISE:3-0-15

RIGHT HEIGHT:3-0-15

LOADING (PSF)			MAX STRESSES		MINIMUM GRADE OF LUMBER	
TOP	30	15	TOP	1-2=0.852	TOP CHORD:2X4	No.2ND 19 SP
BOTT	0	10	BOTT	3-1=0.125	BOT CHORD:2X4	No.2ND 19 SP
			LL.DEFL. < L/360		WEBS	:2X4 No.3 19 SP

STR.INC.: LUMB = 1.33 PLATE = 1.33
REPETITIVE STRESSES USED

SPACING : 24.0 in. o. c.
NO. OF MEMBERS = 1

TRUSS HAS BEEN CHECKED FOR 145 M.P.H. WIND LOAD, WALL HEIGHT 16 FT, ENCLOSED BLDG. TYPE, ASSUMING 10 PSF TOTAL D.L.(7+3) (SBC-97).
DEFLECTION(IN.) L.L= 0.00,D.L=0.00,T.L=0.00

PLATES ARE MITEK M20-249,200 MANUFACTURED FROM ASTM A 446 GRD A GALVANIZED STEEL(EXCEPT AS SHOWN)

PLATE MUST BE INSTALLED ON EA. FACE OF JOINT,SYMMETRICALLY(EXCEPT AS SHOWN)DESIGN CONFORMS W/NDS DESIGN SPECS, SBC,ANSI/TPI-95

THIS DESIGN IS FOR TRUSS FABRICATION ONLY.FOR PERMANENT AND TEMPORARY BRACING(WHICH IS ALWAYS REQD)CONSULT BLDG ARCHITECT OR ENGINEER.

MAR 01 2001

====<<<ACES-32 Ver. 1.1>>>===== [001870] =====<<<FL.QUALITY-MMH>>>=====

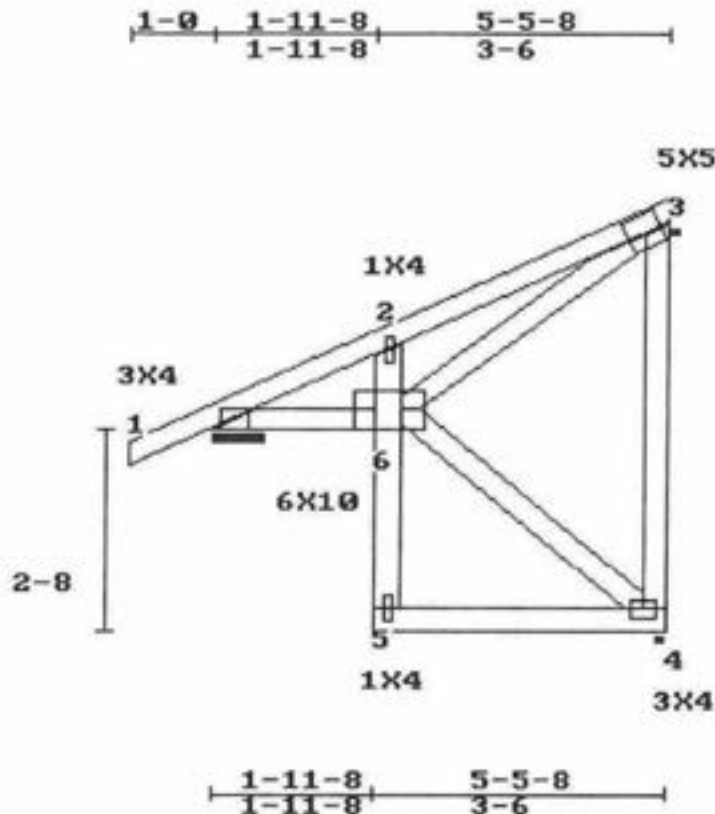
Customer : FOGLIA Truss ID : CF5 Thu Mar 1 09:58:21 2001
 Project #: 00469 Quantity : 1 Family #: special
 Span : 5-5-8 Top Pitch : 6/12
 Bot pitch : 90/12

==Bld(1/21/2000),v1.1=====

FORCES - GRAVITY LOADS			REACTIONS - SIZE	
1-2=353	4-5= 0	2-6=176	1=388	8.00
2-3=305	5-6= 33	3-6= 354	4=33	1.50
3-4= 0	6-1= 299	6-4= 0	3=246	1.50

(5)

PROVIDE FOR 289 LBS UPLIFT AT JOINT 1 (0.74)
 PROVIDE FOR 341 LBS UPLIFT AT JOINT 3 (1.39)
 PROVIDE FOR 317 LBS HORIZ. REACTION AT JOINT 1
 PLATE OFFSETS (X=LEFT,Y=TOP):[j3=2.5,3],(SYMMETRIC PLATING TURNED ON)



L. HL TO PK:6-1-4		SPAN:5-5-8		RISE:5-8-15		RIGHT HEIGHT:5-8-15	
LEFT HEIGHT:0-4-3							
LOADING (PSF)		MAX STRESSES		MINIMUM GRADE OF LUMBER			
TOP	30 15	TOP	2-3=0.364	TOP CHORD:	2X4	No.2ND	19 SP
BOTT	0 10	BOTT	6-1=0.068	BOT CHORD:	2X4	No.2ND	19 SP
		LL.DEFL.@6=0.00	< L/360	WEBS	:2X4	No.3	19 SP
STR.INC.: LUMB = 1.33 PLATE = 1.33				SPACING : 24.0 in. o. c.			
REPETITIVE STRESSES USED				NO. OF MEMBERS = 1			

CHORD: 5-6 TO BE 2X4 No.3 19 SP
 RIGHT END VERTICAL IS NOT EXPOSED.
 TRUSS HAS BEEN CHECKED FOR 145 M.P.H. WIND LOAD, WALL HEIGHT 16 FT, ENCLOSED BLDG. TYPE, ASSUMING 10 PSF TOTAL D.L.(7+3) (SBC-97).
 DEFLECTION(IN.) L.L= 0.00,D.L=0.00,T.L=0.01
 PLATES ARE MITEK M20-249,200 MANUFACTURED FROM ASTM A 446 GRD A GALVANIZED STEEL(EXCEPT AS SHOWN)
 PLATE MUST BE INSTALLED ON EA. FACE OF JOINT,SYMMETRICALLY(EXCEPT AS SHOWN)DESIGN CONFORMS W/NDS DESIGN SPECS, S58C,ANSI/TPI-95
 THIS DESIGN IS FOR TRUSS FABRICATION ONLY.FOR PERMANENT AND TEMPORARY BRACING(WHICH IS ALWAYS REQD)CONSULT BLDG ARCHITECT OR ENGINEER.

MAR 01 2001

====<<<ACES-32 Ver. 1.1>>>===== [001871] =====<<<FL.QUALITY-MMH>>>=====

Customer : FOGLIA Truss ID : S21 Thu Mar 1 09:58:41 2001

Project #: 00469 Quantity : 3 Family # : special

Span : 4-9 Top Pitch : 6/12

Bot pitch : 90/12

====Bld(1/21/2000),v1.1=====

FORCES - GRAVITY LOADS			REACTIONS - SIZE	
1-2= 19	5-6= 86	2-6= 137	2=-446	2.00
2-3=-92	6-7= 19	3-6=-62	5=-139	1.50
3-4= 0	7-1= 0	3-5=-140	4=-119	1.50
4-5= 0		6-1= 0		

(60)

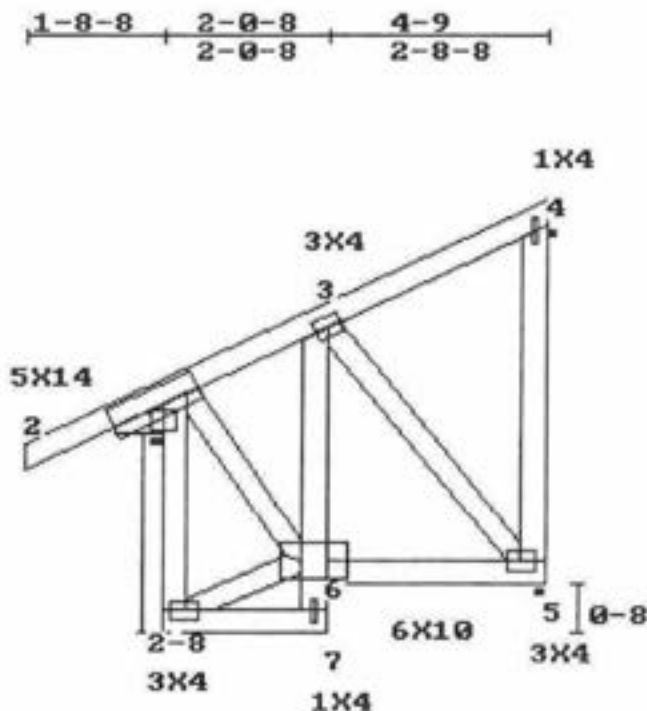
PROVIDE FOR 382 LBS UPLIFT AT JOINT 2 (0.86)

PROVIDE FOR 118 LBS UPLIFT AT JOINT 5 (0.85)

PROVIDE FOR 174 LBS UPLIFT AT JOINT 4 (1.46)

PROVIDE FOR 262 LBS HORIZ. REACTION AT JOINT 2

PLATE OFFSETS (X=LEFT,Y=TOP): [2=12,3], (SYMMETRIC PLATING TURNED ON)



2-0-8 4-9
2-0-8 2-8-8

L. HL TO PK:5-3-12 INTERNAL RISE:0-8

LEFT HEIGHT:3-4-7 SPAN:4-9 RISE:5-8-15 RIGHT HEIGHT:5-0-15

LOADING (PSF)		MAX STRESSES		MINIMUM GRADE OF LUMBER	
L	D	TOP	BOTT	TOP CHORD	BOT CHORD
30	15	2-3=0.153	5-6=0.051	2X4 No.2ND 19 SP	2X4 No.2ND 19 SP
0	10	LL.DEFL.@7=0.00 < L/360		WEBS :2X4 No.3 19 SP	

STR.INC.: LUMB = 1.33 PLATE = 1.33 SPACING : 24.0 in. o. c.

REPETITIVE STRESSES USED NO. OF MEMBERS = 1

CHORD: 6-7 TO BE 2X4 No.3 19 SP

BOTH END VERTICALS ARE NOT EXPOSED.

TRUSS HAS BEEN CHECKED FOR 145 M.P.H. WIND LOAD, WALL HEIGHT 16 FT, ENCLOSED BLDG. TYPE, ASSUMING 10 PSF TOTAL D.L.(7+3) (SBC-97).

DEFLECTION(IN.) L.L= 0.00,D.L=0.00,T.L=0.00

PLATES ARE MITEK M20-249,200 MANUFACTURED FROM ASTM A 446 GRD A GALVANIZED STEEL(EXCEPT AS SHOWN)

PLATE MUST BE INSTALLED ON EA. FACE OF JOINT,SYMMETRICALLY(EXCEPT AS SHOWN)DESIGN CONFORMS W/NDS DESIGN SPECS, SBC,ANSI/TP1-95

THIS DESIGN IS FOR TRUSS FABRICATION ONLY.FOR PERMANENT AND TEMPORARY BRACING(WHICH IS ALWAYS REQD)CONSULT BLDG ARCHITECT OR ENGINEER.

[Signature]

MAR 01 2001

====<<<<ACES-32 Ver. 1.1>>>>===== [001872] =====<<<<FL.QUALITY-MMH>>>>=====

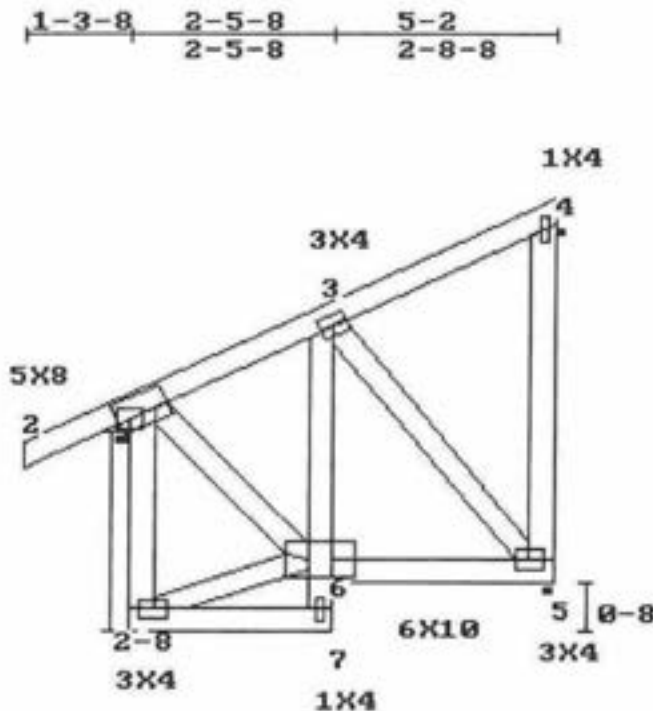
Customer : FOGLIA	Truss ID : S21A	Thu Mar 1 09:58:51 2001
Project #: 00469	Quantity : 2	Family # : special
Span : 5-2		Top Pitch : 6/12
		Bot pitch : 90/12

==Bld(1/21/2000),v1.1=====

FORCES - GRAVITY LOADS			REACTIONS - SIZE	
1-2= 23	5-6= 104	2-6= 142	2=-423	2.00
2-3=-112	6-7= 23	3-6=-48	5=-162	1.50
3-4= 0	7-1= 0	3-5=-169	4=-119	1.50
4-5= 0		6-1= 0		

(2)

PROVIDE FOR 343 LBS UPLIFT AT JOINT 2 (0.81)
 PROVIDE FOR 144 LBS UPLIFT AT JOINT 5 (0.89)
 PROVIDE FOR 174 LBS UPLIFT AT JOINT 4 (1.46)
 PROVIDE FOR 286 LBS HORIZ. REACTION AT JOINT 2
 PLATE OFFSETS (X=LEFT,Y=TOP):[J2=6,3],(SYMMETRIC PLATING TURNED ON)



2-5-8 5-2
2-5-8 2-8-8

L. HL TO PK:5-9-5 INTERNAL RISE:0-8
 LEFT HEIGHT:3-1-15 SPAN:5-2 RISE:5-8-15 RIGHT HEIGHT:5-0-15

LOADING (PSF)		MAX STRESSES		MINIMUM GRADE OF LUMBER	
TOP	30 15	TOP	2-3=0.171	TOP CHORD:2X4	No.2ND 19 SP
BOTT	0 10	BOTT	5-6=0.054	BOT CHORD:2X4	No.2ND 19 SP
		LL.DEFL.@7=0.00 < L/360		WEBS	:2X4 No.3 19 SP

STR.INC.: LUMB = 1.33 PLATE = 1.33
 REPETITIVE STRESSES USED

SPACING : 24.0 in. o. c.
 NO. OF MEMBERS = 1

CHORD: 6-7 TO BE 2X4 No.3 19 SP
 BOTH END VERTICALS ARE NOT EXPOSED.
 TRUSS HAS BEEN CHECKED FOR 145 M.P.H. WIND LOAD, WALL HEIGHT 16 FT, ENCLOSED BLDG. TYPE, ASSUMING 10 PSF TOTAL D.L.(7+3) (SBC-97).
 DEFLECTION(IN.) L.L= 0.00,D.L=0.00,T.L=0.00
 PLATES ARE MITEK M20-249,200 MANUFACTURED FROM ASTM A 446 GRD A GALVANIZED STEEL(EXCEPT AS SHOWN)
 PLATE MUST BE INSTALLED ON EA. FACE OF JOINT,SYMMETRICALLY(EXCEPT AS SHOWN)DESIGN CONFORMS W/NDS DESIGN SPECS, SBC,ANSI/TPI-95
 THIS DESIGN IS FOR TRUSS FABRICATION ONLY.FOR PERMANENT AND TEMPORARY BRACING(WHICH IS ALWAYS REQD)CONSULT BLDG ARCHITECT OR ENGINEER.

[Signature]
 MAR 01 2001

====<<<ACES-32 Ver. 1.1>>>=====[001873]====<<<FL.QUALITY-MMH>>>=====
 Customer : FOGLIA Thu Mar 1 09:59:02 2001
 Project #: 00469 Truss ID : S21B Family #: special
 Span : 5-2 Quantity : 1 Top Pitch : 6/12
 Bot pitch : 90/12

==Bld(1/21/2000),v1.1=====

FORCES - GRAVITY LOADS

1-2= 23 5-6= 104 2-6= 142
 2-3= -112 6-7= 23 3-6= -48
 3-4= 0 7-1= 0 3-5= -169
 4-5= 0 6-1= 0

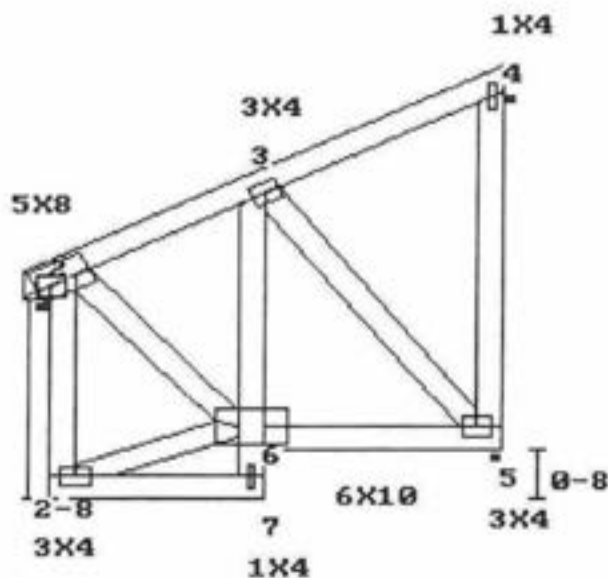
REACTIONS - SIZE

2= 313 2.00
 5= 162 1.50
 4= 119 1.50

(68)

PROVIDE FOR 217 LBS UPLIFT AT JOINT 2 (0.69)
 PROVIDE FOR 144 LBS UPLIFT AT JOINT 5 (0.89)
 PROVIDE FOR 174 LBS UPLIFT AT JOINT 4 (1.46)
 PROVIDE FOR 286 LBS HORIZ. REACTION AT JOINT 2
 PLATE OFFSETS (X=LEFT,Y=TOP):[J2=6,3],(SYMMETRIC PLATING TURNED ON)

0-3-8 2-5-8 5-2
 2-5-8 2-8-8



2-5-8 5-2
 2-5-8 2-8-8

L. HL TO PK:5-9-5 INTERNAL RISE:0-8
 LEFT HEIGHT:3-1-15 SPAN:5-2 RISE:5-8-15 RIGHT HEIGHT:5-0-15

LOADING (PSF)		MAX STRESSES		MINIMUM GRADE OF LUMBER	
TOP	30	TOP	2-3=0.171	TOP CHORD:	2X4 No.2ND 19 SP
BOTT	0	BOTT	5-6=0.054	BOT CHORD:	2X4 No.2ND 19 SP
		LL.DEFL.@7=0.00	< L/360	WEBS	:2X4 No.3 19 SP

STR.INC.: LUMB = 1.33 PLATE = 1.33 SPACING : 24.0 in. o. c.
 REPETITIVE STRESSES USED NO. OF MEMBERS = 1

CHORD: 6-7 TO BE 2X4 No.3 19 SP

BOTH END VERTICALS ARE NOT EXPOSED.

TRUSS HAS BEEN CHECKED FOR 145 M.P.H. WIND LOAD, WALL HEIGHT 16 FT, ENCLOSED BLDG. TYPE, ASSUMING 10 PSF TOTAL D.L.(7+3) (SBC-97).

DEFLECTION(IN.) L.L= 0.00,D.L=0.00,T.L=0.00

PLATES ARE MITEK M20-249,200 MANUFACTURED FROM ASTM A 446 GRD A GALVANIZED STEEL(EXCEPT AS SHOWN)

PLATE MUST BE INSTALLED ON EA. FACE OF JOINT,SYMMETRICALLY(EXCEPT AS SHOWN)DESIGN CONFORMS W/NDS DESIGN SPECS, SSB,ANSI/TP1-95

THIS DESIGN IS FOR TRUSS FABRICATION ONLY.FOR PERMANENT AND TEMPORARY BRACING(WHICH IS ALWAYS REQD)CONSULT BLDG ARCHITECT OR ENGINEER.

[Signature]
 MAR 01 2001

====<<<<ACES-32 Ver. 1.1>>>>===== [002013] =====<<<<FL.QUALITY-MMH>>>>=====

Customer : FOGLIA Wed Mar 7 14:53:43 2001

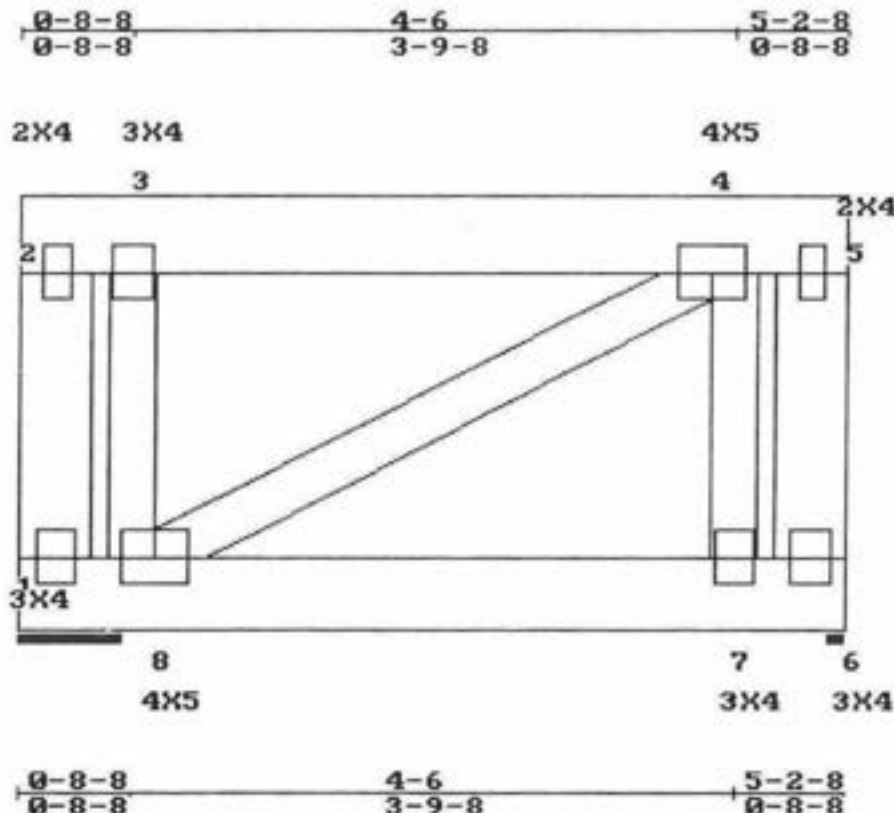
Project #: 00469 Truss ID : FG1 Family #: 301

Span : 5-2-8 B.Span : 5-10-8 Quant. : 2 Top Pitch : /12

==pld(1/21/2000),v1.1=====

REACTIONS - SIZE
1--270 8.00
6--270 1.50

PROVIDE FOR 240 LBS UPLIFT AT JOINT 1 (0.89)
PROVIDE FOR 240 LBS UPLIFT AT JOINT 6 (0.89)



LEFT HEIGHT:2-8 SPAN:5-2-8 RISE:2-8 RIGHT HEIGHT:2-8

LOADING (PSF)		MAX STRESSES		MINIMUM GRADE OF LUMBER	
L	D	TOP	BOTT	TOP CHORD	BOT CHORD
30	15	3-4=0.080	7-8=0.057	2X6 No.2D 19 SP	2X6 No.2D 19 SP
0	10	LL.DEFL.@7=0.01 < L/360		WEBS :2X4	No.3 19 SP

REPETITIVE STRESSES NOT USED

SPACING : 24.0 in. o. c.
NO. OF MEMBERS = 2

LOADING	STRESS INCREASE	LOADING	PANEL(PLF) / JOINTS(LBS)
LUMBER	PLATE	TYPE	
1	1.33	1.33	UNIFORM 2- 5= 90 6- 1= 20

WEB: 1-2; 5-6 TO BE 2X6 No.2D 19 SP

BOTH END VERTICALS ARE NOT EXPOSED.

2 MEMBERS NAILED TOG. W/1 ROW(S) OF .131x3 in. NAILS 12 in. o.c.(TOP CHS.),AND 1 ROW(S) OF .131x3 in. NAILS 12 in. o.c(BOTT. CHS.)

For Webs use 1 ROW of NAILS 12 in. o.c.

TRUSS HAS BEEN CHECKED FOR 145 M.P.H. WIND LOAD, WALL HEIGHT 16 FT, ENCLOSED BLDG. TYPE, ASSUMING 10 PSF TOTAL D.L.(7+3) (SBC-97).

DEFLECTION(IN.) L.L= 0.01,D.L=0.00,T.L=0.01

PLATES ARE MITEK M20-249,200 MANUFACTURED FROM ASTM A 446 GRD A GALVANIZED STEEL(EXCEPT AS SHOWN)

PLATE MUST BE INSTALLED ON EA. FACE OF JOINT,SYMMETRICALLY(EXCEPT AS SHOWN)DESIGN CONFORMS W/NDS DESIGN SPECS, SIBC,NDS,ANSI/TPI-95

THIS DESIGN IS FOR TRUSS FABRICATION ONLY.FOR PERMANENT AND TEMPORARY BRACING(WHICH IS ALWAYS REQD)CONSULT BLDG ARCHITECT OR ENGINEER.

MAR 07 2001

====<<<<ACES-32 Ver. 1.1>>>>=====[001877]=====<<<<FL.QUALITY-MMH>>>>=====

Customer : FOGLIA

Thu Mar 1 10:00:34 2001

Project #: 00469

Truss ID : CJ7

Family # : 204

Span : 7-6-8

B.Span : 5-4

Quant. : 1

Top Pitch : 4.243

==8(d(1/21/2000),v1.1=====

FORCES - LOAD CASE #1

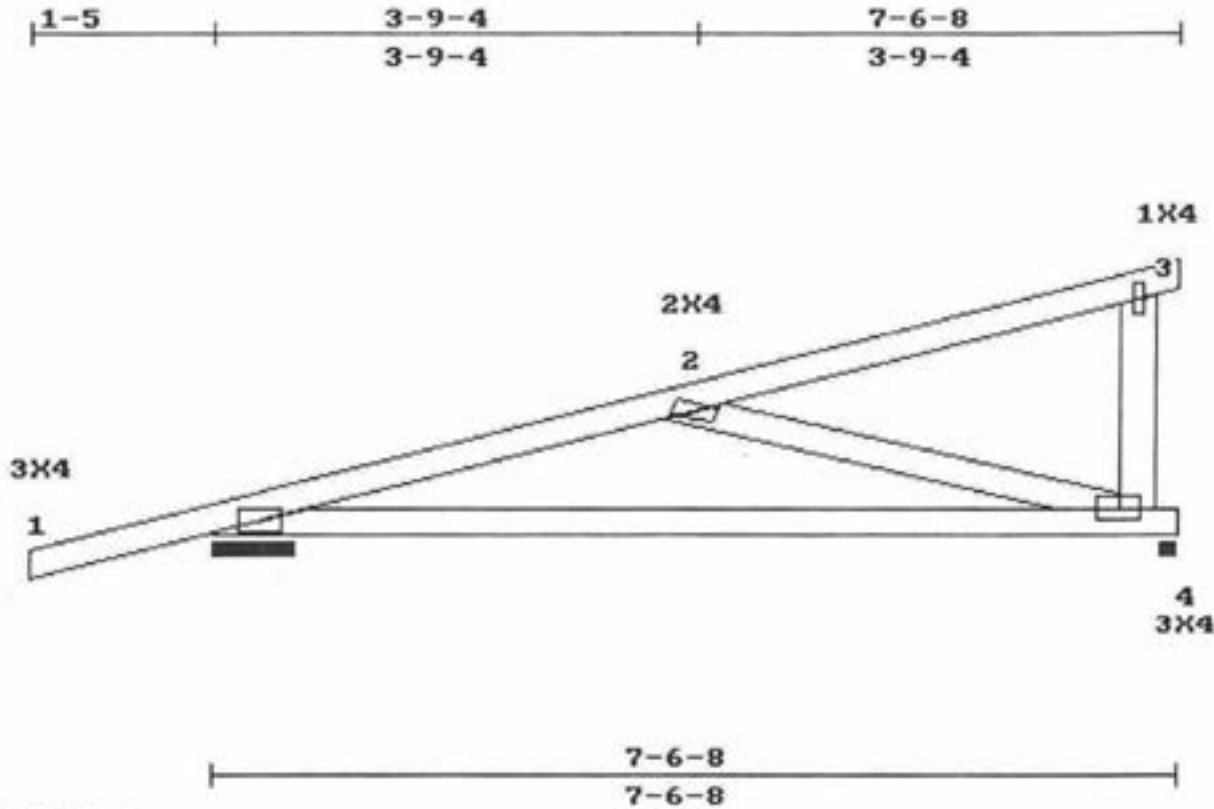
REACTIONS - SIZE

1-2=442 4-1= 411 2-4=438
2-3= 0
3-4=-235

1=468 8.00
4=456 1.50

70

PROVIDE FOR 721 LBS UPLIFT AT JOINT 1 (1.54)
PROVIDE FOR 806 LBS UPLIFT AT JOINT 4 (1.77)
PROVIDE FOR 311 LBS HORIZ. REACTION AT JOINT 1



L. HL TO PK:8-0
LEFT HEIGHT:0-3-15 SPAN:7-6-8 RISE:2-11-15 RIGHT HEIGHT:2-11-15

LOADING (PSF)		MAX STRESSES		MINIMUM GRADE OF LUMBER	
TOP	30	TOP	1-2=0.462	TOP CHORD:2X4	No.2ND 19 SP
BOTT	0	BOTT	4-1=0.353	BOT CHORD:2X4	No.2ND 19 SP
		LL.DEFL.@0=0.00	< L/360	WEBS	:2X4 No.3 19 SP

REPETITIVE STRESSES NOT USED

SPACING : 24.0 in. o. c.
NO. OF MEMBERS = 1

LOADING	STRESS INCREASE	LOADING	PANEL(PLF) / JOINTS(LBS)
LUMBER	1.33	PLATE	1- 2= 46 2- 3= 127 4- 1= 20
TYPE	UNIFORM		

RIGHT END VERTICAL IS NOT EXPOSED.

TRUSS HAS BEEN CHECKED FOR 145 M.P.H. WIND LOAD, WALL HEIGHT 16 FT, ENCLOSED BLDG. TYPE, ASSUMING 10 PSF TOTAL D.L.(7+3) (SBC-97).
DEFLECTION(IN.) L.L= 0.00,D.L=0.00,T.L=0.00

PLATES ARE MITEK M20-249,200 MANUFACTURED FROM ASTM A 446 GRD A GALVANIZED STEEL(EXCEPT AS SHOWN)

PLATE MUST BE INSTALLED ON EA. FACE OF JOINT,SYMMETRICALLY(EXCEPT AS SHOWN)DESIGN CONFORMS W/NDG DESIGN SPECS, SIBC,ANSI/TP1-95

THIS DESIGN IS FOR TRUSS FABRICATION ONLY.FOR PERMANENT AND TEMPORARY BRACING(WHICH IS ALWAYS REQD)CONSULT BLDG ARCHITECT OR ENGINEER.

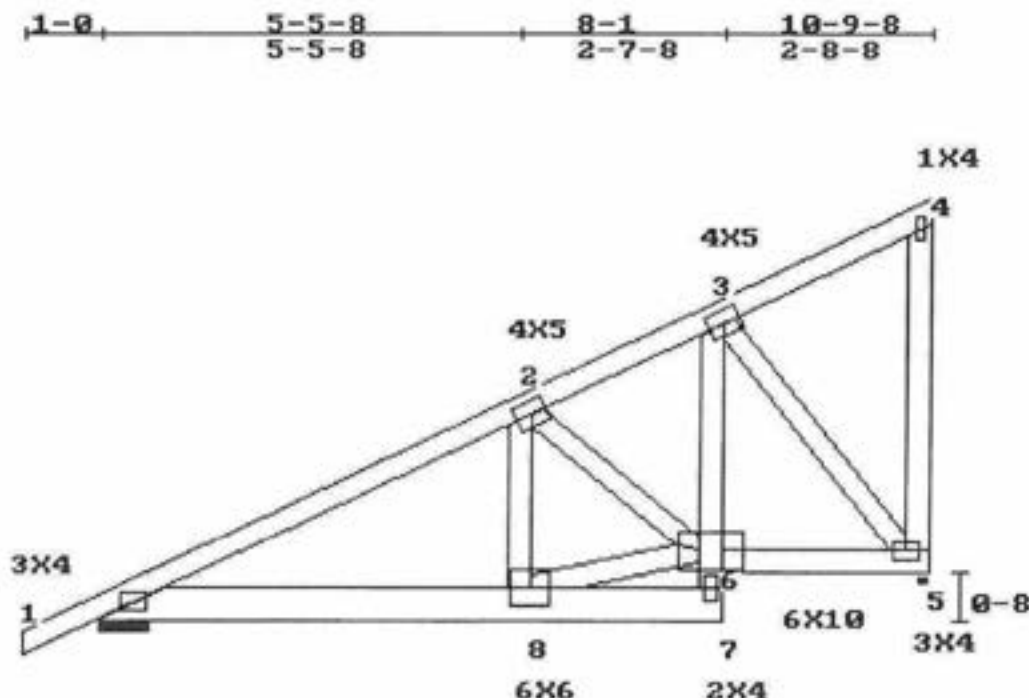
WAR 0 1 2001

====<<<<ACES-32 Ver. 1.1>>>>=====[001878]====<<<<FL.QUALITY-MMH>>>>=====
 Customer : FOGLIA Thu Mar 1 10:00:58 2001
 Project #: 00469 Truss ID : GT10 Family #: special
 Span : 10-9-8 Top Pitch : 6/12
 Bot pitch : 90/12

==Bld(1/21/2000),v1.1=====

FORCES - LOAD CASE #1			REACTIONS - SIZE	
1-2=1798	5-6= 739	2-8= 775	1=1116	8.00
2-3=880	6-7= 25	2-6=1056	5=1101	1.50
3-4= 0	7-8= 0	3-6= 1111		
4-5=119	8-1= 1592	3-5=1207		
		6-8= 1636		

PROVIDE FOR 1480 LBS UPLIFT AT JOINT 1 (1.33)
 PROVIDE FOR 1656 LBS UPLIFT AT JOINT 5 (1.50)
 PROVIDE FOR 620 LBS HORIZ. REACTION AT JOINT 1
 (SYMMETRIC PLATING TURNED ON)



L. HL TO PK:12-0-13		INTERNAL RISE:0-8		SPAN:10-9-8		RISE:5-8-15		RIGHT HEIGHT:5-0-15	
LEFT HEIGHT:0-4-3									
LOADING (PSF)		MAX STRESSES		MINIMUM GRADE OF LUMBER					
TOP	30 15	TOP	1-2=0.355	TOP CHORD:2X4	No.2ND 19 SP				
BOTT	0 10	BOTT	8-1=0.115	BOT CHORD:2X4	No.2ND 19 SP				
		LL.DEFL.@8=0.02 < L/360		WEBS	:2X4 No.3 19 SP				

REPETITIVE STRESSES NOT USED

SPACING : 24.0 in. o. c.
 NO. OF MEMBERS = 2

LOADING	STRESS INCREASE	LOADING	PANEL(PLF) / JOINTS(LBS)
LUMBER	1.33	TYPE	
PLATE	1.33	UNIFORM	1- 4= 90 5- 6= 20 7- 1= 20
		CONCENTRATED	8= 1074

CHORD: 6-7 TO BE 2X4 No.3 19 SP
 CHORD: 7-8; 8-1 TO BE 2X6 No.2D 19 SP
 RIGHT END VERTICAL IS NOT EXPOSED.
 2 MEMBERS NAILED TOG. W/1 ROW(S) OF .131x3 in. NAILS 12 in. o.c.(TOP CHS.),AND 1 ROW(S) OF .131x3 in. NAILS 10 in. o.c(BOTT. CHS.)
 For Webs use 1 ROW of NAILS 12 in. o.c.
 TRUSS HAS BEEN CHECKED FOR 145 M.P.H. WIND LOAD, WALL HEIGHT 16 FT, ENCLOSED BLDG. TYPE, ASSUMING 10 PSF TOTAL D.L.(7+3) (SBC-97).
 DEFLECTION(IN.) L.L= 0.02,D.L=0.01,T.L=0.03
 PLATES ARE MITEK M20-249,200 MANUFACTURED FROM ASTM A 446 GRD A GALVANIZED STEEL(EXCEPT AS SHOWN)
 PLATE MUST BE INSTALLED ON EA. FACE OF JOINT,SYMMETRICALLY(EXCEPT AS SHOWN)DESIGN CONFORMS W/NDS DESIGN SPECS, 55SBC,ANSI/TPI-95
 THIS DESIGN IS FOR TRUSS FABRICATION ONLY.FOR PERMANENT AND TEMPORARY BRACING(WHICH IS ALWAYS REQD)CONSULT BLDG ARCHITECT OR ENGINEER.

MAR 01 2001

====<<<<ACES-32 Ver. 1.1>>>>===== [001879] =====<<<<FL.QUALITY-MMH>>>>=====

Customer : FOGLIA Thu Mar 1 10:01:10 2001

Project #: 00469 Truss ID : S17 Family #: special

Span : 10-9-8 Top Pitch : 6/12

Bot pitch : 90/12

==Bld(1/21/2000),v1.1=====

FORCES - GRAVITY LOADS			REACTIONS - SIZE	
1-2=-659	5-6= 207	2-8=-13	1=-682	8.00
2-3=-245	6-7= 39	2-6=-399	5=-572	1.50
3-4= 0	7-8= 0	3-6= 330		
4-5=-76	8-1= 579	3-5=-520		
		6-8= 587		

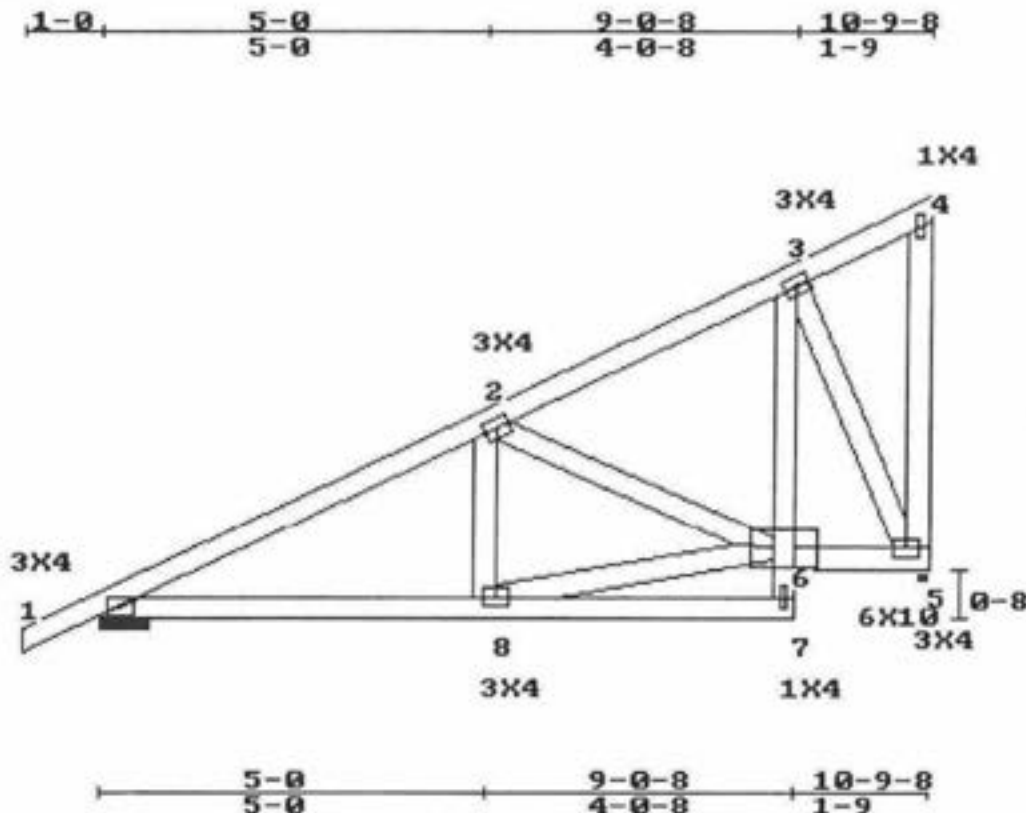
(12)

PROVIDE FOR 471 LBS UPLIFT AT JOINT 1 (0.69)

PROVIDE FOR 670 LBS UPLIFT AT JOINT 5 (1.17)

PROVIDE FOR 630 LBS HORIZ. REACTION AT JOINT 1

(SYMMETRIC PLATING TURNED ON)



LOADING (PSF)		MAX STRESSES		MINIMUM GRADE OF LUMBER	
TOP	30	TOP	1-2=0.615	TOP CHORD:2X4	No.2ND 19 SP
BOTT	0	BOTT	8-1=0.197	BOT CHORD:2X4	No.2ND 19 SP
		LL.DEFL.@8=0.01	< L/360	WEBS	:2X4 No.3 19 SP

STR.INC.: LUMB = 1.33 PLATE = 1.33

REPETITIVE STRESSES USED

SPACING : 24.0 in. o. c.

NO. OF MEMBERS = 1

CHORD: 6-7 TO BE 2X4 No.3 19 SP

RIGHT END VERTICAL IS NOT EXPOSED.

TRUSS HAS BEEN CHECKED FOR 145 M.P.H. WIND LOAD, WALL HEIGHT 16 FT, ENCLOSED BLDG. TYPE, ASSUMING 10 PSF TOTAL D.L.(7+3) (SBC-97).

DEFLECTION(IN.) L.L= 0.01,D.L=0.01,T.L=0.02

PLATES ARE MITEK M20-249,200 MANUFACTURED FROM ASTM A 446 GRD A GALVANIZED STEEL(EXCEPT AS SHOWN)

PLATE MUST BE INSTALLED ON EA. FACE OF JOINT,SYMMETRICALLY(EXCEPT AS SHOWN)DESIGN CONFORMS W/ND5 DESIGN SPECS, SSBC,ANSI/TP1-95

THIS DESIGN IS FOR TRUSS FABRICATION ONLY.FOR PERMANENT AND TEMPORARY BRACING(WHICH IS ALWAYS REQD)CONSULT BLDG ARCHITECT OR ENGINEER.

[Signature]

MAR 01 2001

====<<<<ACES-32 Ver. 1.1>>>>=====[001880]====<<<<FL.QUALITY-MMH>>>>=====

Customer : FOGLIA

Project # : 00469

Span : 10-9-8

Truss ID : J10

Quantity : 5

Thu Mar 1 10:01:20 2001

Family # : 201

Top Pitch : 6/12

==Bld(1/21/2000),v1.1=====

FORCES - GRAVITY LOADS

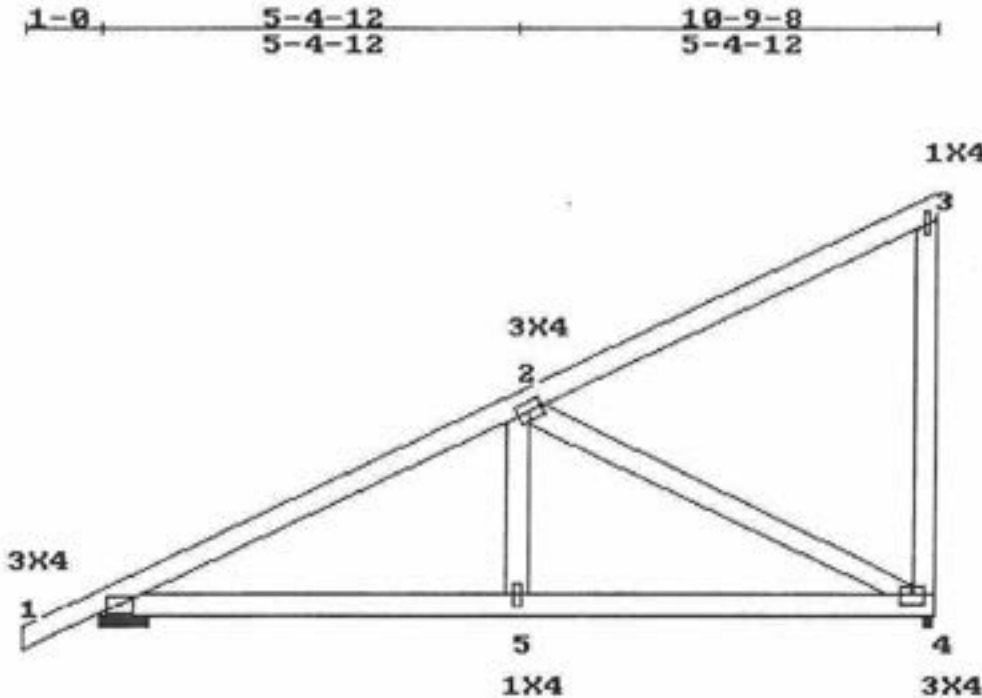
1-2=617 4-5= 543 2-5= 104
2-3= 0 5-1= 543 2-4=610
3-4=240

REACTIONS - SIZE

1=-682 8.00
4=-572 1.50

(13)

PROVIDE FOR 471 LBS UPLIFT AT JOINT 1 (0.69)
PROVIDE FOR 670 LBS UPLIFT AT JOINT 4 (1.17)
PROVIDE FOR 630 LBS HORIZ. REACTION AT JOINT 1
(SYMMETRIC PLATING TURNED ON)



L. HL TO PK:12-0-13
LEFT HEIGHT:0-4-3 SPAN:10-9-8 RISE:5-8-15 RIGHT HEIGHT:5-8-15

LOADING (PSF)		MAX STRESSES		MINIMUM GRADE OF LUMBER	
TOP	30	TOP	1-2=0.867	TOP CHORD:2X4	No.2ND 19 SP
BOTT	0	BOTT	4-5=0.222	BOT CHORD:2X4	No.2ND 19 SP
		LL.DEFL.@5=0.01 < L/360		WEBS	:2X4 No.3 19 SP

STR.INC.: LUMB = 1.33 PLATE = 1.33 SPACING : 24.0 in. o. c.
REPETITIVE STRESSES USED NO. OF MEMBERS = 1

RIGHT END VERTICAL IS NOT EXPOSED.
TRUSS HAS BEEN CHECKED FOR 145 M.P.H. WIND LOAD, WALL HEIGHT 16 FT, ENCLOSED BLDG. TYPE, ASSUMING 10 PSF TOTAL D.L.(7+3) (SBC-97).
DEFLECTION(IN.) L.L= 0.01,D.L=0.01,T.L=0.02
PLATES ARE MITEK M20-249,200 MANUFACTURED FROM ASTM A 446 GRD A GALVANIZED STEEL(EXCEPT AS SHOWN)
PLATE MUST BE INSTALLED ON EA. FACE OF JOINT,SYMMETRICALLY(EXCEPT AS SHOWN)DESIGN CONFORMS W/NDS DESIGN SPECS, SSBC,ANSI/TP1-95
THIS DESIGN IS FOR TRUSS FABRICATION ONLY.FOR PERMANENT AND TEMPORARY BRACING(WHICH IS ALWAYS REQD)CONSULT BLDG ARCHITECT OR ENGINEER.

MAR 01 2001

====<<<<ACES-32 Ver. 1.1>>>>===== [001881] =====<<<<FL.QUALITY-MMH>>>>=====

Customer : FOGLIA
Project #: 00469
Span : 18-0

Truss ID : S15
Quantity : 2

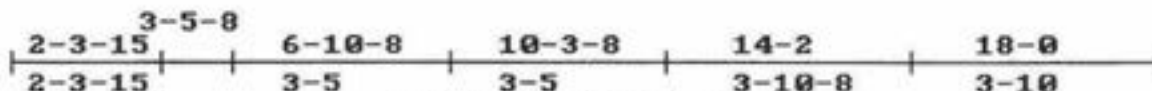
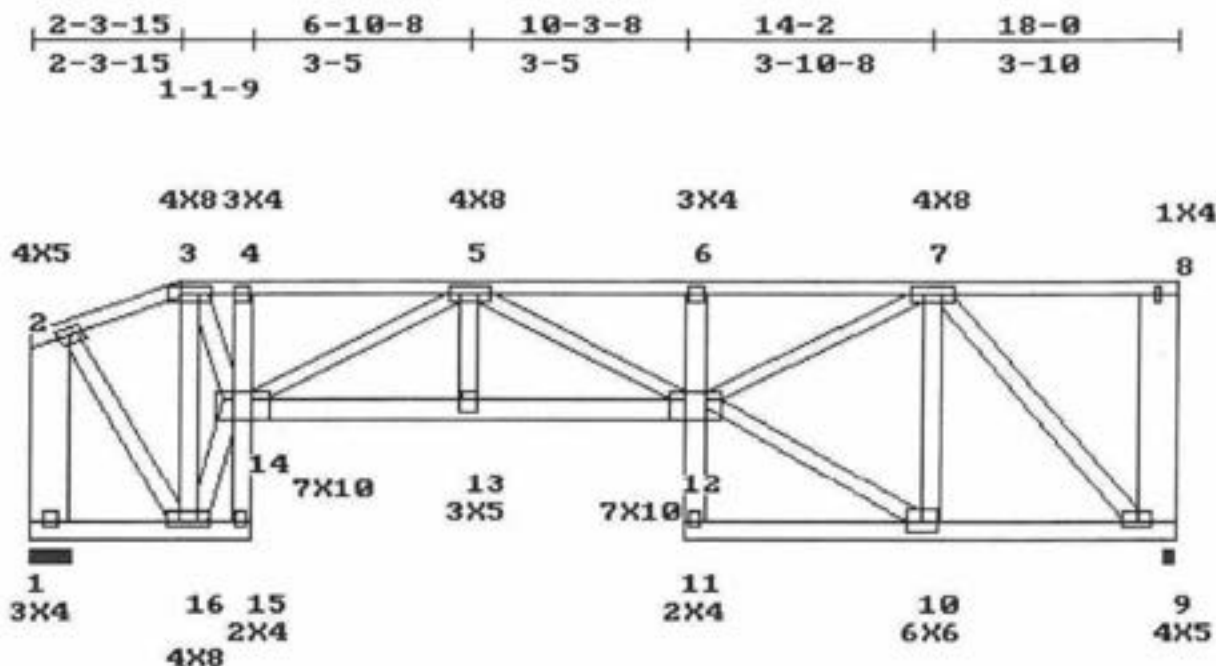
Thu Mar 1 10:06:23 2001
Family #: special
Top Pitch : 6/12
Bot pitch : 90/12

==Bld(1/21/2000),v1.1=====

FORCES - LOAD CASE #1				REACTIONS - SIZE				
1-2=-3209	7-8= 0	9-10= 1936	15-16= 0	2-16= 2781	5-12= 311	14-16= 3397	1=-3523	8.00
2-3=-1320	8-9=-169	10-11= 0	16-1= 0	3-16=-5240	6-12=-329		9=-3523	2.05
3-4=-3250		11-12= 578		3-14= 6062	7-12= 4524			
4-5=-3262		12-13= 5370		4-14=-205	7-10=-225			
5-6=-5617		13-14= 5370		5-14=-2646	7-9=-3382			
6-7=-5600		14-15= 153		5-13= 1104	12-10= 2381			

74

PROVIDE FOR 3997 LBS UPLIFT AT JOINT 1 (1.13)
PROVIDE FOR 4064 LBS UPLIFT AT JOINT 9 (1.15)
PROVIDE FOR 121 LBS HORIZ. REACTION AT JOINT 1
PLATE OFFSETS (X=LEFT,Y=TOP): [j12=5,2], [j13=3,1.5], [j14=5,2], (SYMMETRIC PLATING TURNED ON)



L. HL TO PK:2-7-4 INTERNAL RISE:2-8
LEFT HEIGHT:4-6-15 SPAN:18-0 RISE:5-8-15 RIGHT HEIGHT:5-8-15

LOADING (PSF)		MAX STRESSES		MINIMUM GRADE OF LUMBER	
TOP	30	TOP	6-7=0.702	TOP CHORD:2X4	No.2ND 19 SP
BOTT	0	BOTT	12-13=0.502	BOT CHORD:2X6	No.2D 19 SP
		LL.DEFL.@11=0.11	< L/360	WEBS	:2X4 No.3 19 SP

REPETITIVE STRESSES NOT USED

SPACING : 24.0 in. o. c.
NO. OF MEMBERS = 2

NOTE: PROVIDE FOR -0.28 IN. HORIZONTAL DISPLACEMENT AT ONE BEARING

LOADING	STRESS INCREASE	LOADING	PANEL(PLF) / JOINTS(LBS)
LUMBER	1.33	PLATE	2- 8= 90
TYPE	1.33	TYPE	9-11= 310
			12-14= 310
			15- 1= 310

CHORD: 11-12; 14-15 TO BE 2X4 No.3 19 SP

WEB: 1-2; 8-9 TO BE 2X8 SS 19 SP

BOTH END VERTICALS ARE NOT EXPOSED.

2 MEMBERS NAILED TOG. W/1 ROW(S) OF .131x3 in. NAILS 12 in. o.c.(TOP CHS.),AND 1 ROW(S) OF .131x3 in. NAILS 8 in. o.c.(BOTT. CHS.)

For Webs use 1 ROW of NAILS 12 in. o.c.

TRUSS HAS BEEN CHECKED FOR 145 M.P.H. WIND LOAD, WALL HEIGHT 16 FT, ENCLOSED BLDG. TYPE, ASSUMING 10 PSF TOTAL D.L.(7+3) (SBC-97).

DEFLECTION(IN.) L.L= 0.11,D.L=0.09,T.L=0.20

PLATES ARE MITEK M20-249,200 MANUFACTURED FROM ASTM A 446 GRD A GALVANIZED STEEL(EXCEPT AS SHOWN)

PLATE MUST BE INSTALLED ON EA. FACE OF JOINT,SYMMETRICALLY(EXCEPT AS SHOWN)DESIGN CONFORMS W/MDS DESIGN SPECS, SSSBC,ANSI/TP1-95

THIS DESIGN IS FOR TRUSS FABRICATION ONLY.FOR PERMANENT AND TEMPORARY BRACING(WHICH IS ALWAYS REQD)CONSULT BLDG ARCHITECT OR ENGINEER.

MAR 01 2001

==<<<ACES-32 Ver. 1.1>>>===== [001882] =====<<<FL.QUALITY-MMH>>>=====

Customer : FOGLIA

Project # : 00469

Span : 10-6

Truss ID : S18

Quantity : 4

Thu Mar 1 10:06:33 2001

Family # : 201

Top Pitch : 6/12

==Bld(1/21/2000),v1.1=====

FORCES - GRAVITY LOADS

1-2=590 4-5= 0 2-5=-429
2-3=-577 5-1= 519 3-5= 743
3-4= 51

REACTIONS - SIZE

1=-669 8.00
3=-591 1.50

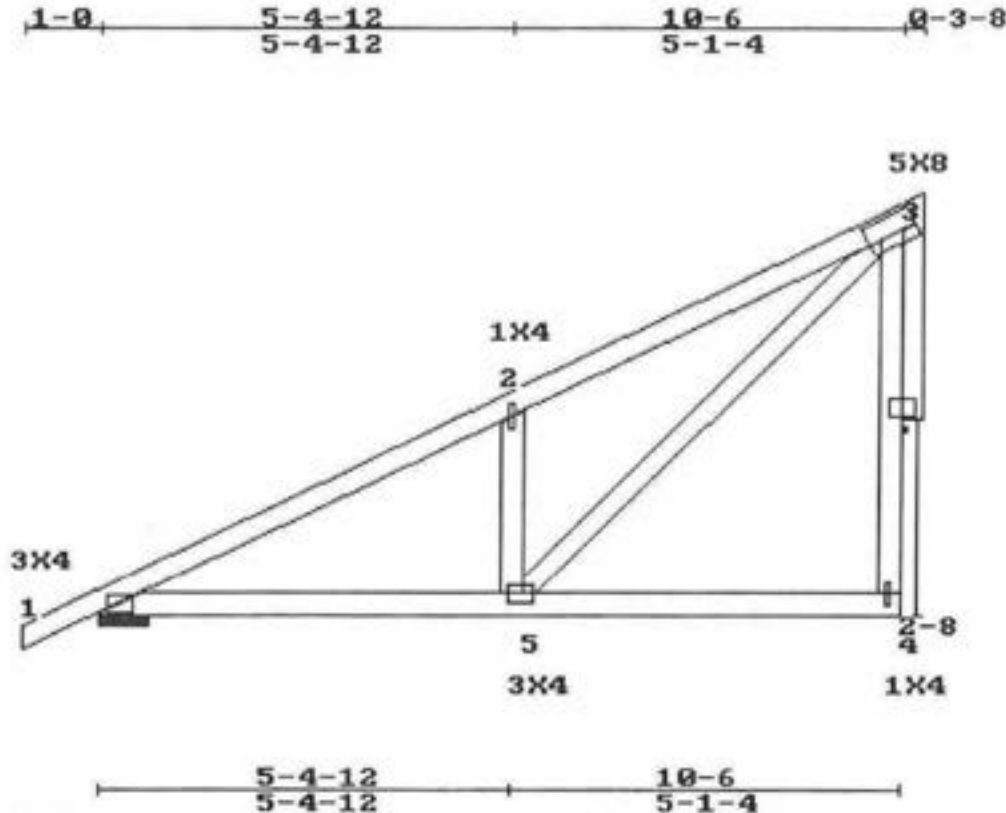
(15)

PROVIDE FOR 465 LBS UPLIFT AT JOINT 1 (0.69)

PROVIDE FOR 690 LBS UPLIFT AT JOINT 3 (1.17)

PROVIDE FOR 613 LBS HORIZ. REACTION AT JOINT 1

PLATE OFFSETS (X=LEFT,Y=TOP):[J3=2,3],(SYMMETRIC PLATING TURNED ON)



L. HL TO PK:11-8-14
LEFT HEIGHT:0-4-3

SPAN:10-6

RISE:5-8-15

RIGHT HEIGHT:5-7-3

LOADING (PSF)

TOP 30 15
BOTT 0 10

MAX STRESSES

TOP 2-3=0.880
BOTT 5-1=0.211
LL.DEFL.@5=0.01 < L/360

MINIMUM GRADE OF LUMBER

TOP CHORD:2X4 No.2ND 19 SP
BOT CHORD:2X4 No.2ND 19 SP
WEBS :2X4 No.3 19 SP

STR.INC.: LUMB = 1.33 PLATE = 1.33
REPETITIVE STRESSES USED

SPACING : 24.0 in. o. c.
NO. OF MEMBERS = 1

RIGHT END VERTICAL IS NOT EXPOSED.

TRUSS HAS BEEN CHECKED FOR 145 M.P.H. WIND LOAD, WALL HEIGHT 16 FT, ENCLOSED BLDG. TYPE, ASSUMING 10 PSF TOTAL D.L.(7+3) (SBC-97).

DEFLECTION(IN.) L.L= 0.01,D.L=0.01,T.L=0.03

PLATES ARE MITEK M20-249,200 MANUFACTURED FROM ASTM A 446 GRD A GALVANIZED STEEL(EXCEPT AS SHOWN)

PLATE MUST BE INSTALLED ON EA. FACE OF JOINT,SYMMETRICALLY(EXCEPT AS SHOWN)DESIGN CONFORMS W/NDS DESIGN SPECS, SIBC,ANSI/TPI-95

THIS DESIGN IS FOR TRUSS FABRICATION ONLY.FOR PERMANENT AND TEMPORARY BRACING(WHICH IS ALWAYS REQD)CONSULT BLDG ARCHITECT OR ENGINEER.

MAR 01 2001

====<<<<ACES-32 Ver. 1.1>>>>===== [001883] =====<<<<FL.QUALITY-MMH>>>>=====

Customer : FOGLIA
 Project # : 00469
 Span : 10-6

Truss ID : S19
 Quantity : 1

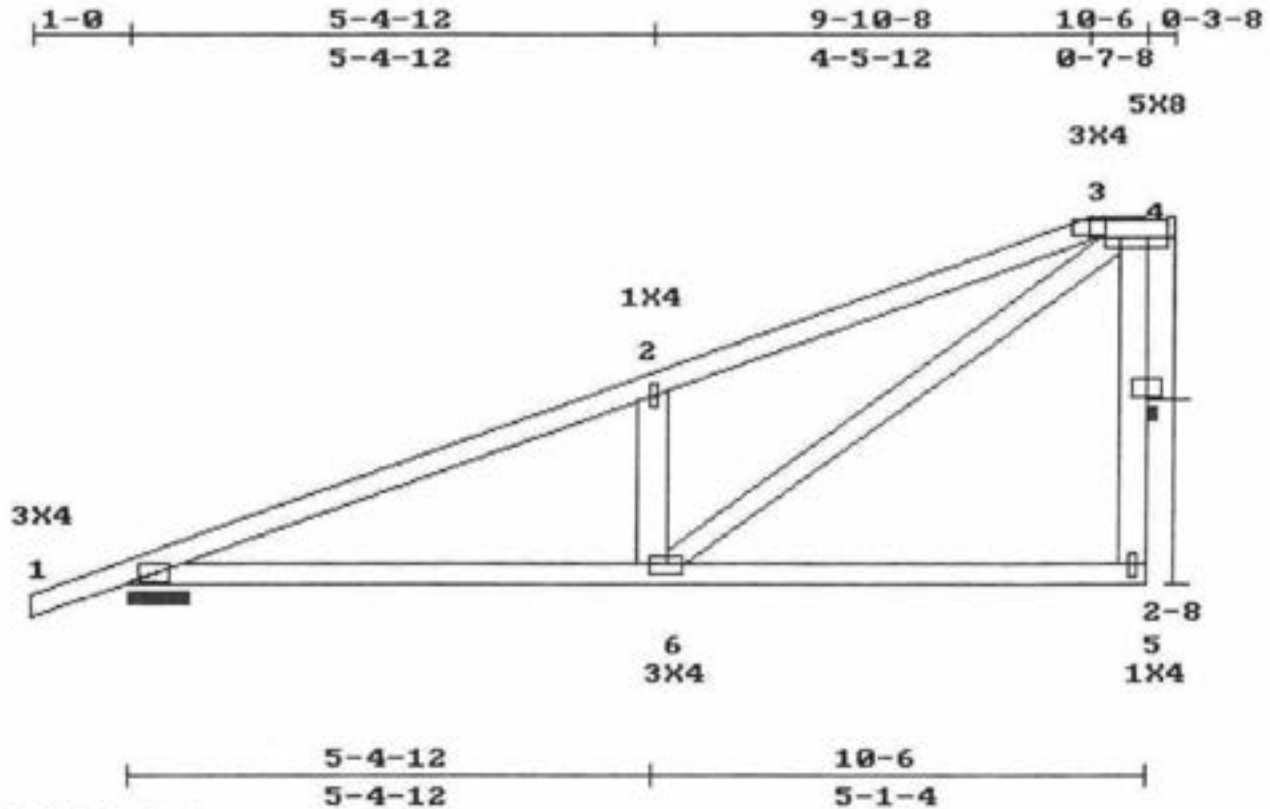
Thu Mar 1 10:06:42 2001
 Family # : 201
 Top Pitch : 6/12

==Bld(1/21/2000),v1.1=====

REACTIONS - SIZE
 1=669 8.00
 4=591 1.50

(76)

PROVIDE FOR 478 LBS UPLIFT AT JOINT 1 (0.71)
 PROVIDE FOR 677 LBS UPLIFT AT JOINT 4 (1.15)
 PROVIDE FOR 587 LBS HORIZ. REACTION AT JOINT 1
 PLATE OFFSETS (X=LEFT,Y=TOP):[J4=2,3],(SYMMETRIC PLATING TURNED ON)



L. HL TO PK:11-0-8
 LEFT HEIGHT:0-4-3
 SPAN:10-6
 RISE:5-3-7
 RIGHT HEIGHT:5-3-7

LOADING (PSF)		MAX STRESSES		MINIMUM GRADE OF LUMBER	
L	D	TOP	BOTT	TOP CHORD	BOT CHORD
30	15	1-2=0.770	6-1=0.303	2X4 No.2ND 19 SP	2X4 No.2ND 19 SP
0	10	LL.DEFL.@5=0.16 < L/360		WEBS :2X4 No.3 19 SP	

STR.INC.: LUMB = 1.33 PLATE = 1.33
 REPETITIVE STRESSES USED
 SPACING : 24.0 in. o. c.
 NO. OF MEMBERS = 1

RIGHT END VERTICAL IS NOT EXPOSED.
 TRUSS HAS BEEN CHECKED FOR 145 M.P.H. WIND LOAD, WALL HEIGHT 16 FT, ENCLOSED BLDG. TYPE, ASSUMING 10 PSF TOTAL D.L.(7+3) (SBC-97).
 DEFLECTION(IN.) L.L= 0.16,D.L=0.13,T.L=0.30
 PLATES ARE MITEK M20-249,200 MANUFACTURED FROM ASTM A 446 GRD A GALVANIZED STEEL(EXCEPT AS SHOWN)
 PLATE MUST BE INSTALLED ON EA. FACE OF JOINT,SYMMETRICALLY(EXCEPT AS SHOWN)DESIGN CONFORMS W/ND S DESIGN SPECS, SSBC,ND S,ANSI/TP1-95
 THIS DESIGN IS FOR TRUSS FABRICATION ONLY.FOR PERMANENT AND TEMPORARY BRACING(WHICH IS ALWAYS REQD)CONSULT BLDG ARCHITECT OR ENGINEER.

MAR 01 2001

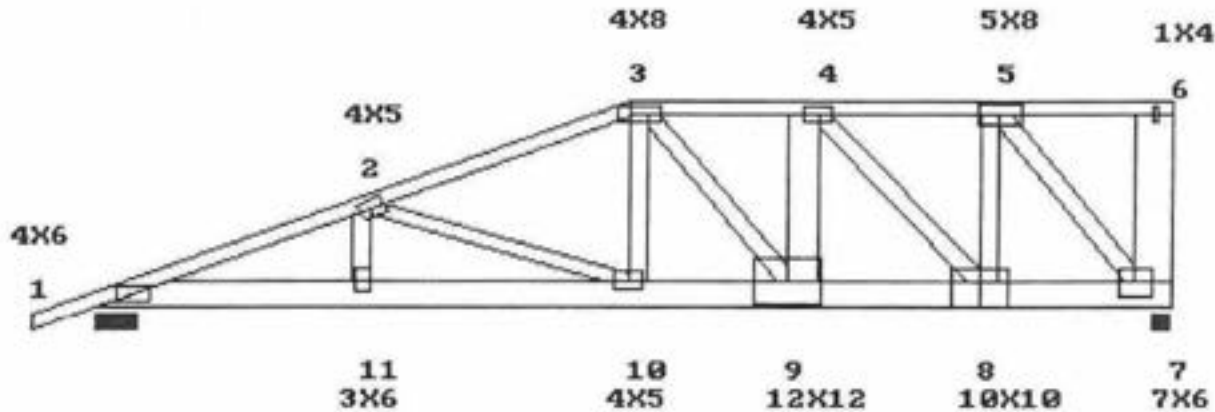
====<<<<ACES-32 Ver. 1.1>>>>====[001884]====<<<<FL.QUALITY-MMH>>>>====
 Customer : FOGLIA Thu Mar 1 10:07:34 2001
 Project #: 00469 Truss ID : S9 Family #: 313
 Span : 16-7-8 B.Span : 16-8 Quant. : 2 Top Pitch : 6/12
 ==8ld(1/21/2000),v1.1=====

FORCES - LOAD CASE #1				REACTIONS - SIZE	
1-2=5636	7-8= 3676	2-11= 79	5-8= 5399	1=2782	8.00
2-3=5171	8-9= 5859	2-10=439	5-7=6635	7=6717	4.07
3-4=5859	9-10= 4617	3-10= 242			
4-5=3676	10-11= 5014	3-9= 2220			
5-6= 0	11-1= 5014	4-9= 2918			
6-7=60		4-8=3745			

(77)

PROVIDE FOR 3358 LBS UPLIFT AT JOINT 1 (1.21)
 PROVIDE FOR 6891 LBS UPLIFT AT JOINT 7 (1.03)
 PROVIDE FOR 471 LBS HORIZ. REACTION AT JOINT 1
 PLATE OFFSETS (X=LEFT,Y=TOP):[j8=5,3],

1-0	4-1-5	8-2-9	10-11	13-9-8	16-7-8
	4-1-5	4-1-5	2-8-7	2-10-8	2-10



4-1-5	8-2-9	10-11	13-9-8	16-7-8
4-1-5	4-1-5	2-8-7	2-10-8	2-10

L. HL TO PK:9-2-3
 LEFT HEIGHT:0-4-3 SPAN:16-7-8 RISE:4-5-8 RIGHT HEIGHT:4-5-7

LOADING (PSF)		MAX STRESSES		MINIMUM GRADE OF LUMBER	
TOP	30 15	TOP	1-2=0.728	TOP CHORD:2X4	No.2ND 19 SP
BOTT	0 10	BOTT	8-9=0.279	BOT CHORD:2X8	SS 19 SP
		LL.DEFL.@9=0.06	< L/360	WEBS	:2X4 No.3 19 SP

REPETITIVE STRESSES NOT USED

SPACING : 24.0 in. o. c.
 NO. OF MEMBERS = 2

LOADING	STRESS INCREASE	LOADING	PANEL(PLF) / JOINTS(LBS)
LUMBER	1.33	PLATE	1.33
TYPE	UNIFORM	TYPE	CONCENTRATED
1	1.33	1	3- 90
		3	6= 45
		7	9= 850
		9	10= 10
		10	1= 20
		9	3523

WEB: 6-7 TO BE 2X8 SS 19 SP
 WEB: 4-9 TO BE 2X6 No.2D 19 SP
 WEB: 5-7 TO BE 2X4 No.2ND 19 SP
 RIGHT END VERTICAL IS NOT EXPOSED.
 2 MEMBERS NAILED TOG. W/1 ROW(S) OF .131x3 in. NAILS 12 in. o.c.(TOP CHS.),AND 1 ROW(S) OF .131x3 in. NAILS 3 in. o.c.(BOTT. CHS.)
 For Webs use 1 ROW of NAILS 12 in. o.c.
 TRUSS HAS BEEN CHECKED FOR 145 M.P.H. WIND LOAD, WALL HEIGHT 16 FT, ENCLOSED BLDG. TYPE, ASSUMING 10 PSF TOTAL D.L.(7+3) (SBC-97).
 DEFLECTION(IN.) L.L= 0.06,D.L=0.05,T.L=0.12
 PLATES ARE MITEK M20-249,200 MANUFACTURED FROM ASTM A 446 GRD A GALVANIZED STEEL(EXCEPT AS SHOWN)
 PLATE MUST BE INSTALLED ON EA. FACE OF JOINT,SYMMETRICALLY(EXCEPT AS SHOWN)DESIGN CONFORMS W/NDS DESIGN SPECS, 55SBC,ANSI/TPI-95
 THIS DESIGN IS FOR TRUSS FABRICATION ONLY.FOR PERMANENT AND TEMPORARY BRACING(WHICH IS ALWAYS REQD)CONSULT BLDG ARCHITECT OR ENGINEER.

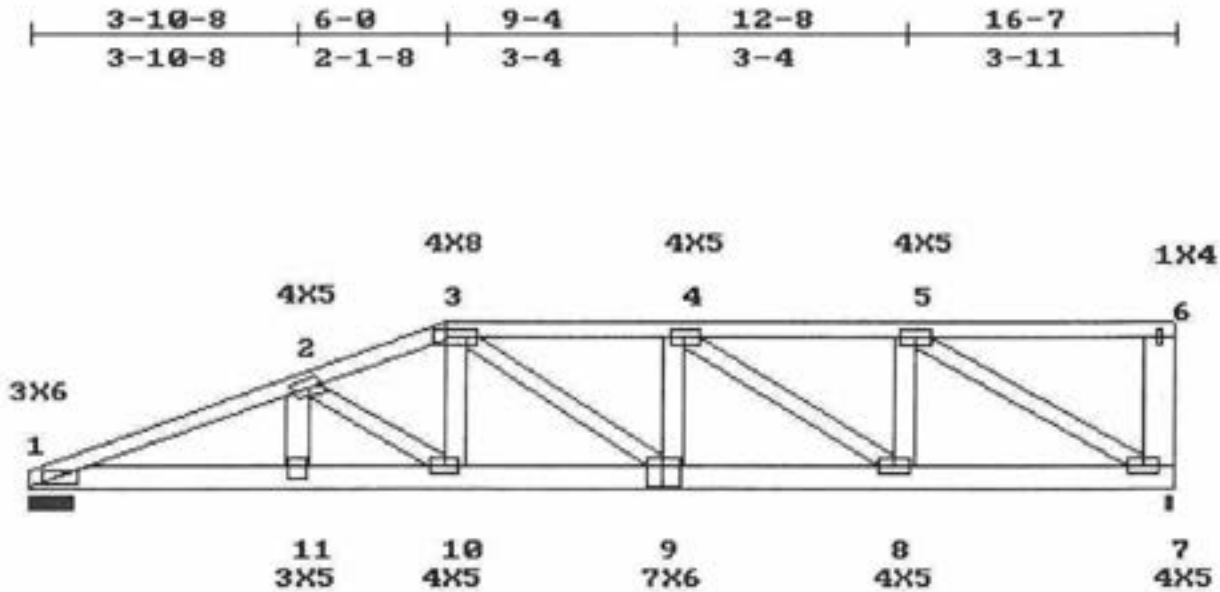
MAR 01 2001

====<<<<ACES-32 Ver. 1.1>>>>=====[001885]====<<<<FL.QUALITY-MMH>>>>=====
 Customer : FOGLIA Thu Mar 1 10:08:10 2001
 Project #: 00469 Truss ID : GT16 Family #: 313
 Span : 16-7 B.Span : 16-8 Quant. : 2 Top Pitch : 6/12
 ==Bld(1/21/2000),v1.1=====

FORCES - LOAD CASE #1				REACTIONS - SIZE	
1-2= 3581	7-8= 1964	2-11= 1082	5-8= 1191	1= 1882	8.00
2-3= 2537	8-9= 2318	2-10= 1201	5-7= 2480	7= 1726	1.60
3-4= 2318	9-10= 2262	3-10= 853			
4-5= 1964	10-11= 3158	3-9= 76			
5-6= 0	11-1= 3158	4-9= 16			
6-7= 173		4-8= 475			

(78)

PROVIDE FOR 3253 LBS UPLIFT AT JOINT 1 (1.73)
 PROVIDE FOR 3177 LBS UPLIFT AT JOINT 7 (1.84)
 PROVIDE FOR 349 LBS HORIZ. REACTION AT JOINT 1
 PLATE OFFSETS (X=LEFT,Y=TOP):(j9=3,2),(j11=3,1.5),



L. HL TO PK:6-8-8
 LEFT HEIGHT:0-4-3 SPAN:16-7 RISE:3-4-3 RIGHT HEIGHT:3-4-3
 =====
 LOADING (PSF) MAX STRESSES MINIMUM GRADE OF LUMBER
 TOP 30 15 TOP 1-2=0.560 TOP CHORD:2X4 No.2ND 19 SP
 BOT 0 10 BOT 11-1=0.196 BOT CHORD:2X6 No.2D 19 SP
 LL.DEFL.@10=0.05 < L/360 WEBS :2X4 No.3 19 SP
 =====
 REPETITIVE STRESSES NOT USED SPACING : 24.0 in. o. c.
 NO. OF MEMBERS = 2

LOADING	STRESS INCREASE	LOADING	PANEL(PLF) / JOINTS(LBS)
LUMBER	PLATE	TYPE	
1	1.33 1.33	UNIFORM	1- 6= 90 7- 1= 20
		CONCENTRATED	8= 803 11= 1025

[Signature]
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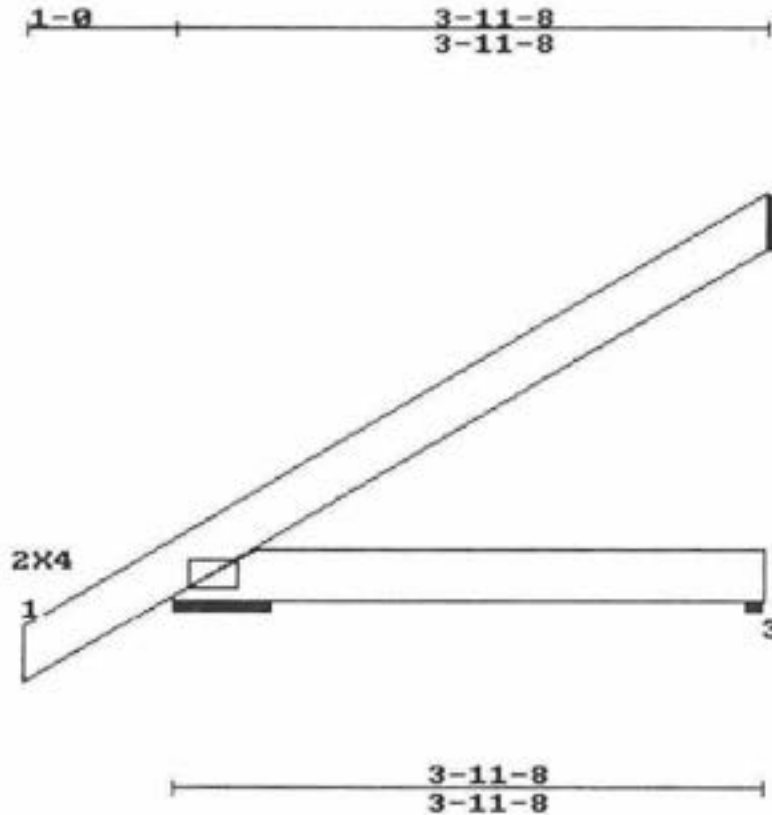
WEB: 6-7 TO BE 2X6 No.2D 19 SP
 RIGHT END VERTICAL IS NOT EXPOSED.
 2 MEMBERS NAILED TOG. W/1 ROW(S) OF .131x3 in. NAILS 12 in. o.c.(TOP CHS.),AND 1 ROW(S) OF .131x3 in. NAILS 10 in. o.c(BOTT. CHS.)
 For Webs use 1 ROW of NAILS 12 in. o.c.
 TRUSS HAS BEEN CHECKED FOR 145 M.P.H. WIND LOAD, WALL HEIGHT 16 FT, ENCLOSED BLDG. TYPE, ASSUMING 10 PSF TOTAL D.L.(7+3) (SBC-97).
 DEFLECTION(IN.) L.L= 0.05,D.L=0.05,T.L=0.10
 PLATES ARE MITEK M20-249,200 MANUFACTURED FROM ASTM A 446 GRD A GALVANIZED STEEL(EXCEPT AS SHOWN)
 PLATE MUST BE INSTALLED ON EA. FACE OF JOINT,SYMMETRICALLY(EXCEPT AS SHOWN)DESIGN CONFORMS W/NDS DESIGN SPECS, SSB,ANSI/TPI-95
 THIS DESIGN IS FOR TRUSS FABRICATION ONLY.FOR PERMANENT AND TEMPORARY BRACING(WHICH IS ALWAYS REQD)CONSULT BLDG ARCHITECT OR ENGINEER.

====<<<ACES-32 Ver. 1.1>>>====[002005]====<<<FL.QUALITY-MMH>>>====
 Customer : FOGLIA Wed Mar 7 07:14:00 2001
 Project #: 00469 Truss ID : C3B Family # : 205
 Span : 3-11-8 B.Span : 3-10-8 Quant. : 1 Top Pitch : 6/12
 ==Bld(1/21/2000),v1.1=====

FORCES - GRAVITY LOADS
 1-2=0 3-1=0
 2-3=0

REACTIONS - SIZE
 1=306 8.00
 3=36 1.50
 2=160 1.50

PROVIDE FOR 237 LBS UPLIFT AT JOINT 1 (0.78)
 PROVIDE FOR 247 LBS UPLIFT AT JOINT 2 (1.54)
 PROVIDE FOR 228 LBS HORIZ. REACTION AT JOINT 1



L. HL TO PK:4-5-2
 LEFT HEIGHT:0-4-3 SPAN:3-11-8 RISE:2-3-15 RIGHT HEIGHT:2-3-15

LOADING (PSF)		MAX STRESSES		MINIMUM GRADE OF LUMBER	
TOP	30	TOP	1-2=0.419	TOP CHORD:2X4	No.2ND 19 SP
BOTT	0	BOTT	3-1=0.062	BOT CHORD:2X4	No.2ND 19 SP
		LL.DEFL.	< L/360	WEBS	:2X4 No.3 19 SP

STR.INC.: LUMB = 1.33 PLATE = 1.33
 REPETITIVE STRESSES USED SPACING : 24.0 in. o. c.
 NO. OF MEMBERS = 1

TRUSS HAS BEEN CHECKED FOR 145 M.P.H. WIND LOAD, WALL HEIGHT 16 FT, ENCLOSED BLDG. TYPE, ASSUMING 10 PSF TOTAL D.L.(7+3) (SBC-97).
 DEFLECTION(IN.) L.L= 0.00,D.L=0.00,T.L=0.00
 PLATES ARE MITEK M20-249,200 MANUFACTURED FROM ASTM A 446 GRD A GALVANIZED STEEL(EXCEPT AS SHOWN)
 PLATE MUST BE INSTALLED ON EA. FACE OF JOINT,SYMMETRICALLY(EXCEPT AS SHOWN)DESIGN CONFORMS W/NDS DESIGN SPECS, SSBC,ANSI/TPI-95
 THIS DESIGN IS FOR TRUSS FABRICATION ONLY.FOR PERMANENT AND TEMPORARY BRACING(WHICH IS ALWAYS REQD)CONSULT BLDG ARCHITECT OR ENGINEER.

MAR 06 2001

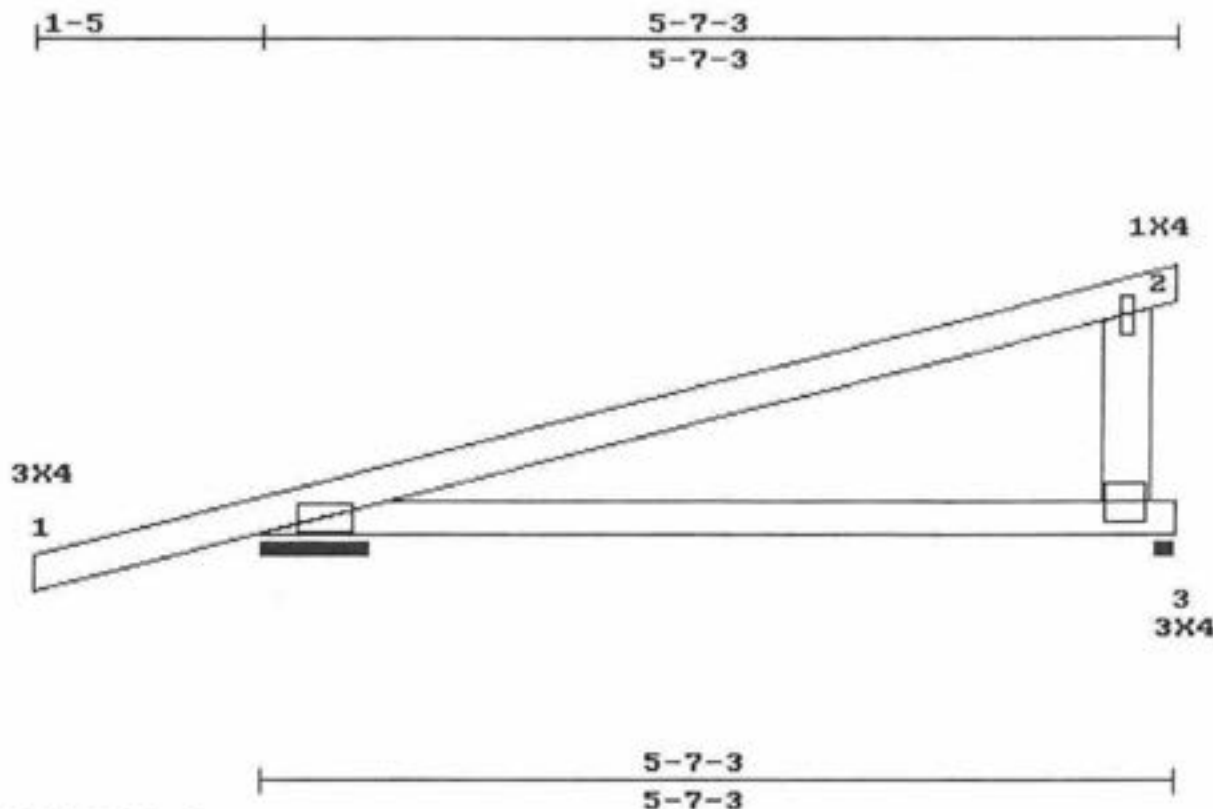
====<<<ACES-32 Ver. 1.1>>>=====[002004]====<<<FL.QUALITY-MMH>>>=====
 Customer : FOGLIA Wed Mar 7 07:13:43 2001
 Project #: 00469 Truss ID : CJ5C Family #: 204
 Span : 5-7-3 B.Span : 3-10-8 Quant. : 1 Top Pitch : 4.243
 ==Bld(1/21/2000),v1.1=====

FORCES - LOAD CASE #1
 1-2=0 3-1=0
 2-3=-172

REACTIONS - SIZE
 1=366 8.00
 3=210 1.50

PROVIDE FOR 568 LBS UPLIFT AT JOINT 1 (1.55)
 PROVIDE FOR 477 LBS UPLIFT AT JOINT 3 (2.27)
 PROVIDE FOR 230 LBS HORIZ. REACTION AT JOINT 1

80



L. HL TO PK:5-11-4
 LEFT HEIGHT:0-3-15

SPAN:5-7-3

RISE:2-3-12

RIGHT HEIGHT:2-3-12

LOADING (PSF)

	L	D
TOP	30	15
BOTT	0	10

MAX STRESSES

TOP	1-2=0.595
BOTT	3-1=0.111
LL.DEFL.	< L/360

MINIMUM GRADE OF LUMBER

TOP CHORD:2X4	No.2ND 19 SP
BOT CHORD:2X4	No.2ND 19 SP
WEBS	:2X4 No.3 19 SP

REPETITIVE STRESSES NOT USED

SPACING : 24.0 in. o. c.
 NO. OF MEMBERS = 1

LOADING	STRESS INCREASE	LOADING	PANEL(PLF) / JOINTS(LBS)
LUMBER	PLATE	TYPE	
1	1.33	1.33	UNIFORM 1- 2= 66 3- 1= 15

RIGHT END VERTICAL IS NOT EXPOSED.

TRUSS HAS BEEN CHECKED FOR 145 M.P.H. WIND LOAD, WALL HEIGHT 16 FT, ENCLOSED BLDG. TYPE, ASSUMING 10 PSF TOTAL D.L.(7+3) (SBC-97).
 DEFLECTION(IN.) L.L= 0.00,D.L=0.00,T.L=0.00

PLATES ARE MITEK M20-249,200 MANUFACTURED FROM ASTM A 446 GRD A GALVANIZED STEEL(EXCEPT AS SHOWN)

PLATE MUST BE INSTALLED ON EA. FACE OF JOINT,SYMMETRICALLY(EXCEPT AS SHOWN)DESIGN CONFORMS W/NDS DESIGN SPECS, S58C,ANSI/TPI-95

THIS DESIGN IS FOR TRUSS FABRICATION ONLY.FOR PERMANENT AND TEMPORARY BRACING(WHICH IS ALWAYS REQD)CONSULT BLDG ARCHITECT OR ENGINEER.

MAR 06 2001

====<<<<ACES-32 Ver. 1.1>>>>===== [001888] =====<<<<FL.QUALITY-MMH>>>>=====

Customer : FOGLIA

Project # : 00469

Span : 3-0

Truss ID : C3E

Quantity : 1

Thu Mar 1 10:08:56 2001

Family # : 205

Top Pitch : 6/12

==Bld(1/21/2000),v1.1=====

FORCES - GRAVITY LOADS

1-2= 0 3-1= 0
2-3= 0

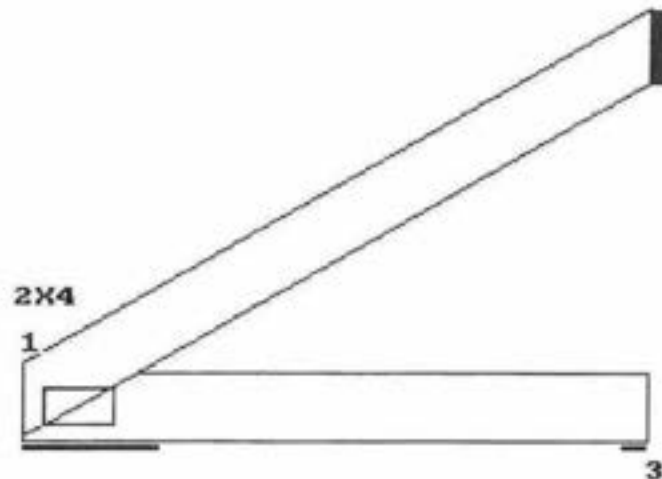
REACTIONS - SIZE

1=-143 8.00
3=-26 1.50
2=-117 1.50

(81)

PROVIDE FOR 79 LBS UPLIFT AT JOINT 1 (0.55)
PROVIDE FOR 183 LBS UPLIFT AT JOINT 2 (1.56)
PROVIDE FOR 172 LBS HORIZ. REACTION AT JOINT 1
(SYMMETRIC PLATING TURNED ON)

3-0
3-0



3-0
3-0

L. HL TO PK:3-4-4
LEFT HEIGHT:0-4-3

SPAN:3-0

RISE:1-10-3

RIGHT HEIGHT:1-10-3

LOADING (PSF)

TOP 30 15
BOTT 0 10

MAX STRESSES

TOP 1-2=0.225
BOTT 3-1=0.034
LL.DEFL. < L/360

MINIMUM GRADE OF LUMBER

TOP CHORD:2X4 No.2ND 19 SP
BOT CHORD:2X4 No.2ND 19 SP
WEBS :2X4 No.3 19 SP

STR.INC.: LUMB = 1.33 PLATE = 1.33
REPETITIVE STRESSES USED

SPACING : 24.0 in. o. c.
NO. OF MEMBERS = 1

TRUSS HAS BEEN CHECKED FOR 145 M.P.H. WIND LOAD, WALL HEIGHT 16 FT, ENCLOSED BLDG. TYPE, ASSUMING 10 PSF TOTAL D.L.(7*3) (SBC-97).

DEFLECTION(IN.) L.L= 0.00,D.L=0.00,T.L=0.00

PLATES ARE MITEK M20-249,200 MANUFACTURED FROM ASTM A 446 GRD A GALVANIZED STEEL(EXCEPT AS SHOWN)

PLATE MUST BE INSTALLED ON EA. FACE OF JOINT,SYMMETRICALLY(EXCEPT AS SHOWN)DESIGN CONFORMS W/NDS DESIGN SPECS, SS8C,ANSI/TPI-95

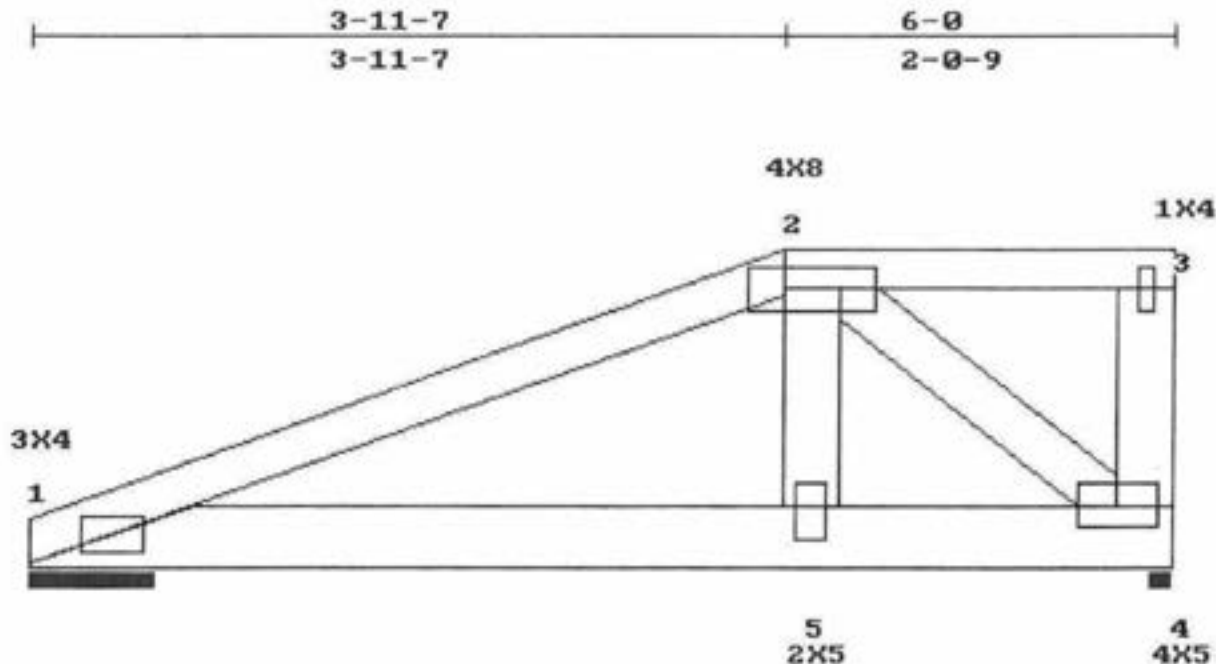
THIS DESIGN IS FOR TRUSS FABRICATION ONLY.FOR PERMANENT AND TEMPORARY BRACING(WHICH IS ALWAYS REQD)CONSULT BLDG ARCHITECT OR ENGINEER.

MAR 01 2001

===<<<ACES-32 Ver. 1.1>>>===== [002003] =====<<<FL.QUALITY-MMH>>>=====
 Customer : FOGLIA Wed Mar 7 07:13:12 2001
 Project #: 00469 Truss ID : GT6 Family # : 313
 Span : 6-0 B.Span : 6-0 Quant. : 1 Top Pitch : 6/12
 ==Bld(1/21/2000),v1.1=====

FORCES - LOAD CASE #1			REACTIONS - SIZE	
1-2=810	4-5= 713	2-5= 790	1=901	8.00
2-3= 0	5-1= 713	2-4=999	4=1026	1.50
3-4=-131				

PROVIDE FOR 1045 LBS UPLIFT AT JOINT 1 (1.16)
 PROVIDE FOR 1236 LBS UPLIFT AT JOINT 4 (1.21)
 PROVIDE FOR 229 LBS HORIZ. REACTION AT JOINT 1
 PLATE OFFSETS (X=LEFT,Y=TOP):[5=3,1],



L. HL TO PK:4-5-1
 LEFT HEIGHT:0-4-3 SPAN:6-0 RISE:2-3-14 RIGHT HEIGHT:2-3-14

LOADING (PSF)		MAX STRESSES		MINIMUM GRADE OF LUMBER	
TOP	30 15	TOP	1-2=0.364	TOP CHORD:2X4	No.2ND 19 SP
BOTT	0 10	BOTT	4-5=0.298	BOT CHORD:2X6	No.2D 19 SP
		LL.DEFL.@5=0.01 < L/360		WEBS	:2X4 No.3 19 SP

REPETITIVE STRESSES NOT USED

SPACING : 24.0 in. o. c.
 NO. OF MEMBERS = 1

LOADING	STRESS INCREASE	LOADING	PANEL(PLF) / JOINTS(LBS)
LUMBER	1.33	PLATE	1- 2= 90 2- 3= 132 4- 1= 196
		TYPE	5= 241
		UNIFORM	
		CONCENTRATED	

RIGHT END VERTICAL IS NOT EXPOSED.
 TRUSS HAS BEEN CHECKED FOR 145 M.P.H. WIND LOAD, WALL HEIGHT 16 FT, ENCLOSED BLDG. TYPE, ASSUMING 10 PSF TOTAL D.L.(7*3) (SBC-97).
 DEFLECTION(IN.) L.L= 0.01,D.L=0.01,T.L=0.01
 PLATES ARE MITEK M20-249,200 MANUFACTURED FROM ASTM A 446 GRD A GALVANIZED STEEL(EXCEPT AS SHOWN)
 PLATE MUST BE INSTALLED ON EA. FACE OF JOINT,SYMMETRICALLY(EXCEPT AS SHOWN)DESIGN CONFORMS W/NDS DESIGN SPECS, SSB,ANSI/TPI-95
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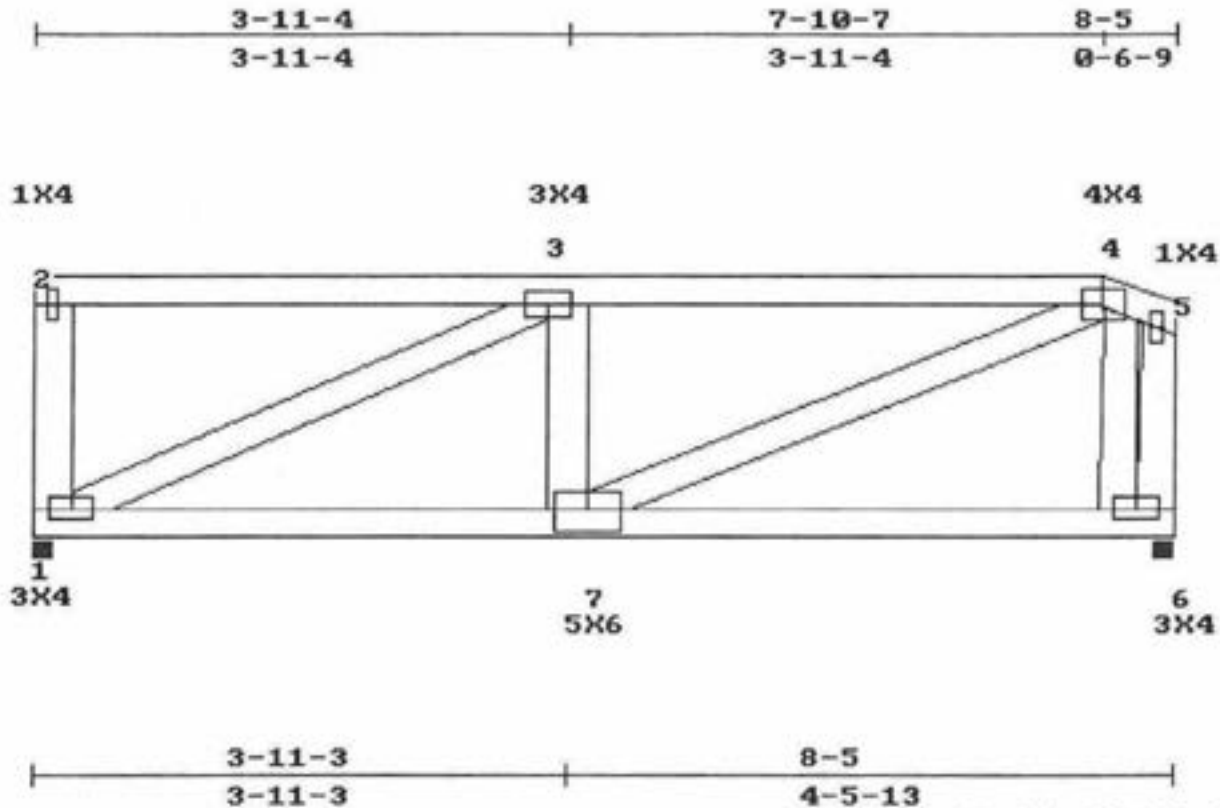
MAR 06 2001

====<<<ACES-32 Ver. 1.1>>>=====[001890]====<<<FL.QUALITY-MMH>>>=====
 Customer : FOGLIA Thu Mar 1 10:09:29 2001
 Project #: 00469 Truss ID : S13 Family # : special
 Span : 8-5 Quantity : 1 Top Pitch : 6/12
 ==Bld(1/21/2000),v1.1=====

FORCES - GRAVITY LOADS			REACTIONS - SIZE	
1-2=173	6-7= 75	3-1=-458	1=-454	2.00
2-3= 0	7-1= 389	3-7=-108	6=-454	2.00
3-4=-389		4-7= 368		
4-5= 0		4-6=-396		
5-6=-21				

(83)

PROVIDE FOR 410 LBS UPLIFT AT JOINT 1 (0.90)
 PROVIDE FOR 396 LBS UPLIFT AT JOINT 6 (0.87)
 PROVIDE FOR 25 LBS HORIZ. REACTION AT JOINT 1
 PLATE OFFSETS (X=LEFT,Y=TOP):[J7=3,2],(SYMMETRIC PLATING TURNED ON)



LEFT HEIGHT:2-8-4		SPAN:8-5	RISE:2-8-4	R. HL TO PK :0-7-5
				RIGHT HEIGHT:2-4-15
=====				
LOADING (PSF)	MAX STRESSES		MINIMUM GRADE OF LUMBER	
TOP 30 15	TOP 3-4=0.446	TOP CHORD:2X4	No.2ND 19 SP	
BOTT 0 10	BOTT 7-1=0.143	BOT CHORD:2X4	No.2ND 19 SP	
	LL.DEFL.@7=0.01 < L/360	WEBS :2X4	No.3 19 SP	
=====				
STR.INC.: LUMB = 1.33 PLATE = 1.33			SPACING : 24.0 in. o. c.	
REPETITIVE STRESSES USED			NO. OF MEMBERS = 1	

BOTH END VERTICALS ARE NOT EXPOSED.
 TRUSS HAS BEEN CHECKED FOR 145 M.P.H. WIND LOAD, WALL HEIGHT 16 FT, ENCLOSED BLDG. TYPE, ASSUMING 10 PSF TOTAL D.L.(7*3) (SBC-97).
 DEFLECTION(IN.) L.L= 0.01,D.L=0.00,T.L=0.01
 PLATES ARE MITEK M20-249,200 MANUFACTURED FROM ASTM A 446 GRD A GALVANIZED STEEL(EXCEPT AS SHOWN)
 PLATE MUST BE INSTALLED ON EA. FACE OF JOINT,SYMMETRICALLY(EXCEPT AS SHOWN)DESIGN CONFORMS W/ND S DESIGN SPECS, SSBC,ANSI/TPI-95
 THIS DESIGN IS FOR TRUSS FABRICATION ONLY.FOR PERMANENT AND TEMPORARY BRACING(WHICH IS ALWAYS REQD)CONSULT BLDG ARCHITECT OR ENGINEER.

[Signature]
 MAR 01 2001

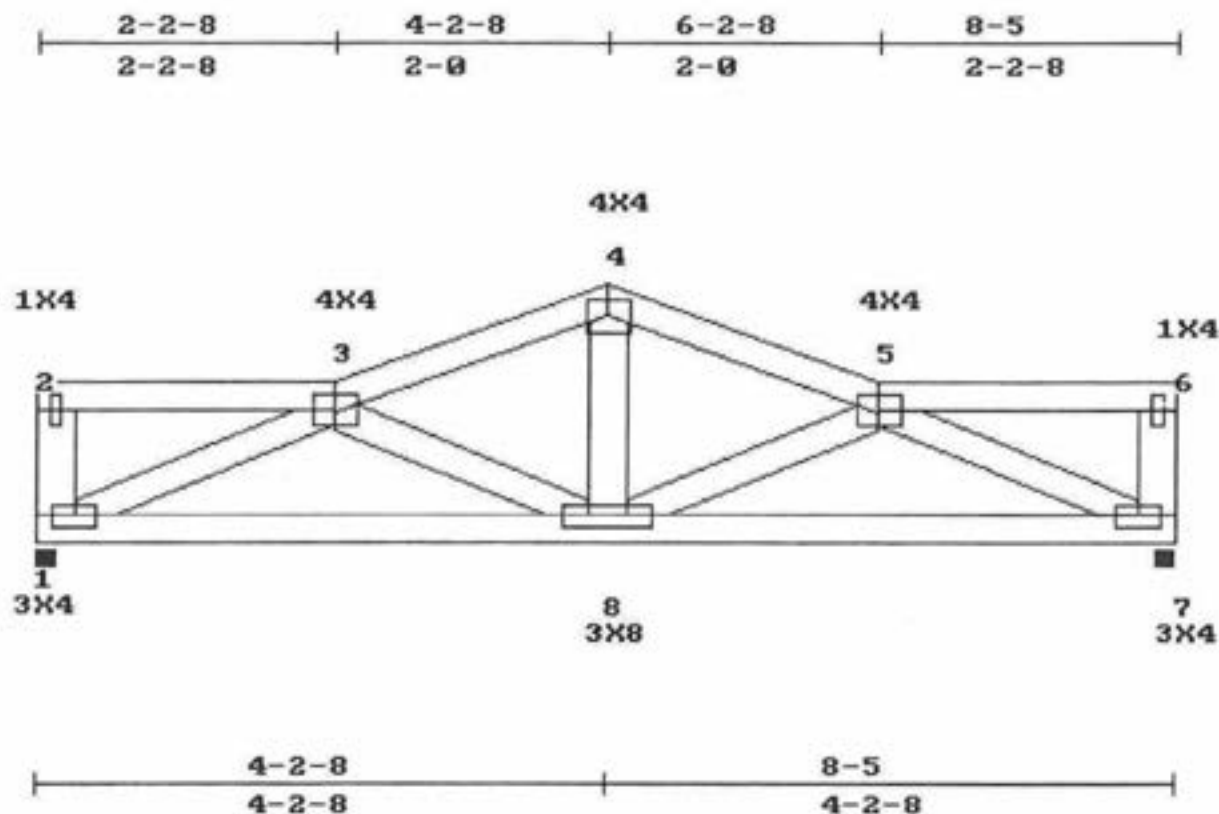
====<<<<ACES-32 Ver. 1.1>>>>=====[001891]====<<<<FL.QUALITY-MMH>>>>=====
 Customer : FOGLIA Thu Mar 1 10:09:37 2001
 Project #: 00469 Truss ID : S12 Family #: special
 Span : 8-5 Quantity : 1 Top Pitch : 6/12

==Bld(1/21/2000),v1.1=====

FORCES - GRAVITY LOADS			REACTIONS - SIZE	
1-2=96	7-8= 484	3-1=579	1=454	2.00
2-3= 0	8-1= 484	3-8=110	7=454	2.00
3-4=439		4-8= 208		
4-5=439		5-8=110		
5-6= 0		5-7=579		
6-7=96				

(84)

PROVIDE FOR 370 LBS UPLIFT AT JOINT 1 (0.81)
 PROVIDE FOR 370 LBS UPLIFT AT JOINT 7 (0.81)
 PROVIDE FOR 33 LBS HORIZ. REACTION AT JOINT 1
 (SYMMETRIC PLATING TURNED ON)



LEFT HEIGHT:1-8-3 SPAN:8-5 RISE:2-8-3 RIGHT HEIGHT:1-8-3

LOADING (PSF)		MAX STRESSES		MINIMUM GRADE OF LUMBER	
TOP	L 30 D 15	TOP	3-4=0.193	TOP CHORD:2X4	No.2ND 19 SP
BOTT	0 10	BOTT	7-8=0.161	BOT CHORD:2X4	No.2ND 19 SP
		LL.DEFL.@8=0.01 < L/360		WEBS	:2X4 No.3 19 SP

STR.INC.: LUMB = 1.33 PLATE = 1.33 SPACING : 24.0 in. o. c.
 REPETITIVE STRESSES USED NO. OF MEMBERS = 1

BOTH END VERTICALS ARE NOT EXPOSED.
 TRUSS HAS BEEN CHECKED FOR 145 M.P.H. WIND LOAD, WALL HEIGHT 16 FT, ENCLOSED BLDG. TYPE, ASSUMING 10 PSF TOTAL D.L.(7*3) (SBC-97).
 DEFLECTION(IN.) L.L= 0.01,D.L=0.01,T.L=0.01
 PLATES ARE MITEK M20-249,200 MANUFACTURED FROM ASTM A 446 GRD A GALVANIZED STEEL(EXCEPT AS SHOWN)
 PLATE MUST BE INSTALLED ON EA. FACE OF JOINT,SYMMETRICALLY(EXCEPT AS SHOWN)DESIGN CONFORMS W/NDS DESIGN SPECS, SBC,ANSI/TPI-95
 THIS DESIGN IS FOR TRUSS FABRICATION ONLY,FOR PERMANENT AND TEMPORARY BRACING(WHICH IS ALWAYS REQD)CONSULT BLDG ARCHITECT OR ENGINEER.

MAR 01 2001

====<<<ACES-32 Ver. 1.1>>>===== [001892] =====<<<FL.QUALITY-MMH>>>=====

Customer : FOGLIA Thu Mar 1 10:09:46 2001

Project #: 00469 Truss ID : S11 Family #: 101

Span : 8-5 Quantity : 1 Top Pitch : 6/12

==Bld(1/21/2000),v1.1=====

FORCES - GRAVITY LOADS

1-2=454 3-4= 393 2-4= 83

2-3=454 4-1= 393

REACTIONS - SIZE

1=454 2.00

3=454 2.00

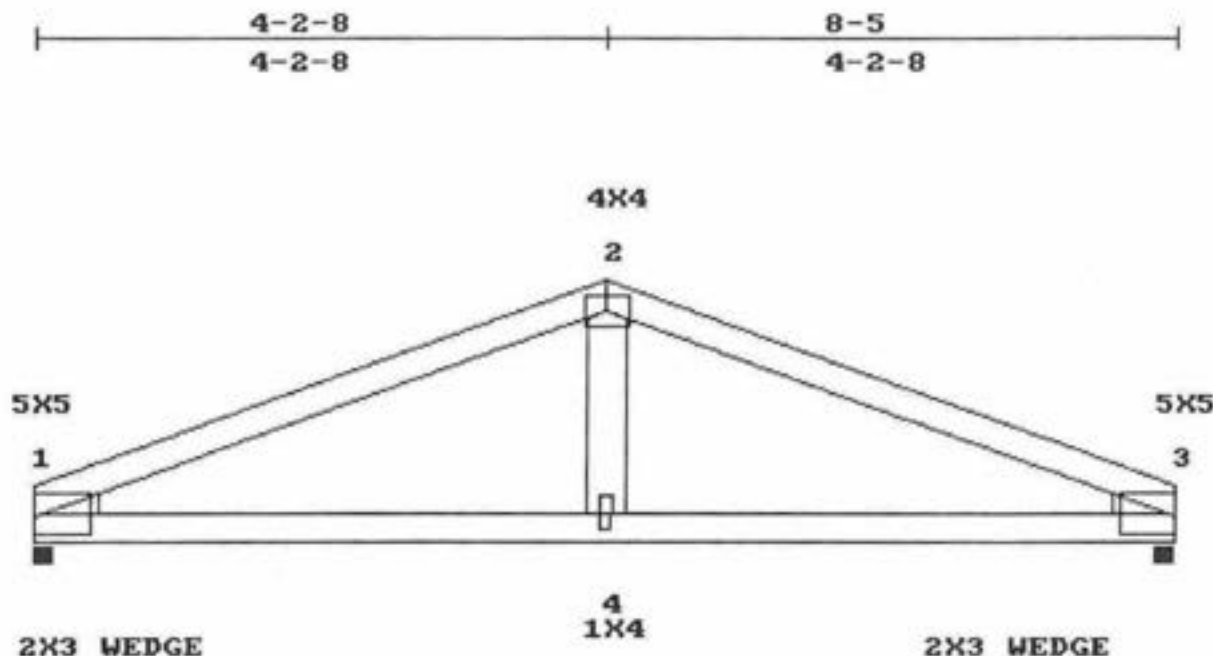
95

PROVIDE FOR 356 LBS UPLIFT AT JOINT 1 (0.79)

PROVIDE FOR 356 LBS UPLIFT AT JOINT 3 (0.79)

PROVIDE FOR 79 LBS HORIZ. REACTION AT JOINT 1

(SYMMETRIC PLATING TURNED ON)



L. HL TO PK:4-8-7

LEFT HEIGHT:0-7

SPAN:8-5

RISE:2-8-4

R. HL TO PK:4-8-7

RIGHT HEIGHT:0-7

LOADING (PSF)

TOP 30 15

BOTT 0 10

MAX STRESSES

TOP 2-3=0.598

BOTT 4-1=0.144

LL.DEFL.@4=0.01 < L/360

MINIMUM GRADE OF LUMBER

TOP CHORD:2X4 No.2ND 19 SP

BOT CHORD:2X4 No.2ND 19 SP

WEBS :2X4 No.3 19 SP

STR.INC.: LUMB = 1.33 PLATE = 1.33

REPETITIVE STRESSES USED

SPACING : 24.0 in. o. c.

NO. OF MEMBERS = 1

TRUSS HAS BEEN CHECKED FOR 145 M.P.H. WIND LOAD, WALL HEIGHT 16 FT, ENCLOSED BLDG. TYPE, ASSUMING 10 PSF TOTAL D.L.(7+3) (SBC-97).

DEFLECTION(IN.) L.L= 0.01,D.L=0.01,T.L=0.01

PLATES ARE MITEK M20-249,200 MANUFACTURED FROM ASTM A 446 GRD A GALVANIZED STEEL(EXCEPT AS SHOWN)

PLATE MUST BE INSTALLED ON EA. FACE OF JOINT,SYMMETRICALLY(EXCEPT AS SHOWN)DESIGN CONFORMS W/NDIS DESIGN SPECS, SSBC,ANSI/TPI-95

THIS DESIGN IS FOR TRUSS FABRICATION ONLY.FOR PERMANENT AND TEMPORARY BRACING(WHICH IS ALWAYS REQD)CONSULT BLDG ARCHITECT OR ENGINEER.

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MAR 01 2001

====<<<<ACES-32 Ver. 1.1>>>>=====[001893]=====<<<<FL.QUALITY-MMH>>>>=====

Customer : FOGLIA

Project # : 00469

Span : 9-4

Truss ID : H3

Quantity : 1

Thu Mar 1 10:09:59 2001

Family # : 314

Top Pitch : 6/12

==Bld(1/21/2000),v1.1=====

FORCES - LOAD CASE #1

1-2=1253 4-5= 1110 2-6= 316
2-3=1110 5-6= 1110 2-5= 0
3-4=1253 6-1= 1110 3-5= 316

REACTIONS - SIZE

1=908 2.00
4=908 2.00

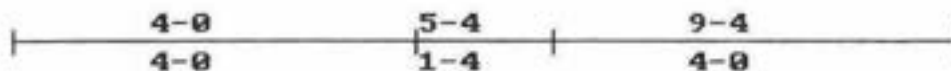
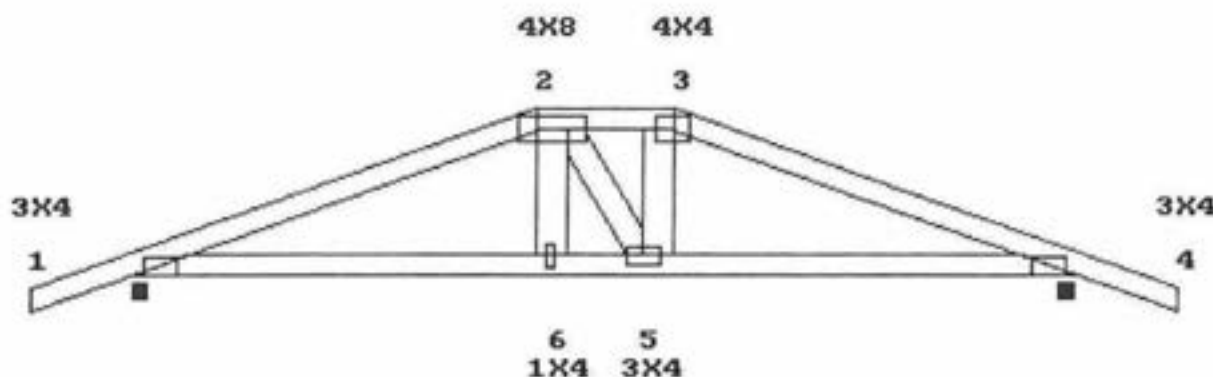
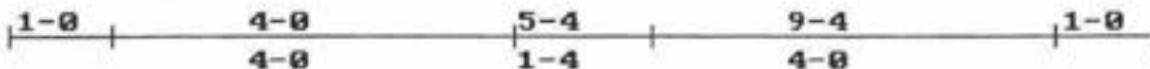
96

PROVIDE FOR 845 LBS UPLIFT AT JOINT 1 (0.93)

PROVIDE FOR 845 LBS UPLIFT AT JOINT 4 (0.93)

PROVIDE FOR 69 LBS HORIZ. REACTION AT JOINT 1

(SYMMETRIC PLATING TURNED ON)



L. HL TO PK:4-5-11

LEFT HEIGHT:0-4-3

SPAN:9-4

RISE:2-4-3

R. HL TO PK :4-5-11

RIGHT HEIGHT:0-4-3

LOADING (PSF)

L D

TOP 30 15

BOTT 0 10

MAX STRESSES

TOP 3-4=0.644

BOTT 5-6=0.294

LL.DEFL.@6=0.02 < L/360

MINIMUM GRADE OF LUMBER

TOP CHORD:2X4 No.2ND 19 SP

BOT CHORD:2X4 No.2ND 19 SP

WEBS :2X4 No.3 19 SP

REPETITIVE STRESSES NOT USED

SPACING : 24.0 in. o. c.

NO. OF MEMBERS = 1

LOADING STRESS INCREASE LOADING PANEL(PLF) / JOINTS(LBS)

LUMBER PLATE TYPE

1 1.33 1.33 UNIFORM

CONCENTRATED

1- 2= 90 2- 3= 135 3- 4= 90 4- 5= 20 5- 6= 30 6- 1= 20

5= 257 6= 257

TRUSS HAS BEEN CHECKED FOR 145 M.P.H. WIND LOAD, WALL HEIGHT 16 FT, ENCLOSED BLDG. TYPE, ASSUMING 10 PSF TOTAL D.L.(7+3) (SBC-97).

DEFLECTION(IN.) L.L= 0.02,D.L=0.02,T.L=0.04

PLATES ARE MITEK M20-249,200 MANUFACTURED FROM ASTM A 446 GRD A GALVANIZED STEEL(EXCEPT AS SHOWN)

PLATE MUST BE INSTALLED ON EA. FACE OF JOINT,SYMMETRICALLY(EXCEPT AS SHOWN)DESIGN CONFORMS W/NDG DESIGN SPECS, SSBG,ANSI/TPI-95

THIS DESIGN IS FOR TRUSS FABRICATION ONLY.FOR PERMANENT AND TEMPORARY BRACING(WHICH IS ALWAYS REQD)CONSULT BLDG ARCHITECT OR ENGINEER.

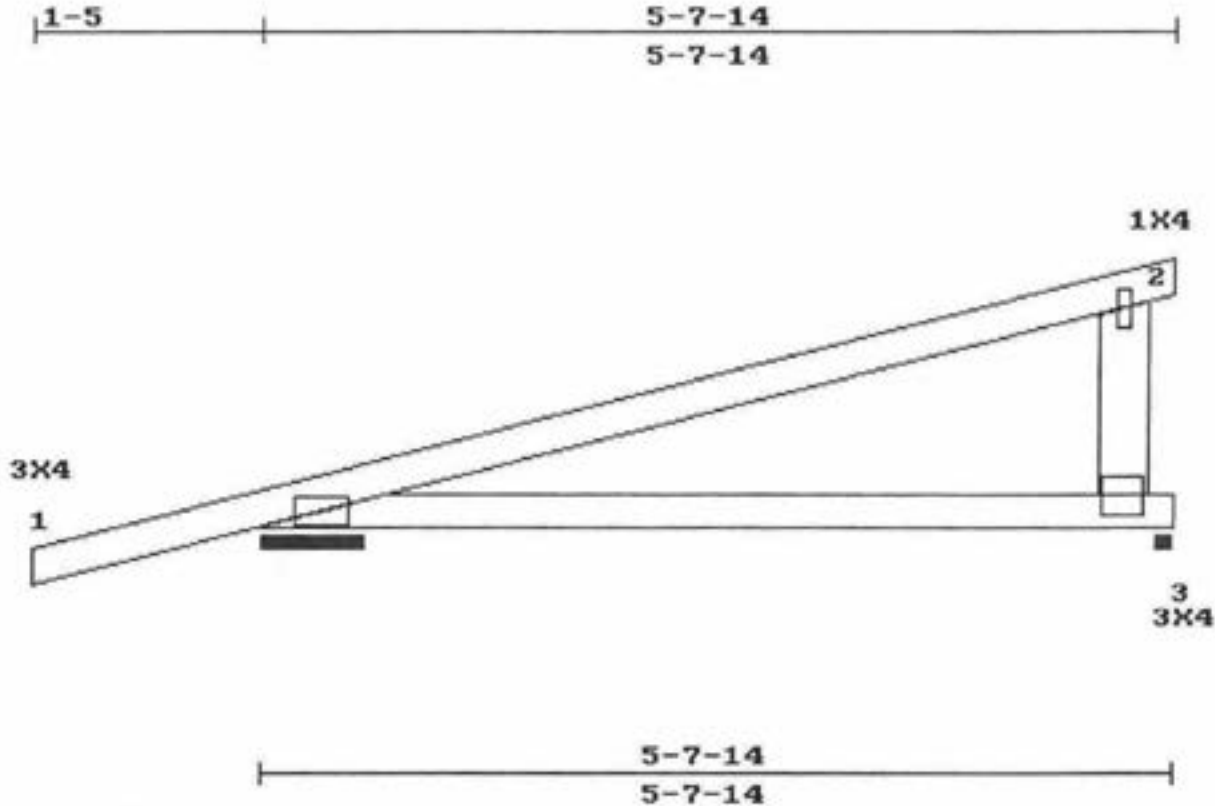
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MAR 0 1 2001

====<<<<ACES-32 Ver. 1.1>>>>=====[001894]====<<<<FL.QUALITY-MMH>>>>====
 Customer : FOGLIA Thu Mar 1 10:10:09 2001
 Project #: 00469 Truss ID : CJ5 Family #: 204
 Span : 5-7-14 Quantity : 2 Top Pitch : 4.243/12
 ==Bld(1/21/2000),v1.1=====

FORCES - LOAD CASE #1		REACTIONS - SIZE	
1-2=0	3-1=0	1=370	8.00
2-3=-175		3=-214	1.50

(91)

PROVIDE FOR 575 LBS UPLIFT AT JOINT 1 (1.55)
 PROVIDE FOR 484 LBS UPLIFT AT JOINT 3 (2.26)
 PROVIDE FOR 232 LBS HORIZ. REACTION AT JOINT 1
 (SYMMETRIC PLATING TURNED ON)



L. HL TO PK:6-0		SPAN:5-7-14		RISE:2-3-15		RIGHT HEIGHT:2-3-15	
LEFT HEIGHT:0-3-15							

LOADING (PSF)		MAX STRESSES		MINIMUM GRADE OF LUMBER	
L	D	TOP	1-2=0.603	TOP CHORD:2X4	No.2ND 19 SP
TOP	30 15	BOTT	3-1=0.115	BOT CHORD:2X4	No.2ND 19 SP
BOTT	0 10	LL.DEFL.	< L/360	WEBS	:2X4 No.3 19 SP

REPETITIVE STRESSES NOT USED

SPACING : 24.0 in. o. c.
 NO. OF MEMBERS = 1

LOADING STRESS INCREASE	LOADING	PANEL(PLF) / JOINTS(LBS)
LUMBER PLATE	TYPE	
1 1.33 1.33	UNIFORM	1- 2= 67 3- 1= 15

RIGHT END VERTICAL IS NOT EXPOSED.
 TRUSS HAS BEEN CHECKED FOR 145 M.P.H. WIND LOAD, WALL HEIGHT 16 FT, ENCLOSED BLDG. TYPE, ASSUMING 10 PSF TOTAL D.L.(7+3) (SBC-97).
 DEFLECTION(IN.) L.L= 0.00,D.L=0.00,T.L=0.00
 PLATES ARE MITEK M20-249,200 MANUFACTURED FROM ASTM A 446 GRD A GALVANIZED STEEL(EXCEPT AS SHOWN)
 PLATE MUST BE INSTALLED ON EA. FACE OF JOINT,SYMMETRICALLY(EXCEPT AS SHOWN)DESIGN CONFORMS W/NDS DESIGN SPECS, S58C,ANSI/TPI-95
 THIS DESIGN IS FOR TRUSS FABRICATION ONLY.FOR PERMANENT AND TEMPORARY BRACING(WHICH IS ALWAYS REQD)CONSULT BLDG ARCHITECT OR ENGINEER.

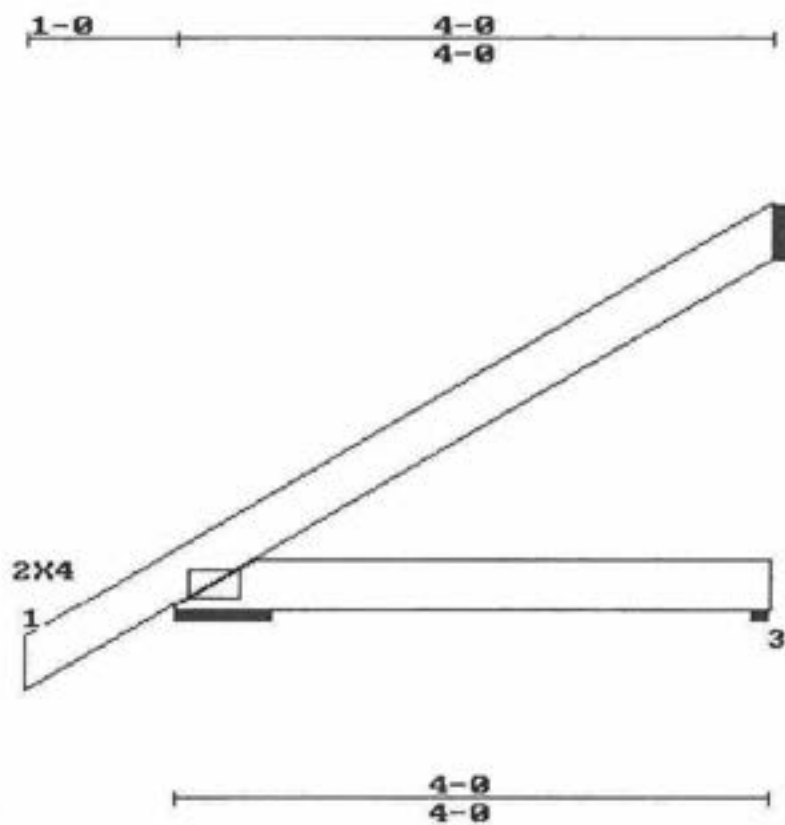
MAR 01 2001

====<<<ACES-32 Ver. 1.1>>>=====[001895]====<<<FL.QUALITY-MMH>>>=====
 Customer : FOGLIA Thu Mar 1 10:10:20 2001
 Project #: 00469 Truss ID : J4 Family #: 205
 Span : 4-0 Quantity : 2 Top Pitch : 6/12
 ==Bld(1/21/2000),v1.1=====

FORCES - GRAVITY LOADS REACTIONS - SIZE
 1-2=0 3-1=0 1=308 8.00
 2-3=0 3=36 1.50
 2=162 1.50



PROVIDE FOR 239 LBS UPLIFT AT JOINT 1 (0.78)
 PROVIDE FOR 250 LBS UPLIFT AT JOINT 2 (1.54)
 PROVIDE FOR 231 LBS HORIZ. REACTION AT JOINT 1
 (SYMMETRIC PLATING TURNED ON)



L. HL TO PK:4-5-11		SPAN:4-0		RISE:2-4-3		RIGHT HEIGHT:2-4-3	
LEFT HEIGHT:0-4-3							
LOADING (PSF)		MAX STRESSES		MINIMUM GRADE OF LUMBER			
L	D	TOP	1-2=0.429	TOP CHORD:2X4	No.2ND	19	SP
TOP	30 15	BOTT	3-1=0.064	BOT CHORD:2X4	No.2ND	19	SP
BOTT	0 10	LL.DEFL. < L/360		WEBS	:2X4	No.3	19 SP
STR.INC.: LUMB = 1.33 PLATE = 1.33				SPACING : 24.0 in. o. c.			
REPETITIVE STRESSES USED				NO. OF MEMBERS = 1			

TRUSS HAS BEEN CHECKED FOR 145 M.P.H. WIND LOAD, WALL HEIGHT 16 FT, ENCLOSED BLDG. TYPE, ASSUMING 10 PSF TOTAL D.L.(7+3) (SBC-97).
 DEFLECTION(IN.) L.L= 0.00,D.L=0.00,T.L=0.00
 PLATES ARE NITEK M20-249,200 MANUFACTURED FROM ASTM A 446 GRD A GALVANIZED STEEL(EXCEPT AS SHOWN)
 PLATE MUST BE INSTALLED ON EA. FACE OF JOINT,SYMMETRICALLY(EXCEPT AS SHOWN)DESIGN CONFORMS W/NOS DESIGN SPECS, S58C,ANSI/TPI-95
 THIS DESIGN IS FOR TRUSS FABRICATION ONLY.FOR PERMANENT AND TEMPORARY BRACING(WHICH IS ALWAYS REQD)CONSULT BLDG ARCHITECT OR ENGINEER.

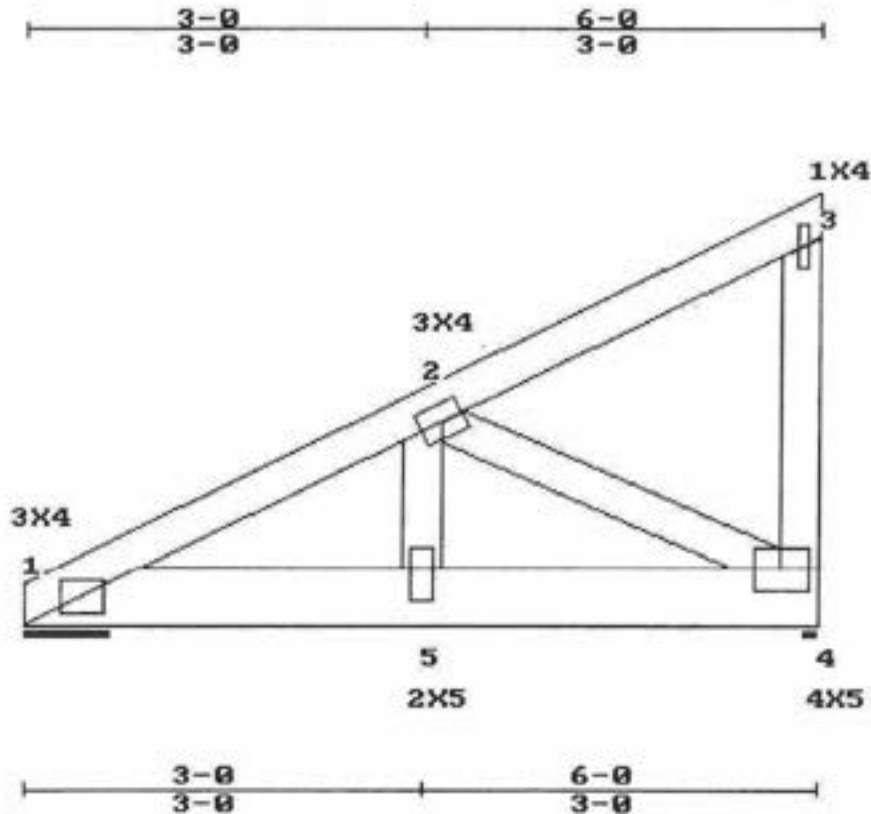
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 MAR 01 2001

====<<<ACES-32 Ver. 1.1>>>=====[001896]====<<<FL.QUALITY-MMH>>>=====
 Customer : FOGLIA Thu Mar 1 10:10:36 2001
 Project #: 00469 Truss ID : GT6A Family #: 201
 Span : 6-0 Quantity : 1 Top Pitch : 6/12
 ==5ld(1/21/2000),v1.1=====

FORCES - LOAD CASE #1			REACTIONS - SIZE	
1-2=877	4-5= 770	2-5= 549	1=801	8.00
2-3= 0	5-1= 770	2-4=859	4=801	1.50
3-4=-132				

89

PROVIDE FOR 908 LBS UPLIFT AT JOINT 1 (1.13)
 PROVIDE FOR 1082 LBS UPLIFT AT JOINT 4 (1.35)
 PROVIDE FOR 338 LBS HORIZ. REACTION AT JOINT 1
 PLATE OFFSETS (X=LEFT,Y=TOP):[J5=3,1],(SYMMETRIC PLATING TURNED ON)



L. HL TO PK:6-8-8		SPAN:6-0		RISE:3-4-3		RIGHT HEIGHT:3-4-3	
LEFT HEIGHT:0-4-3							
LOADING (PSF)		MAX STRESSES		MINIMUM GRADE OF LUMBER			
L	D	TOP	1-2=0.327	TOP CHORD:2X4	No.2ND 19 SP		
30	15	BOTT	4-5=0.234	BOT CHORD:2X6	No.2D 19 SP		
BOTT	0	LL.DEFL. @5=0.01	< L/360	WEBS	:2X4 No.3 19 SP		
				SPACING : 24.0 in. o. c.			
REPETITIVE STRESSES NOT USED				NO. OF MEMBERS = 1			

LOADING	STRESS INCREASE	LOADING	PANEL(PLF) / JOINTS(LBS)
LUMBER	PLATE	TYPE	
1	1.33	1.33	UNIFORM 1- 3= 90 4- 1= 196

RIGHT END VERTICAL IS NOT EXPOSED.
 TRUSS HAS BEEN CHECKED FOR 145 M.P.H. WIND LOAD, WALL HEIGHT 16 FT, ENCLOSED BLDG. TYPE, ASSUMING 10 PSF TOTAL D.L.(7+3) (SBC-97).
 DEFLECTION(IN.) L.L= 0.01,D.L=0.01,T.L=0.02
 PLATES ARE MITEK M20-249,200 MANUFACTURED FROM ASTM A 446 GRD A GALVANIZED STEEL(EXCEPT AS SHOWN)
 PLATE MUST BE INSTALLED ON EA. FACE OF JOINT,SYMMETRICALLY(EXCEPT AS SHOWN)DESIGN CONFORMS W/NDS DESIGN SPECS, SIBC,ANSI/TPI-95
 THIS DESIGN IS FOR TRUSS FABRICATION ONLY.FOR PERMANENT AND TEMPORARY BRACING(WHICH IS ALWAYS REQD)CONSULT BLDG ARCHITECT OR ENGINEER.

[Signature]
 MAR 01 2001

====<<<<ACES-32 Ver. 1.1>>>>=====[001897]=====<<<<FL.QUALITY-MMH>>>>=====

Customer : FOGLIA

Project # : 00469

Span : 6-0

Truss ID : CA5

Quantity : 1

Thu Mar 1 10:10:48 2001

Family # : 205

Top Pitch : 6/12

==Bld(1/21/2000),v1.1=====

FORCES - GRAVITY LOADS

1-2=0

3-1=0

REACTIONS - SIZE

1=418 8.00

3=56 1.50

2=252 1.50

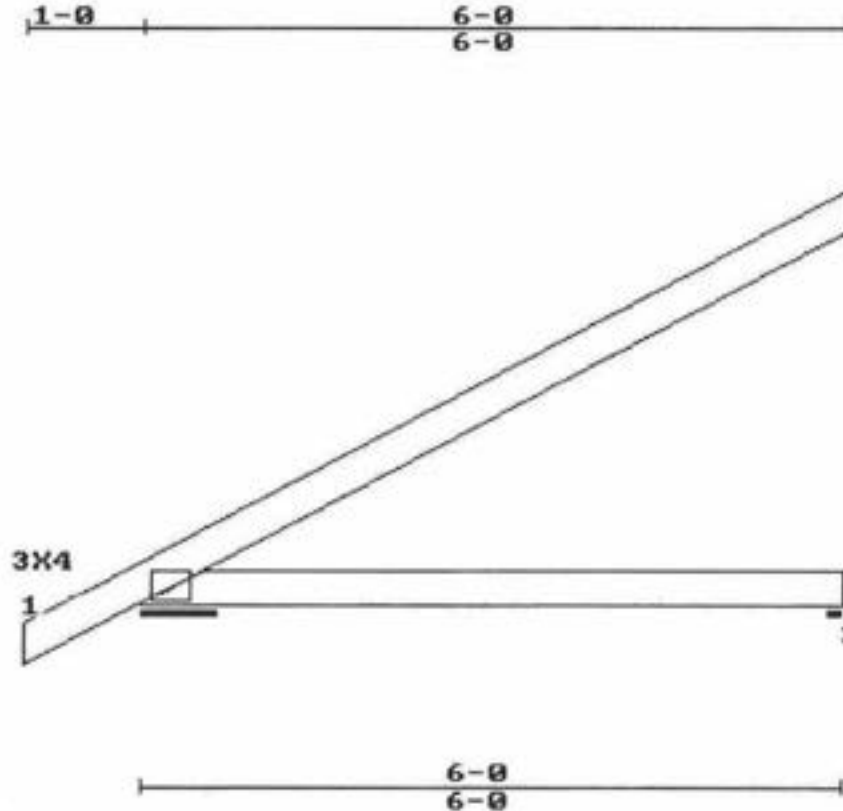
90

PROVIDE FOR 307 LBS UPLIFT AT JOINT 1 (0.73)

PROVIDE FOR 383 LBS UPLIFT AT JOINT 2 (1.52)

PROVIDE FOR 348 LBS HORIZ. REACTION AT JOINT 1

(SYMMETRIC PLATING TURNED ON)



L. HL TO PK:6-8-8

LEFT HEIGHT:0-4-3

SPAN:6-0

RISE:3-4-3

RIGHT HEIGHT:3-4-3

LOADING (PSF)

L D

MAX STRESSES

TOP 1-2=0.712

BOTT 3-1=0.152

LL.DEFL. < L/360

MINIMUM GRADE OF LUMBER

TOP CHORD:2X4 No.1D 19 SP

BOT CHORD:2X4 No.2ND 19 SP

WEBS :2X4 No.3 19 SP

STR.INC.: LUMB = 1.33 PLATE = 1.33

REPETITIVE STRESSES USED

SPACING : 24.0 in. o. c.

NO. OF MEMBERS = 1

TRUSS HAS BEEN CHECKED FOR 145 M.P.H. WIND LOAD, WALL HEIGHT 16 FT, ENCLOSED BLDG. TYPE, ASSUMING 10 PSF TOTAL D.L.(7+3) (SBC-97).

DEFLECTION(IN.) L.L= 0.00,D.L=0.00,T.L=0.00

PLATES ARE MITEK M20-249,200 MANUFACTURED FROM ASTM A 446 GRD A GALVANIZED STEEL(EXCEPT AS SHOWN)

PLATE MUST BE INSTALLED ON EA. FACE OF JOINT,SYMMETRICALLY(EXCEPT AS SHOWN)DESIGN CONFORMS W/NDS DESIGN SPECS, S58C,ANSI/TP1-95

THIS DESIGN IS FOR TRUSS FABRICATION ONLY.FOR PERMANENT AND TEMPORARY BRACING(WHICH IS ALWAYS REQD)CONSULT BLDG ARCHITECT OR ENGINEER.

[Signature]
MAR 01 2001

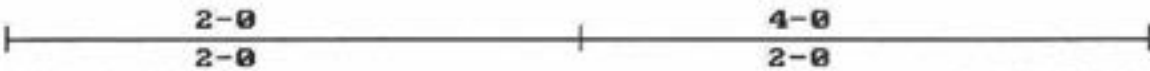
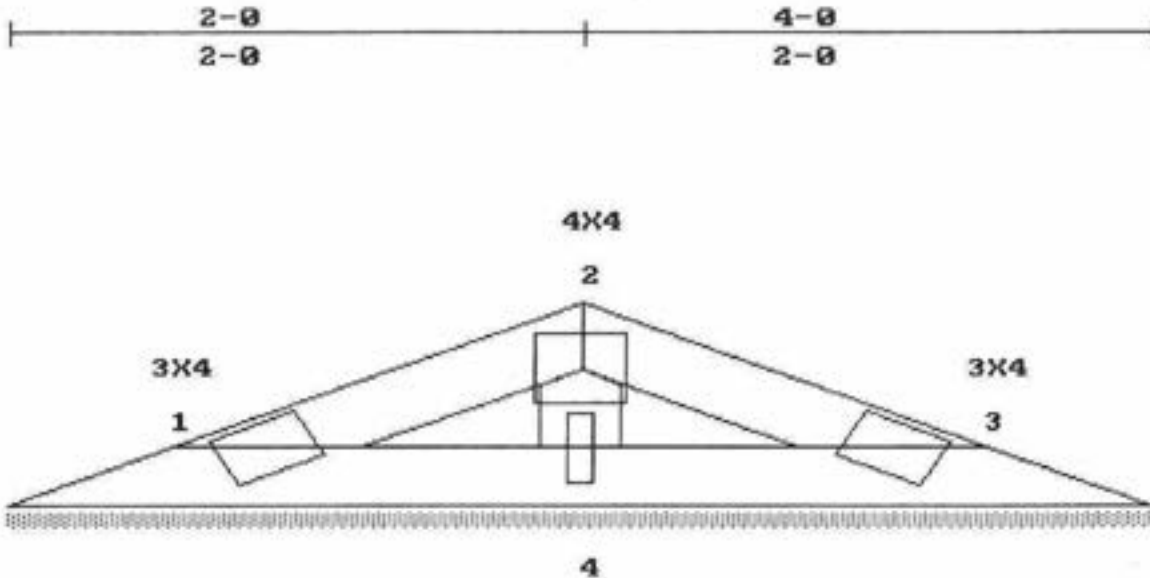
====<<<<ACES-32 Ver. 1.1>>>>===== [001898] =====<<<<FL.QUALITY-MMH>>>>=====

Customer : FOGLIA Thu Mar 1 10:10:57 2001
 Project #: 00469 Truss ID : V4 Family # : 101
 Span : 4-0 Quantity : 1 Top Pitch : 6/12

==81d(1/21/2000),v1.1=====

NOTES: (1) -Gable studs spaced at 24 inches o.c.
 (2) -Brace vertical studs in accordance with standard gable end detail (91)
 (3) -Continuous bearing provided along entire bottom chord
 (4) -Provide 1X4 plates at each end of gable stud unless otherwise noted

(SYMMETRIC PLATING TURNED ON)



L. HL TO PK:2-2-13 LEFT HEIGHT:0-0 SPAN:4-0 RISE:1-0 R. HL TO PK :2-2-13 RIGHT HEIGHT:0-0

LOADING (PSF)		MAX STRESSES	MINIMUM GRADE OF LUMBER	
TOP	30 15		TOP CHORD:2X4	No.2ND 19 SP
BOTT	0 10		BOT CHORD:2X4	No.2ND 19 SP
		LL.DEFL.@0=0.00 < L/360	WEBS	:2X4 No.3 19 SP

STR.INC.: LUMB = 1.33 PLATE = 1.33 SPACING : 24.0 in. o. c.
 REPETITIVE STRESSES USED NO. OF MEMBERS = 1

TRUSS HAS BEEN CHECKED FOR 145 M.P.H. WIND LOAD, WALL HEIGHT 16 FT, ENCLOSED BLDG. TYPE, ASSUMING 10 PSF TOTAL D.L.(7+3) (SBC-97).
 DEFLECTION(IN.) L.L= 0.00,D.L=0.00,T.L=0.00
 PLATES ARE MITEK M20-249,200 MANUFACTURED FROM ASTM A 446 GRD A GALVANIZED STEEL(EXCEPT AS SHOWN)
 PLATE MUST BE INSTALLED ON EA. FACE OF JOINT,SYMMETRICALLY(EXCEPT AS SHOWN)DESIGN CONFORMS W/ND5 DESIGN SPECS, SSBC,ANSI/TPI-95
 THIS DESIGN IS FOR TRUSS FABRICATION ONLY.FOR PERMANENT AND TEMPORARY BRACING(WHICH IS ALWAYS REQD)CONSULT BLDG ARCHITECT OR ENGINEER.

[Signature]
 9 MAR 6 1 2001

====<<<<ACES-32 Ver. 1.1>>>>=====[001899]====<<<<FL.QUALITY-MMH>>>>=====

Customer : FOGLIA

Project #: 00469

Span : 8-0

Truss ID : V8

Quantity : 1

Thu Mar 1 10:11:08 2001

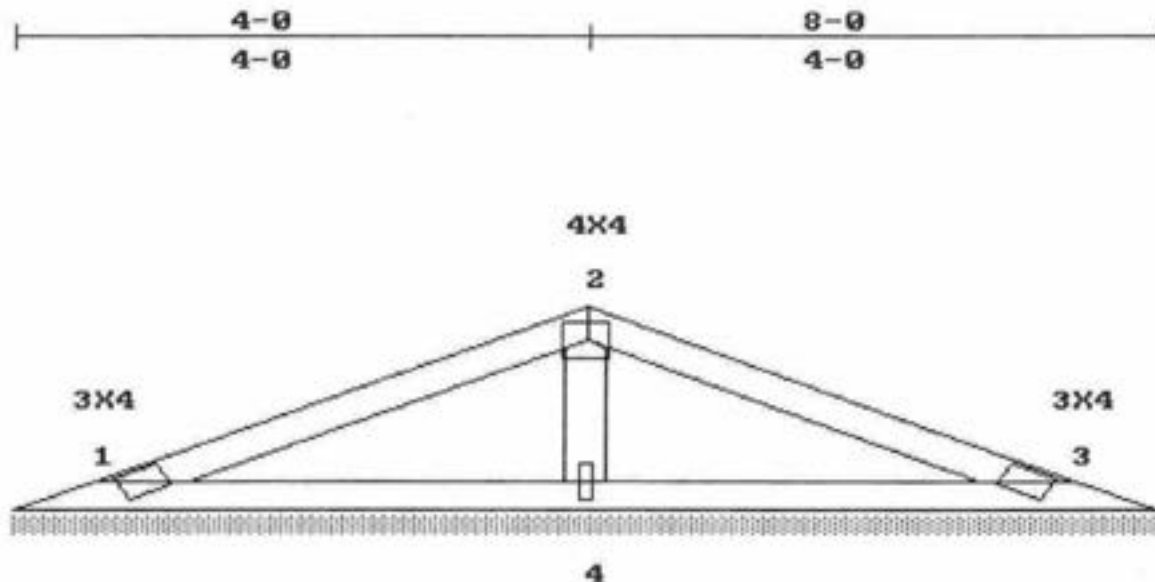
Family # : 101

Top Pitch : 6/12

==81d(1/21/2000),v1.1=====

NOTES: (1) -Gable studs spaced at 24 inches o.c.
(2) -Brace vertical studs in accordance with standard gable end detail (92)
(3) -Continuous bearing provided along entire bottom chord
(4) -Provide 1X4 plates at each end of gable stud unless otherwise noted

(SYMMETRIC PLATING TURNED ON)



L. HL TO PK:4-5-11

LEFT HEIGHT:0-0

SPAN:8-0

RISE:2-0

R. HL TO PK:4-5-11

RIGHT HEIGHT:0-0

LOADING (PSF)

MAX STRESSES

MINIMUM GRADE OF LUMBER

TOP 30 15

BOTT 0 10

LL.DEFL.@0=0.00 < L/360

TOP CHORD:2X4 No.2ND 19 SP

BOT CHORD:2X4 No.2ND 19 SP

WEBS :2X4 No.3 19 SP

STR.INC.: LUMB = 1.33 PLATE = 1.33

SPACING : 24.0 in. o. c.

REPETITIVE STRESSES USED

NO. OF MEMBERS = 1

TRUSS HAS BEEN CHECKED FOR 145 M.P.H. WIND LOAD, WALL HEIGHT 16 FT, ENCLOSED BLDG. TYPE, ASSUMING 10 PSF TOTAL D.L.(7*3) (SBC-97).

DEFLECTION(IN.) L.L= 0.00,D.L=0.00,T.L=0.00

PLATES ARE MITEK M20-249,200 MANUFACTURED FROM ASTM A 446 GRD A GALVANIZED STEEL(EXCEPT AS SHOWN)

PLATE MUST BE INSTALLED ON EA. FACE OF JOINT,SYMMETRICALLY(EXCEPT AS SHOWN)DESIGN CONFORMS W/NDS DESIGN SPECS, SIBC,ANSI/TP1-95

THIS DESIGN IS FOR TRUSS FABRICATION ONLY.FOR PERMANENT AND TEMPORARY BRACING(WHICH IS ALWAYS REQD)CONSULT BLDG ARCHITECT OR ENGINEER.

[Signature]

MAR 01 2001

====<<<ACES-32 Ver. 1.1>>>===== [001900] =====<<<FL.QUALITY-MMH>>>=====

Customer : FOGLIA

Project #: 00469

Span : 12-0

Truss ID : V12

Quantity : 1

Thu Mar 1 10:11:17 2001

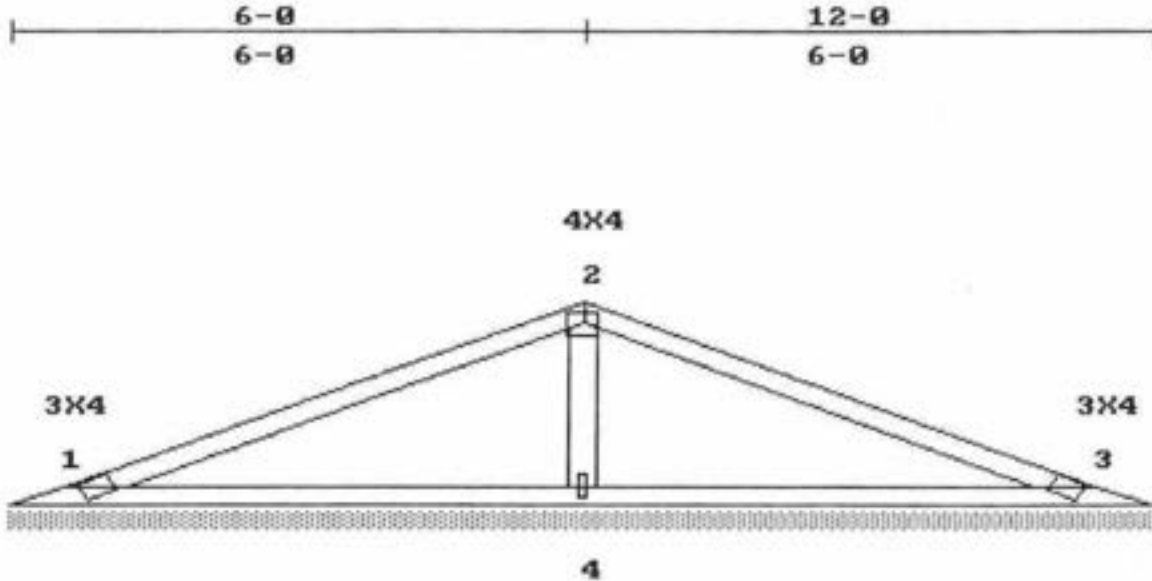
Family # : 101

Top Pitch : 6/12

==Bld(1/21/2000),v1.1=====

NOTES: (1) - Gable studs spaced at 48 inches o.c.
(2) - Brace vertical studs in accordance with standard gable end detail (93)
(3) - Continuous bearing provided along entire bottom chord
(4) - Provide 1X4 plates at each end of gable stud unless otherwise noted

(SYMMETRIC PLATING TURNED ON)



L. HL TO PK: 6-8-8

LEFT HEIGHT: 0-0

SPAN: 12-0

RISE: 3-0

R. HL TO PK: 6-8-8

RIGHT HEIGHT: 0-0

LOADING (PSF)

MAX STRESSES

MINIMUM GRADE OF LUMBER

TOP 30 15

BOTT 0 10

LL.DEFL.@0=0.00 < L/360

TOP CHORD: 2X4 No.2ND 19 SP

BOT CHORD: 2X4 No.2ND 19 SP

WEBS : 2X4 No.3 19 SP

STR.INC.: LUMB = 1.33 PLATE = 1.33

SPACING : 24.0 in. o. c.

REPETITIVE STRESSES USED

NO. OF MEMBERS = 1

TRUSS HAS BEEN CHECKED FOR 145 M.P.H. WIND LOAD, WALL HEIGHT 16 FT, ENCLOSED BLDG. TYPE, ASSUMING 10 PSF TOTAL D.L.(7+3) (SBC-97).

DEFLECTION(IN.) L.L= 0.00,D.L=0.00,T.L=0.00

PLATES ARE MITEK M20-249,200 MANUFACTURED FROM ASTM A 446 GRD A GALVANIZED STEEL(EXCEPT AS SHOWN)

PLATE MUST BE INSTALLED ON EA. FACE OF JOINT,SYMMETRICALLY(EXCEPT AS SHOWN)DESIGN CONFORMS W/NDS DESIGN SPECS, SBC,ANSI/TPI-95

THIS DESIGN IS FOR TRUSS FABRICATION ONLY.FOR PERMANENT AND TEMPORARY BRACING(WHICH IS ALWAYS REQD)CONSULT BLDG ARCHITECT OR ENGINEER.

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MAR 01 2001

====<<<<ACES-32 Ver. 1.1>>>>===== [001901]=====<<<<FL.QUALITY-MMH>>>>=====

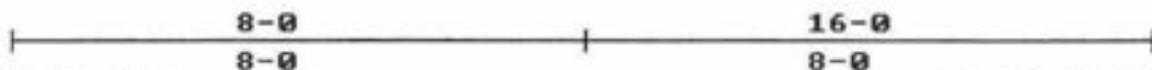
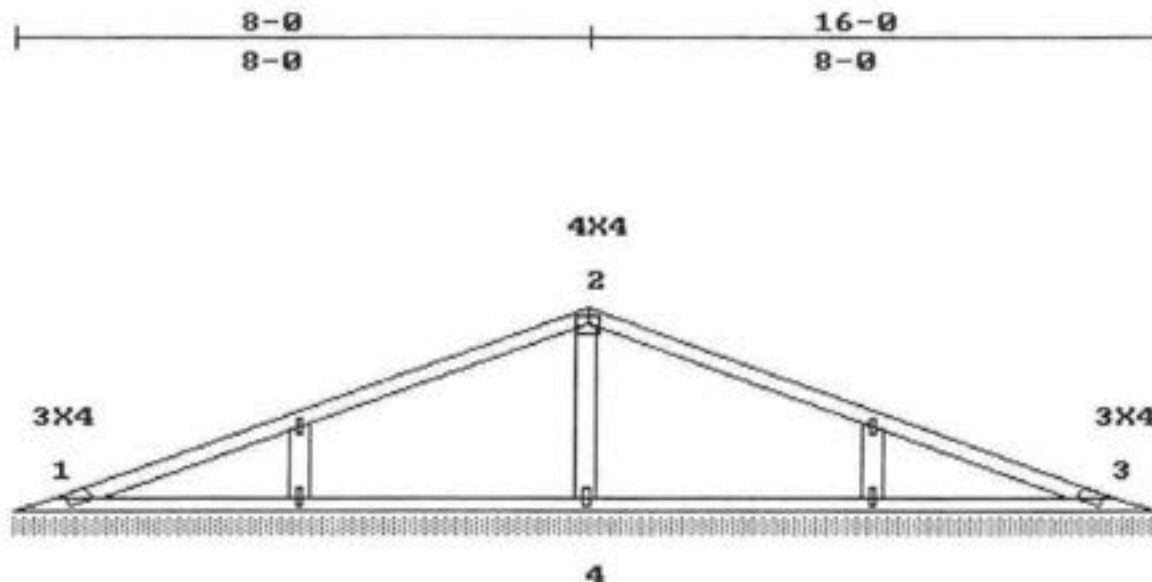
Customer : FOGLIA	Truss ID : V16	Thu Mar 1 10:11:25 2001
Project #: 00469	Quantity : 1	Family # : 101
Span : 16-0		Top Pitch : 6/12

==Bld(1/21/2000), v1.1=====

NOTES: (1) -Gable studs spaced at 48 inches o.c.
 (2) -Brace vertical studs in accordance with standard gable end detail
 (3) -Continuous bearing provided along entire bottom chord
 (4) -Provide 1X4 plates at each end of gable stud unless otherwise noted

94

(SYMMETRIC PLATING TURNED ON)



L. HL TO PK:8-11-5 LEFT HEIGHT:0-0 SPAN:16-0 RISE:4-0 R. HL TO PK:8-11-5 RIGHT HEIGHT:0-0

LOADING (PSF)		MAX STRESSES	MINIMUM GRADE OF LUMBER	
L	D		TOP CHORD:2X4	No.2ND 19 SP
TOP	30	15	BOT CHORD:2X4	No.2ND 19 SP
BOTT	0	10	WEBS	:2X4 No.3 19 SP
LL.DEFL.@0=0.00 < L/360				
STR.INC.: LUMB = 1.33 PLATE = 1.33			SPACING : 24.0 in. o. c.	
REPETITIVE STRESSES USED			NO. OF MEMBERS = 1	

TRUSS HAS BEEN CHECKED FOR 145 M.P.H. WIND LOAD, WALL HEIGHT 16 FT, ENCLOSED BLDG. TYPE, ASSUMING 10 PSF TOTAL D.L.(7+3) (SBC-97).
 DEFLECTION(IN.) L.L= 0.00,D.L=0.00,T.L=0.00
 PLATES ARE MITEK M20-249,200 MANUFACTURED FROM ASTM A 446 GRD A GALVANIZED STEEL(EXCEPT AS SHOWN)
 PLATE MUST BE INSTALLED ON EA. FACE OF JOINT,SYMMETRICALLY(EXCEPT AS SHOWN)DESIGN CONFORMS W/NDS DESIGN SPECS, SSBC,ANSI/TP1-95
 THIS DESIGN IS FOR TRUSS FABRICATION ONLY.FOR PERMANENT AND TEMPORARY BRACING(WHICH IS ALWAYS REQD)CONSULT BLDG ARCHITECT OR ENGINEER.

[Signature]
 MAR 01 2001

====<<<ACES-32 Ver. 1.1>>>=====[002000]====<<<FL.QUALITY-MMH>>>=====
 Customer : FOGLIA Truss ID : T9A Wed Mar 7 07:11:38 2001
 Project #: 00469 Family #: 101
 Span : 14-8 B.Span : 14-8 Quant. : 1 Top Pitch : 6/12
 ==Bld(1/21/2000),v1.1=====

FORCES - GRAVITY LOADS

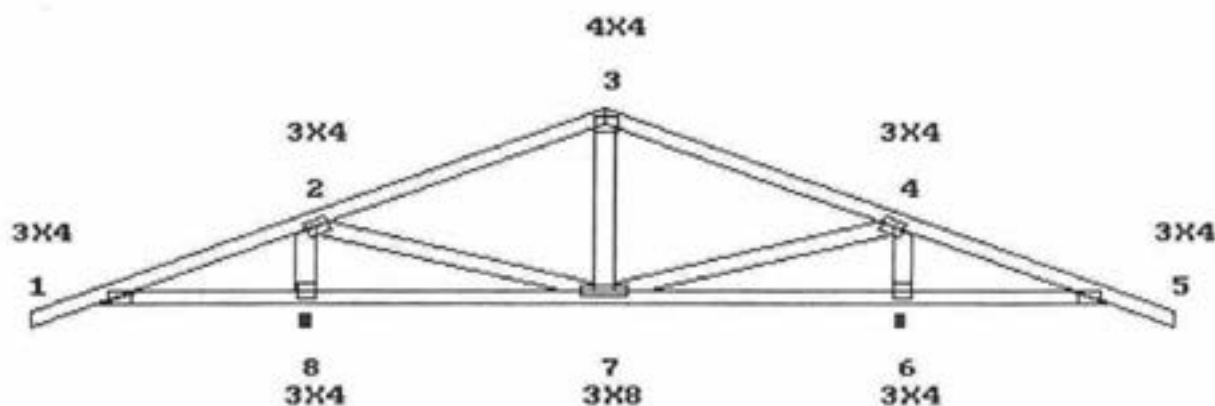
1-2= 603 5-6=-536 2-8=-843
 2-3=-63 6-7=-536 2-7= 629
 3-4=-63 7-8=-536 3-7=-334
 4-5= 603 8-1=-536 4-7= 629
 4-6=-843

REACTIONS - SIZE

8=917 2.00
 6=917 2.00

95

PROVIDE FOR 1174 LBS UPLIFT AT JOINT 8 (1.28)
 PROVIDE FOR 1174 LBS UPLIFT AT JOINT 6 (1.28)
 PROVIDE FOR 124 LBS HORIZ. REACTION AT JOINT 8



L. HL TO PK: 8-2-6
 LEFT HEIGHT: 0-4-3

SPAN: 14-8

RISE: 4-0-3

R. HL TO PK: 8-2-6
 RIGHT HEIGHT: 0-4-3

LOADING (PSF)

TOP 30 15
 BOTT 0 10

MAX STRESSES

TOP 1-2=0.436
 BOTT 5-6=0.616
 LL.DEFL.@5=0.02 < L/360

MINIMUM GRADE OF LUMBER

TOP CHORD: 2X4 No.2ND 19 SP
 BOT CHORD: 2X4 No.2ND 19 SP
 WEBS : 2X4 No.3 19 SP

STR.INC.: LUMB = 1.33 PLATE = 1.33
 REPETITIVE STRESSES USED

SPACING : 24.0 in. o. c.
 NO. OF MEMBERS = 1

TRUSS HAS BEEN CHECKED FOR 145 M.P.H. WIND LOAD, WALL HEIGHT 16 FT, ENCLOSED BLDG. TYPE, ASSUMING 10 PSF TOTAL D.L.(7+3) (SBC-97).
 DEFLECTION(IN.) L.L= 0.02,D.L=0.02,T.L=0.05
 PLATES ARE MITEK M20-249,200 MANUFACTURED FROM ASTM A 446 GRD A GALVANIZED STEEL(EXCEPT AS SHOWN)
 PLATE MUST BE INSTALLED ON EA. FACE OF JOINT,SYMMETRICALLY(EXCEPT AS SHOWN)DESIGN CONFORMS W/ND'S DESIGN SPECS, SSB,ANSI/TP1-95
 THIS DESIGN IS FOR TRUSS FABRICATION ONLY.FOR PERMANENT AND TEMPORARY BRACING(WHICH IS ALWAYS REQD)CONSULT BLDG ARCHITECT OR ENGINEER.

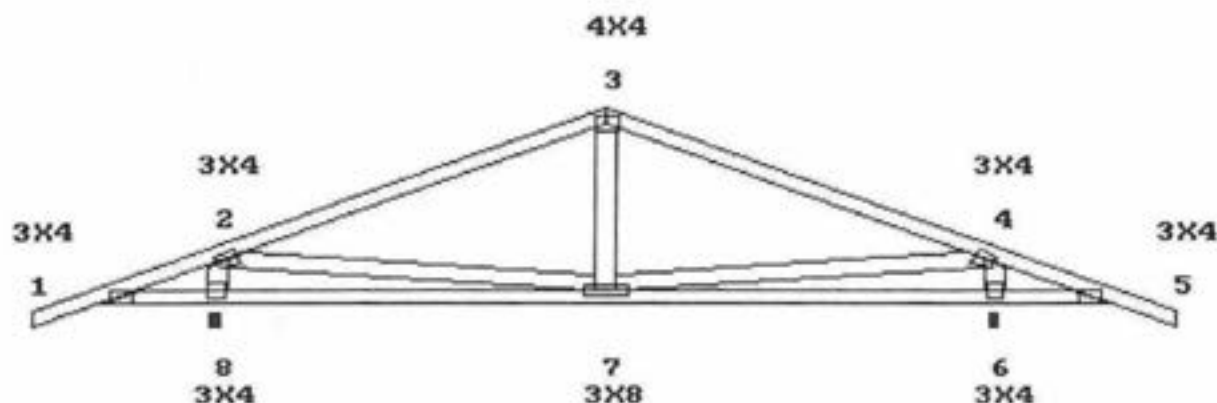
MAR 06 2001

====<<<ACES-32 Ver. 1.1>>>=====[002001]====<<<FL.QUALITY-MMH>>>=====
 Customer : FOGLIA Wed Mar 7 07:11:48 2001
 Project #: 00469 Truss ID : T9 Family #: 101
 Span : 14-8 B.Span : 14-8 Quant. : 1 Top Pitch : 6/12
 ==Bld(1/21/2000),v1.1=====

FORCES - GRAVITY LOADS			REACTIONS - SIZE	
1-2= 435	5-6=386	2-8=843	8=917	2.00
2-3=432	6-7=386	2-7= 781	6=917	2.00
3-4=432	7-8=386	3-7=124		
4-5= 435	8-1=386	4-7= 781		
		4-6=843		

PROVIDE FOR 988 LBS UPLIFT AT JOINT 8 (1.08)
 PROVIDE FOR 988 LBS UPLIFT AT JOINT 6 (1.08)
 PROVIDE FOR 124 LBS HORIZ. REACTION AT JOINT 8

1-0 1-8 7-4 13-0 14-8 1-0
 1-8 5-8 5-8 1-8



1-8 7-4 13-0 14-8
 1-8 5-8 5-8 1-8

L. HL TO PK:8-2-6 LEFT HEIGHT:0-4-3 SPAN:14-8 RISE:4-0-3 R. HL TO PK:8-2-6 RIGHT HEIGHT:0-4-3

LOADING (PSF)		MAX STRESSES		MINIMUM GRADE OF LUMBER	
L	D	TOP	BOTT	TOP CHORD	BOT CHORD
30	15	2-3=0.869	5-6=0.860	2X4 No.2ND 19 SP	2X4 No.2ND 19 SP
0	10	LL.DEFL.@5=0.02 < L/360		WEBS :2X4 No.3 19 SP	

STR.INC.: LUMB = 1.33 PLATE = 1.33 SPACING : 24.0 in. o. c.
 REPETITIVE STRESSES USED NO. OF MEMBERS = 1

TRUSS HAS BEEN CHECKED FOR 145 M.P.H. WIND LOAD, WALL HEIGHT 16 FT, ENCLOSED BLDG. TYPE, ASSUMING 10 PSF TOTAL D.L.(7+3) (SBC-97).
 DEFLECTION(IN.) L.L= 0.02,D.L=0.01,T.L=0.03
 PLATES ARE MITEK M20-249,200 MANUFACTURED FROM ASTM A 446 GRD A GALVANIZED STEEL(EXCEPT AS SHOWN)
 PLATE MUST BE INSTALLED ON EA. FACE OF JOINT,SYMMETRICALLY(EXCEPT AS SHOWN)DESIGN CONFORMS W/NDIS DESIGN SPECS, S5BC,ANSI/TPI-95
 THIS DESIGN IS FOR TRUSS FABRICATION ONLY.FOR PERMANENT AND TEMPORARY BRACING(WHICH IS ALWAYS REQD)CONSULT BLDG ARCHITECT OR ENGINEER.

MAR 06 2001

====<<<ACES-32 Ver. 1.1>>>===== [002002] =====<<<FL.QUALITY-MMH>>>=====

Customer : FOGLIA Wed Mar 7 07:11:58 2001

Project #: 00469 Truss ID : T8 Family # : 101

Span : 14-8 B.Span : 14-8 Quant. : 1 Top Pitch : 6/12

==Bld(1/21/2000),v1.1=====

FORCES - GRAVITY LOADS REACTIONS - SIZE

1-2=876 3-4= 780 2-4= 145 1=908 2.00

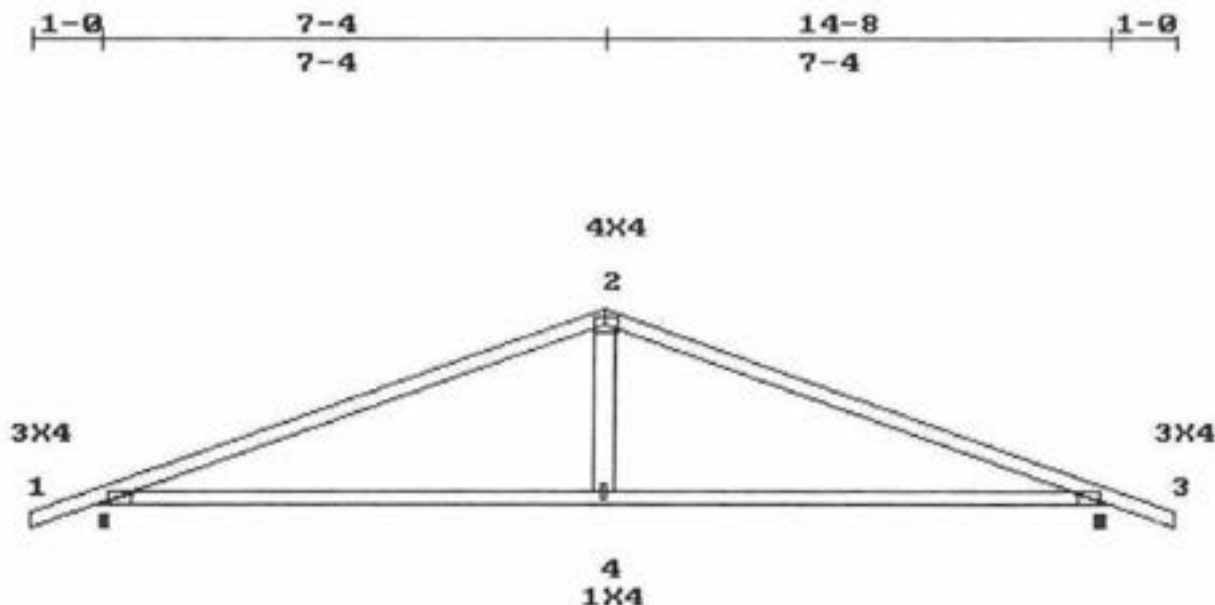
2-3=876 4-1= 780 3=908 2.00

PROVIDE FOR 757 LBS UPLIFT AT JOINT 1 (0.83)

PROVIDE FOR 757 LBS UPLIFT AT JOINT 3 (0.83)

PROVIDE FOR 124 LBS HORIZ. REACTION AT JOINT 1

97



L. HL TO PK:8-2-6 SPAN:14-8 RISE:4-0-3 R. HL TO PK:8-2-6

LEFT HEIGHT:0-4-3 RIGHT HEIGHT:0-4-3

LOADING (PSF)		MAX STRESSES		MINIMUM GRADE OF LUMBER	
TOP	30	TOP	2-3=0.760	TOP CHORD:2X4	SS 19 SP
BOTT	0	BOTT	4-1=0.377	BOT CHORD:2X4	No.2ND 19 SP
		LL.DEFL.@4=0.02 < L/360		WEBS	:2X4 No.3 19 SP

STR.INC.: LUMB = 1.33 PLATE = 1.33 SPACING : 24.0 in. o. c.

REPETITIVE STRESSES USED NO. OF MEMBERS = 1

TRUSS HAS BEEN CHECKED FOR 145 M.P.H. WIND LOAD, WALL HEIGHT 16 FT, ENCLOSED BLDG. TYPE, ASSUMING 10 PSF TOTAL D.L.(7*3) (SBC-97).

DEFLECTION(IN.) L.L= 0.02,D.L=0.02,T.L=0.04

PLATES ARE MITEK M20-249,200 MANUFACTURED FROM ASTM A 446 GRD A GALVANIZED STEEL(EXCEPT AS SHOWN)

PLATE MUST BE INSTALLED ON EA. FACE OF JOINT,SYMMETRICALLY(EXCEPT AS SHOWN)DESIGN CONFORMS W/NDS DESIGN SPECS, S58C,ANSI/TPI-95

THIS DESIGN IS FOR TRUSS FABRICATION ONLY.FOR PERMANENT AND TEMPORARY BRACING(WHICH IS ALWAYS REQD)CONSULT BLDG ARCHITECT OR ENGINEER.

MAR 06 2001

====<<<<ACES-32 Ver. 1.1>>>>===== [002009] =====<<<<FL.QUALITY-MMH>>>>=====

Customer : FOGLIA
 Project # : 00469
 Span : 14-8 B.Span : 14-8 Truss ID : T1
 2-4 4.26/12, 5-7 4.26/12, Quant. : 1 Family # : spec.
 Wed Mar 7 07:17:19 2001 Top Pitch : 6/12

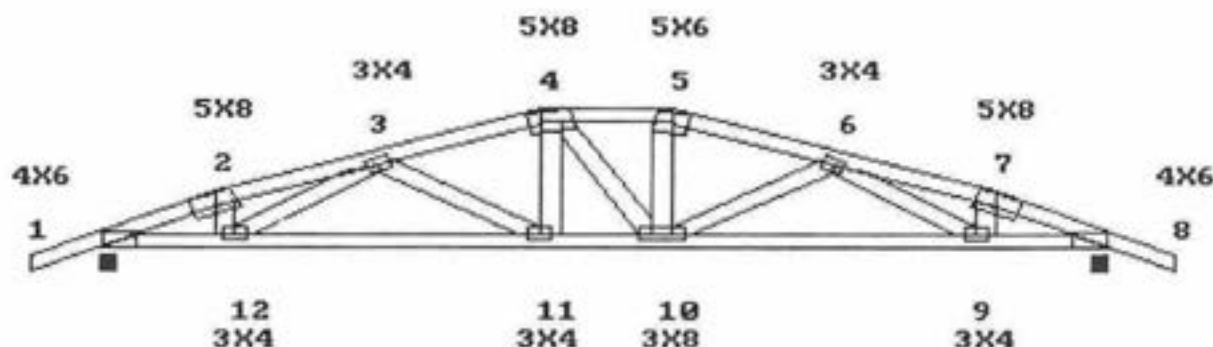
==Bld(1/21/2000),v1.1=====

FORCES - LOAD CASE #1				REACTIONS - SIZE	
1-2=2852	7-8=2853	8-9= 2477	2-12= 266	6-10=371	1=1609 3.04
2-3=2628		9-10= 2706	3-12=283	6-9=283	8=1609 3.03
3-4=2556		10-11= 2406	3-11=370	7-9= 266	
4-5=2406		11-12= 2705	4-11= 501		
5-6=2556		12-1= 2477	4-10= 0		
6-7=2629			5-10= 501		

98

PROVIDE FOR 2184 LBS UPLIFT AT JOINT 1 (1.36)
 PROVIDE FOR 2184 LBS UPLIFT AT JOINT 8 (1.36)
 PROVIDE FOR 86 LBS HORIZ. REACTION AT JOINT 1
 PLATE OFFSETS (X=LEFT,Y=TOP): [j2=4,3], [j7=4,3],

1-0 1-8 4-0-5 6-4-9 8-3-7 10-7-12 13-0 14-8 1-0
 1-8 2-4-5 2-4-5 1-10-14 2-4-5 2-4-5 1-8



1-8 6-4-9 8-3-7 13-0 14-8
 1-8 4-8-9 1-10-14 4-8-9 1-8

L. HL TO PK:1-10-6 LEFT HEIGHT:0-4-3 SPAN:14-8 RISE:2-10-4 R. HL TO PK:1-10-6 RIGHT HEIGHT:0-4-3

LOADING (PSF)		MAX STRESSES		MINIMUM GRADE OF LUMBER	
L	D	TOP	BOTT	TOP CHORD	NO.2ND 19 SP
30	15	7-8=0.736	9-10=0.623	2X4	No.2ND 19 SP
0	10	LL.DEFL.@11=0.10 < L/360		2X4	No.3 19 SP

REPETITIVE STRESSES NOT USED

SPACING : 24.0 in. o. c.
 NO. OF MEMBERS = 1

LOADING	STRESS INCREASE	LOADING	PANEL(PLF) / JOINTS(LBS)
LUMBER	PLATE	TYPE	
1	1.33 1.33	UNIFORM	1- 2= 90 2- 7= 170 7- 8= 90 8- 9= 20 9-12= 25 12- 1= 20
		CONCENTRATED	9= 25 10= 200 11= 200 12= 25

TRUSS HAS BEEN CHECKED FOR 145 M.P.H. WIND LOAD, WALL HEIGHT 16 FT, ENCLOSED BLDG. TYPE, ASSUMING 10 PSF TOTAL D.L.(7+3) (SBC-97).
 DEFLECTION(IN.) L.L.= 0.10,D.L.=0.09,T.L.=0.19
 PLATES ARE MITEK M20-249,200 MANUFACTURED FROM ASTM A 446 GRD A GALVANIZED STEEL(EXCEPT AS SHOWN)
 PLATE MUST BE INSTALLED ON EA. FACE OF JOINT,SYMMETRICALLY(EXCEPT AS SHOWN)DESIGN CONFORMS W/NDG DESIGN SPECS, SSB,ANSI/TPI-95
 THIS DESIGN IS FOR TRUSS FABRICATION ONLY.FOR PERMANENT AND TEMPORARY BRACING(WHICH IS ALWAYS REQD)CONSULT BLDG ARCHITECT OR ENGINEER.

MMH 06 2001

====<<<ACES-32 Ver. 1.1>>>=====[001907]====<<<FL.QUALITY-MMH>>>=====

Customer : FOGLIA

Project #: 00469

Span : 2-4

Truss ID : JD2

Quantity : 1

Thu Mar 1 10:13:44 2001

Family #: 205

Top Pitch : 6/12

==Bld(1/21/2000),v1.1=====

FORCES - GRAVITY LOADS

1-2= 0

3-1= 0

2-3= 0

REACTIONS - SIZE

1=-229 2.00

3=-22 2.00

2=-97 1.50

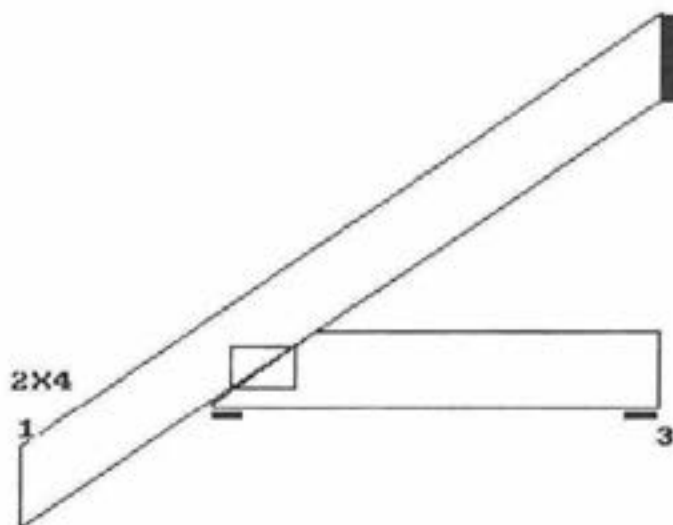
99

PROVIDE FOR 197 LBS UPLIFT AT JOINT 1 (0.86)

PROVIDE FOR 147 LBS UPLIFT AT JOINT 2 (1.51)

PROVIDE FOR 133 LBS HORIZ. REACTION AT JOINT 1
(SYMMETRIC PLATING TURNED ON)

1-0 2-4
2-4



2-4
2-4

L. HL TO PK:2-7-5

LEFT HEIGHT:0-4-3

SPAN:2-4

RISE:1-6-3

RIGHT HEIGHT:1-6-3

LOADING (PSF)

L D

TOP 30 15

BOTT 0 10

MAX STRESSES

TOP 1-2=0.126

BOTT 3-1=0.019

LL.DEFL. < L/360

MINIMUM GRADE OF LUMBER

TOP CHORD:2X4 No.2ND 19 SP

BOT CHORD:2X4 No.2ND 19 SP

WEBS :2X4 No.3 19 SP

STR.INC.: LUMB = 1.33 PLATE = 1.33

REPETITIVE STRESSES USED

SPACING : 24.0 in. o. c.

NO. OF MEMBERS = 1

TRUSS HAS BEEN CHECKED FOR 145 M.P.H. WIND LOAD, WALL HEIGHT 16 FT, ENCLOSED BLDG. TYPE, ASSUMING 10 PSF TOTAL D.L.(7+3) (SBC-97).
DEFLECTION(IN.) L.L= 0.00,D.L=0.00,T.L=0.00

PLATES ARE MITEK M20-249,200 MANUFACTURED FROM ASTM A 446 GRD A GALVANIZED STEEL(EXCEPT AS SHOWN)

PLATE MUST BE INSTALLED ON EA. FACE OF JOINT,SYMMETRICALLY(EXCEPT AS SHOWN)DESIGN CONFORMS W/NDS DESIGN SPECS, S88C,ANSI/TPI-95

THIS DESIGN IS FOR TRUSS FABRICATION ONLY.FOR PERMANENT AND TEMPORARY BRACING(WHICH IS ALWAYS REQD)CONSULT BLDG ARCHITECT OR ENGINEER.

[Signature]

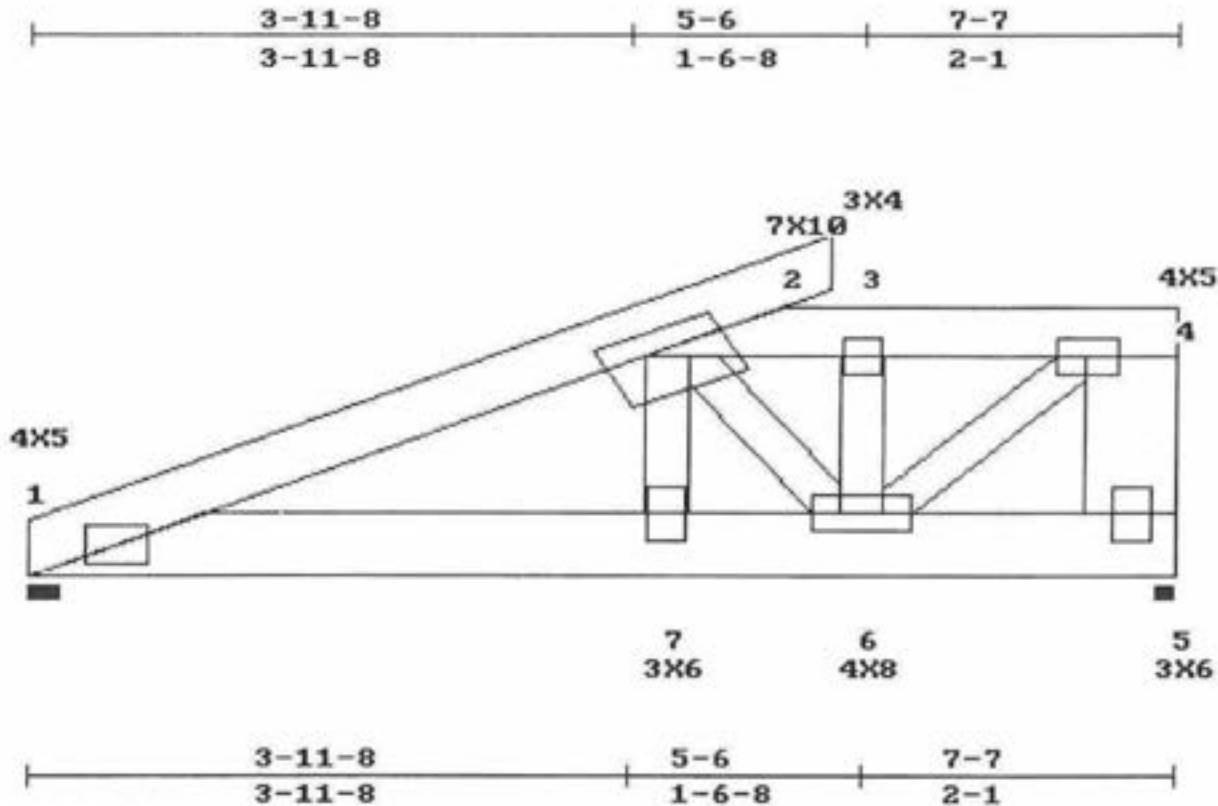
MAR 01 2001

====<<<<ACES-32 Ver. 1.1>>>>=====[002006]====<<<<FL.QUALITY-MMH>>>>=====
 Customer : FOGLIA Wed Mar 7 07:14:51 2001
 Project #: 00469
 Span : 7-7 B.Span : 8-3-8 Truss ID : T10 Family #: 313
 Quant. : 4 Top Pitch : 6/12
 ==Bld(1/21/2000),v1.1=====

FORCES - LOAD CASE #1			REACTIONS - SIZE	
1-2=-2420	5-6= 0	2-7= 54	1=-4368	2.58
2-3=-2467	6-7= 2152	2-6= 513	5=-2692	1.66
3-4=-2467	7-1= 2152	3-6=-2040		
4-5=-2672		4-6= 3463		

100

PROVIDE FOR 4262 LBS UPLIFT AT JOINT 1 (0.98)
 PROVIDE FOR 2814 LBS UPLIFT AT JOINT 5 (1.05)
 PROVIDE FOR 233 LBS HORIZ. REACTION AT JOINT 1
 PLATE OFFSETS (X=LEFT,Y=TOP):[j2=5,3],



L. HL TO PK:4-5-2
 LEFT HEIGHT:0-6-7 SPAN:7-7 RISE:3-2-3 RIGHT HEIGHT:2-6-3

LOADING (PSF)		MAX STRESSES		MINIMUM GRADE OF LUMBER	
TOP	30	TOP	1-2=0.182	TOP CHORD:2X6	No.2D 19 SP
BOTT	0	BOTT	7-1=0.061	BOT CHORD:2X8	SS 19 SP
		LL.DEFL.@6=0.01	< L/360	WEBS	:2X4 No.3 19 SP

REPETITIVE STRESSES NOT USED

SPACING : 24.0 in. o. c.
 NO. OF MEMBERS = 2

LOADING	STRESS INCREASE	LOADING	PANEL(PLF) / JOINTS(LBS)
LUMBER	PLATE	TYPE	
1	1.33	1.33	1- 4= 240 5- 1= 20
		UNIFORM	1= 2760 3= 1613 6= 760
		CONCENTRATED	

WEB: 4-5 TO BE 2X8 SS 19 SP

RIGHT END VERTICAL IS NOT EXPOSED.

2 MEMBERS NAILED TOG. W/1 ROW(S) OF .131x3 in. NAILS 3 in. o.c.(TOP CHS.),AND 1 ROW(S) OF .131x3 in. NAILS 12 in. o.c(BOTT. CHS.)
 For Webs use 1 ROW of NAILS 12 in. o.c.

TRUSS HAS BEEN CHECKED FOR 145 M.P.H. WIND LOAD, WALL HEIGHT 16 FT, ENCLOSED BLDG. TYPE, ASSUMING 10 PSF TOTAL D.L.(7+3) (SBC-97).

DEFLECTION(IN.) L.L= 0.01,D.L=0.01,T.L=0.02

PLATES ARE MITEK M20-249,200 MANUFACTURED FROM ASTM A 446 GRD A GALVANIZED STEEL(EXCEPT AS SHOWN)

PLATE MUST BE INSTALLED ON EA. FACE OF JOINT,SYMMETRICALLY(EXCEPT AS SHOWN)DESIGN CONFORMS W/NDS DESIGN SPECS, S58C,ANSI/TP1-95

THIS DESIGN IS FOR TRUSS FABRICATION ONLY.FOR PERMANENT AND TEMPORARY BRACING(WHICH IS ALWAYS REGD)CONSULT BLDG ARCHITECT OR ENGINEER.

MAR 00 2001

====<<<<ACES-32 Ver. 1.1>>>>===== [002008] =====<<<<FL.QUALITY-MMH>>>>=====

Customer : FOGLIA

Project # : 00469

Span : 7-11-8 B.Span : 8-8

Truss ID : S1

Quant. : 1

Wed Mar 7 07:17:03 2001

Family # : 201

Top Pitch : 6/12

==Bld(1/21/2000),v1.1=====

FORCES - GRAVITY LOADS

1-2=-433 4-5= 0 2-5=-339

2-3=-426 5-1= 384 3-5= 568

3-4= 36

REACTIONS - SIZE

1=-543 2.00

3=-484 2.00

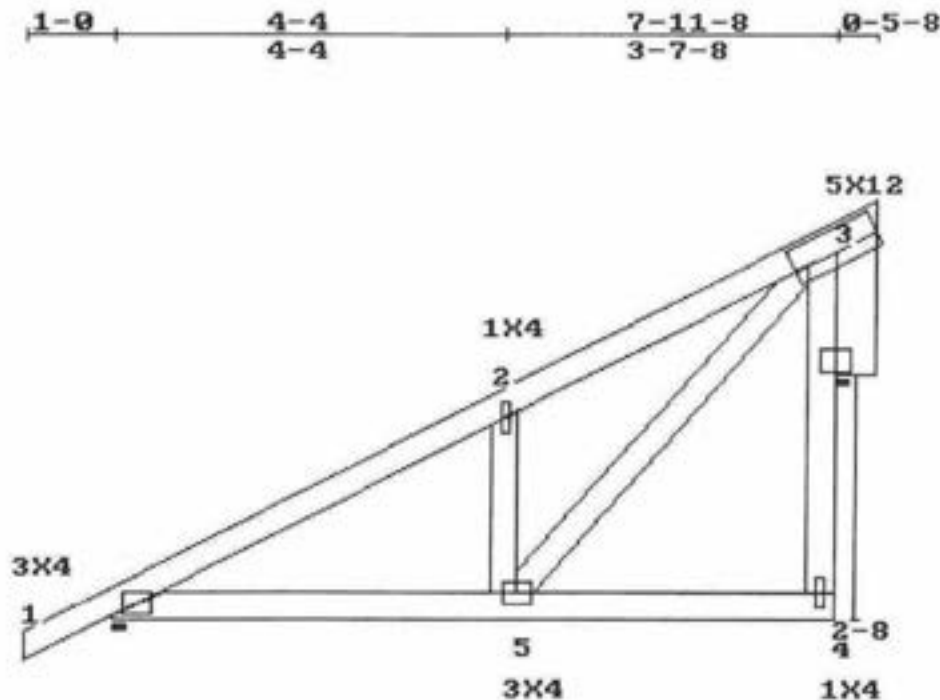
101

PROVIDE FOR 394 LBS UPLIFT AT JOINT 1 (0.73)

PROVIDE FOR 558 LBS UPLIFT AT JOINT 3 (1.15)

PROVIDE FOR 463 LBS HORIZ. REACTION AT JOINT 1

PLATE OFFSETS (X=LEFT,Y=TOP):[J3=2,3],



L. HL TO PK:8-10-12

LEFT HEIGHT:0-4-3

SPAN:7-11-8

RISE:4-6-11

RIGHT HEIGHT:4-3-15

LOADING (PSF)

TOP 30 15

BOTT 0 10

MAX STRESSES

TOP 2-3=0.505

BOTT 5-1=0.134

LL.DEFL.@5=0.01 < L/360

MINIMUM GRADE OF LUMBER

TOP CHORD:2X4 No.2ND 19 SP

BOT CHORD:2X4 No.2ND 19 SP

WEBS :2X4 No.3 19 SP

STR.INC.: LUMB = 1.33 PLATE = 1.33

REPETITIVE STRESSES USED

SPACING : 24.0 in. o. c.

NO. OF MEMBERS = 1

RIGHT END VERTICAL IS NOT EXPOSED.

TRUSS HAS BEEN CHECKED FOR 145 M.P.H. WIND LOAD, WALL HEIGHT 16 FT, ENCLOSED BLDG. TYPE, ASSUMING 10 PSF TOTAL D.L.(7+3) (SBC-97).

DEFLECTION(IN.) L.L= 0.01,D.L=0.01,T.L=0.01

PLATES ARE MITEK M20-249,200 MANUFACTURED FROM ASTM A 446 GRD A GALVANIZED STEEL(EXCEPT AS SHOWN)

PLATE MUST BE INSTALLED ON EA. FACE OF JOINT,SYMMETRICALLY(EXCEPT AS SHOWN)DESIGN CONFORMS W/NDS DESIGN SPECS, SSB,ANSI/TPI-95

THIS DESIGN IS FOR TRUSS FABRICATION ONLY.FOR PERMANENT AND TEMPORARY BRACING(WHICH IS ALWAYS REQD)CONSULT BLDG ARCHITECT OR ENGINEER.

MAR 06 2001

====<<<<ACES-32 Ver. 1.1>>>>===== [001910] =====<<<<FL.QUALITY-MMH>>>>=====

Customer : FOGLIA

Project #: 00469

Span : 1-9-5

Truss ID : CJ1A

Quantity : 2

Thu Mar 1 10:14:29 2001

Family # : 201

Top Pitch : 5.54/12

==Bld(1/21/2000),v1.1=====

FORCES - LOAD CASE #1

REACTIONS - SIZE

1-2= 0

3-1= 0

1=139

2.00

2-3=-16

3=-20

1.50

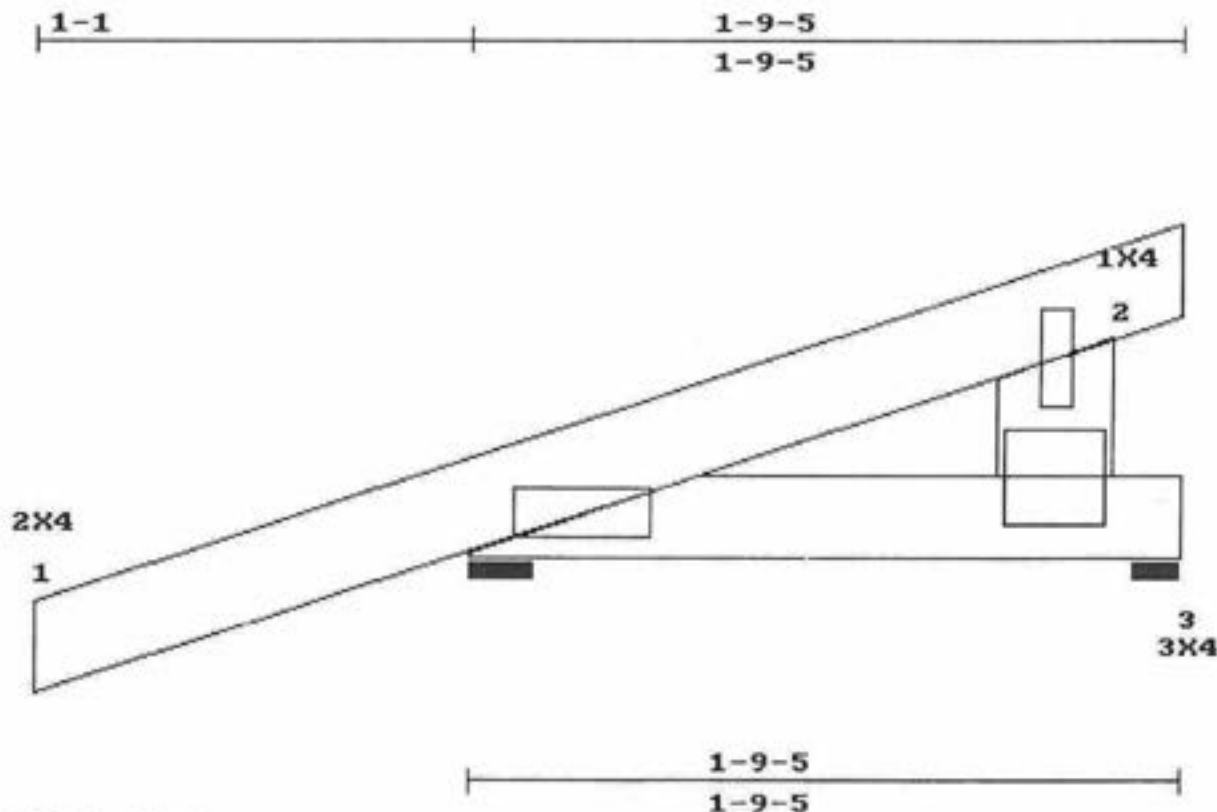
102

PROVIDE FOR 213 LBS UPLIFT AT JOINT 1 (1.53)

PROVIDE FOR 122 LBS UPLIFT AT JOINT 3 (6.07)

PROVIDE FOR 93 LBS HORIZ. REACTION AT JOINT 1

(SYMMETRIC PLATING TURNED ON)



L. HL TO PK:1-11-8

LEFT HEIGHT:0-4-3

SPAN:1-9-5

RISE:1-2

RIGHT HEIGHT:1-2

LOADING (PSF)

L D

TOP 30 15

BOTT 0 10

MAX STRESSES

TOP 1-2=0.075

BOTT 3-1=0.003

LL.DEFL. < L/360

MINIMUM GRADE OF LUMBER

TOP CHORD:2X4 No.2ND 19 SP

BOT CHORD:2X4 No.2ND 19 SP

WEBS :2X4 No.3 19 SP

REPETITIVE STRESSES NOT USED

SPACING : 24.0 in. o. c.

NO. OF MEMBERS = 1

LOADING STRESS INCREASE LOADING

LUMBER PLATE TYPE

1 1.33 1.33 UNIFORM

PANEL(PLF) / JOINTS(LBS)

1- 2= 20 3- 1= 4

RIGHT END VERTICAL IS NOT EXPOSED.

TRUSS HAS BEEN CHECKED FOR 145 M.P.H. WIND LOAD, WALL HEIGHT 16 FT, ENCLOSED BLDG. TYPE, ASSUMING 10 PSF TOTAL D.L.(7+3) (SBC-97).

DEFLECTION(IN.) L.L= 0.00,D.L=0.00,T.L=0.00

PLATES ARE MITEK M20-249,200 MANUFACTURED FROM ASTM A 446 GRD A GALVANIZED STEEL(EXCEPT AS SHOWN)

PLATE MUST BE INSTALLED ON EA. FACE OF JOINT,SYMMETRICALLY(EXCEPT AS SHOWN)DESIGN CONFORMS W/ND S DESIGN SPECS, S58C,ANSI/TPI-95

THIS DESIGN IS FOR TRUSS FABRICATION ONLY.FOR PERMANENT AND TEMPORARY BRACING(WHICH IS ALWAYS REQD)CONSULT BLDG ARCHITECT OR ENGINEER.

[Signature]

MAR 01 2001

====<<<<ACES-32 Ver. 1.1>>>>===== [001911] =====<<<<FL.QUALITY-MMH>>>>=====

Customer : FOGLIA

Project # : 00469

Span : 2-2-8

Truss ID : CC1

Quantity : 2

Thu Mar 1 10:14:41 2001

Family # : 205

Top Pitch : 6/12

==Bld(1/21/2000),v1.1=====

FORCES - GRAVITY LOADS

1-2= 0 3-1= 0
2-3= 0

REACTIONS - SIZE

1=223 2.00
3=21 1.50
2=93 1.50

103

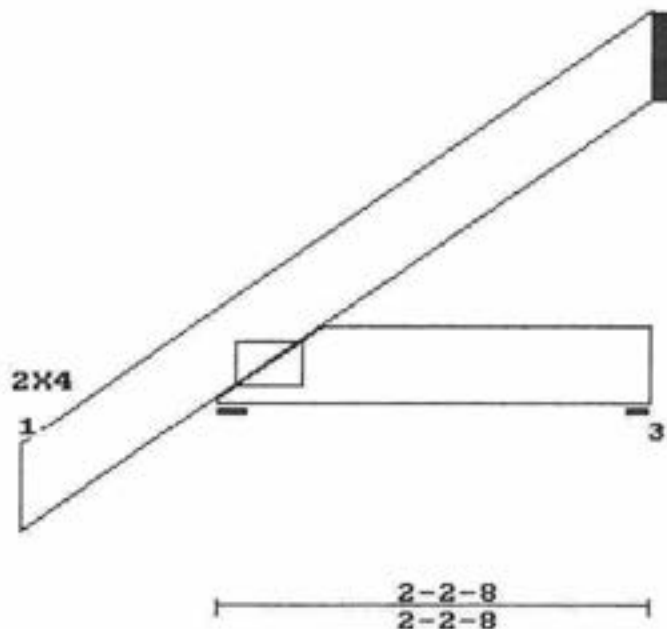
PROVIDE FOR 194 LBS UPLIFT AT JOINT 1 (0.87)

PROVIDE FOR 139 LBS UPLIFT AT JOINT 2 (1.50)

PROVIDE FOR 125 LBS HORIZ. REACTION AT JOINT 1

(SYMMETRIC PLATING TURNED ON)

1-0 2-2-8
2-2-8



L. HL TO PK:2-5-10
LEFT HEIGHT:0-4-3

SPAN:2-2-8

RISE:1-5-7

RIGHT HEIGHT:1-5-7

LOADING (PSF)

TOP 30 15

BOTT 0 10

MAX STRESSES

TOP 1-2=0.111

BOTT 3-1=0.017

LL.DEFL. < L/360

MINIMUM GRADE OF LUMBER

TOP CHORD:2X4 No.2ND 19 SP

BOT CHORD:2X4 No.2ND 19 SP

WEBS :2X4 No.3 19 SP

STR.INC.: LUMB = 1.33 PLATE = 1.33

REPETITIVE STRESSES USED

SPACING : 24.0 in. o. c.

NO. OF MEMBERS = 1

TRUSS HAS BEEN CHECKED FOR 145 M.P.H. WIND LOAD, WALL HEIGHT 16 FT, ENCLOSED BLDG. TYPE, ASSUMING 10 PSF TOTAL D.L.(7+3) (SBC-97).

DEFLECTION(IN.) L.L= 0.00,D.L=0.00,T.L=0.00

PLATES ARE MITEK M20-249,200 MANUFACTURED FROM ASTM A 446 GRD A GALVANIZED STEEL(EXCEPT AS SHOWN)

PLATE MUST BE INSTALLED ON EA. FACE OF JOINT,SYMMETRICALLY(EXCEPT AS SHOWN)DESIGN CONFORMS W/NDS DESIGN SPECS, SSBC,ANSI/TP1-95

THIS DESIGN IS FOR TRUSS FABRICATION ONLY.FOR PERMANENT AND TEMPORARY BRACING(WHICH IS ALWAYS REQD)CONSULT BLDG ARCHITECT OR ENGINEER.

[Signature]
MAR 01 2001

====<<<ACES-32 Ver. 1.1>>>====[001912]====<<<FL.QUALITY-MMH>>>=====

Customer : FOGLIA

Project #: 00469

Span : 4-1

Truss ID : CC3

Quantity : 2

Thu Mar 1 10:14:51 2001

Family # : 205

Top Pitch : 6/12

==Qd(1/21/2000),v1.1=====

FORCES - GRAVITY LOADS

1-2= 0

3-1= 0

2-3= 0

REACTIONS - SIZE

1=-327 2.00

3=-39 1.50

2=-177 1.50

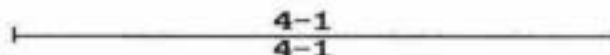
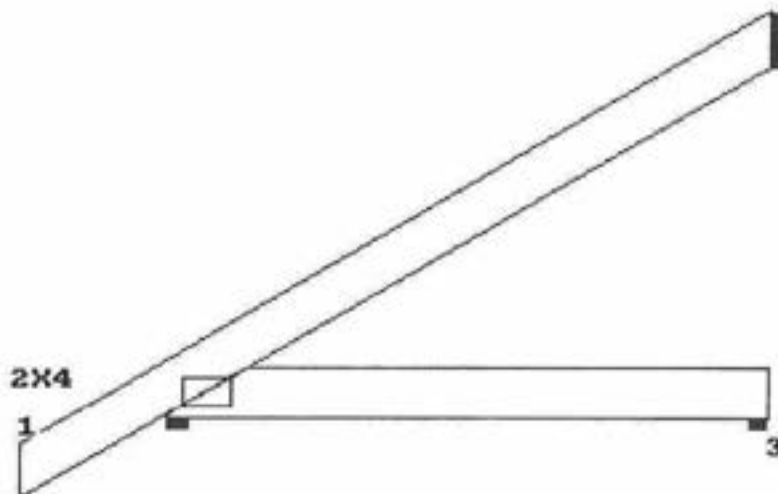
(109)

PROVIDE FOR 258 LBS UPLIFT AT JOINT 1 (0.79)

PROVIDE FOR 264 LBS UPLIFT AT JOINT 2 (1.49)

PROVIDE FOR 235 LBS HORIZ. REACTION AT JOINT 1

(SYMMETRIC PLATING TURNED ON)



L. HL TO PK:4-6-13

LEFT HEIGHT:0-4-3

SPAN:4-1

RISE:2-4-11

RIGHT HEIGHT:2-4-11

LOADING (PSF)

L D

TOP 30 15

BOTT 0 10

MAX STRESSES

TOP 1-2=0.449

BOTT 3-1=0.067

LL.DEFL. < L/360

MINIMUM GRADE OF LUMBER

TOP CHORD:2X4 No.2ND 19 SP

BOT CHORD:2X4 No.2ND 19 SP

WEBS :2X4 No.3 19 SP

STR.INC.: LUMB = 1.33 PLATE = 1.33

REPETITIVE STRESSES USED

SPACING : 24.0 in. o. c.

NO. OF MEMBERS = 1

TRUSS HAS BEEN CHECKED FOR 145 M.P.H. WIND LOAD, WALL HEIGHT 16 FT, ENCLOSED BLDG. TYPE, ASSUMING 10 PSF TOTAL D.L.(7+3) (SBC-97).

DEFLECTION(IN.) L.L= 0.00,D.L=0.00,T.L=0.00

PLATES ARE MITEK M20-249,200 MANUFACTURED FROM ASTM A 446 GRD A GALVANIZED STEEL(EXCEPT AS SHOWN)

PLATE MUST BE INSTALLED ON EA. FACE OF JOINT,SYMMETRICALLY(EXCEPT AS SHOWN)DESIGN CONFORMS W/NDG DESIGN SPECS, SSSC,ANSI/TPI-95

THIS DESIGN IS FOR TRUSS FABRICATION ONLY.FOR PERMANENT AND TEMPORARY BRACING(WHICH IS ALWAYS REQD)CONSULT BLDG ARCHITECT OR ENGINEER.

[Signature]

MAR 01 2001

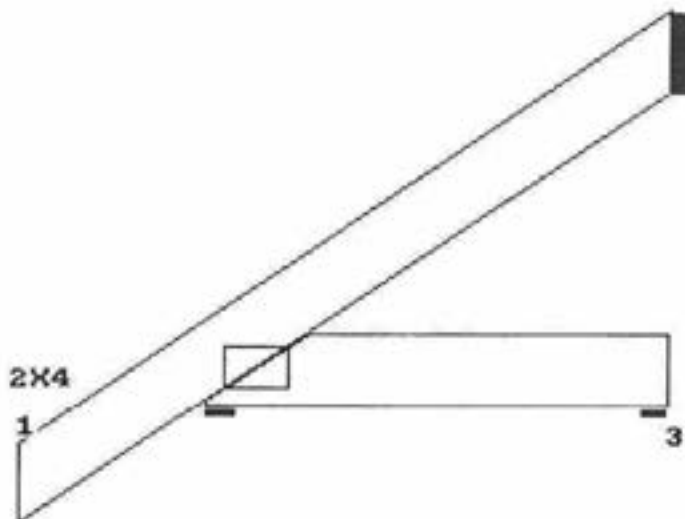
====<<<<ACES-32 Ver. 1.1>>>>=====[001913]====<<<<FL.QUALITY-MMH>>>>=====
 Customer : FOGLIA Thu Mar 1 10:15:00 2001
 Project #: 00469 Truss ID : C2 Family #: 205
 Span : 2-5-9 Quantity : 8 Top Pitch : 6/12
 ==Bld(1/21/2000),v1.1=====

FORCES - GRAVITY LOADS		REACTIONS - SIZE	
1-2= 0	3-1= 0	1=-237	2.00
2-3= 0		3=-23	1.50
		2=-104	1.50

105

PROVIDE FOR 203 LBS UPLIFT AT JOINT 1 (0.85)
 PROVIDE FOR 156 LBS UPLIFT AT JOINT 2 (1.50)
 PROVIDE FOR 140 LBS HORIZ. REACTION AT JOINT 1
 (SYMMETRIC PLATING TURNED ON)

1-0 2-5-9
 2-5-9



2-5-9
 2-5-9

L. HL TO PK:2-9-1
 LEFT HEIGHT:0-4-3 SPAN:2-5-9 RISE:1-6-15 RIGHT HEIGHT:1-6-15

LOADING (PSF)		MAX STRESSES		MINIMUM GRADE OF LUMBER	
L	D	TOP	1-2=0.144	TOP CHORD:2X4	No.2ND 19 SP
TOP	30	BOTT	3-1=0.022	BOT CHORD:2X4	No.2ND 19 SP
BOTT	0	LL.DEFL.	< L/360	WEBS	:2X4 No.3 19 SP

STR.INC.: LUMB = 1.33 PLATE = 1.33 SPACING : 24.0 in. o. c.
 REPETITIVE STRESSES USED NO. OF MEMBERS = 1

TRUSS HAS BEEN CHECKED FOR 145 M.P.H. WIND LOAD, WALL HEIGHT 16 FT, ENCLOSED BLDG. TYPE, ASSUMING 10 PSF TOTAL D.L.(7+3) (SBC-97).
 DEFLECTION(IN.) L.L= 0.00,D.L=0.00,T.L=0.00
 PLATES ARE MITEK M20-249,200 MANUFACTURED FROM ASTM A 446 GRD A GALVANIZED STEEL(EXCEPT AS SHOWN)
 PLATE MUST BE INSTALLED ON EA. FACE OF JOINT,SYMMETRICALLY(EXCEPT AS SHOWN)DESIGN CONFORMS W/NDS DESIGN SPECS, SIBC,ANSI/TPI-95
 THIS DESIGN IS FOR TRUSS FABRICATION ONLY.FOR PERMANENT AND TEMPORARY BRACING(WHICH IS ALWAYS REQD)CONSULT BLDG ARCHITECT OR ENGINEER.

[Signature]

MAR 01 2001

====<<<ACBS-32 Ver. 1.1>>>===== [001914] =====<<<FL.QUALITY-MMH>>>=====

Customer : FOGLIA

Project #: 00469

Span : 5-10

Truss ID : FG8

Quantity : 4

Thu Mar 1 10:15:21 2001

Family # : 301

Top Pitch : /12

==Bld(1/21/2000),v1.1=====

FORCES - LOAD CASE #1

1-2=-340 5-6= 0 3-1=-601
2-3= 0 6-1= 473 3-6=-311
3-4=-473 4-6= 599
4-5=-711

REACTIONS - SIZE

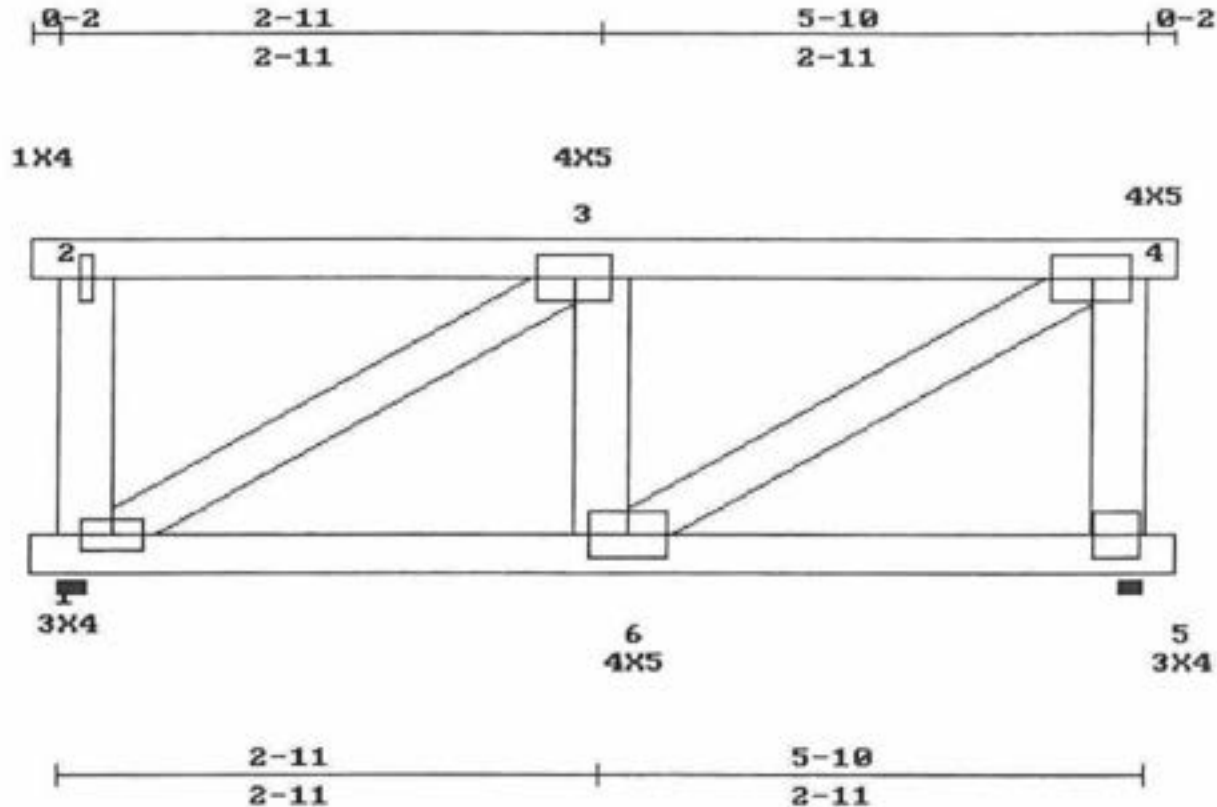
1=-739 2.00
5=-739 1.50

106

PROVIDE FOR 964 LBS UPLIFT AT JOINT 1 (1.30)

PROVIDE FOR 964 LBS UPLIFT AT JOINT 5 (1.30)

(SYMMETRIC PLATING TURNED ON)



LEFT HEIGHT:2-6-3 SPAN:5-10 RISE:2-6-3 RIGHT HEIGHT:2-6-3

LOADING (PSF)		MAX STRESSES		MINIMUM GRADE OF LUMBER	
L	D	TOP	BOTT	TOP CHORD	BOT CHORD
30	15	3-4=0.228	6-1=0.062	2X4 No.2ND 19 SP	2X4 No.2ND 19 SP
0	10	LL.DEFL.@6=0.00 < L/360		WEBS :2X4 No.3 19 SP	

REPETITIVE STRESSES NOT USED

SPACING : 24.0 in. o. c.
NO. OF MEMBERS = 2

LOADING	STRESS INCREASE	LOADING	PANEL(PLF) / JOINTS(LBS)
LUMBER	PLATE	TYPE	
1	1.33	1.33	UNIFORM 2- 4= 240 5- 1= 20

BOTH END VERTICALS ARE NOT EXPOSED.

2 MEMBERS NAILED TOG. W/1 ROW(S) OF .131x3 in. NAILS 11 in. o.c.(TOP CHS.),AND 1 ROW(S) OF .131x3 in. NAILS 12 in. o.c(BOTT. CHS.)

For Webs use 1 ROW of NAILS 12 in. o.c.

TRUSS HAS BEEN CHECKED FOR 145 M.P.H. WIND LOAD, WALL HEIGHT 16 FT, ENCLOSED BLDG. TYPE, ASSUMING 10 PSF TOTAL D.L.(7+3) (SBC-97).

DEFLECTION(IN.) L.L= 0.00,D.L=0.00,T.L=0.01

PLATES ARE MITEK M20-249,200 MANUFACTURED FROM ASTM A 446 GRD A GALVANIZED STEEL(EXCEPT AS SHOWN)

PLATE MUST BE INSTALLED ON EA. FACE OF JOINT,SYMMETRICALLY(EXCEPT AS SHOWN)DESIGN CONFORMS W/ND5 DESIGN SPECS, S5BC,ANSI/TPI-95

THIS DESIGN IS FOR TRUSS FABRICATION ONLY.FOR PERMANENT AND TEMPORARY BRACING(WHICH IS ALWAYS REQD)CONSULT BLDG ARCHITECT OR ENGINEER.

[Signature]
MAR 01 2001

====<<<<ACES-32 Ver. 1.1>>>>===== [001915] =====<<<<FL.QUALITY-MMH>>>>=====

Customer : FOGLIA Thu Mar 1 10:15:32 2001

Project #: 00469 Truss ID : CJ5A Family #: 201

Span : 5-4-9 Quantity : 2 Top Pitch : 5.54/12

==Bld(1/21/2000),v1.1=====

FORCES - LOAD CASE #1 REACTIONS - SIZE

1-2= 0 3-1= 0 1=-314 2.00

2-3=-159 3=-194 1.50

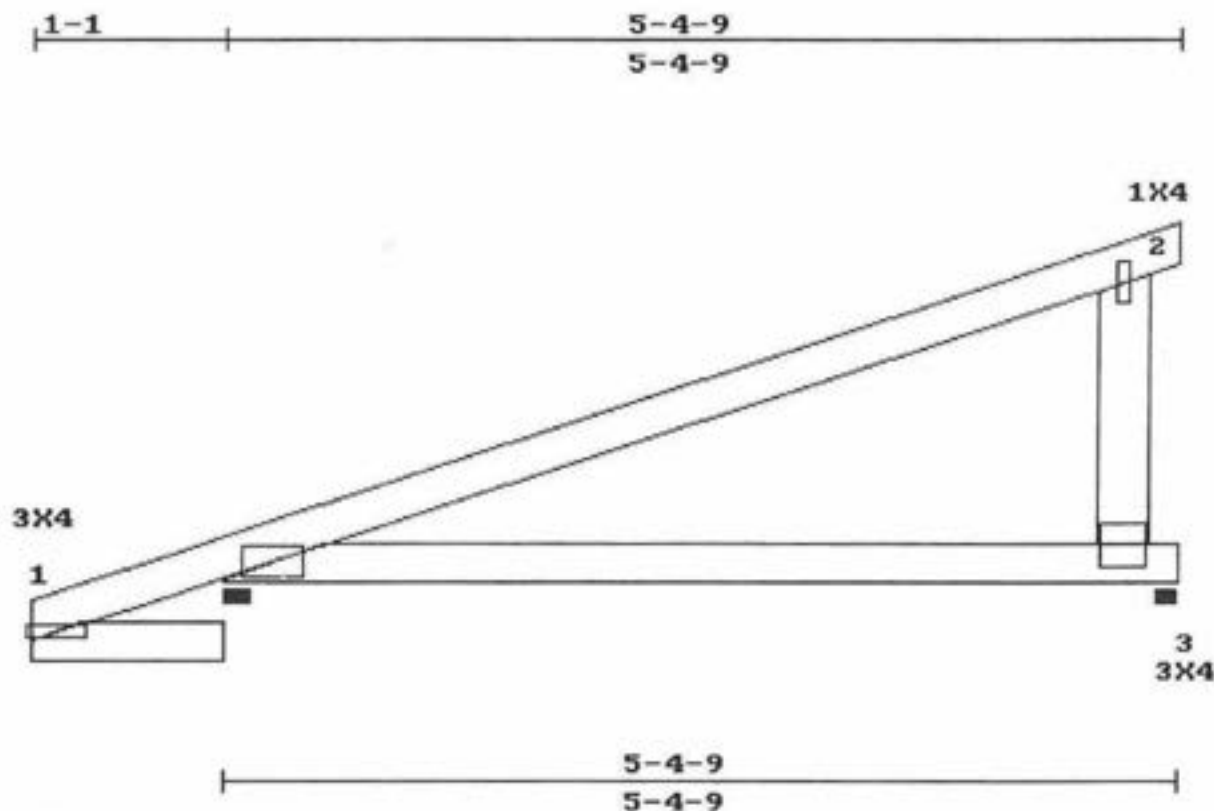
107

PROVIDE FOR 502 LBS UPLIFT AT JOINT 1 (1.60)

PROVIDE FOR 501 LBS UPLIFT AT JOINT 3 (2.58)

PROVIDE FOR 289 LBS HORIZ. REACTION AT JOINT 1

(SYMMETRIC PLATING TURNED ON)



L. HL TO PK:5-11-2 SPAN:5-4-9 RISE:2-10 RIGHT HEIGHT:2-10

LEFT HEIGHT:0-4-3

LOADING (PSF) MAX STRESSES MINIMUM GRADE OF LUMBER

TOP 30 15 TOP CHORD:2X4 No.2ND 19 SP

BOTT 0 10 BOT CHORD:2X4 No.2ND 19 SP

LL.DEFL. < L/360 WEBS :2X4 No.3 19 SP

REPETITIVE STRESSES NOT USED

SPACING : 24.0 in. o. c.

NO. OF MEMBERS = 1

LOADING	STRESS INCREASE	LOADING	PANEL(PLF) / JOINTS(LBS)
LUMBER	PLATE	TYPE	
1	1.33	1.33	UNIFORM 1- 2= 61 3- 1= 14

RIGHT END VERTICAL IS NOT EXPOSED.

TRUSS HAS BEEN CHECKED FOR 145 M.P.H. WIND LOAD, WALL HEIGHT 16 FT, ENCLOSED BLDG. TYPE, ASSUMING 10 PSF TOTAL D.L.(7+3) (SBC-97).

DEFLECTION(IN.) L.L= 0.00,D.L=0.00,T.L=0.00

PLATES ARE MITEK M20-249,200 MANUFACTURED FROM ASTM A 446 GRD A GALVANIZED STEEL(EXCEPT AS SHOWN)

PLATE MUST BE INSTALLED ON EA. FACE OF JOINT,SYMMETRICALLY(EXCEPT AS SHOWN)DESIGN CONFORMS W/MDS DESIGN SPECS, SSSC,ANSI/TPI-95

THIS DESIGN IS FOR TRUSS FABRICATION ONLY.FOR PERMANENT AND TEMPORARY BRACING(WHICH IS ALWAYS REQD)CONSULT BLDG ARCHITECT OR ENGINEER.

[Signature]

MAR 01 2001

====<<<ACES-32 Ver. 1.1>>>===== [001916] =====<<<FL.QUALITY-MMH>>>=====

Customer : FOGLIA

Project #: 00469

Span : 5-0

Truss ID : CC5

Quantity : 1

Thu Mar 1 10:15:42 2001

Family # : 205

Top Pitch : 6/12

==sld(1/21/2000),v1.1=====

FORCES - GRAVITY LOADS

1-2= 0 3-1= 0

2-3= 0

REACTIONS - SIZE

1=-363 8.00

3=-46 1.50

2=-207 1.50

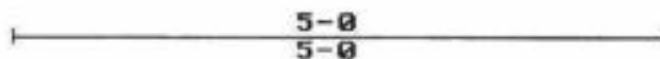
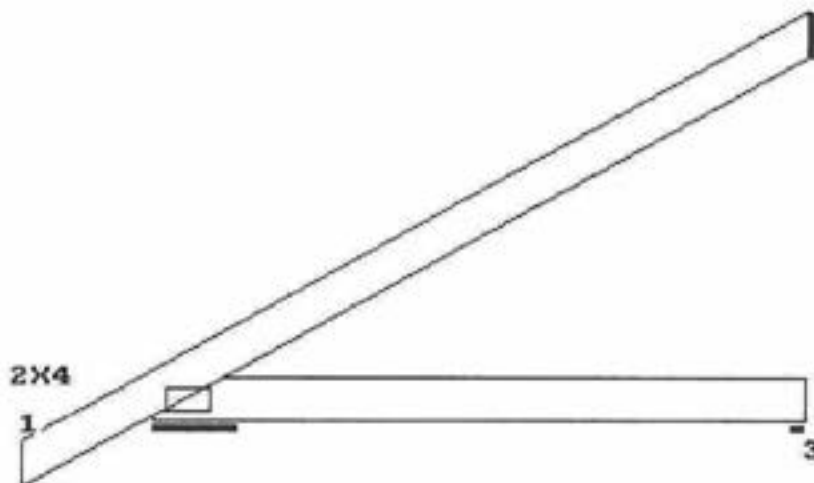
(108)

PROVIDE FOR 273 LBS UPLIFT AT JOINT 1 (0.75)

PROVIDE FOR 316 LBS UPLIFT AT JOINT 2 (1.53)

PROVIDE FOR 289 LBS HORIZ. REACTION AT JOINT 1

(SYMMETRIC PLATING TURNED ON)



L. HL TO PK:5-7-1

LEFT HEIGHT:0-4-3

SPAN:5-0

RISE:2-10-3

RIGHT HEIGHT:2-10-3

LOADING (PSF)

L D

TOP 30 15

BOTT 0 10

MAX STRESSES

TOP 1-2=0.702

BOTT 3-1=0.103

LL.DEFL. < L/360

MINIMUM GRADE OF LUMBER

TOP CHORD:2X4 No.2ND 19 SP

BOT CHORD:2X4 No.2ND 19 SP

WEBS :2X4 No.3 19 SP

STR.INC.: LUMB = 1.33 PLATE = 1.33

REPETITIVE STRESSES USED

SPACING : 24.0 in. o. c.

NO. OF MEMBERS = 1

TRUSS HAS BEEN CHECKED FOR 145 M.P.H. WIND LOAD, WALL HEIGHT 16 FT, ENCLOSED BLDG. TYPE, ASSUMING 10 PSF TOTAL D.L.(7+3) (SBC-97).

DEFLECTION(IN.) L.L= 0.00,D.L=0.00,T.L=0.00

PLATES ARE MITEK M20-249,200 MANUFACTURED FROM ASTM A 446 GRD A GALVANIZED STEEL(EXCEPT AS SHOWN)

PLATE MUST BE INSTALLED ON EA. FACE OF JOINT,SYMMETRICALLY(EXCEPT AS SHOWN)DESIGN CONFORMS W/ND5 DESIGN SPECS, SSBC,ANSI/TPI-95

THIS DESIGN IS FOR TRUSS FABRICATION ONLY.FOR PERMANENT AND TEMPORARY BRACING(WHICH IS ALWAYS REQD)CONSULT BLDG ARCHITECT OR ENGINEER.

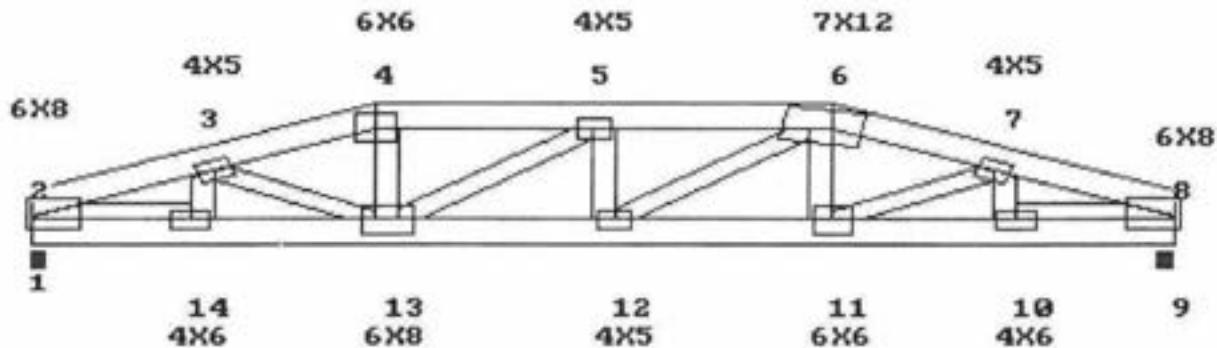
[Signature]
MAR 01 2001

====<<<<ACES-32 Ver. 1.1>>>>=====[002007]====<<<<FL.QUALITY-MMH>>>>=====
 Customer : FOGLIA Wed Mar 7 07:16:10 2001
 Project #: 00469 Truss ID : GT14 Family #: 314
 Span : 14-5 B.Span : 14-5 Quant. : 2 Top Pitch : 4.24/
 ==8(d(1/21/2000),v1.1=====

FORCES - LOAD CASE #1				REACTIONS - SIZE	
1-2=-3727	7-8=-5826	9-10= 0	2-14= 5675	6-12= 909	1=-3759 2.68
2-3=-5787	8-9=-3723	10-11= 5517	3-14=-1431	6-11= 621	9=-3756 2.75
3-4=-6505		11-12= 6137	3-13= 752	7-11= 720	
4-5=-6121		12-13= 6876	4-13= 1145	7-10=-1427	
5-6=-6876		13-14= 5474	5-13=-928	8-10= 5716	
6-7=-6522		14-1= 0	5-12=-440		

PROVIDE FOR 5310 LBS UPLIFT AT JOINT 1 (1.41)
 PROVIDE FOR 5453 LBS UPLIFT AT JOINT 9 (1.45)
 PROVIDE FOR 50 LBS HORIZ. REACTION AT JOINT 1
 PLATE OFFSETS (X=LEFT,Y=TOP):[J2=4,2.5],[J6=6,4],[J8=4,2.5],

2-1-15	4-3-15	7-2-8	10-1-1	12-3	14-5
2-1-15	2-1-15	2-10-9	2-10-9	2-1-15	2-1-15



2-1-15	4-3-15	7-2-8	10-1-1	12-3	14-5
2-1-15	2-1-15	2-10-9	2-10-9	2-1-15	2-1-15

L. HL TO PK:4-7-1 LEFT HEIGHT:0-11-14 SPAN:14-5 RISE:2-6-4 R. HL TO PK:4-7-1 RIGHT HEIGHT:0-11-14

LOADING (PSF)		MAX STRESSES		MINIMUM GRADE OF LUMBER	
TOP	30	TOP	5-6=0.666	TOP CHORD:2X6	No.2D 19 SP
BOTT	0	BOTT	12-13=0.418	BOT CHORD:2X6	No.2D 19 SP
		LL.DEFL.@12=0.08 < L/360	WEBS	:2X4	No.3 19 SP

REPETITIVE STRESSES NOT USED

SPACING : 24.0 in. o. c.
 NO. OF MEMBERS = 2

LOADING STRESS INCREASE	LOADING LUMBER PLATE TYPE	PANEL(PLF) / JOINTS(LBS)
1	1.33 1.33	UNIFORM 2- 8= 340 9- 1= 31
		CONCENTRATED 4= 200 6= 200 11= 910 13= 910

BOTH END VERTICALS ARE NOT EXPOSED.
 2 MEMBERS NAILED TOG. W/1 ROW(S) OF .131x3 in. NAILS 7 in. o.c.(TOP CHS.),AND 1 ROW(S) OF .131x3 in. NAILS 6 in. o.c(BOTT. CHS.)
 For Webs use 1 ROW of NAILS 12 in. o.c.
 TRUSS HAS BEEN CHECKED FOR 145 M.P.H. WIND LOAD, WALL HEIGHT 16 FT, ENCLOSED BLDG. TYPE, ASSUMING 10 PSF TOTAL D.L.(7+3) (SBC-97).
 DEFLECTION(IN.) L.L= 0.08,D.L=0.07,T.L=0.15
 PLATES ARE MITEK M20-249,200 MANUFACTURED FROM ASTM A 446 GRD A GALVANIZED STEEL(EXCEPT AS SHOWN)
 PLATE MUST BE INSTALLED ON EA. FACE OF JOINT,SYMMETRICALLY(EXCEPT AS SHOWN)DESIGN CONFORMS W/NDOS DESIGN SPECS, SSSC,ANSI/TPI-95
 THIS DESIGN IS FOR TRUSS FABRICATION ONLY.FOR PERMANENT AND TEMPORARY BRACING(WHICH IS ALWAYS REQD)CONSULT BLDG ARCHITECT OR ENGINEER.

MAR 06 2001

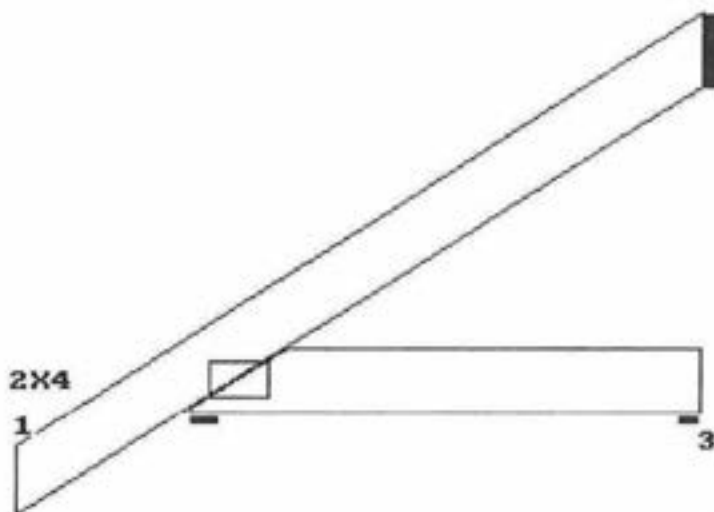
====<<<ACES-32 Ver. 1.1>>>=====[001918]====<<<FL.QUALITY-MMH>>>=====
 Customer : FOGLIA Thu Mar 1 10:16:36 2001
 Project #: 00469 Truss ID : CC2 Family #: 205
 Span : 2-11 Quantity : 2 Top Pitch : 6/12
 ==Rld(1/21/2000),v1.1=====

FORCES - GRAVITY LOADS		REACTIONS - SIZE	
1-2= 0	3-1= 0	1=-262	2.00
2-3= 0		3=-28	1.50
		2=-125	1.50

(110)

PROVIDE FOR 218 LBS UPLIFT AT JOINT 1 (0.83)
 PROVIDE FOR 187 LBS UPLIFT AT JOINT 2 (1.50)
 PROVIDE FOR 167 LBS HORIZ. REACTION AT JOINT 1
 (SYMMETRIC PLATING TURNED ON)

1-0 2-11
2-11



2-11
2-11

L. HL TO PK:3-3-2
 LEFT HEIGHT:0-4-3 SPAN:2-11 RISE:1-9-11 RIGHT HEIGHT:1-9-11

LOADING (PSF)		MAX STRESSES		MINIMUM GRADE OF LUMBER	
	L D	TOP	1-2=0.211	TOP CHORD:2X4	No.2ND 19 SP
TOP	30 15	BOTT	3-1=0.032	BOT CHORD:2X4	No.2ND 19 SP
BOTT	0 10	LL.DEFL.	< L/360	WEBS	:2X4 No.3 19 SP
STR.INC.: LUMB = 1.33 PLATE = 1.33				SPACING : 24.0 in. o. c.	
REPETITIVE STRESSES USED				NO. OF MEMBERS = 1	

TRUSS HAS BEEN CHECKED FOR 145 M.P.H. WIND LOAD, WALL HEIGHT 16 FT, ENCLOSED BLDG. TYPE, ASSUMING 10 PSF TOTAL D.L.(7+3) (SBC-97).
 DEFLECTION(IN.) L.L= 0.00,D.L=0.00,T.L=0.00
 PLATES ARE MITEK M20-249,200 MANUFACTURED FROM ASTM A 446 GRD A GALVANIZED STEEL(EXCEPT AS SHOWN)
 PLATE MUST BE INSTALLED ON EA. FACE OF JOINT,SYMMETRICALLY(EXCEPT AS SHOWN)DESIGN CONFORMS W/ND S DESIGN SPECS, SSBC,ANSI/TP1-95
 THIS DESIGN IS FOR TRUSS FABRICATION ONLY.FOR PERMANENT AND TEMPORARY BRACING(WHICH IS ALWAYS REQD)CONSULT BLDG ARCHITECT OR ENGINEER.

[Signature]

MAR 01 2001

====<<<<ACES-32 Ver. 1.1>>>>=====[001919]=====<<<<FL.QUALITY-MMH>>>>=====

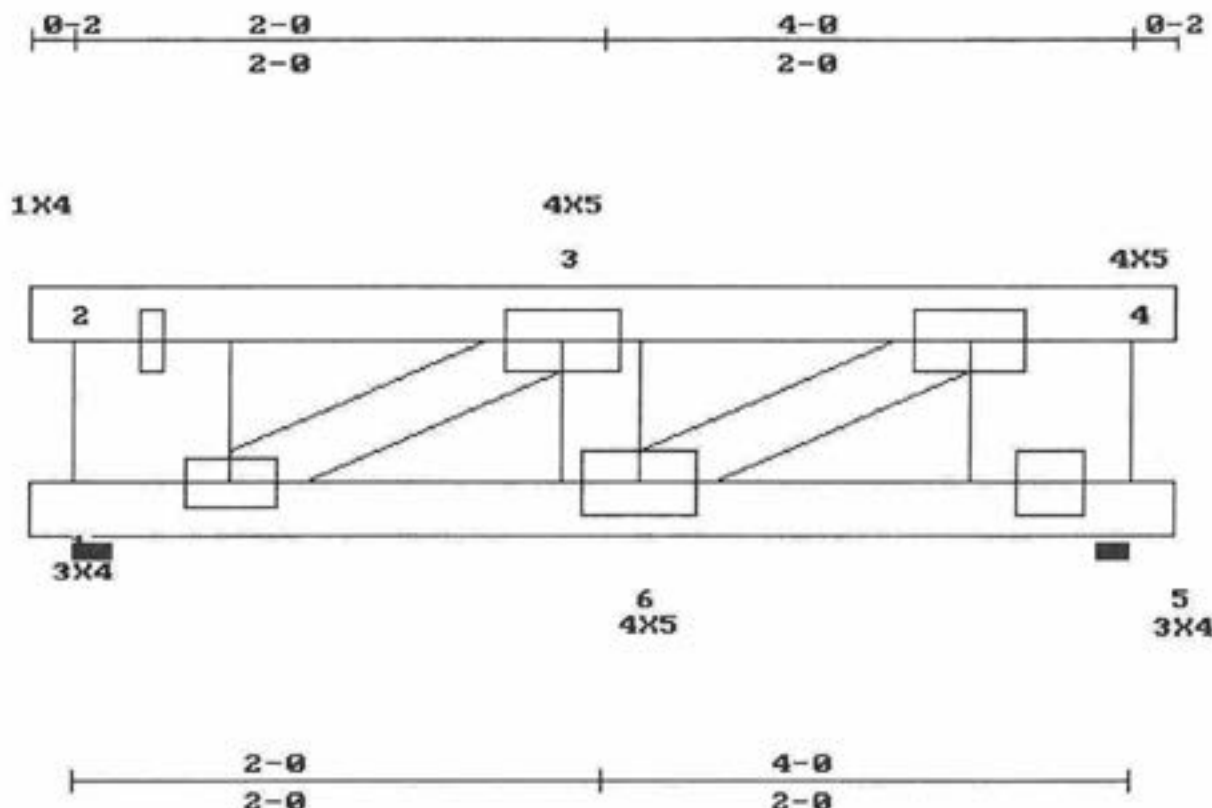
Customer : FOGLIA Truss ID : FG5 Thu Mar 1 10:16:50 2001
 Project # : 00469 Quantity : 4 Family # : 301
 Span : 4-0 Top Pitch : /12

==Bld(1/21/2000),v1.1=====

FORCES - LOAD CASE #1			REACTIONS - SIZE	
1-2=-326	5-6= 0	3-1=-730	1=-694	2.00
2-3= 0	6-1= 642	3-6=-306	5=-694	1.50
3-4=-642		4-6= 729		
4-5=-674				

(11)

PROVIDE FOR 815 LBS UPLIFT AT JOINT 1 (1.17)
 PROVIDE FOR 852 LBS UPLIFT AT JOINT 5 (1.23)
 PROVIDE FOR 139 LBS HORIZ. REACTION AT JOINT 1
 (SYMMETRIC PLATING TURNED ON)



LEFT HEIGHT:1-4		SPAN:4-0		RISE:1-4		RIGHT HEIGHT:1-4	
LOADING (PSF)				MAX STRESSES			
TOP	30	15		TOP	3-4=0.164	MINIMUM GRADE OF LUMBER	
BOTT	0	10		BOTT	6-1=0.067	TOP CHORD:2X4 No.2ND 19 SP	
				LL.DEFL.@6=0.00 < L/360		BOT CHORD:2X4 No.2ND 19 SP	
						WEBS :2X4 No.3 19 SP	
REPETITIVE STRESSES NOT USED				SPACING : 24.0 in. o. c.			
				NO. OF MEMBERS = 2			

LOADING	STRESS INCREASE	LOADING	PANEL(PLF) / JOINTS(LBS)
LUMBER	PLATE	TYPE	
1	1.33	1.33	2- 4= 340 5- 1= 20

WEB: 1-2; 4-5 TO BE 2X8 SS 19 SP

BOTH END VERTICALS ARE EXPOSED.

2 MEMBERS NAILED TOG. W/1 ROW(S) OF .131x3 in. NAILS 12 in. o.c.(BOTT. CHS.),AND 16d NAILS 12 in. o.c.(TOP CHORD)

For Webs use 1 ROW of NAILS 12 in. o.c.

****NOTE:Special hanger/connection is reqd. to dist. top chord uniform load(s) to all plies.****

TRUSS HAS BEEN CHECKED FOR 145 M.P.H. WIND LOAD, WALL HEIGHT 16 FT, ENCLOSED BLDG. TYPE, ASSUMING 10 PSF TOTAL D.L.(7+3) (SBC-97).

DEFLECTION(IN.) L.L= 0.00,D.L=0.00,T.L=0.01

PLATES ARE MITEK M20-249,200 MANUFACTURED FROM ASTM A 446 GRD A GALVANIZED STEEL(EXCEPT AS SHOWN)

PLATE MUST BE INSTALLED ON EA. FACE OF JOINT,SYMMETRICALLY(EXCEPT AS SHOWN)DESIGN CONFORMS W/NDG DESIGN SPECS, SBC,ANSI/TP1-95

THIS DESIGN IS FOR TRUSS FABRICATION ONLY.FOR PERMANENT AND TEMPORARY BRACING(WHICH IS ALWAYS REQD)CONSULT BLDG ARCHITECT OR ENGINEER.

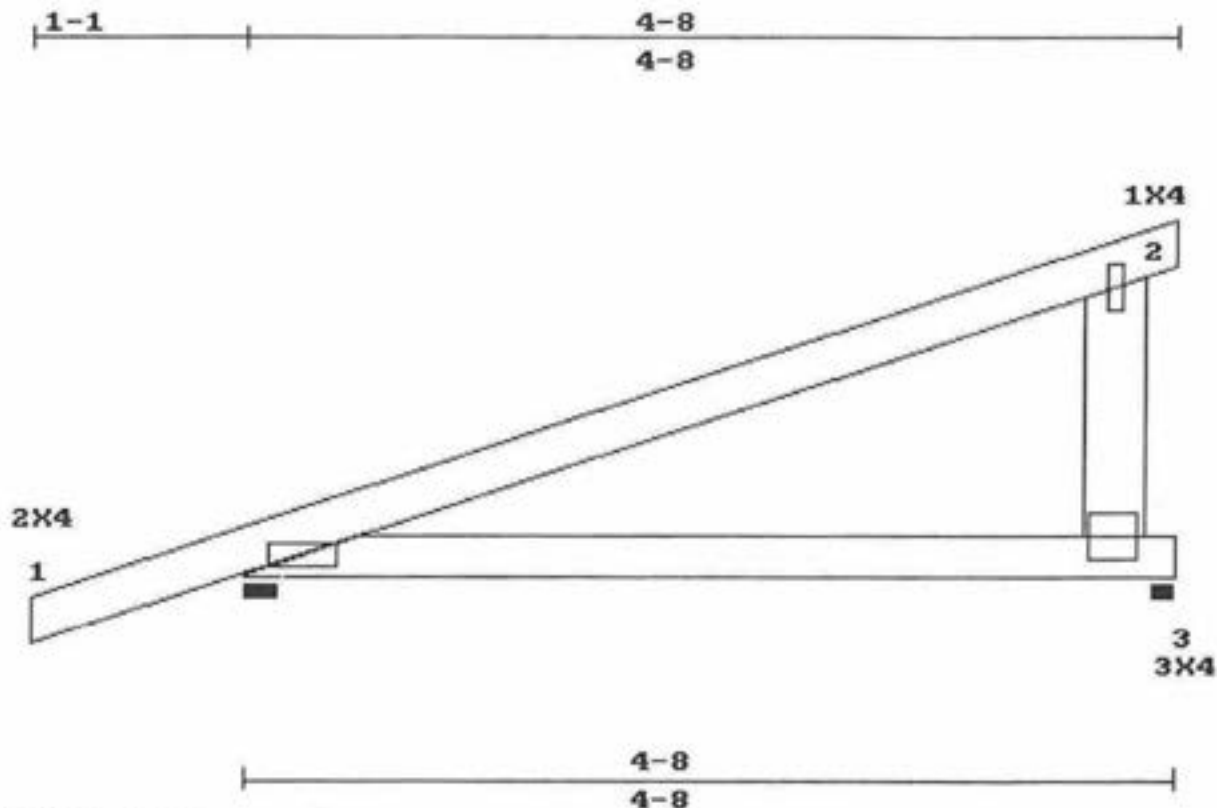
[Signature]
 MAR 01 2001

====<<<ACES-32 Ver. 1.1>>>=====[001920]====<<<FL.QUALITY-MMH>>>=====
 Customer : FOGLIA Thu Mar 1 10:17:01 2001
 Project #: 00469 Truss ID : CJ4 Family # : 201
 Span : 4-8 Quantity : 2 Top Pitch : 5.54/12
 ==81d(1/21/2000),v1.1=====

FORCES - LOAD CASE #1		REACTIONS - SIZE
1-2= 0	3-1= 0	1=-265 2.00
2-3=-119		3=-146 1.50

(12)

PROVIDE FOR 432 LBS UPLIFT AT JOINT 1 (1.63)
 PROVIDE FOR 414 LBS UPLIFT AT JOINT 3 (2.84)
 PROVIDE FOR 250 LBS HORIZ. REACTION AT JOINT 1
 (SYMMETRIC PLATING TURNED ON)



L. HL TO PK:5-1-11		SPAN:4-8		RISE:2-6-1		RIGHT HEIGHT:2-6-1	
LEFT HEIGHT:0-4-3							
LOADING (PSF)		MAX STRESSES		MINIMUM GRADE OF LUMBER			
	L D	TOP	1-2=0.507	TOP CHORD:2X4	No.2ND	19	SP
TOP	30 15	BOTT	3-1=0.060	BOT CHORD:2X4	No.2ND	19	SP
BOTT	0 10	LL.DEFL.	< L/360	WEBS	:2X4	No.3	19 SP

REPETITIVE STRESSES NOT USED
 SPACING : 24.0 in. o. c.
 NO. OF MEMBERS = 1

LOADING	STRESS INCREASE	LOADING	PANEL(PLF) / JOINTS(LBS)
LUMBER	PLATE	TYPE	
1	1.33	1.33	UNIFORM 1- 2= 53 3- 1= 12

RIGHT END VERTICAL IS NOT EXPOSED.
 TRUSS HAS BEEN CHECKED FOR 145 M.P.H. WIND LOAD, WALL HEIGHT 16 FT, ENCLOSED BLDG. TYPE, ASSUMING 10 PSF TOTAL D.L.(7+3) (SBC-97).
 DEFLECTION(IN.) L.L= 0.00,D.L=0.00,T.L=0.00
 PLATES ARE MITEK M20-249,200 MANUFACTURED FROM ASTM A 446 GRD A GALVANIZED STEEL(EXCEPT AS SHOWN)
 PLATE MUST BE INSTALLED ON EA. FACE OF JOINT,SYMMETRICALLY(EXCEPT AS SHOWN)DESIGN CONFORMS W/NDS DESIGN SPECS, SSB,ANSI/TP1-95
 THIS DESIGN IS FOR TRUSS FABRICATION ONLY.FOR PERMANENT AND TEMPORARY BRACING(WHICH IS ALWAYS REQD)CONSULT BLDG ARCHITECT OR ENGINEER.

[Signature]
 MAR 01 2001

====<<<ACES-32 Ver. 1.1>>>=====[001921]====<<<FL.QUALITY-MMH>>>=====
 Customer : FOGLIA Thu Mar 1 10:17:13 2001
 Project #: 00469 Truss ID : CD4 Family # : 205
 Span : 4-4 Quantity : 3 Top Pitch : 6/12

==Bld(1/21/2000),v1.1=====

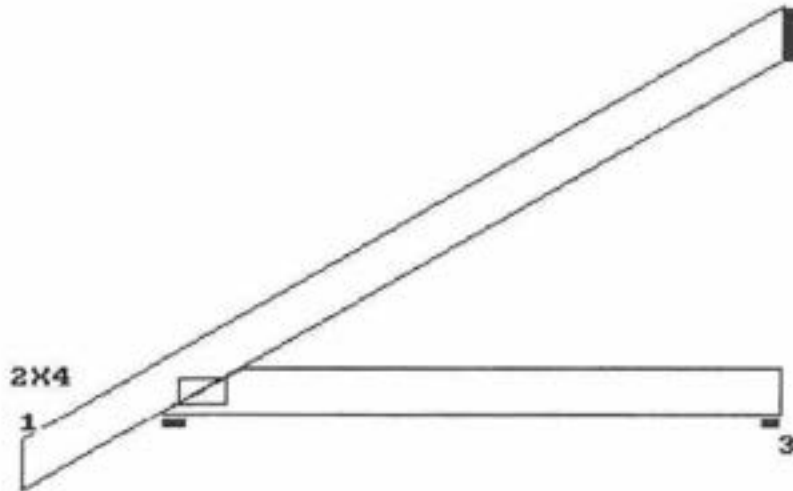
FORCES - GRAVITY LOADS
 1-2= 0 3-1= 0
 2-3= 0

REACTIONS - SIZE
 1=340 2.00
 3=42 1.50
 2=188 1.50

113

PROVIDE FOR 267 LBS UPLIFT AT JOINT 1 (0.78)
 PROVIDE FOR 281 LBS UPLIFT AT JOINT 2 (1.49)
 PROVIDE FOR 250 LBS HORIZ. REACTION AT JOINT 1
 (SYMMETRIC PLATING TURNED ON)

1-0 4-4
 4-4



4-4
 4-4

L. HL TO PK:4-10-2
 LEFT HEIGHT:0-4-3

SPAN:4-4

RISE:2-6-3

RIGHT HEIGHT:2-6-3

LOADING (PSF)

MAX STRESSES

MINIMUM GRADE OF LUMBER

TOP 30 15

TOP 1-2=0.512

TOP CHORD:2X4 No.2ND 19 SP

BOTT 0 10

BOTT 3-1=0.076

BOT CHORD:2X4 No.2ND 19 SP

LL.DEFL. < L/360

WEBS :2X4 No.3 19 SP

STR.INC.: LUMB = 1.33 PLATE = 1.33

SPACING : 24.0 in. o. c.

REPETITIVE STRESSES USED

NO. OF MEMBERS = 1

TRUSS HAS BEEN CHECKED FOR 145 M.P.H. WIND LOAD, WALL HEIGHT 16 FT, ENCLOSED BLDG. TYPE, ASSUMING 10 PSF TOTAL D.L.(7+3) (SBC-97).

DEFLECTION(IN.) L.L= 0.00,D.L=0.00,T.L=0.00

PLATES ARE MITEK M20-249,200 MANUFACTURED FROM ASTM A 446 GRD A GALVANIZED STEEL(EXCEPT AS SHOWN)

PLATE MUST BE INSTALLED ON EA. FACE OF JOINT,SYMMETRICALLY(EXCEPT AS SHOWN)DESIGN CONFORMS W/NDS DESIGN SPECS, SSB,ANSI/TP1-95

THIS DESIGN IS FOR TRUSS FABRICATION ONLY.FOR PERMANENT AND TEMPORARY BRACING(WHICH IS ALWAYS REQD)CONSULT BLDG ARCHITECT OR ENGINEER.

[Signature]

MAR 01 2001

====<<<ACES-32 Ver. 1.1>>>=====[001922]=====<<<FL.QUALITY-MMH>>>=====

Customer : FOGLIA Truss ID : FG4 Thu Mar 1 10:17:26 2001

Project # : 00469 Quantity : 2 Family # : 301

Span : 8-4 Top Pitch : /12

== ald(1/21,2000),v1.1 ==

FORCES - LOAD CASE #1			REACTIONS - SIZE	
1-2=-525	6-7= 0	3-1=-3123	1=-1678	2.00
2-3= 0	7-8= 2924	3-8= 59	6=-1678	1.50
3-4=-2913	8-1= 2913	4-8=-12		
4-5=-2924		4-7=-1067		
5-6=-1651		5-7= 3132		

114

PROVIDE FOR 1922 LBS UPLIFT AT JOINT 1 (1.15)

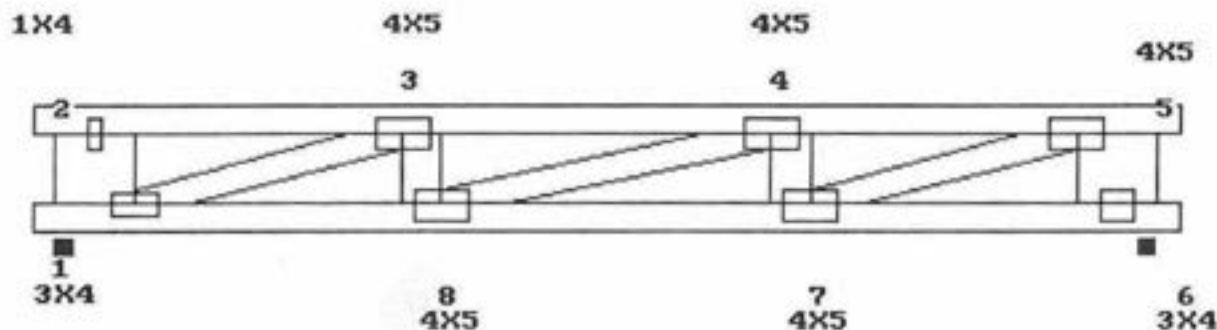
PROVIDE FOR 1940 LBS UPLIFT AT JOINT 6 (1.16)

PROVIDE FOR 139 LBS HORIZ. REACTION AT JOINT 1

(SYMMETRIC PLATING TURNED ON)

0-2 2-9-5 5-6-11 8-4 0-2

 2-9-5 2-9-5 2-9-5



2-9-5 5-6-11 8-4

2-9-5 2-9-5 2-9-5

LEFT HEIGHT:1-4 SPAN:8-4 RISE:1-4 RIGHT HEIGHT:1-4

LOADING (PSF)			MAX STRESSES		MINIMUM GRADE OF LUMBER	
TOP	30	15	TOP	3-4=0.538	TOP CHORD:2X4	No.2ND 19 SP
BOTT	0	10	BOTT	7-8=0.289	BOT CHORD:2X4	No.2ND 19 SP
			LL.DEFL.@8=0.03 < L/360		WEBS	:2X4 No.3 19 SP

REPETITIVE STRESSES NOT USED

SPACING : 24.0 in. o. c.

NO. OF MEMBERS = 2

LOADING	STRESS INCREASE	LOADING	PANEL(PLF) / JOINTS(LBS)
LUMBER	PLATE	TYPE	
1	1.33	1.33	UNIFORM 2- 5= 390 6- 1= 20

WEB: 1-2; 5-6 TO BE 2X8 SS 19 SP

BOTH END VERTICALS ARE EXPOSED.

2 MEMBERS NAILED TOG. W/1 ROW(S) OF .131x3 in. NAILS 12 in. o.c.(BOTT. CHS.),AND 16d NAILS 12 in. o.c.(TOP CHORD)

For Webs use 1 ROW of NAILS 12 in. o.c.

****NOTE:Special hanger/connection is reqd. to dist. top chord uniform load(s) to all plies.****

TRUSS HAS BEEN CHECKED FOR 145 M.P.H. WIND LOAD, WALL HEIGHT 16 FT, ENCLOSED BLDG. TYPE, ASSUMING 10 PSF TOTAL D.L.(7+3) (SBC-97).

DEFLECTION(IN.) L.L= 0.03,D.L=0.03,T.L=0.06

PLATES ARE MITEK M20-249,200 MANUFACTURED FROM ASTM A 446 GRD A GALVANIZED STEEL(EXCEPT AS SHOWN)

PLATE MUST BE INSTALLED ON EA. FACE OF JOINT,SYMMETRICALLY(EXCEPT AS SHOWN)DESIGN CONFORMS W/NDOS DESIGN SPECS, SSBC,ANSI/TPI-95

THIS DESIGN IS FOR TRUSS FABRICATION ONLY.FOR PERMANENT AND TEMPORARY BRACING(WHICH IS ALWAYS REQD)CONSULT BLDG ARCHITECT OR ENGINEER.

[Signature]

MAR 01 2001

BUILDING CODE COMPLIANCE OFFICE
METRO-DADE FLAGLER BUILDING
140 WEST FLAGLER STREET, SUITE 1603
MIAMI, FLORIDA 33130-1583
(305) 375-2901
FAX (305) 375-2908

PRODUCT CONTROL DIVISION
(305) 375-2902
FAX (305) 372-8339

PRODUCT CONTROL NOTICE OF ACCEPTANCE

Southeastern Metals Manufacturing Co., Inc.
11801 Industry Drive
Jacksonville, FL 32226

Prod. examine

Product application for Product Approval of:

→ "5-V Crimp" Metal Roofing Panels

under Chapter 8 of the Miami-Dade County Code governing the use of Alternate Materials and Types of Construction, and completely described in the plans, specifications and calculations as submitted by: Construction Research Laboratory, Inc. and Hurricane Test Laboratory, Inc. has been recommended for acceptance by the Building Code Compliance Office to be used in Dade County, Florida under the specific conditions set forth on pages 2-4 and the standard conditions on page 5.

This approval shall not be valid after the expiration date stated below. The Building Code Compliance Office reserves the right to secure this product or material at any time from a jobsite or manufacturer's plant for quality control testing. If this product or material fails to perform in the approved manner, the Building Code Compliance Office may revoke, modify, or suspend the use of such product or material immediately. The Building Code Compliance Office reserves the right to revoke this approval, if it is determined by the Building Code Compliance Office that this product or material fails to meet the requirements of the South Florida Building Code.

The expense of such testing will be incurred by the manufacturer.

→ ACCEPTANCE NO.: 98-0429.09 Renews & Revises: 97-0404.05
EXPIRES: 06/23/01

Raul Rodriguez
Raul Rodriguez
Product Control Supervisor

THIS IS THE COVERSHEET, SEE ADDITIONAL PAGES FOR SPECIFIC AND GENERAL CONDITIONS BUILDING CODE COMMITTEE

This application for Product Approval has been reviewed by the Miami-Dade County Building Code Compliance Office and approved by the Building Code Committee to be used in Dade County, Florida under the conditions set forth above.

*6/11/01 TOWN OF SEWALL'S POINT
REVIEW*

BLOG OFFICIAL

*TOWN COPY
102 ABKIE CT.*

APPROVED: 06/23/98

FILE

Charles Danger
Charles Danger, P.E.
Director
Building Code Compliance Dept.
Miami-Dade County

PN 5228

(RPG. SUB 5250)



BUILDING CODE COMPLIANCE OFFICE
METRO-DADE FLAGLER BUILDING
140 WEST FLAGLER STREET, SUITE 1803
MIAMI, FLORIDA 33130-1563
(305) 375-2901
FAX (305) 375-2908**PRODUCT CONTROL NOTICE OF ACCEPTANCE****Binnings Pan American****2805 N.E. 185 St.****Miami****FL 33163**

PRODUCT CONTROL DIVISION

(305) 375-2902

FAX (305) 372-6339

Your application for Product Approval of:

*Series PA-14 Aluminum Single Hung Window*under Chapter 8 of the Metropolitan Dade County Code governing the use of Alternate Materials and Types of Construction, and completely described in the plans, specifications and calculations as submitted by: *Applicant, along with Drawing No. PA-14DC-01, Sheets 1 thru 8 of 8.*

has been recommended for acceptance by the Building Code Compliance office to be used in Dade County, Florida under the specific conditions set forth on pages 2 et. seq. and the Standard Conditions on page 3.

This approval shall not be valid after the expiration date stated below. The Office of Code Compliance reserves the right to secure this product or material at anytime from a jobsite or manufacturer's plant for quality control testing. If this product or material fails to perform in the approved manner, the Code Compliance Office may revoke, modify, or suspend the use of such product or material immediately. The applicant shall re-evaluate this product or material should any ammendments to the South Florida Building Code be enacted affecting this product or material. The Building Code Compliance Office reserves the the right to revoke this approval, if it is determined by the Building Code Compliance Office that this product or material fails to meet the requirements of the South Florida Building Code. The expense of such testing will be incurred by the manufacturer.

Acceptance No.: 97-1027.09 (Revises No.: 96-0220.01)Expires: 07/09/01

Raul Rodriguez
Product Control Supervisor**THIS IS THE COVERSHEET, SEE ADDITIONAL PAGES FOR SPECIFIC AND GENERAL CONDITIONS****BUILDING CODE COMMITTEE**

This application for Product Approval has been reviewed by the Metropolitan Dade County Building Code Compliance Department and approved by the Building Code Committee to be used in Dade County, Florida under the conditions set forth above.

1/15/01 TOWN OF SEVILLE'S POINT
Review:

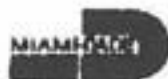
BLOG OFFICIAL


Charles Danger, P.E.
Director
Building Code Compliance Dept.
Metropolitan Dade CountyApproved: 07/09/98

FILE

TOWN - COPY
102 ABILE CT.

PN 5228

IMPACT FRENCH DOOR @ REAR PATIO

MIAMI-DADE COUNTY, FLORIDA
METRO-DADE FLAGLER BUILDING

BUILDING CODE COMPLIANCE OFFICE
METRO-DADE FLAGLER BUILDING
140 WEST FLAGLER STREET, SUITE 1603
MIAMI, FLORIDA 33130-1363
(305) 375-2901 FAX (305) 375-2908

PRODUCT CONTROL NOTICE OF ACCEPTANCE

Vinyl Tech/Progressive Glass Technology
1070 Technology Drive
Nokomis FL 34275

CONTRACTOR LICENSING SECTION
(305) 375-2337 FAX (305) 375-2338

CONTRACTOR ENFORCEMENT SECTION
(305) 375-2946 FAX (305) 375-2909

PRODUCT CONTROL DIVISION
(305) 375-2902 FAX (305) 375-4339

Your application for Product Approval of:

SWD-101 Outswing Aluminum French Door

under Chapter 8 of the Code of Miami-Dade County governing the use of Alternate Materials and Types of Construction, and completely described herein, has been recommended for acceptance by the Miami-Dade County Building Code Compliance Office (BCCO) under the conditions specified herein.

This approval shall not be valid after the expiration date stated below. BCCO reserves the right to secure this product or material at anytime from a jobsite or manufacturer's plant for quality control testing.

If this product or material fails to perform in the approved manner, BCCO may revoke, modify, or suspend the use of such product or material immediately. BCCO reserves the right to revoke this approval, if it is determined BCCO that this product or material fails to meet the requirements of the South Florida Building Code.

The expense of such testing will be incurred by the manufacturer.

Acceptance No.: 99-0128.01 (Revises No.: 98-0506.02)

Expires: 11/22/2001

Raul Rodriguez
Chief Product Control Division

THIS IS THE COVERSHEET, SEE ADDITIONAL PAGES FOR SPECIFIC AND GENERAL CONDITIONS

BUILDING CODE & PRODUCT REVIEW COMMITTEE

This application for Product Approval has been reviewed by the BCCO and approved by the Building Code and Product Review Committee to be used in Dade County, Florida under the conditions set forth above.

Francisco J. Quintana, R.A.
Director
Miami-Dade County
Building Code Compliance Office

Approved: 02/18/1999

1 of 3



BUILDING CODE COMPLIANCE OFFICE
METRO-DADE FLAGLER BUILDING
140 WEST FLAGLER STREET, SUITE 1603
MIAMI, FLORIDA 33130-1563
(305) 375-2901 FAX (305) 375-2908**PRODUCT CONTROL NOTICE OF ACCEPTANCE**T.M. Window & Door Co.
601 N.W. 12th Ave.
Pompano Beach FL 33069CONTRACTOR LICENSING SECTION
(305) 375-2527 FAX (305) 375-2558CONTRACTOR ENFORCEMENT SECTION
(305) 375-2966 FAX (305) 375-2908PRODUCT CONTROL DIVISION
(305) 375-2902 FAX (305) 372-6339

Your application for Product Approval of:

Series 200 Outswing Aluminum French Door w/Sidelites(Heavy Duty Reinforced)

under Chapter 8 of the Code of Miami-Dade County governing the use of Alternate Materials and Types of Construction, and completely described herein, has been recommended for acceptance by the Miami-Dade County Building Code Compliance Office (BCCO) under the conditions specified herein.

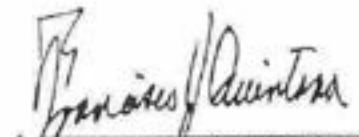
This approval shall not be valid after the expiration date stated below. BCCO reserves the right to secure this product or material at anytime from a jobsite or manufacturer's plant for quality control testing.

If this product or material fails to perform in the approved manner, BCCO may revoke, modify, or suspend the use of such product or material immediately. BCCO reserves the right to revoke this approval, if it is determined BCCO that this product or material fails to meet the requirements of the South Florida Building Code.

The expense of such testing will be incurred by the manufacturer.

Acceptance No.: 99-0524.05Expires: 06/30/2000Raul Rodriguez
Chief Product Control Division**THIS IS THE COVERSHEET, SEE ADDITIONAL PAGES FOR SPECIFIC AND GENERAL
CONDITIONS****BUILDING CODE & PRODUCT REVIEW COMMITTEE**

This application for Product Approval has been reviewed by the BCCO and approved by the Building Code and Product Review Committee to be used in Dade County, Florida under the conditions set forth above.

Francisco J. Quintana, R.A.
Director
Miami-Dade County
Building Code Compliance OffiApproved: 07/15/1999

1 of 3

BUILDING CODE COMPLIANCE OFFICE
METRO-DADE FLAGLER BUILDING
140 WEST FLAGLER STREET, SUITE 1603
MIAMI, FLORIDA 33130-1563
(305) 375-2901 FAX (305) 375-2908CONTRACTOR LICENSING SECTION
(305) 375-2527 FAX (305) 375-2558CONTRACTOR ENFORCEMENT SECTION
(305) 375-2966 FAX (305) 375-2908PRODUCT CONTROL DIVISION
(305) 375-2902 FAX (305) 372-6339**PRODUCT CONTROL NOTICE OF ACCEPTANCE**Binnings Pan American
2805 N.E. 185 St.
Miami

EL 33163

Your application for Product Approval of:

Series PA-9, Models A, B, and C Aluminum Sliding Glass Door

under Chapter 8 of the Metropolitan Dade County Code governing the use of Alternate Materials and Types of Construction, and completely described in the plans, specifications and calculations as submitted by Applicant, along with Drawing No. PA-09-DC, Sheets 1 thru 6 of 6. (This NOA renews NOA No. 95-0831.01.)

has been recommended for acceptance by the Building Code Compliance office to be used in Dade County, Florida under the specific conditions set forth on pages 2 et. seq. and the Standard Conditions on page 3.

This approval shall not be valid after the expiration date stated below. The Office of Code Compliance reserves the right to secure this product or material at anytime from a jobsite or manufacturer's plant for quality control testing. If this product or material fails to perform in the approved manner, the Code Compliance Office may revoke, modify, or suspend the use of such product or material immediately. The applicant shall re-evaluate this product or material should any amendments to the South Florida Building Code be enacted affecting this product or material. The Building Code Compliance Office reserves the right to revoke this approval, if it is determined by the Building Code Compliance Office that this product or material fails to meet the requirements of the South Florida Building Code. The expense of such testing will be incurred by the manufacturer.

Acceptance No.: 98-1110.06Expires: 11/16/01

Raul Rodriguez
Product Control Supervisor

**THIS IS THE COVERSHEET, SEE ADDITIONAL PAGES FOR SPECIFIC AND GENERAL
CONDITIONS**

BUILDING CODE COMMITTEE

This application for Product Approval has been reviewed by the Metropolitan Dade County Building Code Compliance Department and approved by the Building Code Committee to be used in Dade County, Florida under the conditions set forth above.


Charles Danger, P.E.
Director
Building Code Compliance Dep
Metropolitan Dade CountyApproved: 12/03/98



MIAMI-DADE COUNTY, FLORIDA
METRO-DADE FLAGLER BUILDING

BUILDING CODE COMPLIANCE OFFICE
METRO-DADE FLAGLER BUILDING
140 WEST FLAGLER STREET, SUITE 1603
MIAMI, FLORIDA 33130-1563
(305) 375-2901 FAX (305) 375-2908

PRODUCT CONTROL NOTICE OF ACCEPTANCE

Alufab Hurricane Shutters, Inc.
2353 Ali Baba Avenue
Opa Locka FL 33054

CONTRACTOR LICENSING SECTION
(305) 375-2527 FAX (305) 375-2558

CONTRACTOR ENFORCEMENT SECTION
(305) 375-2966 FAX (305) 375-2908

PRODUCT CONTROL DIVISION
(305) 375-2902 FAX (305) 372-6339

Your application for Product Approval of:

20 gauge Galvanized Steel Storm Panel Shutter

under Chapter 8 of the Code of Miami-Dade County governing the use of Alternate Materials and Types of Construction, and completely described herein, has been recommended for acceptance by the Miami-Dade County Building Code Compliance Office (BCCO) under the conditions specified herein.

This approval shall not be valid after the expiration date stated below. BCCO reserves the right to secure this product or material at anytime from a jobsite or manufacturer's plant for quality control testing.

If this product or material fails to perform in the approved manner, BCCO may revoke, modify, or suspend the use of such product or material immediately. BCCO reserves the right to revoke this approval, if it is determined BCCO that this product or material fails to meet the requirements of the South Florida Building Code.

The expense of such testing will be incurred by the manufacturer.

Acceptance No.: 98-0915.03 (Revises No.: 96-1107.02)

Expires: 07/17/2000

Raul Rodriguez
Chief Product Control Division

THIS IS THE COVERSHEET, SEE ADDITIONAL PAGES FOR SPECIFIC AND GENERAL CONDITIONS

BUILDING CODE & PRODUCT REVIEW COMMITTEE

This application for Product Approval has been reviewed by the BCCO and approved by the Building Code and Product Review Committee to be used in Dade County, Florida under the conditions set forth above.

Francisco J. Quintana, R.A.
Director
Miami-Dade County
Building Code Compliance Of

Approved: 01/20/1999

1 of 3





16-0129
MIAMI-DADE COUNTY, FLORIDA
METRO-DADE FLAGLER BUILDING

BUILDING CODE COMPLIANCE OFFICE
METRO-DADE FLAGLER BUILDING
140 WEST FLAGLER STREET, SUITE 1603
MIAMI, FLORIDA 33130-1563
(305) 375-2901 FAX (305) 375-2908

PRODUCT CONTROL NOTICE OF ACCEPTANCE

DAB Door Company, Inc.
12195 NW 98th Avenue
Hialeah Gardens FL 33016

CONTRACTOR LICENSING SECTION
(305) 375-2527 FAX (305) 375-2558

CONTRACTOR ENFORCEMENT SECTION
(305) 375-2966 FAX (305) 375-2908

PRODUCT CONTROL DIVISION
(305) 375-2902 FAX (305) 372-6339

Your application for Product Approval of:

Sectional Residential Garage Door

under Chapter 8 of the Metropolitan Dade County Code governing the use of Alternate Materials and Types of Construction, and completely described in the plans, specifications and calculations as submitted by: *applicant, along with Dab Door Co. Inc. drawing No 98-05, sheet 1 to 3 of 3, dated 02/23/98, revised on 08/24/98, signed and sealed by H. Farooq-PE.*

has been recommended for acceptance by the Building Code Compliance office to be used in Dade County, Florida under the specific conditions set forth on pages 2 et. seq. and the Standard Conditions on page 3.

This approval shall not be valid after the expiration date stated below. The Office of Code Compliance reserves the right to secure this product or material at anytime from a jobsite or manufacturer's plant for quality control testing. If this product or material fails to perform in the approved manner, the Code Compliance Office may revoke, modify, or suspend the use of such product or material immediately. The applicant shall re-evaluate this product or material should any amendments to the South Florida Building Code be enacted affecting this product or material. The Building Code Compliance Office reserves the right to revoke this approval, if it is determined by the Building Code Compliance Office that this product or material fails to meet the requirements of the South Florida Building Code. The expense of such testing will be incurred by the manufacturer.

Acceptance No.: 98-0901.09 (Revises No.: 98-0409.04)

Expires: 08/14/01


Raul Rodriguez
Product Control Supervisor

THIS IS THE COVERSHEET, SEE ADDITIONAL PAGES FOR SPECIFIC AND GENERAL CONDITIONS

BUILDING CODE COMMITTEE

This application for Product Approval ~~has been~~ reviewed by the Metropolitan Dade County Building Code Compliance Department and approved by the Building Code Committee to be used in Dade County, Florida under the conditions set forth above.


Charles Danger, P.E.
Director
Building Code Compliance Dept.
Metropolitan Dade County

Approved: 10/22/98

-1-





MIAMI-DADE COUNTY, FLORIDA
METRO-DADE FLAGLER BUILDING

BUILDING CODE COMPLIANCE OFFICE
METRO-DADE FLAGLER BUILDING
140 WEST FLAGLER STREET, SUITE 1603
MIAMI, FLORIDA 33130-1563
(305) 375-2901 FAX (305) 375-2908

CONTRACTOR LICENSING SECTION
(305) 375-2527 FAX (305) 375-2558

CONTRACTOR ENFORCEMENT SECTION
(305) 375-2966 FAX (305) 375-2908

PRODUCT CONTROL DIVISION
(305) 375-2902 FAX (305) 372-6339

PRODUCT CONTROL NOTICE OF ACCEPTANCE

All Weather Control, Inc.
1505 South 30 Avenue
Hollywood FL 33020

Your application for Product Approval of:

"Hurri Down" Aluminum Bahama Shutter

under Chapter 8 of the Code of Miami-Dade County governing the use of Alternate Materials and Types of Construction, and completely described herein, has been recommended for acceptance by the Miami-Dade County Building Code Compliance Office (BCCO) under the conditions specified herein.

This approval shall not be valid after the expiration date stated below. BCCO reserves the right to secure this product or material at anytime from a jobsite or manufacturer's plant for quality control testing.

If this product or material fails to perform in the approved manner, BCCO may revoke, modify, or suspend the use of such product or material immediately. BCCO reserves the right to revoke this approval, if it is determined BCCO that this product or material fails to meet the requirements of the South Florida Building Code.

The expense of such testing will be incurred by the manufacturer.

Acceptance No.: 99-0326.12

Expires: 04/18/2002

Raul Rodriguez
Chief Product Control Division

THIS IS THE COVERSHEET, SEE ADDITIONAL PAGES FOR SPECIFIC AND GENERAL
CONDITIONS

BUILDING CODE & PRODUCT REVIEW COMMITTEE

This application for Product Approval has been reviewed by the BCCO and approved by the Building Code and Product Review Committee to be used in Dade County, Florida under the conditions set forth above.

Francisco J. Quintana, R.A.
Director
Miami-Dade County
Building Code Compliance Office

Approved: 06/03/1999

1 of 3





METROPOLITAN DADE COUNTY, FLORIDA
METRO-DADE FLAGLER BUILDING

BUILDING CODE COMPLIANCE OFFICE
METRO-DADE FLAGLER BUILDING
140 WEST FLAGLER STREET, SUITE 1803
MIAMI, FLORIDA 33130-1583

(305) 375-2901
FAX (305) 375-2908

PRODUCT CONTROL NOTICE OF ACCEPTANCE

Binnings Pan American

2805 N.E. 185 St.

Miami

FL 33163

PRODUCT CONTROL DIVISION

(305) 375-2902

FAX (305) 372-6339

Your application for Product Approval of:

Series PA-14 Aluminum Single Hung Window

under Chapter 8 of the Metropolitan Dade County Code governing the use of Alternate Materials and Types of Construction, and completely described in the plans, specifications and calculations as submitted by:

Applicant, along with Drawing No. PA-14DC-01, Sheets 1 thru 8 of 8.

has been recommended for acceptance by the Building Code Compliance office to be used in Dade County, Florida under the specific conditions set forth on pages 2 et. seq. and the Standard Conditions on page 3.

This approval shall not be valid after the expiration date stated below. The Office of Code Compliance reserves the right to secure this product or material at anytime from a jobsite or manufacturer's plant for quality control testing. If this product or material fails to perform in the approved manner, the Code Compliance Office may revoke, modify, or suspend the use of such product or material immediately. The applicant shall re-evaluate this product or material should any amendments to the South Florida Building Code be enacted affecting this product or material. The Building Code Compliance Office reserves the right to revoke this approval, if it is determined by the Building Code Compliance Office that this product or material fails to meet the requirements of the South Florida Building Code. The expense of such testing will be incurred by the manufacturer.

Acceptance No.: 97-1027.09 (Revises No.: 96-0220.01)

Expires: 07/09/01

Raul Rodriguez
Product Control Supervisor

THIS IS THE COVERSHEET, SEE ADDITIONAL PAGES FOR SPECIFIC AND GENERAL CONDITIONS

BUILDING CODE COMMITTEE

This application for Product Approval has been reviewed by the Metropolitan Dade County Building Code Compliance Department and approved by the Building Code Committee to be used in Dade County, Florida under the conditions set forth above.

1/15/01 TOWN OF SEWELL'S POINT
REVIEWED:
BLOG OFFICIAL

Charles Danger, P.E.
Director
Building Code Compliance Dept.
Metropolitan Dade County

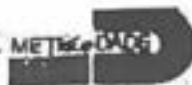
Approved: 07/09/98

FILE

TOWN - COPY
102 ABBIE CT.

PN 5228



**METROPOLITAN DADE COUNTY, FLORIDA
METRO-DADE FLAGLER BUILDING****BUILDING CODE COMPLIANCE OFFICE
METRO-DADE FLAGLER BUILDING
140 WEST FLAGLER STREET, SUITE 1603
MIAMI, FLORIDA 33130-1563
(305) 375-2901
FAX (305) 375-2908****PRODUCT CONTROL NOTICE OF ACCEPTANCE****Designer Windows of Palm Beaches
6451 E. Rogers Cir.
Boca Raton FL 33487****PRODUCT CONTROL DIVISION
(305) 375-2902
FAX (305) 372-6339**

Your application for Product Approval of:

72" x 72" Rectangular Aluminum Fixed Window

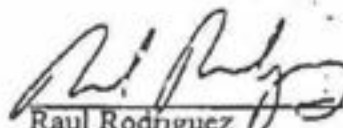
under Chapter 8 of the Metropolitan Dade County Code governing the use of Alternate Materials and Types of Construction, and completely described in the plans, specifications and calculations as submitted by: *Applicant, along with Drawing No. W97-001, Sheet 1 of 1.*

has been recommended for acceptance by the Building Code Compliance office to be used in Dade County, Florida under the specific conditions set forth on pages 2 et. seq. and the Standard Conditions on page 3.

This approval shall not be valid after the expiration date stated below. The Office of Code Compliance reserves the right to secure this product or material at anytime from a jobsite or manufacturer's plant for quality control testing. If this product or material fails to perform in the approved manner, the Code Compliance Office may revoke, modify, or suspend the use of such product or material immediately. The applicant shall re-evaluate this product or material should any amendments to the South Florida Building Code be enacted affecting this product or material. The Building Code Compliance Office reserves the right to revoke this approval, if it is determined by the Building Code Compliance Office that this product or material fails to meet the requirements of the South Florida Building Code. The expense of such testing will be incurred by the manufacturer.

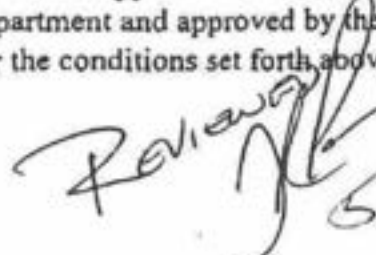
Acceptance No.: 97-0723.04

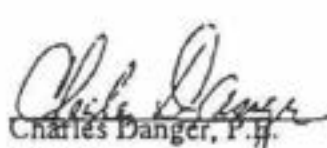
Expires: 04/09/01


Raul Rodriguez
Product Control Supervisor

**THIS IS THE COVERSHEET, SEE ADDITIONAL PAGES FOR SPECIFIC AND GENERAL
CONDITIONS****BUILDING CODE COMMITTEE**

This application for Product Approval has been reviewed by the Metropolitan Dade County Building Code Compliance Department and approved by the Building Code Committee to be used in Dade County, Florida under the conditions set forth above.


5.18.99


Charles Danger, P.E.

Director
Building Code Compliance Dept.
Metropolitan Dade County

Approved: 04/09/98

-1-



Designer Windows of Palm BeachesACCEPTANCE No.: 97-0723.04APPROVED : APR 09 1998EXPIRES : APR 09 2001NOTICE OF ACCEPTANCE: SPECIFIC CONDITIONS**1. SCOPE**

- 1.1 This approves an aluminum fixed window as described in section 2 of this Notice of Acceptance, designed to comply with the South Florida Building Code, (SFBC), 1994 Edition for Miami-Dade County, for the locations where the pressure requirements, as determined by SFBC Chapter 23, do not exceed the Design Pressure Rating values indicated in the approved drawings.

2. PRODUCT DESCRIPTION

The 72" wide x 72" Rectangular Aluminum Fixed Window" and its components shall be constructed in strict compliance with the following document: Drawing No. W97-001, titled "72" wide x 72" Rectangular Window", Sheet 1 of 1, dated 01/31/97, revised on 02/14/98, signed and sealed by K. Venkatesan, P.E., bearing the Miami-Dade County Product Control approval stamp with the Notice of Acceptance number and approval date by the Miami-Dade County Product Control Division. These documents shall hereinafter be referred to as the approved drawings.

3. LIMITATIONS

- 3.1 This approval applies to single unit applications of aluminum fixed window, as shown in approved drawings.
- 3.2 Units with dimensions equal to or smaller than those shown in approved drawings shall qualify under this approval.
- 3.3 Installation is limited by the Design Pressure Rating shown in approved drawings.

4. INSTALLATION:

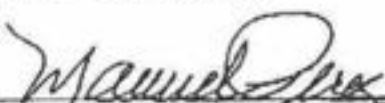
- 4.1 The aluminum fixed window and its components shall be constructed in strict compliance with the approved drawings.
- 4.2 Hurricane protection system (shutters): the installation of this unit will require a hurricane protection system.

5. LABELING

- 5.1 Each unit shall bear a permanent label with the manufacturer's name or logo, city, state and following statement: "Miami-Dade County Product Control Approved".

6. BUILDING PERMIT REQUIREMENTS

- 6.1 Application for building permit shall be accompanied by two copies of the following:
- 6.1.1 This Notice of Acceptance.
- 6.1.2 Duplicate copies of the approved drawings, as identified in section 2 of this Notice of Acceptance, clearly marked to show the components selected for the proposed installation.
- 6.1.3 Any other documents required by the Building Official or the South Florida Building Code (SFBC) in order to properly evaluate the installation of this system.


Manuel Perez, P.E. Product Control Examiner
Product Control Division



MIAMI-DADE COUNTY, FLORIDA
METRO-DADE FLAGLER BUILDING

BUILDING CODE COMPLIANCE OFFICE
METRO-DADE FLAGLER BUILDING
140 WEST FLAGLER STREET, SUITE 1603
MIAMI, FLORIDA 33130-1563
(305) 375-2901 FAX (305) 375-2908

PRODUCT CONTROL NOTICE OF ACCEPTANCE

T.M. Window & Door Co.
601 N.W. 12th Ave.
Pompano Beach FL 33069

CONTRACTOR LICENSING SECTION
(305) 375-2527 FAX (305) 375-2558

CONTRACTOR ENFORCEMENT SECTION
(305) 375-2966 FAX (305) 375-2908

PRODUCT CONTROL DIVISION
(305) 375-2902 FAX (305) 372-6339

Your application for Product Approval of:

Series 200 Outswing Aluminum French Door w/Sidelites(Heavy Duty Reinforced)

under Chapter 8 of the Code of Miami-Dade County governing the use of Alternate Materials and Types of Construction, and completely described herein, has been recommended for acceptance by the Miami-Dade County Building Code Compliance Office (BCCO) under the conditions specified herein.

This approval shall not be valid after the expiration date stated below. BCCO reserves the right to secure this product or material at anytime from a jobsite or manufacturer's plant for quality control testing.

If this product or material fails to perform in the approved manner, BCCO may revoke, modify, or suspend the use of such product or material immediately. BCCO reserves the right to revoke this approval, if it is determined BCCO that this product or material fails to meet the requirements of the South Florida Building Code.

The expense of such testing will be incurred by the manufacturer.

Acceptance No.: 99-0524.05

Expires: 06/30/2000

Raul Rodriguez
Chief Product Control Division

THIS IS THE COVERSHEET, SEE ADDITIONAL PAGES FOR SPECIFIC AND GENERAL
CONDITIONS

BUILDING CODE & PRODUCT REVIEW COMMITTEE

This application for Product Approval has been reviewed by the BCCO and approved by the Building Code and Product Review Committee to be used in Dade County, Florida under the conditions set forth above.

Francisco J. Quintana, R.A.
Director
Miami-Dade County
Building Code Compliance Office

Approved: 07/15/1999

1 of 3



LOT
IMPACT FRENCH DOOR @ REAR PATIO



MIAMI-DADE COUNTY, FLORIDA
METRO-DADE FLAGLER BUILDING

BUILDING CODE COMPLIANCE OFFICE
METRO-DADE FLAGLER BUILDING
140 WEST FLAGLER STREET, SUITE 1403
MIAMI, FLORIDA 33130-1343
(305) 375-2901 FAX (305) 375-2908

PRODUCT CONTROL NOTICE OF ACCEPTANCE

Vinyl Tech/Progressive Glass Technology
1070 Technology Drive
Nokomis FL 34275

CONTRACTOR LICENSING SECTION
(305) 375-2527 FAX (305) 375-2558

CONTRACTOR ENFORCEMENT SECTION
(305) 375-2946 FAX (305) 375-2908

PRODUCT CONTROL DIVISION
(305) 375-2902 FAX (305) 375-4339

Your application for Product Approval of:

SWD-101 Outswing Aluminum French Door

under Chapter 8 of the Code of Miami-Dade County governing the use of Alternate Materials and Types of Construction, and completely described herein, has been recommended for acceptance by the Miami-Dade County Building Code Compliance Office (BCCO) under the conditions specified herein.

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The expense of such testing will be incurred by the manufacturer.

Acceptance No. 99-0128.01 (Revises No.: 98-0506.02)

Expires: 11/22/2001

Raul Rodriguez
Chief Product Control Division

THIS IS THE COVERSHEET, SEE ADDITIONAL PAGES FOR SPECIFIC AND GENERAL CONDITIONS

BUILDING CODE & PRODUCT REVIEW COMMITTEE

This application for Product Approval has been reviewed by the BCCO and approved by the Building Code and Product Review Committee to be used in Dade County, Florida under the conditions set forth above.

Francisco J. Quintana, R.A.

Director
Miami-Dade County
Building Code Compliance Office

Approved: 02/18/1999

1 of 3





MIAMI-DADE COUNTY, FLORIDA
METRO-DADE FLAGLER BUILDING

BUILDING CODE COMPLIANCE OFFICE
METRO-DADE FLAGLER BUILDING
140 WEST FLAGLER STREET, SUITE 1603
MIAMI, FLORIDA 33130-1563
(305) 375-2901 FAX (305) 375-2908

PRODUCT CONTROL NOTICE OF ACCEPTANCE

T.M. Window & Door Co.
601 N.W. 12th Ave.
Pompano Beach FL 33069

CONTRACTOR LICENSING SECTION
(305) 375-2527 FAX (305) 375-2558

CONTRACTOR ENFORCEMENT SECTION
(305) 375-2966 FAX (305) 375-2908

PRODUCT CONTROL DIVISION
(305) 375-2902 FAX (305) 372-6339

Your application for Product Approval of:

Series 200 Outswing Aluminum French Door w/Sidelites(Heavy Duty Reinforced)

under Chapter 8 of the Code of Miami-Dade County governing the use of Alternate Materials and Types of Construction, and completely described herein, has been recommended for acceptance by the Miami-Dade County Building Code Compliance Office (BCCO) under the conditions specified herein.

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The expense of such testing will be incurred by the manufacturer.

Acceptance No.: 99-0524.05

Expires: 06/30/2000

Raul Rodriguez
Chief Product Control Division

THIS IS THE COVERSHEET, SEE ADDITIONAL PAGES FOR SPECIFIC AND GENERAL CONDITIONS

BUILDING CODE & PRODUCT REVIEW COMMITTEE

This application for Product Approval has been reviewed by the BCCO and approved by the Building Code and Product Review Committee to be used in Dade County, Florida under the conditions set forth above.

Francisco J. Quintana, R.A.
Director
Miami-Dade County
Building Code Compliance Office

Approved: 07/15/1999

1 of 3





MIAMI-DADE COUNTY, FLORIDA
METRO-DADE FLAGLER BUILDING

BUILDING CODE COMPLIANCE OFFICE
METRO-DADE FLAGLER BUILDING
140 WEST FLAGLER STREET, SUITE 1603
MIAMI, FLORIDA 33130-1563
(305) 375-2901 FAX (305) 375-2908

PRODUCT CONTROL NOTICE OF ACCEPTANCE

Binnings Pan American

2805 N.E. 185 St.

Miami

FL 33163

CONTRACTOR LICENSING SECTION
(305) 375-2527 FAX (305) 375-2558

CONTRACTOR ENFORCEMENT SECTION
(305) 375-2966 FAX (305) 375-2908

PRODUCT CONTROL DIVISION
(305) 375-2902 FAX (305) 372-6339

Your application for Product Approval of:

Series PA-9, Models A, B, and C Aluminum Sliding Glass Door

under Chapter 8 of the Metropolitan Dade County Code governing the use of Alternate Materials and Types of Construction, and completely described in the plans, specifications and calculations as submitted by: *Applicant, along with Drawing No. PA-09-DC, Sheets 1 thru 6 of 6. (This NOA renews NOA No. 95-0831.01.)*

has been recommended for acceptance by the Building Code Compliance office to be used in Dade County, Florida under the specific conditions set forth on pages 2 et. seq. and the Standard Conditions on page 3.

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Acceptance No.: 98-1110.06

Expires: 11/16/01

Raul Rodriguez
Product Control Supervisor

THIS IS THE COVERSHEET, SEE ADDITIONAL PAGES FOR SPECIFIC AND GENERAL
CONDITIONS

BUILDING CODE COMMITTEE

This application for Product Approval has been reviewed by the Metropolitan Dade County Building Code Compliance Department and approved by the Building Code Committee to be used in Dade County, Florida under the conditions set forth above.

Charles Danger, P.E.
Director
Building Code Compliance Dept.
Metropolitan Dade County

Approved: 12/03/98



MIAMI-DADE COUNTY, FLORIDA
METRO-DADE FLAGLER BUILDING

BUILDING CODE COMPLIANCE OFFICE
METRO-DADE FLAGLER BUILDING
140 WEST FLAGLER STREET, SUITE 1603
MIAMI, FLORIDA 33130-1563
(305) 375-2901 FAX (305) 375-2908

PRODUCT CONTROL NOTICE OF ACCEPTANCE

Alufab Hurricane Shutters, Inc.
2353 Ali Baba Avenue
Opa Locka FL 33054

CONTRACTOR LICENSING SECTION
(305) 375-2527 FAX (305) 375-2558

CONTRACTOR ENFORCEMENT SECTION
(305) 375-2966 FAX (305) 375-2908

PRODUCT CONTROL DIVISION
(305) 375-2902 FAX (305) 372-6339

Your application for Product Approval of:

20 gauge Galvanized Steel Storm Panel Shutter

under Chapter 8 of the Code of Miami-Dade County governing the use of Alternate Materials and Types of Construction, and completely described herein, has been recommended for acceptance by the Miami-Dade County Building Code Compliance Office (BCCO) under the conditions specified herein.

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The expense of such testing will be incurred by the manufacturer.

Acceptance No.: 98-0915.03 (Revises No.: 96-1107.02)

Expires: 07/17/2000

Raul Rodriguez
Chief Product Control Division

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CONDITIONS

BUILDING CODE & PRODUCT REVIEW COMMITTEE

This application for Product Approval has been reviewed by the BCCO and approved by the Building Code and Product Review Committee to be used in Dade County, Florida under the conditions set forth above.

Francisco J. Quintana, R.A.
Director

Miami-Dade County
Building Code Compliance Office

Approved: 01/20/1999

1 of 3





160 DRS.
MIAMI-DADE COUNTY, FLORIDA
METRO-DADE FLAGLER BUILDING

BUILDING CODE COMPLIANCE OFFICE
METRO-DADE FLAGLER BUILDING
140 WEST FLAGLER STREET, SUITE 1603
MIAMI, FLORIDA 33130-1363
(305) 375-2901 FAX (305) 375-2908

PRODUCT CONTROL NOTICE OF ACCEPTANCE

DAB Door Company, Inc.
12195 NW 98th Avenue
Hialeah Gardens FL 33016

CONTRACTOR LICENSING SECTION
(305) 375-2527 FAX (305) 375-2558

CONTRACTOR ENFORCEMENT SECTION
(305) 375-2966 FAX (305) 375-2908

PRODUCT CONTROL DIVISION
(305) 375-2902 FAX (305) 372-6339

Your application for Product Approval of:

Sectional Residential Garage Door

under Chapter 8 of the Metropolitan Dade County Code governing the use of Alternate Materials and Types of Construction, and completely described in the plans, specifications and calculations as submitted by: *applicant, along with Dab Door Co. Inc. drawing No 98-05, sheet 1 to 3 of 3, dated 02/23/98, revised on 08/24/98, signed and sealed by H. Farooq-PE.*

has been recommended for acceptance by the Building Code Compliance office to be used in Dade County, Florida under the specific conditions set forth on pages 2 et. seq. and the Standard Conditions on page 3.

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Acceptance No.: 98-0901.09 (Revises No.: 98-0409.04)

Expires: 08/14/01


Raul Rodriguez
Product Control Supervisor

THIS IS THE COVERSHEET, SEE ADDITIONAL PAGES FOR SPECIFIC AND GENERAL CONDITIONS

BUILDING CODE COMMITTEE

This application for Product Approval ~~has~~ been reviewed by the Metropolitan Dade County Building Code Compliance Department and approved by the Building Code Committee to be used in Dade County, Florida under the conditions set forth above.


Charles Danger, P.E.
Director
Building Code Compliance Dept.
Metropolitan Dade County

Approved: 10/22/98

-1-





MIAMI-DADE COUNTY, FLORIDA
METRO-DADE FLAGLER BUILDING

BUILDING CODE COMPLIANCE OFFICE
METRO-DADE FLAGLER BUILDING
140 WEST FLAGLER STREET, SUITE 1603
MIAMI, FLORIDA 33130-1563
(305) 375-2901 FAX (305) 375-2908

CONTRACTOR LICENSING SECTION
(305) 375-2527 FAX (305) 375-2558

CONTRACTOR ENFORCEMENT SECTION
(305) 375-2966 FAX (305) 375-2908

PRODUCT CONTROL DIVISION
(305) 375-2002 FAX (305) 375-6339

PRODUCT CONTROL NOTICE OF ACCEPTANCE

All Weather Control, Inc.
1505 South 30 Avenue
Hollywood FL 33020

Your application for Product Approval of:
"Hurri Down" Aluminum Bahama Shutter

under Chapter 8 of the Code of Miami-Dade County governing the use of Alternate Materials and Types of Construction, and completely described herein, has been recommended for acceptance by the Miami-Dade County Building Code Compliance Office (BCCO) under the conditions specified herein.

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The expense of such testing will be incurred by the manufacturer.

Acceptance No.: 99-0326.12

Expires: 04/18/2002

Raul Rodriguez
Chief Product Control Division

**THIS IS THE COVERSHEET, SEE ADDITIONAL PAGES FOR SPECIFIC AND GENERAL
CONDITIONS**

BUILDING CODE & PRODUCT REVIEW COMMITTEE

This application for Product Approval has been reviewed by the BCCO and approved by the Building Code and Product Review Committee to be used in Dade County, Florida under the conditions set forth above.

Francisco J. Quintana, R.A.

Director
Miami-Dade County
Building Code Compliance Office

Approved: 06/03/1999

1 of 3



MASTER PERMIT NO. 5228

TOWN OF SEWALL'S POINT

Date 8/29/01

BUILDING PERMIT NO. 5521

Building to be erected for FOGLIA CONTR. CORP.

Type of Permit IRRIGATION

Applied for by FOGLIA CONTR. CORP.

(Contractor)

Building Fee \$130.00

Subdivision SEWALL'S MEADOW Lot 22

Block _____

Radon Fee 60.00

Address 102 AIRBIE COURT

Impact Fee _____

Type of structure S.F.R. (UNDER CONST.)

A/C Fee _____

8/29/01 "AFTER PART" PERMIT
- DOUBLE FEE

Electrical Fee _____

Parcel Control Number:

Plumbing Fee _____

13-38-41-013-000-00220-00000

Roofing Fee 6.00

Amount Paid \$66.00 Check # 4581 Cash _____

Other Fees (RV) 3.00

Total Construction Cost \$ 2,450.00

TOTAL Fees \$330.00
\$66.00

Signed [Signature]
Applicant

Signed _____

Town Building Inspector OFFICIAL

BUILDING PERMIT

FORM BOARD SURVEY DATE _____
COMPACTION TESTS DATE _____
GROUND ROUGH DATE _____
SOIL POISONING DATE _____
FOOTINGS / PIERS DATE _____
SLAB ON GRADE DATE _____
TIE-BEAMS & COLUMNS DATE _____
STRAPS AND ANCHORS DATE _____
DRIVEWAY DATE _____
AS-BUILT SURVEY DATE _____

SHEATHING DATE _____
FRAMING DATE _____
INSULATION DATE _____
ROOF DRY-IN DATE _____
ROOF FINAL DATE _____
METER FINAL DATE _____
AS BUILT SURVEY DATE _____
STORM PANELS DATE _____
LANDCAPE & GRADE DATE _____
FINAL INSPECTION DATE 9/5/01

FLOOD ZONE _____

LOWEST HABITABLE FLOOR ELEV. _____

24 HOURS NOTICE REQUIRED FOR INSPECTIONS.

CALL 287-2455

WORK HOURS - 8:00 AM UNTIL 5:00 PM

MONDAY THROUGH SATURDAY

☐ New Construction ☐ Remodel ☐ Addition ☐ Demolition

This permit must be visible from the street, accessible to the inspector.

FURTHER CONDITIONS ARE SET FORTH IN THE APPLICATION FOR PERMIT,
NOTATIONS ON THE APPROVED SUBMITTALS, AND ATTACHMENTS IN THE PERMIT FILE.
DO NOT FASTEN THIS OR ANY OTHER SIGN TO A TREE!

TOWN OF SEWALL'S POINT

Building Department - Inspection Log

Date of Inspection: ☐ Mon ☐ Wed ☒ Fri AUGUST 31, 2001; Page 3 of 3.

PERMIT	OWNER/ADDRESS/CONTR.	INSPECTION TYPE	RESULTS	NOTES/COMMENTS:
✓ 5380	GIFFORD	POOL DECK	Passed	~~Co. per. Test.~~
(16)	85 N. SEWALL'S POINT RD A&G CONCRETE POOLS			Waterfall area only INSPECTOR: <u>J 8/31</u>
✓ 5489	STRACUZZI (285-2815) (260-1443 con)	U/G PLUMBING	Passed	
S (7)	12 RIO VISTA DR. RUCCOLO			INSPECTOR: <u>J 8/31</u>
✓ 5324	HECKENBERG	MAS. WALL - FINAL	Passed	PN 5405 - ENTRY COL. (STATUS?)
(13)	SLAGOON ISLAND CT. U/B (JAMIE 718-2777)	ENTRY COL. - FINAL	Passed	INSPECTOR: <u>J 8/31</u>
✓ T/R	TENNY	FIELD VERIF.	Passed	(AMENDED PLAN -)
S (9)	105 SEWALL'S POINT RD GORDON'S TREE SERVICE		PN 0494 8/31/01	(W/O GUMBO LIMBO) INSPECTOR: <u>J 8/31</u>
✓ T/R	AYRES	FIELD VERIF.	Passed	
S (8)	155 RIVER RD. MONTE'S TREE SERVICE		PN 0493 8/31/01	INSPECTOR: <u>J 8/31</u>
✓ 5482	RAO	IRRIGATION - FINAL	Passed	MPN 5192
(15)	30 CASTLE HILL WAY FRITZ IRRIGATION			INSPECTOR: <u>J 8/31</u>
✓ 5521	FOGLIA	IRRIGATION - FINAL	Passed	MPN 5228
S (12)	102 ABBIE COURT FOGLIA CONTRACTING			INSPECTOR: <u>J 8/31</u>

OTHER: _____

Southern Irrigation, Inc.
5207 SW Moore Street
Palm City, FL 34990
561-288-1883
561-288-1894 fax



FILE
IRRG.
PERMIT
5521
(MPN 5228)

August 21, 2001

Town Of Sewalls Point
15 Sewalls Point Road
Sewalls Point, FL 34996

Mr. Edward Arnold:

Lot 22 Sewalls Meadow, 102 Abbie Court, as per section 22-146 is installed as a low volume irrigation system with a rain sensor devise.

Martin County Competency #SP00734

Sincerely,

Robin G. Henn
Sec. / Tres.



**Town of Sewall's Point
BUILDING PERMIT APPLICATION**

RECEIVED
AUG 21 2001

Bldg. Permit Number: 5521

Owner or Titleholder's Name FCGUA CONTRACTING CORP Phone No. (FH) 75-8808
 Street: 7428 WYDS Rd. City CORAL SPRINGS State: FL Zip 33007
 Legal Description of Property: 102 ABBIE COURT
 Parcel Number: 13-38-4-013-000-00220-00000

Location of Job Site: LOT 22 SEWALLS MEADOW
 TYPE OF WORK TO BE DONE: IRRIGATION SYSTEM

CONTRACTOR/Company Name: SEWELL IRRIGATION Phone No. (FH) 308-7083
 Street: SAME AS ABOVE City _____ State: _____ Zip _____
 State Registration: _____ State License: _____

ARCHITECT: _____ Phone No. () _____
 Street: _____ City _____ State: _____ Zip _____

ENGINEER: _____ Phone No. () _____
 Street: _____ City _____ State: _____ Zip _____

AREA SQUARE FOOTAGE - SEWER - ELECTRIC:
 Living Area: _____ Garage Area: _____ Carport: _____ Accessory Bldg: _____
 Covered Patio: _____ Scr. Porch: _____ Wood Deck: _____
 Type Sewage: _____ Septic Tank Permit # from Health Dept. _____
 New Electrical Service Size: _____ AMPS

FLOOD HAZARD INFORMATION
 Flood zone: _____ Minimum Base Flood Elevation (BFE): _____ NGVD
 Proposed first habitable floor finished elevation: _____ NGVD (minimum 1 foot above BFE)

COSTS AND VALUES
 Estimated cost of construction or Improvement: \$ 2450⁰⁰
 Estimated Fair Market Value (FMV) prior to improvement: \$ _____
 If Improvement is cost greater than 50% of Fair Market Value? YES _____ NO _____

Approved by Town Engineer _____ (if required)
 Date: _____
 Approved by Building Official: [Signature]
 Date: 8/24/01

NOTICE: In addition to the requirements of this permit, there may be additional restrictions applicable to this property that may be found in the public records of COUNTY OF MARTIN, and there may be additional permits required from other governmental entities such as water management districts, state and federal agencies.
 Replat required upon completion of slab or footing inspection And Prior to any further inspections.
 8.

AC 5910992

STATE OF FLORIDA
DEPARTMENT OF BUSINESS AND PROFESSIONAL REGULATION
CONSTRUCTION INDUSTRY LICENSING BOARD

DATE BATCH NUMBER LICENSE NO.

07/03/2002 00900145
The Florida Building & Construction Trades Council
Local Union No. 1000
Under the provisions of Chapter 489
Expiration date: AUG 31, 2002

FOGLIA, JOSEPH N.
FOGLIA CONTRACTING CORP
7428 WILES RD
CORAL SPRINGS

JEB BUSH
GOVERNOR

DISPLAY AS REQUIRED BY LAW

CYNTHIA A. HENDERSON
SECRETARY

CG-C038434

MASTER PERMIT NO. N/A

TOWN OF SEWALL'S POINT

Date 1/16/01

BUILDING PERMIT NO. 5228

Building to be erected for FOGLIA CONTRACTING CORP Type of Permit BLDG. - S.F.R.

Applied for by FOGLIA CONTRACTING CORP (Contractor) Building Fee \$1,844.35

Subdivision SEWALL'S MEADOW Lot 22 Block _____ Radon Fee 37.72

Address 102 ABBIE CT Impact Fee 4,024.92

Type of structure S.F.R. A/C Fee 120.00

Parcel Control Number: 13-38-41-013-000-00220-00000 Electrical Fee 120.00

Amount Paid \$6,571.42 Check # 4326 Cash _____ Other Fees (PLUMB) 120.00

Total Construction Cost \$ 192,120.00 Roofing Fee 120.00

Signed [Signature] Signed [Signature] TOTAL Fees \$6,571.42

Applicant

Town Building Inspector OFFICIAL

BUILDING PERMIT

FORM BOARD SURVEY DATE _____
COMPACTION TESTS DATE _____
GROUND ROUGH DATE _____
SOIL POISONING DATE _____
FOOTINGS / PIERS DATE _____
SLAB ON GRADE DATE _____
TIE-BEAMS & COLUMNS DATE _____
STRAPS AND ANCHORS DATE _____
DRIVEWAY DATE _____
AS-BUILT SURVEY DATE _____

SHEATHING DATE _____
FRAMING DATE _____
INSULATION DATE _____
ROOF DRY-IN DATE _____
ROOF FINAL DATE _____
METER FINAL DATE _____
AS BUILT SURVEY DATE _____
STORM PANELS DATE _____
LANDCAPE & GRADE DATE _____
FINAL INSPECTION DATE _____

FLOOD ZONE _____

LOWEST HABITABLE FLOOR ELEV. _____

24 HOURS NOTICE REQUIRED FOR INSPECTIONS.

CALL 287-2455

WORK HOURS - 8:00 AM UNTIL 5:00 PM

MONDAY THROUGH SATURDAY

☐ New Construction ☐ Remodel ☐ Addition ☐ Demolition

This permit must be visible from the street, accessible to the inspector.

FURTHER CONDITIONS ARE SET FORTH IN THE APPLICATION FOR PERMIT,
NOTATIONS ON THE APPROVED SUBMITTALS, AND ATTACHMENTS IN THE PERMIT FILE.
DO NOT FASTEN THIS OR ANY OTHER SIGN TO A TREE!

AC# 5910992

STATE OF FLORIDA
DEPARTMENT OF BUSINESS AND PROFESSIONAL REGULATION
CONSUMER INDUSTRY LICENSING BOARD

DATE	BATCH NUMBER	LICENSE #

07/03/2002	00900142	00000000

For BEER, WINE, DISTILLERS, and other alcoholic beverages, the licensee must be a resident of the State of Florida.
Under the provisions of Chapter 489, F.S.
Expiration date: AUG 31, 2002

FOGLIA, JOSEPH N.
FOGLIA CONTRACTING CORP
7428 MILES RD
CORAL SPRINGS

JEB JUSH
GOVERNOR

DISPLAY AS REQUIRED BY LAW

CYNTHIA A. HENDERSON
SECRETARY



TEL: (954) 975-3384
FAX: (954) 978-8980
3120 N.W. 16TH TERRACE
POMPANO BEACH, FL. 33064

Customer FOGILA CUSTOM HOMES

Job # 00469

PROJECT LOT 22

TOWN COPY
102 ABBIE CT.

PN 5228



Designer : MH

MASTER PERMIT NO. 5228

TOWN OF SEWALL'S POINT

Date 8/29/01

BUILDING PERMIT NO. 5521

Building to be erected for FOGLIA CONTR. CORP.

Type of Permit IRRIGATION

Applied for by FOGLIA CONTR. CORP.

(Contractor)

Building Fee \$130.00

Subdivision SEWALL'S MEADOW Lot 22

Block _____

Radon Fee 60.00

Address 102 AIRBIE COURT

Impact Fee _____

Type of structure S.F.R. (UNDER CONST.)

A/C Fee _____

8/29/01 "AFTER PART" PERMIT
- DOUBLE FEE

Electrical Fee _____

Parcel Control Number:

Plumbing Fee _____

13-38-41-013-000-00220-00000

Roofing Fee 6.00

Amount Paid \$66.00 Check # 4581 Cash _____

Other Fees (RV) 3.00

Total Construction Cost \$ 2,450.00

TOTAL Fees \$330.00
\$66.00

Signed [Signature]
Applicant

Signed _____

Town Building Inspector OFFICIAL

BUILDING PERMIT

FORM BOARD SURVEY DATE _____
COMPACTION TESTS DATE _____
GROUND ROUGH DATE _____
SOIL POISONING DATE _____
FOOTINGS / PIERS DATE _____
SLAB ON GRADE DATE _____
TIE-BEAMS & COLUMNS DATE _____
STRAPS AND ANCHORS DATE _____
DRIVEWAY DATE _____
AS-BUILT SURVEY DATE _____

SHEATHING DATE _____
FRAMING DATE _____
INSULATION DATE _____
ROOF DRY-IN DATE _____
ROOF FINAL DATE _____
METER FINAL DATE _____
AS BUILT SURVEY DATE _____
STORM PANELS DATE _____
LANDCAPE & GRADE DATE _____
FINAL INSPECTION DATE 9/5/01

FLOOD ZONE _____

LOWEST HABITABLE FLOOR ELEV. _____

24 HOURS NOTICE REQUIRED FOR INSPECTIONS.

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TOWN OF SEWALL'S POINT

Building Department - Inspection Log

Date of Inspection: ☐ Mon ☐ Wed ☒ Fri AUGUST 31, 2001; Page 3 of 3.

PERMIT	OWNER/ADDRESS/CONTR.	INSPECTION TYPE	RESULTS	NOTES/COMMENTS:
✓ 5380	GIFFORD	POOL DECK	Passed	~~Co. per. Test.~~
(16)	85 N. SEWALL'S POINT RD A&G CONCRETE POOLS			Waterfall area only INSPECTOR: <u>J 8/31</u>
✓ 5489	STRACUZZI (285-2815) (260-1443 con)	U/G PLUMBING	Passed	
S (7)	12 RIO VISTA DR. RUCCOLO			INSPECTOR: <u>J 8/31</u>
✓ 5324	HECKENBERG	MAS. WALL - FINAL	Passed	PN 5405 - ENTRY COL. (STATUS?)
(13)	SLAGOON ISLAND CT. U/B (JAMIE 718-2777)	ENTRY COL. - FINAL	Passed	INSPECTOR: <u>J 8/31</u>
✓ T/R	TENNY	FIELD VERIF.	Passed	(AMENDED PLAN -)
S (9)	105 SEWALL'S POINT RD GORDON'S TREE SERVICE		PN 0494 8/31/01	(W/O GUMBO LIMBO) INSPECTOR: <u>J 8/31</u>
✓ T/R	AYRES	FIELD VERIF.	Passed	
S (8)	155 RIVER RD. MONTE'S TREE SERVICE		PN 0493 8/31/01	INSPECTOR: <u>J 8/31</u>
✓ 5482	RAO	IRRIGATION - FINAL	Passed	MPN 5192
(15)	30 CASTLE HILL WAY FRITZ IRRIGATION			INSPECTOR: <u>J 8/31</u>
✓ 5521	FOGLIA	IRRIGATION - FINAL	Passed	MPN 5228
S (12)	102 ABBIE COURT FOGLIA CONTRACTING			INSPECTOR: <u>J 8/31</u>

OTHER: _____

AC 5910992

STATE OF FLORIDA
DEPARTMENT OF BUSINESS AND PROFESSIONAL REGULATION
CONSTRUCTION INDUSTRY LICENSING BOARD

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JEB BUSH
GOVERNOR

DISPLAY AS REQUIRED BY LAW

CYNTHIA A. HENDERSON
SECRETARY

CG-C038434

ACORD CERTIFICATE OF LIABILITY INSURANCE

BSR AL
SOUTH-2

02/06/01

Gary Insurance Agency
& Associates, Inc.
117 East Seminole Street
Stuart FL 34994
Phone: 561-283-2609 Fax: 561-220-8107

FILE
permit

THIS CERTIFICATE IS ISSUED AS A MATTER OF INFORMATION
ONLY AND CONFERS NO RIGHTS UPON THE CERTIFICATE
HOLDER. THIS CERTIFICATE DOES NOT AMEND, EXTEND OR
ALTER THE COVERAGE AFFORDED BY THE POLICIES BELOW.

INSURERS AFFORDING COVERAGE

INSURED

Southern Irrigation, Inc.
Mark J. & Robin O. Mann
5207 SW Moore Street
Palm City FL 34990

FILE
lic/ins.

INSURER A: The Hartford

INSURER B:

INSURER C:

INSURER D:

INSURER E:

RECEIVED

FEB - 6 2001

BY:

COVERAGES

THE POLICIES OF INSURANCE LISTED BELOW HAVE BEEN ISSUED TO THE INSURED NAMED ABOVE FOR THE POLICY PERIOD INDICATED. NOTWITHSTANDING
ANY REQUIREMENT, TERM OR CONDITION OF ANY CONTRACT OR OTHER DOCUMENT WITH RESPECT TO WHICH THIS CERTIFICATE MAY BE ISSUED OR
MAY PERTAIN, THE INSURANCE AFFORDED BY THE POLICIES DESCRIBED HEREIN IS SUBJECT TO ALL THE TERMS, EXCLUSIONS AND CONDITIONS OF SUCH
POLICIES. AGGREGATE LIMITS SHOWN MAY HAVE BEEN REDUCED BY PAID CLAIMS.

INSR LTR	TYPE OF INSURANCE	POLICY NUMBER	POLICY EFFECTIVE DATE (MM/DD/YY)	POLICY EXPIRATION DATE (MM/DD/YY)	LIMITS
A	GENERAL LIABILITY ☐ COMMERCIAL GENERAL LIABILITY ☐ CLAIMS MADE ☐ OCCUR GEN'L AGGREGATE LIMIT APPLIES PER: ☐ POLICY ☐ PRO- JECT ☐ LOC	21 SRA LY1604	11/10/00	11/10/01	EACH OCCURRENCE \$ 1,000,000 FIRE DAMAGE (Any one fire) \$ 300,000 MED EXP (Any one person) \$ 10,000 PERSONAL & ADV INJURY \$ 1,000,000 GENERAL AGGREGATE \$ 2,000,000 PRODUCTS - COMPROP AGG \$ 2,000,000
A	AUTOMOBILE LIABILITY ☒ ANY AUTO ☐ ALL OWNED AUTOS ☐ SCHEDULED AUTOS ☐ HERED AUTOS ☐ NON-OWNED AUTOS	21 UEC LI5180	11/10/00	11/10/01	COMBINED SINGLE LIMIT (Per accident) \$ 1,000,000 BODILY INJURY (Per person) \$ BODILY INJURY (Per accident) \$ PROPERTY DAMAGE (Per accident) \$
	GARAGE LIABILITY ☐ ANY AUTO				AUTO ONLY - EA ACCIDENT \$ OTHER THAN EA ACC \$ AUTO ONLY: AGG \$
	EXCESS LIABILITY ☐ OCCUR ☐ CLAIMS MADE DEDUCTIBLE \$ RETENTION \$				EACH OCCURRENCE \$ AGGREGATE \$ \$ \$ \$
A	WORKERS COMPENSATION AND EMPLOYERS LIABILITY	21 WEC EV7905	04/01/00	04/01/01	WC STATUTORY LIMITS EL EACH ACCIDENT \$ 100,000 EL DISEASE - EA EMPLOYEE \$ 100,000 EL DISEASE - POLICY LIMIT \$ 500,000
	OTHER				

DESCRIPTION OF OPERATIONS/LOCATIONS/EXCLUSIONS ADDED BY ENDORSEMENTS/SPECIAL PROVISIONS

1999 Ford F250 1FTNF20L3XEB65354
1998 Ford Explorer 1FMEU32E2WEA75918
1995 Ford F250 1FTEF25Y5SNA57041

CERTIFICATE HOLDER

N

ADDITIONAL INSURED; INSURER LETTER:

SEWALLS

Town of Sewalls Point
Building Department
1 S. Sewalls Point Road
Sewalls Point FL 34996

CANCELLATION

SHOULD ANY OF THE ABOVE DESCRIBED POLICIES BE CANCELLED BEFORE THE EXPIRATION
DATE THEREOF, THE ISSUING INSURER WILL ENDEAVOR TO MAIL 30 DAYS WRITTEN
NOTICE TO THE CERTIFICATE HOLDER NAMED TO THE LEFT, BUT FAILURE TO DO SO SHALL
IMPOSE NO OBLIGATION OR LIABILITY OF ANY KIND UPON THE INSURER, ITS AGENTS OR
REPRESENTATIVES.

Andre J. Lambros, CIC



MARTIN COUNTY, FLORIDA
Construction Industry Lic Bd
Certificate of Competency

License: SP00734
Expires September 30, 2001

HENN, MARK J

SOUTHERN IRRIGATION INC

5207 SW MOORE ST

PALM CITY, FL 34990

IRRIGATION SPRINKLERS

FILE

ref 10/1

RECEIVED

OCT 25 2000

BY: *[Signature]*

MARTIN COUNTY ORIGINAL
2000 COUNTY OCCUPATIONAL LICENSE 2001
Larry C. O'Steen, Tax Collector, P.O. Box 9013, Stuart, FL 34996
(888) 286-0604

CHARACTER COUNTS IN MARTIN COUNTY

LICENSE 1996 520 015 SP000734

PHONE 561 286 1883 EXT NO 1711

LOCATION:

5207 SW MOORE ST

HOME OFFICE NO STORAGE

PREV YR \$	<u>0.00</u>	LIC. FEE \$	<u>25.00</u>
\$	<u>0.00</u>	PENALTY \$	<u>0.00</u>
\$	<u>0.00</u>	COL. FEE \$	<u>0.00</u>
\$		TRANSFER \$	<u>0.00</u>
TOTAL			<u>25.00</u>

IF HEREBY LICENSED TO ENGAGE IN THE BUSINESS, PROFESSION OR OCCUPATION
OF IRRIGATION SPRINKLER

AT ABOVE ADDRESS FOR THE PERIOD BEGINNING ON THE

1 DAY OF OCTOBER 2001 12 82601 2755 PAID

SOUTHERN IRRIGATION INC
MARK J HENN
5207 SW MOORE STREET
PALM CITY FL 34990

FLORIDA QUALITY TRUSS INC.

Customer:
Fagila Custom Homes
Project:
Lot 22.....

Truss ID.

1	CJ7C
2	C1
3	C3
4	C5
5	J7
6	H8
7	H1
8	H2
9	S2
10	S3
11	S4
12	S5
13	S6
14	GT25
15	FG3
16	GT36
17	S26
18	S25
19	S24
20	S23
21	S22
22	HH2
23	CJ9
24	J6
25	FG2
26	C4
27	NG2
28	C1A
29	C3A
30	C5A

31	H4
32	GT7
33	T2
34	T3
35	S27
36	S28
37	T4
38	H5
39	H6
40	T5
41	T6
42	S30
43	S31
44	S32
45	T7
46	T7A
47	S33
48	S34
49	S42
50	S41
51	H40A
52	S40
53	G1
54	G1A
55	S39
56	S38A
57	S38
58	S37
59	S36
60	S35
61	S14
62	CJ7A
63	NG1
64	CE5
65	CF5
66	S21

67	S21A
68	S21B
69	FG1
70	C57
71	GT10
72	S17
73	J10
74	S15
75	S18
76	S19
77	S9
78	GT16
79	C3B
80	CJ5C
81	C3E
82	GT6
83	S13
84	S12
85	S11
86	H3
87	CJ5
88	J4
89	GT6A
90	CA5
91	V4
92	V8
93	V12
94	V16
95	T9A
96	T9
97	T8
98	T1
99	J02
100	T10
101	S1
102	CJ1A

103	CC1
104	CC3
105	C2
106	FG8
107	CJ5A
108	CC5
109	CT14
110	CC2
111	FG5
112	CJ4
113	C04
114	FG4
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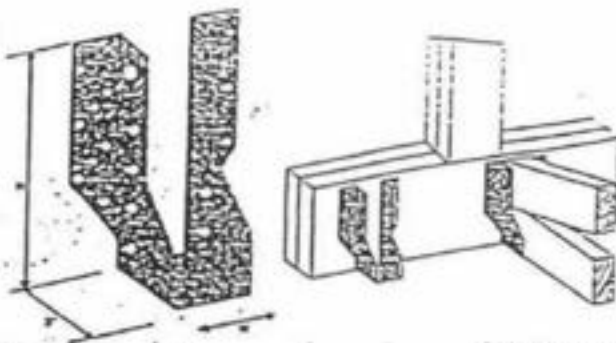
TRUSS/JOIST HANGERS, TUH - TYPE

- Featuring Load Share Fastening System
- Develops full Truss/Joist capacity
- 16 gauge Galvanized Steel
- Engineered and Tested

Load Share Fastening System



Diamond shaped spacing allows for angular "Load Share" fastening. Four direct force distribution double the bearing load to the connector. Conventional connectors transfer the bearing load to a single point.

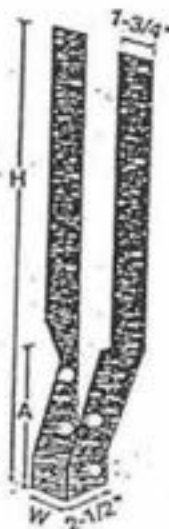
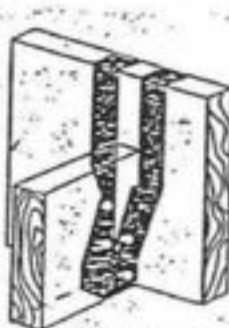


Load Share Nailin

Product Code	Joist Size	Dimensions		Qty/ Box	Fasteners		Design Loads (lbs)			
		W	H		Girder Header	Truss/ Joist	Normal 100%	Snow 115%	Roof 125%	Uplift
TUH 26	2X6	1-9/16"	5-5/16"	50	(16) 16d	(8) 16d	2500	3500	2500	2000

JOIST/TRUSS HANGERS, ADJUSTABLE

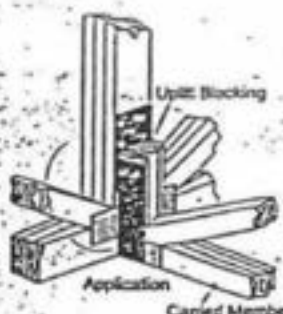
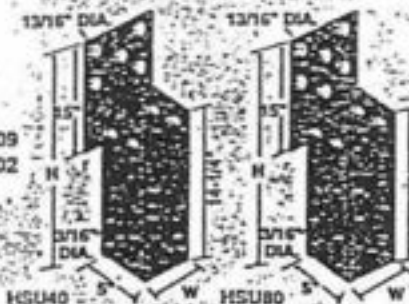
- Extra long straps allow maximum number of nails for maximum strength
- Can be field formed for top flange hanger convenience
- A few sizes satisfies all your hanger requirements
- No bolts required
- A full 2-1/2" seat bearing depth
- Galvanized steel
- SBCCI PST & ESI No. 94139
- Dade County Approval No. 95-1023.02



Product Code	Gauge	Qty/ Box	Wgt/ Box	Dimensions			Fasteners		Design Loads (lbs)	
				W	H	A	Header	Joist	Normal	Uplift
JHA 413	18 ga	50	40	3-5/8"	12-1/4"	4-3/4"	18-16d	6-10d	2440	820
JHA 418	16 ga	30	41	3-5/8"	16-1/4"	4-3/4"	24-16d	6-10d	3250	820

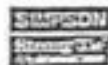
GIRDER HANGERS (HEAVY)

- Material: 7 gauge (3/16") H. R. Steel
- 3/4" diameter bolts required with 3" of wood penetration and minimum vertical width 5-1/2"
- Used where heavy design loads and uplift are required
- SBCCI PST & ESI No. 94139
- Dade County Approval No. 94-0706.09
- Dade County Approval No. 94-0328.02



Product Code	Description	W	H	Fasteners			Design Loads (lbs)		
				Bolts		Nails	Normal	Max	Uplift
				Girder	Block	Carried Member			
HSU 40	Girder Hgr(Heavy)	3-1/4"	23"	(5)-3/4"	(2)-3/4"	4-10d	6250	6250	9110
HSU 80	Girder Hgr(Heavy)	4-7/8"	23-1/2"	(6)-3/4"	(2)-3/4"	4-10d	9280	10670	10000
HSU 8016	Girder Hgr(Heavy)	4-7/8"	16"	(6)-3/4"	(2)-3/4"	4-10d	9280	10670	10000

LUS/HUS/HHUS/HGUS DOUBLE SHEAR JOIST HANGERS

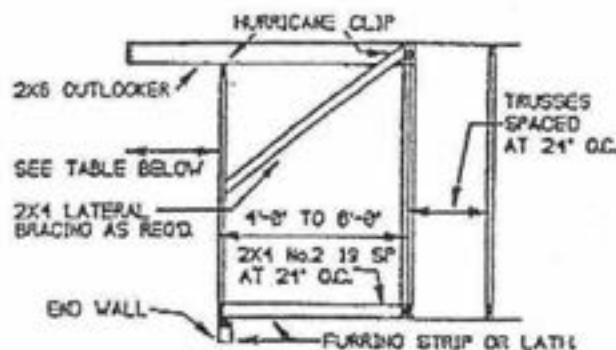
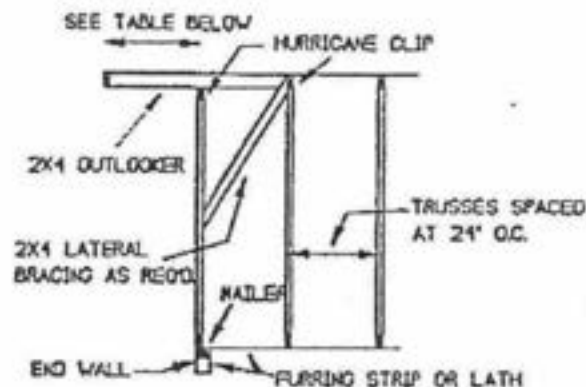
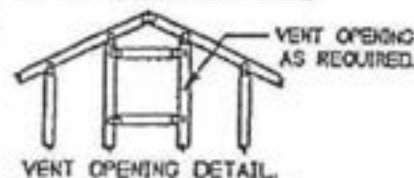
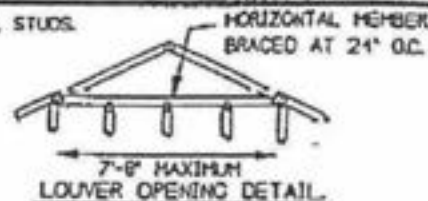
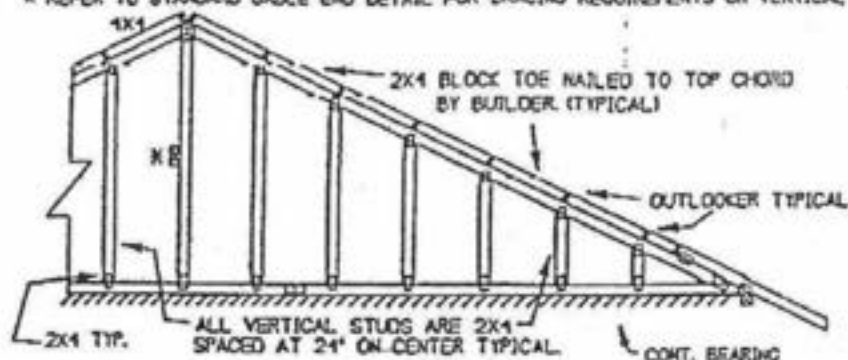


Model No.	Dimensions			Fasteners		Avg Us	Doug Fir-Larch/Southern Pine Allowable Loads						Spruce-Pine-Fir Allowable Loads					
	W	H	B	Carrying Member	Carried Member		Uplift* (13)	Uplift* (14)	Flwr (14)	Snow (11)	Coast (12)	Wind (13)	Uplift* (13)	Uplift* (14)	Flwr (14)	Snow (11)	Coast (12)	Wind (13)
HUS25-2	3 1/4"	5 1/2"	3	14-16d	8-16d	14667	1550	1550	2580	2965	3225	3440	1385	1550	2165	2490	2710	2890
HUS25-2	3 1/4"	6 1/2"	4	20-16d	8-16d	14667	2000	2000	3735	4320	4625	4835	1865	2000	3185	3660	3990	4245
HUS25-2	3 1/4"	8"	3	22-16d	8-16d	19620	2000	2000	3855	4465	4855	5180	1860	2000	3275	3765	4095	4370
HUS25-2	3 1/4"	7 1/4"	4	36-16d	10-16d	25180	2650	2650	6245	7185	7810	8000	2330	2650	5335	6070	6150	6220

STANDARD DROPPED GABLE END DETAIL.

PREPARED BY H&M OFFICE

K REFER TO STANDARD GABLE END DETAIL FOR BRACING REQUIREMENTS ON VERTICAL STUDS.



MAXIMUM OUTLOOKERS EXTENSION.

LUMBER	WITH PLASTER		WITHOUT PLASTER	
	2x4	2x6	2x4	2x6
No.2ND 19 SP	2-8-3	3-10-11	2-10-13	1-2-9
No.2 19 SP	2-9-15	4-8-11	3-8-12	4-4-11
No.2D 19 SP	3-8-2	4-4-7	3-3-2	4-8-12
No.1ND 19 SP	3-8-2	4-5-6	3-3-2	4-9-11
No.1 19 SP	3-1-11	4-7-15	3-1-13	5-8-8
No.1D 19 SP	3-3-3	4-9-18	3-6-7	5-2-5
ND SS 19 SP	3-7-3	5-6-12	3-7-3	5-7-14
SS 19 SP	3-8-1	5-6-3	3-8-1	5-9-3
DSS 18 SP	3-8-13	5-10-7	3-8-13	5-10-7

NOTE 1

1. CONNECTION BETWEEN BOTTOM CHORD OF GABLE END TRUSS AND WALL TO BE PROVIDED BY PROJECT ENGINEER OR ARCHITECT.
2. FURNISH COPY OF THIS DRAWING TO CONTRACTOR FOR INSTALLATION OF OUTLOOKERS.
3. REFER TO STANDARD GABLE END DETAIL FOR BRACING REQUIREMENTS OF VERTICAL STUDS.

MINIMUM GRADE OF LUMBER

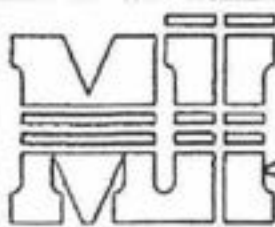
T.C. 2x4 No.2ND 19 SP
B.C. 2x4 No.2ND 19 SP
Webs 2x4 No.3 19 SP

TPJ-92 Cr11.

SSBC-91

LOADING (PSF)
L 0
TOP 30 15
BOTTOM 0 10
SPACING: 24 Inch O.C.

STR. INCR. 33 %
DRAWN BY: JB
CHECKED BY: JAI
REP. STRESS: YES



MiTek Industries Inc.

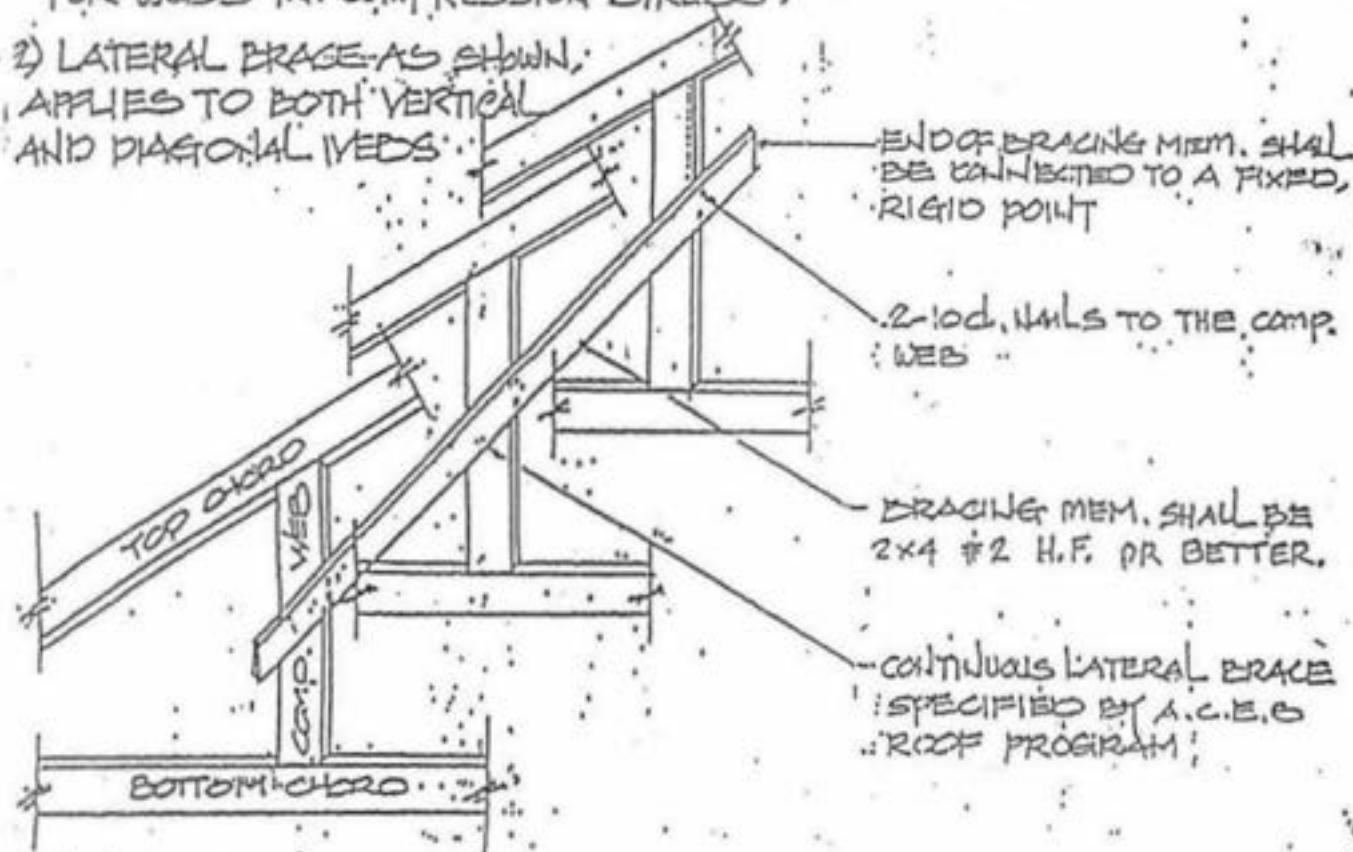


Approved: Mehmet Y. Tilter, Registered Engineer Inc.

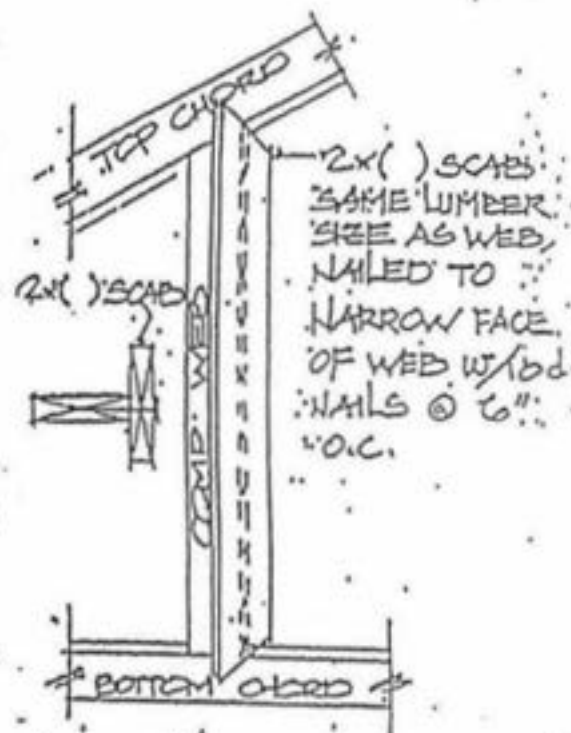
NOTE :

1) THIS DETAIL IS VALID ONLY FOR COMPUTER OUTPUT OF A.C.E.G. ROOF PROGRAM & SERVES AS AN INTERPRETATION OF CONTINUOUS LATERAL BRACING FOR WEBS IN COMPRESSION STRESS.

2) LATERAL BRACE AS SHOWN, APPLIES TO BOTH VERTICAL AND DIAGONAL WEBS.



MIN. LENGTH OF SCAB = $0.8 \times L$ (LENGTH OF WEB)



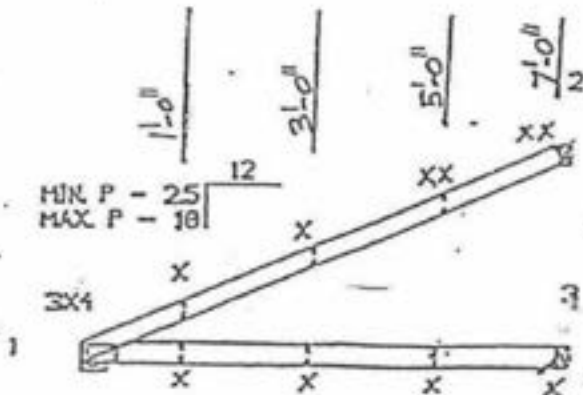
1/2 POINTS OR 1/2 POINTS BRACING

OPTIONAL T-BRACE IN CASE THAT CONTINUOUS LATERAL BRACING CAN NOT BE ACHIEVED

WIG (PSF)	SCALE	BEHAX OF FLORIDA	APPROVED FOR BEHAX FLA.	DESIGNED BY
IVE 1	SPACING	STANDARD LATERAL BRACE	STATE OF	CHECKED BY
PEAD 1	TPI. OF DSGH CRIT.	DETAIL FOR WEB	NEG. ENG. 10000 FLA.	DATE 7/22/83
PEAD 1	ALLOWABLE STRESS			DRG. 1 ALB-DFV

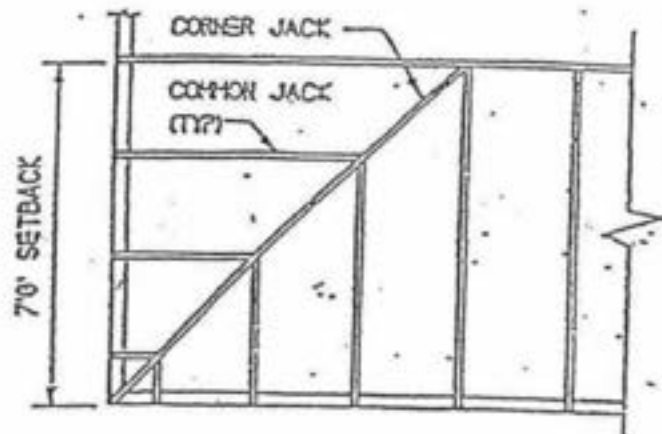
7'0" CORNER SET

OC. 08/11/1986

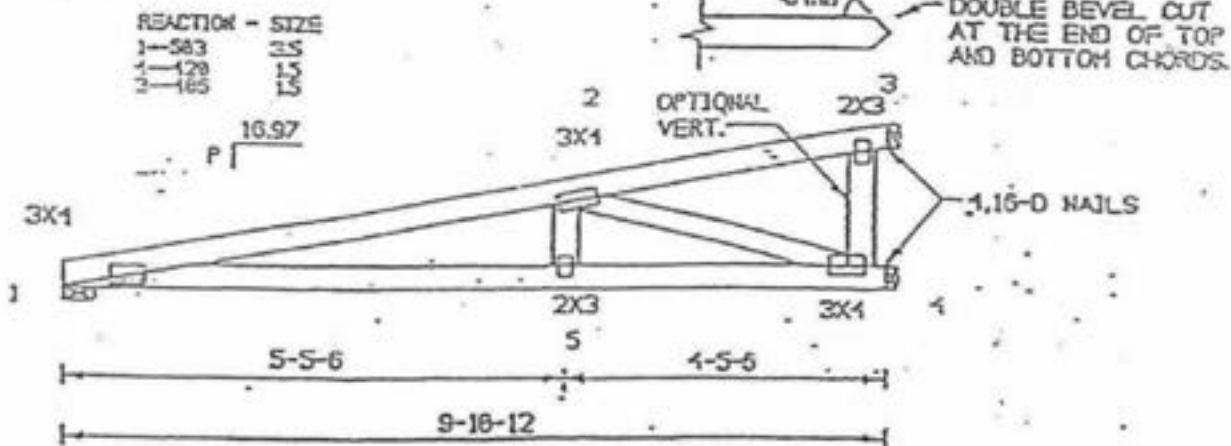


(X) 2-16d
(XX) 3-16d

COMMON JACKS



PARTIAL ROOF LAYOUT



CORNER JACK

NOTE: THIS DESIGN HAS BEEN CHECKED WITH 200 PSF WIND LOAD, WALL HEIGHT 10 FT MAX.
PROVIDE FOR UPLIFT AT SEARING WALL CONNECTIONS: 150 LBS AT COMMON JACKS & 250 LBS AT CORNER JACKS.

MINIMUM GRADE OF LUMBER

T.C. 2X4 1b2 19 SP
B.C. 2X4 1b2 19 SP
Joists 2X4 1b3 19 SP

TPI-92 C-11
SSBC-91



MITek Industries Inc.



Approved for use by MITek Industries Inc.

LOADING (PSF)
D 0
OP 30 15
OTOM 0 10
PACING 24' OC

STR. INCR: 33 X
DRAWN BY: TAM
CHECKED BY: JAJ
REP. STRESS: NO

BACK DETAIL

12 191 Southern Pine

2x4 13 191 Southern Pine

Space vertical @ 5'0" o.c. max.

CONTINUOUS LATERAL BRACING ATTACHED TO TOP OR BOTTOM OF TOP CHORD OF TRUSS SUPPORTING PIGGYBACK AS SHOWN ON TRUSS ENGINEERING.
x4-12 HF, @ 24" & C w/ 2-16 ds

OVERALL SUPPORTING SPAN UP TO:

ALPINE	TYPE	42'0"	52'0"	WOOD	TYPE	42'0"	52'0"	WOOD	TYPE	42'0"	52'0"
	A	3x4	3x5		A	24	26		A	3-4,5	3-5
	B	3x6	3x7		B	46	48		B	5-6,3	5-7
	C	2x4	2x4		C	24	24		C	2-4,5	2-4
	D	5x6	5x6		D	46	47		D	5-6,3	5-7

Web Bracing Chart

Web Length	Bracing Required
To 7'9"	No Bracing
To 9'9"	1"x4" T-Bracing. Same grade and length as web; attach with 8d nails @ 8" p.o.
To 11'3"	2"x4" T-Bracing. Same grade and length as web; attach with 16d nails @ 8" o.c.

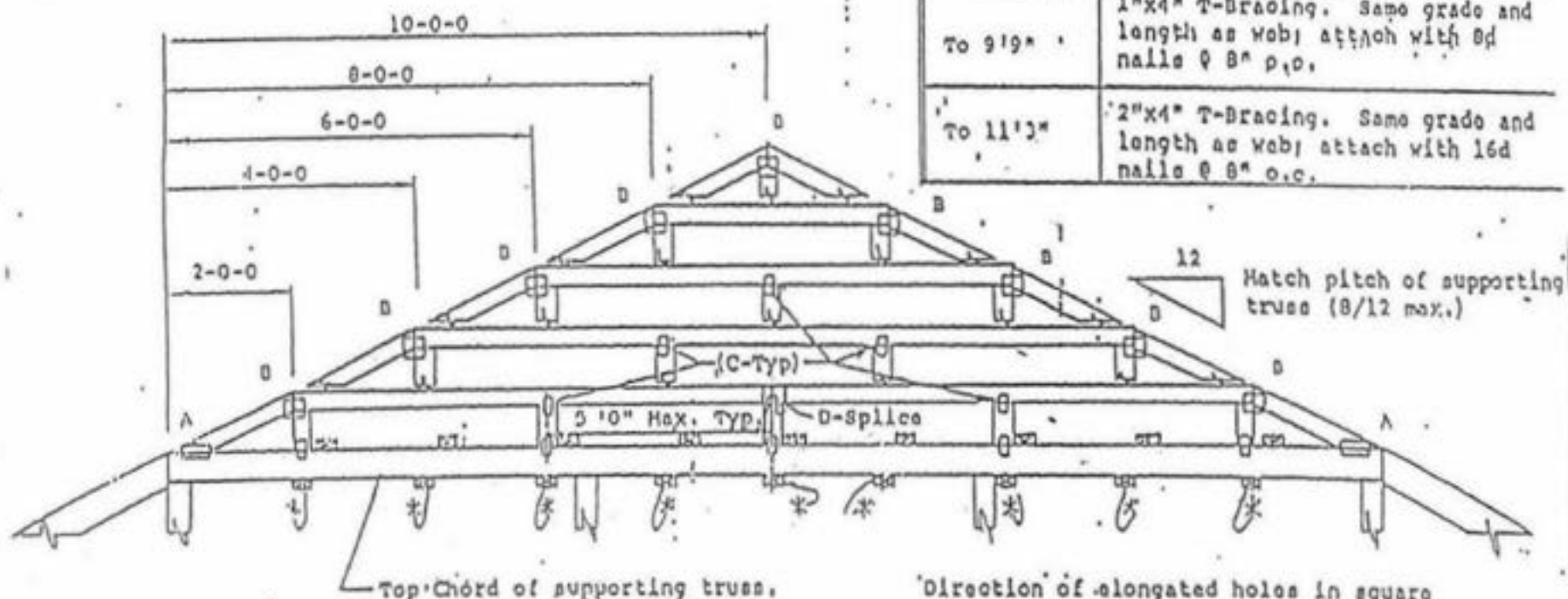


PLATE TYPE

Handwritten signature and date: 6-17-91

FURNISH A COPY OF THIS DESIGN TO ERECTION CONSTRUCTION

IMPORTANT! ALPINE ENGINEERING PRODUCTS, INC. SHALL NOT BE RESPONSIBLE FOR ANY CONSTRUCTION ERRORS OR OMISSIONS ON ANY CONSTRUCTION FROM THIS DESIGN. IT IS THE RESPONSIBILITY OF THE ERECTOR TO MAINTAIN THE TRUSS IN CONFORMANCE WITH THE DESIGN. CONTROL VARIATION BY THE ALPINE CONSTRUCTION AND

WARNING! THESE TRUSSES ARE DESIGNED FOR USE IN A SINGLE TRUSS AND NOT FOR USE IN A TRUSS SYSTEM. THE TRUSS IS NOT TO BE USED FOR ANY OTHER PURPOSE. THE TRUSS IS NOT TO BE USED FOR ANY OTHER PURPOSE. THE TRUSS IS NOT TO BE USED FOR ANY OTHER PURPOSE.

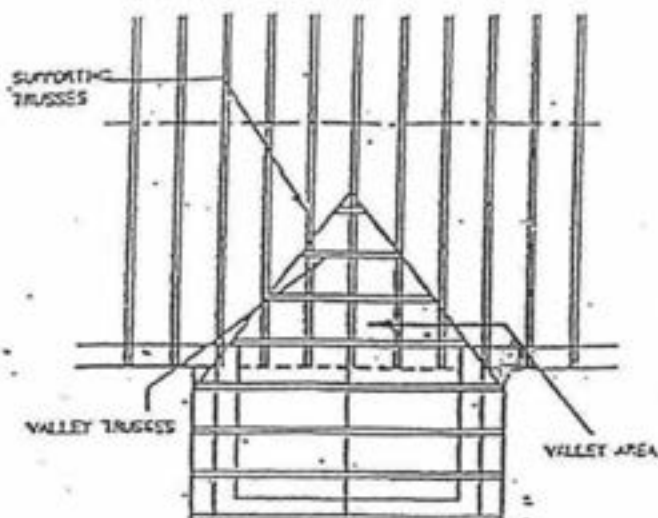
Handwritten signature: J. J. J.

DESIGN NO. TPI-70
TO LL 30 MF
TO OL 15 MF

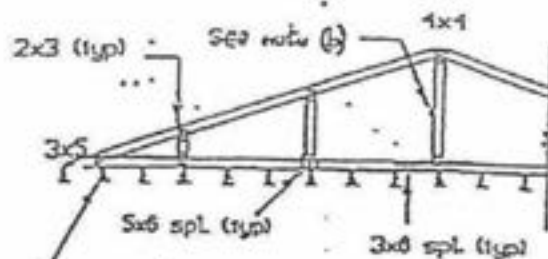
REF: 0439 EOK
DATE: 2/23/82
CNS: 660,182
(3/17/01)

STANDARD ROOF VALLEY DETAIL

PREPARED BY PEARS OFFICE



PARTIAL ROOF LAYOUT

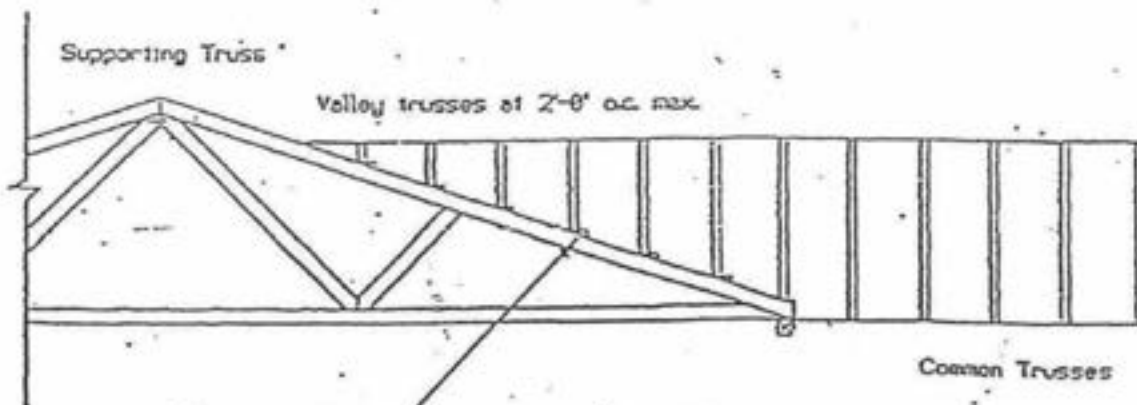


Supporting trusses at 2'-0" o.c. max

NOTES

- (a) Provide continuous bracing on verticals over 6'-3". Connect bracing to verticals w/2-8d nails and bracing must be tied to a fixed point at each end.
- (b) On spans over 24'-0", add verticals at 6'-0" o.c. max.

(c) STAYS @ 48" o/c. MAX.



6" wedge nailed to truss w/2-8d toe nails or Ewel cut bottom chord of valley truss

MINIMUM GRADE OF LUMBER

2X4 No. 2/D 19 SP
2X4 No. 2/D 19 SP
2X4 No. 3 19 S

TP1-92 Cr11.
SS8C-91



Mitek Industries Inc.

WIND (PSF)
L D
30 15
OH 8 10
WIND 24 Inch O.C.

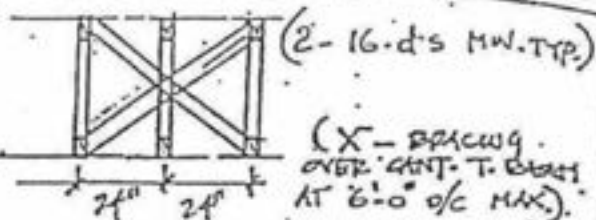
STR. INCR. : 33 %
DRAWN BY : P.G.
CHECKED BY : J.A.J.
REP. STRESS: YES



Approved for Mitek Industries Inc.

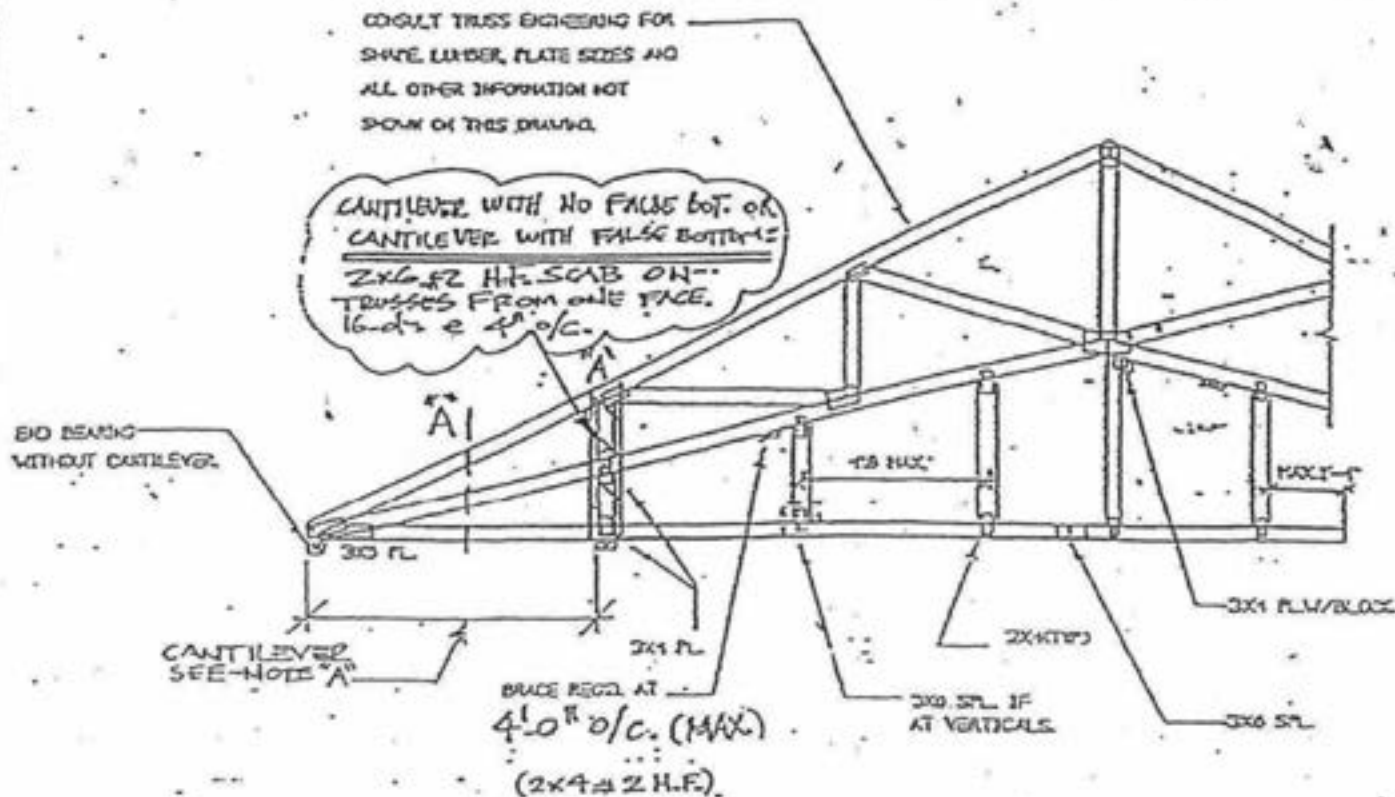
PREPARED BY NAME OFFICE

FOR CANTILEVERS UP TO 6'-0" LENGTH PROVIDE
BOTTOM CHORD BRACING AT 24" O/C MAX.
FOR 6'-0" AND GREATER CANTILEVERS, ADDITIONALLY
PROVIDE X-BRACING STARTING ABOVE THE TIE BEAM,
SPACED AT 6'-0" O/C MAX.



CONSULT TRUSS ENGINEERING FOR
SHAKE, LUMBER, PLATE SIZES AND
ALL OTHER INFORMATION NOT
SHOWN ON THIS DRAWING.

CANTILEVER WITH NO FALSE BOT. OR
CANTILEVER WITH FALSE BOTTOMS
2x6 @ 12" Ht. SCAB ON
TRUSSES FROM ONE FACE.
16' or e 20' o/c.



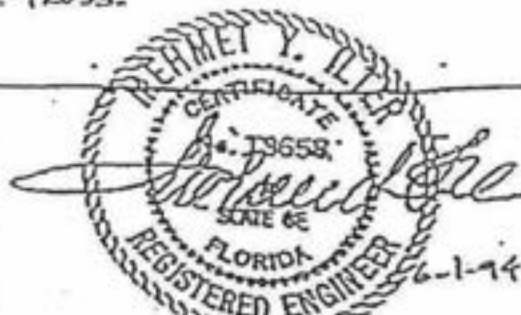
NOTE: TOP AND BOTTOM CHORD FILLERS ARE NOT A STRUCTURAL COMPONENT OF A TRUSS. PORTIONS OF, OR THE ENTIRE FILLER MAY BE REMOVED, OR ADDED TO, WITHOUT AFFECTING THE STRUCTURAL INTEGRITY OF THE TRUSS, PROVIDED THEY ARE NOT AFFECTING BEARING LOCATIONS OF THE TRUSS.

MINIMUM GRADE OF LUMBER					
T.C.	2X4	NO.2ND	19	SP	TP1-92 Cr11. SSAC-91
B.C.	2X4	NO.2ND	19	SP	
Webs	2X4	NO.3	19	SP	

LOADING	(PSF)	STR. INCR. 33 %
TOP	0	DRAWN BY: B.V.
BOTTOM	30 15	CHECKED BY: JAI
SPACING	8 10	REF. STRESS YES
24 inch O.C.		



HiTek Industries Inc.



Approved for Mulek Industries Inc.

WARNING: This safety alert symbol is used to attract your attention. PERSONAL SAFETY IS INVOLVED! When you see this symbol - BECOME ALERT - HEED ITS MESSAGE.

CAUTION: A CAUTION identifies safe operating practices or indicates unsafe conditions that could result in personal injury or damage to structures.

HIB-91 Summary Sheet

COMMENTARY and RECOMMENDATIONS for HANDLING, INSTALLING & BRACING METAL PLATE CONNECTED WOOD TRUSSES*

It is the responsibility of the builder, architect, building contractor, building engineer, owner or architect to ensure that the trusses are installed, handled, stored and braced in accordance with the design and specifications. The trusses must remain in the same high degree of safety as when they were manufactured. TPI does not intend these recommendations to be interpreted as a warranty of the product or the Engineer's design. For further information, consult the project architect and/or Engineer's design specifications for handling, installing and bracing wood trusses in a particular project. These recommendations are based upon the collective experience of leading truss fabricators.

CAUTION: The builder, building contractor, licensed contractor, erector or erection contractor is advised to obtain and read the entire booklet "Commentary and Recommendations for Handling, Installing & Bracing Metal Plate Connected Wood Trusses, HIB-91" from the Truss Plate Institute.

DANGER: A DANGER designates a condition where failure to follow instructions or heed warnings will most likely result in serious personal injury or death or damage to structures.

WARNING: A WARNING describes a condition where failure to follow instructions could result in severe personal injury or damage to structures.



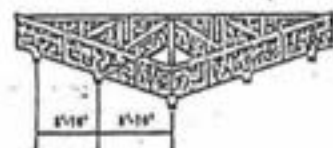
personnel in the wood truss industry, but must, due to the nature of the product, be presented as a guide to the use of a qualified building designer or erector. Thus, the Truss Plate Institute, Inc. accepts no responsibility for damage or injury from the use, application or reliance on the recommendations and information contained herein by building designers, erectors, and others. Copyright © by Truss Plate Institute, Inc. All rights reserved. This document or any part thereof must not be reproduced in any form without written permission of the publisher, printed in the United States of America.

CAUTION: All temporary bracing should be no less than 2x4 grade marked lumber. All connections should be made with minimum of 2-16d nails. All trusses assumed 2" on-center or less. All multi-ply trusses should be connected together in accordance with design drawings prior to installation.

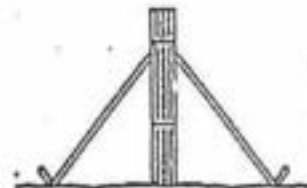
TRUSS STORAGE



CAUTION: Trusses should not be unloaded on rough terrain or uneven surfaces which could cause damage to the truss.



Trusses stored horizontally should be supported on blocking to prevent excessive lateral bending and lessen moisture gain.



Trusses stored vertically should be braced to prevent toppling or tipping.

WARNING: Do not break banding until installation begins or lift bundled trusses by the bands.

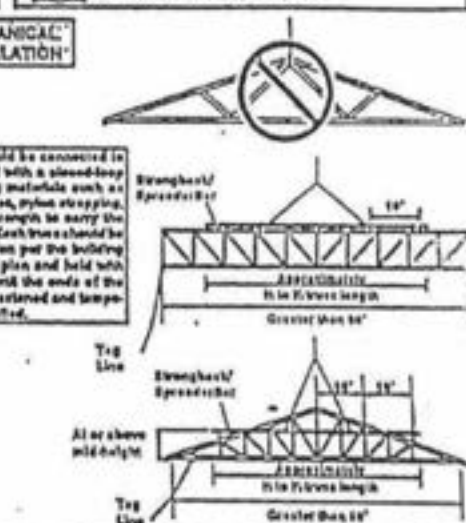
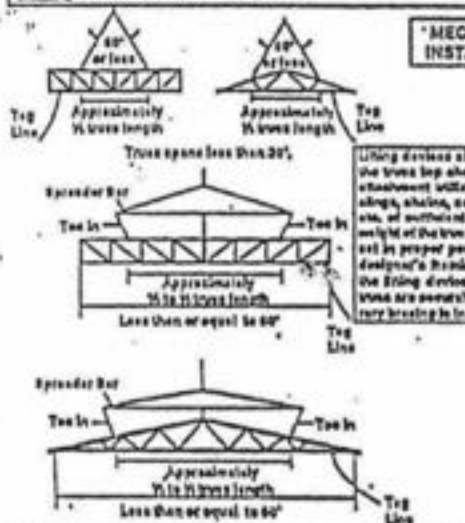
WARNING: Do not use damaged trusses.

DANGER: Do not store bundles upright unless properly braced.

DANGER: Walking on trusses which are lying flat is extremely dangerous and should be strictly prohibited.

WARNING: Do not attach cables, chains, or hooks to the web members.

WARNING: Do not lift single trusses with spans greater than 36' by the peak.



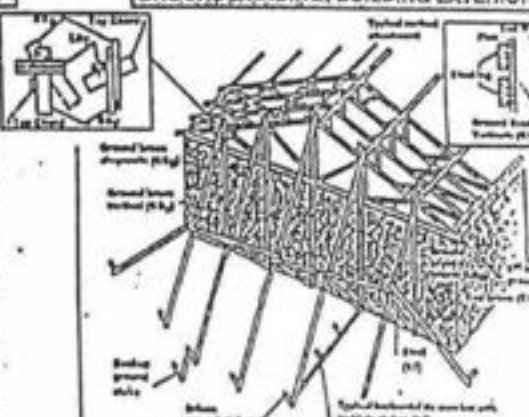
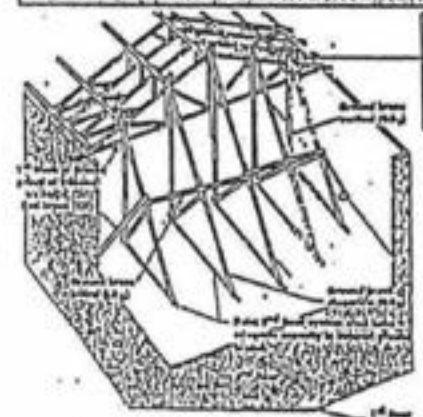
MECHANICAL INSTALLATION

Lifting devices should be connected to the truss top chord with a spreader-bar attachment utilizing materials such as slings, chains, cables, nylon slings, etc. of sufficient strength to carry the weight of the truss. Each truss should be set in proper position per the building designer's bracing plan and held with the string device until the ends of the truss are securely fastened and temporary bracing is installed.

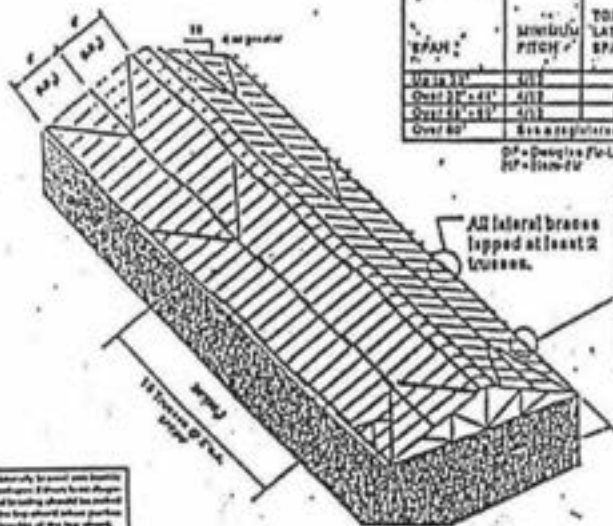
CAUTION: Temporary bracing shown in this summary sheet is adequate for the installation of trusses with similar configurations. Consult a registered professional engineer if a different bracing arrangement is desired. The engineer may design bracing in accordance with TPI's Recommended Design Specification for Temporary Bracing of Metal Plate Connected Wood Trusses, DSD-89, and in some cases determine that a wider spacing is possible.

GROUND BRACING BUILDING INTERIOR

GROUND BRACING BUILDING EXTERIOR

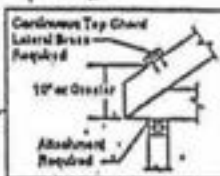


CAUTION: Ground bracing required for all installations.



SPAN	MINIMUM PITCH	TOP CHORD LATERAL BRACE SPACING (S _L)	TOP CHORD DIAGONAL BRACE SPACING (S _D)
Up to 30'	4/12	10'	10'
Over 30' to 40'	4/12	10'	10'
Over 40' to 50'	4/12	10'	10'
Over 50'	4/12	10'	10'

See a registered professional engineer
 DP = Douglas Fir-Larch
 SP = Southern Pine
 LP = Larch-Pine-Fir



All lateral braces lapped at least 2 trusses.

Top chords that are laterally braced are used to transfer and resist forces. If forces are applied to the chords, they should be braced to the structure at the top chord where forces are applied to the top chord.

PITCHED TRUSS

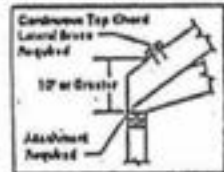


WARNING: Failure to follow these recommendations could result in severe personal injury or damage to trusses or buildings.

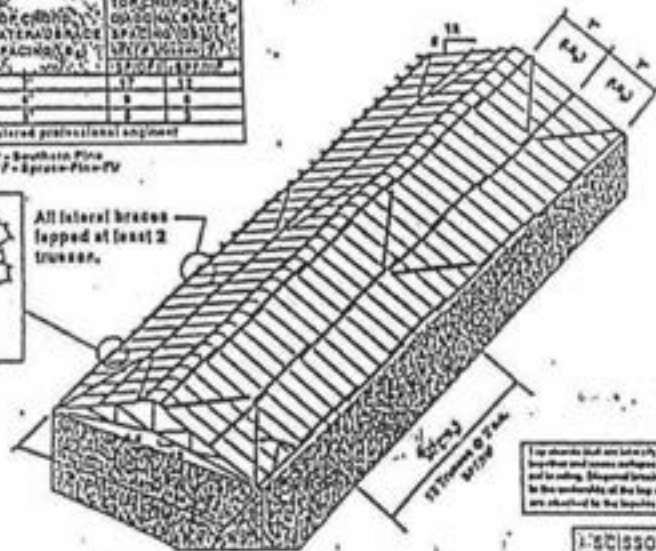


SPAN	MINIMUM PITCH	TOP CHORD LATERAL BRACE SPACING (S _L)	TOP CHORD DIAGONAL BRACE SPACING (S _D)
Up to 30'	4/12	10'	10'
Over 30' to 40'	4/12	10'	10'
Over 40' to 50'	4/12	10'	10'
Over 50'	4/12	10'	10'

See a registered professional engineer
 DP = Douglas Fir-Larch
 SP = Southern Pine
 LP = Larch-Pine-Fir



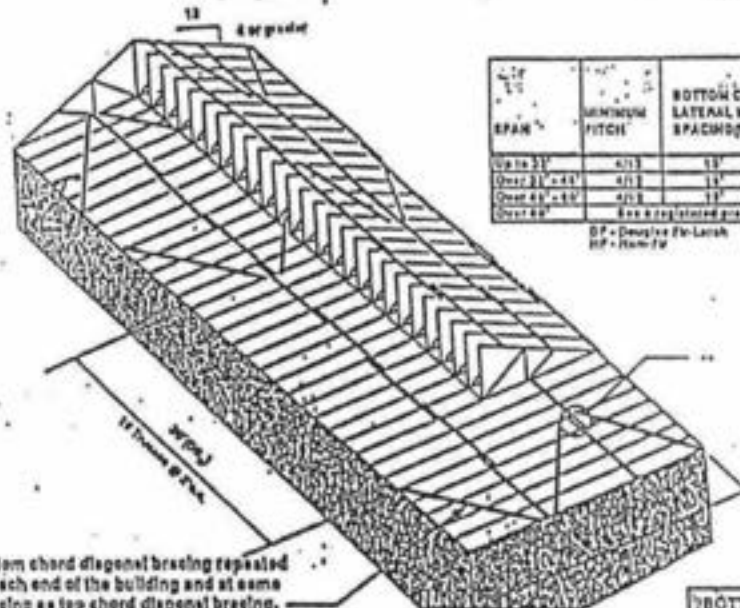
All lateral braces lapped at least 2 trusses.



Frame 3

Top chords that are laterally braced are used to transfer and resist forces. If forces are applied to the chords, they should be braced to the structure at the top chord where forces are applied to the top chord.

SCISSORS TRUSS



SPAN	MINIMUM PITCH	BOTTOM CHORD LATERAL BRACE SPACING (S _L)	BOTTOM CHORD DIAGONAL BRACE SPACING (S _D)
Up to 30'	4/12	10'	10'
Over 30' to 40'	4/12	10'	10'
Over 40' to 50'	4/12	10'	10'
Over 50'	4/12	10'	10'

See a registered professional engineer
 DP = Douglas Fir-Larch
 SP = Southern Pine
 LP = Larch-Pine-Fir

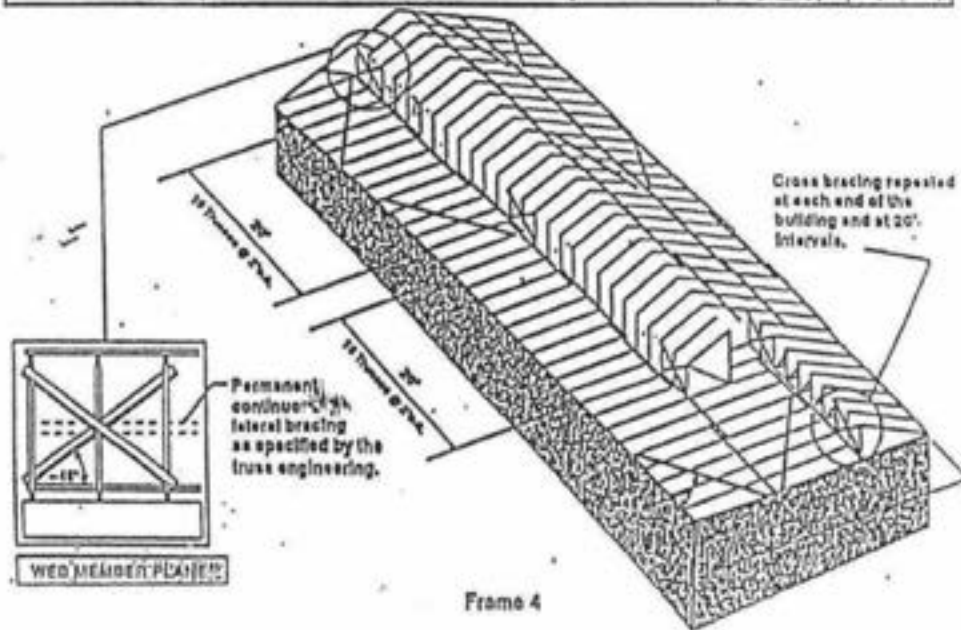
All lateral braces lapped at least 2 trusses.

Bottom chord diagonal bracing repeated at each end of the building and at same spacing as top chord diagonal bracing.

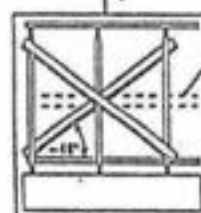
BOTTOM CHORD PLANE



WARNING: Failure to follow these recommendations could result in severe personal injury or damage to trusses or buildings.



Frame 4



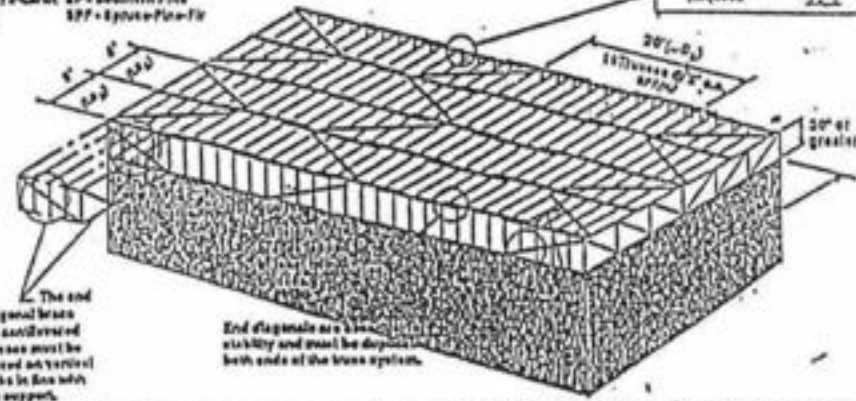
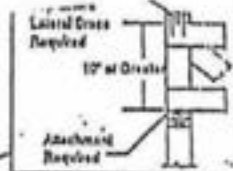
Permanent continuous lateral bracing as specified by the truss engineering.

Cross bracing repeated at each end of the building and at 20' intervals.

SPACE	MINIMUM PITCH	TOP CHORD LATERAL BRACE SPACING (S _B) (F Trusses)	TOP CHORD DIAGONAL BRACE SPACING (S _D) (F Trusses)
Up to 24"	3/12	8'	12'
Over 24" to 48"	4/12	7'	10'
Over 48" to 60"	5/12	6'	8'
Over 60"	6/12	5'	6'

DF = Douglas Fir-Larch
SP = Southern Pine
SPP = Spruce-Pine-Fir

Top chords that are laterally braced and braced together and secure endframes if there is no diagonal bracing. Diagonal bracing should be added to the endframes of the top chord where possible and attached to the trusses of the top chord.

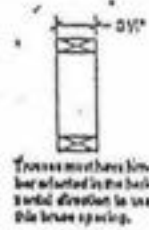
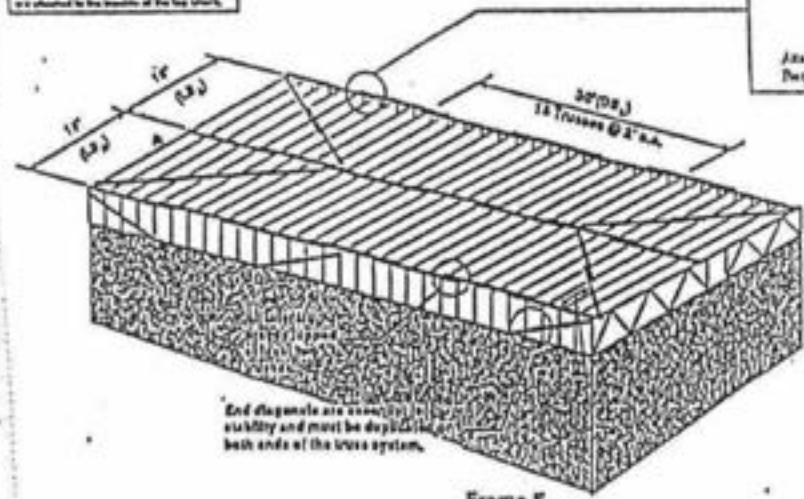
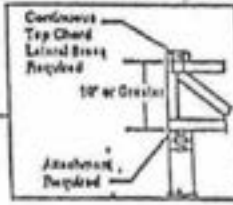


The end diagonal braces for endbraced trusses must be placed on vertical webs in line with the supports.

WARNING: Failure to follow these recommendations could result in severe personal injury or damage to trusses or buildings.

4x2 PARALLEL CHORD TRUSS TOP CHORD

Top chords that are laterally braced and braced together and secure endframes if there is no diagonal bracing. Diagonal bracing should be added to the endframes of the top chord where possible and attached to the trusses of the top chord.



Frame 5

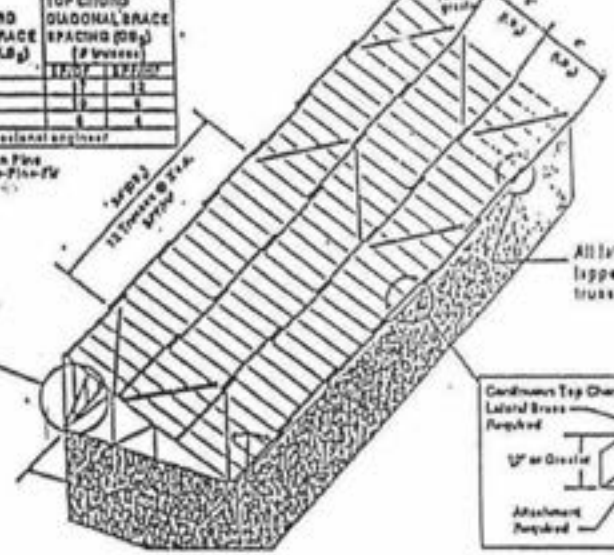
SPACE	MINIMUM PITCH	TOP CHORD LATERAL BRACE SPACING (S _B) (F Trusses)	TOP CHORD DIAGONAL BRACE SPACING (S _D) (F Trusses)
Up to 24"	3/12	8'	12'
Over 24" to 48"	4/12	7'	10'
Over 48" to 60"	5/12	6'	8'
Over 60"	6/12	5'	6'

DF = Douglas Fir-Larch
SP = Southern Pine
SPP = Spruce-Pine-Fir

Top chords that are laterally braced and braced together and secure endframes if there is no diagonal bracing. Diagonal bracing should be added to the endframes of the top chord where possible and attached to the trusses of the top chord.

SPRUE TRUSSES

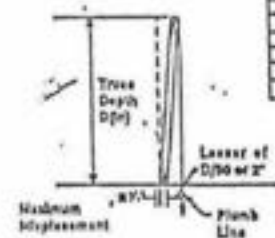
Diagonal brace also required on end verticals.



WARNING: Failure to follow these recommendations could result in severe personal injury or damage to trusses or buildings.

INSTALLATION TOLERANCES

PLUMB	Column	Diaphragm	Diaphragm
12"	1/16"	1/8"	1/8"
24"	1/16"	1/8"	1/8"
36"	1/16"	1/8"	1/8"
48"	1/16"	1/8"	1/8"
72"	1/16"	1/8"	1/8"
96"	1/16"	1/8"	1/8"
120"	1/16"	1/8"	1/8"
144"	1/16"	1/8"	1/8"



Length L (ft)	Lesser of L/700 or 2"
12'	1/8"
24'	1/8"
36'	1/8"
48'	1/8"
72'	1/8"
96'	1/8"
120'	1/8"
144'	1/8"

OUT-OF-PLUMB INSTALLATION TOLERANCES

WARNING: Do not cut trusses.

OUT-OF-PLANE INSTALLATION TOLERANCES

DANGER: Under no circumstances should construction loads of any description be placed on unbraced trusses.

Frame 6

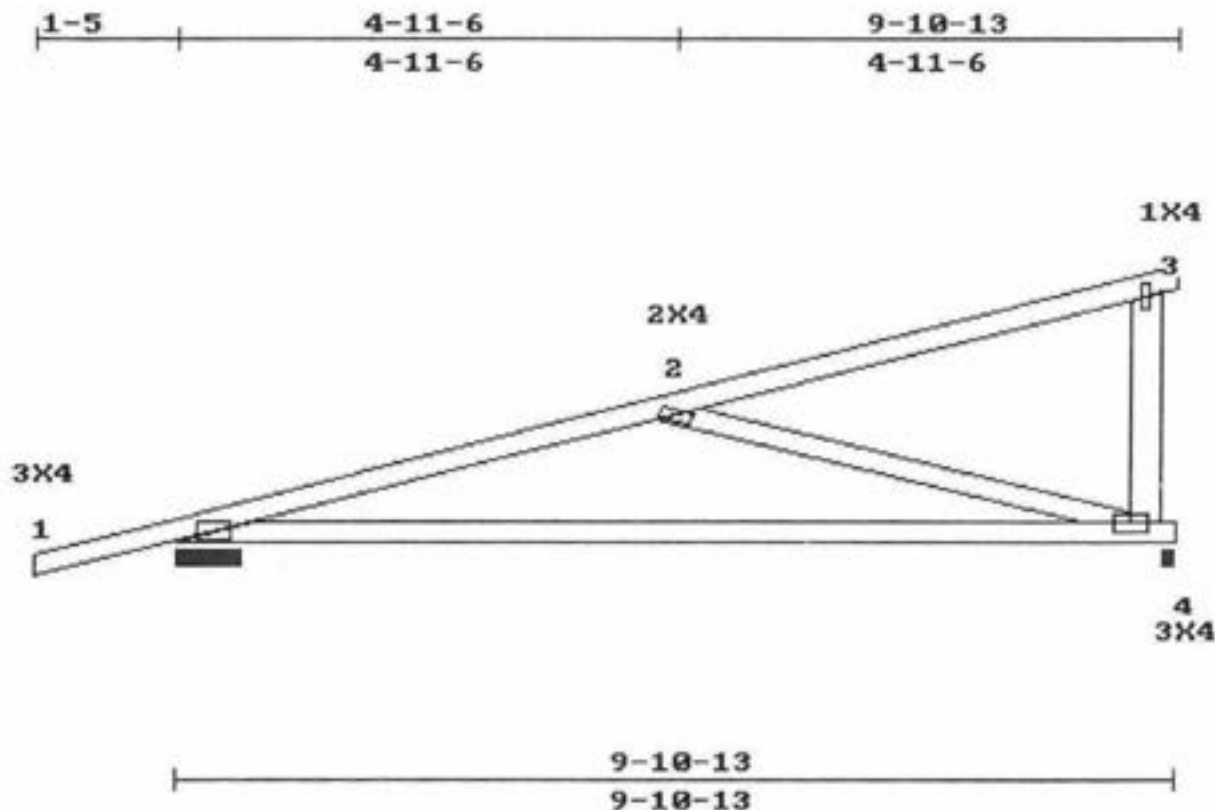
====<<<ACES-32 Ver. 1.1>>>=====[001807]====<<<FL.QUALITY-MMH>>>=====
 Customer : FOGLIA Thu Mar 1 08:52:09 2001
 Project #: 00469 Truss ID : CJ7C Family #: 204
 Span : 9-10-13 B.Span : 7-0 Quant. : 2 Top Pitch : 4.243
 ==Bld(1/21/2000),v1.1=====

FORCES - LOAD CASE #1
 1-2=-777 4-1= 725 2-4=-772
 2-3= 0
 3-4=-406

REACTIONS - SIZE
 1=-693 8.00
 4=-791 1.50

(1)

PROVIDE FOR 996 LBS UPLIFT AT JOINT 1 (1.44)
 PROVIDE FOR 1229 LBS UPLIFT AT JOINT 4 (1.55)
 PROVIDE FOR 409 LBS HORIZ. REACTION AT JOINT 1



L. HL TO PK:10-6
 LEFT HEIGHT:0-3-15 SPAN:9-10-13 RISE:3-9-15 RIGHT HEIGHT:3-9-15

LOADING (PSF)		MAX STRESSES		MINIMUM GRADE OF LUMBER	
TOP	30	TOP	2-3=0.767	TOP CHORD:2X4	No.2ND 19 SP
BOTT	0	BOTT	4-1=0.769	BOT CHORD:2X4	No.1D 19 SP
		LL.DEFL.@0=0.00	< L/360	WEBS	:2X4 No.3 19 SP

REPETITIVE STRESSES NOT USED

SPACING : 24.0 in. o. c.
 NO. OF MEMBERS = 1

LOADING	STRESS INCREASE	LOADING	PANEL(PLF) / JOINTS(LBS)
LUMBER	1.33	PLATE	1- 2= 59 2- 3= 166 4- 1= 25
1	1.33	UNIFORM	

RIGHT END VERTICAL IS NOT EXPOSED.
 TRUSS HAS BEEN CHECKED FOR 145 M.P.H. WIND LOAD, WALL HEIGHT 16 FT, ENCLOSED BLDG. TYPE, ASSUMING 10 PSF TOTAL D.L.(7*3) (SBC-97).
 DEFLECTION(IN.) L.L= 0.00,D.L=0.00,T.L=0.00
 PLATES ARE MITEK M20-249,200 MANUFACTURED FROM ASTM A 446 GRD A GALVANIZED STEEL(EXCEPT AS SHOWN)
 PLATE MUST BE INSTALLED ON EA. FACE OF JOINT,SYMMETRICALLY(EXCEPT AS SHOWN)DESIGN CONFORMS W/NDS DESIGN SPECS, SSB,ANSI/TPI-95
 THIS DESIGN IS FOR TRUSS FABRICATION ONLY.FOR PERMANENT AND TEMPORARY BRACING(WHICH IS ALWAYS REQD)CONSULT BLDG ARCHITECT OR ENGINEER.

MAR 01 2001

====<<<ACES-32 Ver. 1.1>>>=====[001808]====<<<FL.QUALITY-MMH>>>=====
 Customer : FOGLIA Thu Mar 1 08:52:29 2001
 Project # : 00469 Truss ID : C1 Family # : 205
 Span : 1-0 B.Span : 1-0 Quant. : 18 Top Pitch : 6/12
 ==Bld(1/21/2000),v1.1=====

FORCES - GRAVITY LOADS

1-2= 0
 2-3= 0

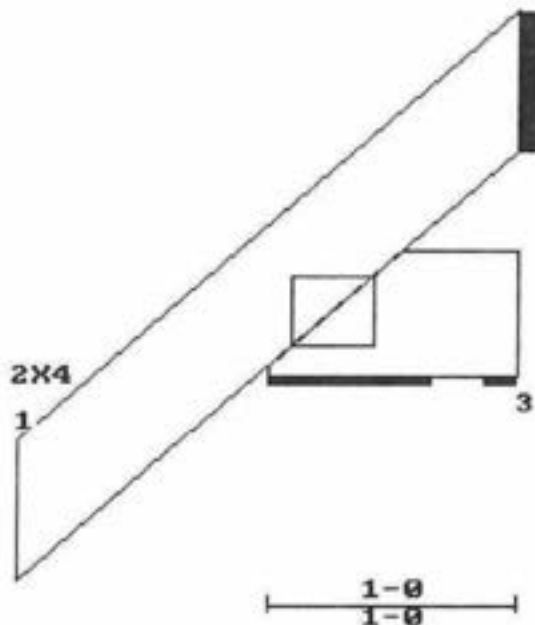
REACTIONS - SIZE

1=143 8.00
 3=6 1.50
 2=27 1.50

②

PROVIDE FOR 134 LBS UPLIFT AT JOINT 1 (0.94)
 PROVIDE FOR 52 LBS UPLIFT AT JOINT 2 (1.93)
 PROVIDE FOR 54 LBS HORIZ. REACTION AT JOINT 1

1-0 1-0
 1-0



L. HL TO PK:1-1-7
 LEFT HEIGHT:0-4-3

SPAN:1-0

RISE:0-10-3

RIGHT HEIGHT:0-10-3

LOADING (PSF)

TOP 30 15
 BOTT 0 10

MAX STRESSES

TOP 1-2=0.014
 BOTT 3-1=0.002
 LL.DEFL. < L/360

MINIMUM GRADE OF LUMBER

TOP CHORD:2X4 No.2ND 19 SP
 BOT CHORD:2X4 No.2ND 19 SP
 WEBS :2X4 No.3 19 SP

STR.INC.: LUMB = 1.33 PLATE = 1.33
 REPETITIVE STRESSES USED

SPACING : 24.0 in. o. c.
 NO. OF MEMBERS = 1

TRUSS HAS BEEN CHECKED FOR 145 M.P.H. WIND LOAD, WALL HEIGHT 16 FT, ENCLOSED BLDG. TYPE, ASSUMING 10 PSF TOTAL D.L.(7+3) (SBC-97).
 DEFLECTION(IN.) L.L= 0.00,D.L=0.00,T.L=0.00

PLATES ARE MITEK M20-249,200 MANUFACTURED FROM ASTM A 446 GRD A GALVANIZED STEEL(EXCEPT AS SHOWN)

PLATE MUST BE INSTALLED ON EA. FACE OF JOINT,SYMMETRICALLY(EXCEPT AS SHOWN)DESIGN CONFORMS W/ND S DESIGN SPECS, SSBC,ANSI/TP1-95

THIS DESIGN IS FOR TRUSS FABRICATION ONLY.FOR PERMANENT AND TEMPORARY BRACING(WHICH IS ALWAYS REQD)CONSULT BLDG ARCHITECT OR ENGINEER.

MAR 01 2001

====<<<<ACES-32 Ver. 1.1>>>>=====[001809]=====<<<<FL.QUALITY-MMH>>>>=====

Customer : FOGLIA	Truss ID : C3	Thu Mar 1 08:52:51 2001
Project #: 00469	Quantity : 17	Family # : 205
Span : 3-0		Top Pitch : 6/12

==Bld(1/21/2000),v1.1=====

FORCES - GRAVITY LOADS

1-2= 0 3-1= 0
2-3= 0

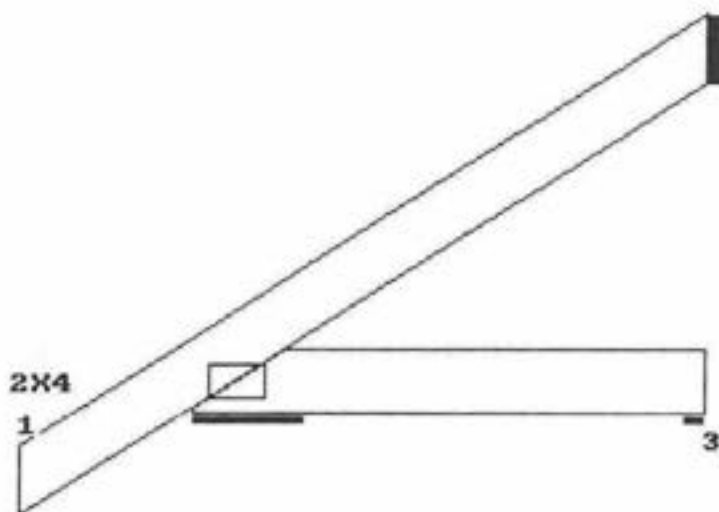
REACTIONS - SIZE

1=-253 8.00
3=-26 1.50
2=-117 1.50

②

PROVIDE FOR 205 LBS UPLIFT AT JOINT 1 (0.81)
PROVIDE FOR 183 LBS UPLIFT AT JOINT 2 (1.56)
PROVIDE FOR 172 LBS HORIZ. REACTION AT JOINT 1
(SYMMETRIC PLATING TURNED ON)

1-0 3-0
3-0



3-0
3-0

L. HL TO PK:3-4-4
LEFT HEIGHT:0-4-3

SPAN:3-0

RISE:1-10-3

RIGHT HEIGHT:1-10-3

LOADING (PSF)

	L	D
TOP	30	15
BOTT	0	10

MAX STRESSES

TOP	1-2=0.225
BOTT	3-1=0.034
LL.DEFL.	< L/360

MINIMUM GRADE OF LUMBER

TOP CHORD:2X4	No.2ND 19 SP
BOT CHORD:2X4	No.2ND 19 SP
WEBS	:2X4 No.3 19 SP

STR.INC.: LUMB = 1.33 PLATE = 1.33
REPETITIVE STRESSES USED

SPACING : 24.0 in. o. c.
NO. OF MEMBERS = 1

TRUSS HAS BEEN CHECKED FOR 145 M.P.H. WIND LOAD, WALL HEIGHT 16 FT, ENCLOSED BLDG. TYPE, ASSUMING 10 PSF TOTAL D.L.(7+3) (SBC-97).
DEFLECTION(IN.) L.L.= 0.00,D.L.=0.00,T.L.=0.00

PLATES ARE MITEK M20-249,200 MANUFACTURED FROM ASTM A 446 GRD A GALVANIZED STEEL(EXCEPT AS SHOWN)

PLATE MUST BE INSTALLED ON EA. FACE OF JOINT,SYMMETRICALLY(EXCEPT AS SHOWN)DESIGN CONFORMS W/NDS DESIGN SPECS, SSB,ANSI/TPI-95

THIS DESIGN IS FOR TRUSS FABRICATION ONLY.FOR PERMANENT AND TEMPORARY BRACING(WHICH IS ALWAYS REQD)CONSULT BLDG ARCHITECT OR ENGINEER.

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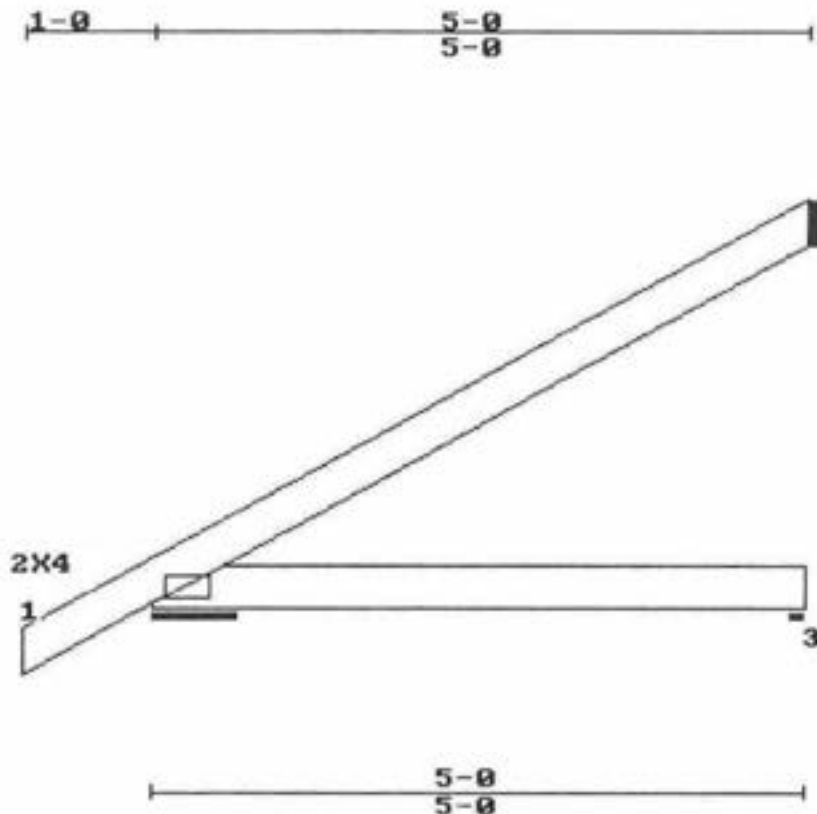
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====<<<<ACES-32 Ver. 1.1>>>>=====[001810]====<<<<FL.QUALITY-MMH>>>>=====
 Customer : FOGLIA Thu Mar 1 08:53:02 2001
 Project #: 00469 Truss ID : C5 Family #: 205
 Span : 5-0 Quantity : 11 Top Pitch : 6/12
 ==Bld(1/21/2000),v1.1=====

FORCES - GRAVITY LOADS		REACTIONS - SIZE	
1-2= 0	3-1= 0	1=363	8.00
2-3= 0		3=46	1.50
		2=207	1.50

④

PROVIDE FOR 273 LBS UPLIFT AT JOINT 1 (0.75)
 PROVIDE FOR 316 LBS UPLIFT AT JOINT 2 (1.53)
 PROVIDE FOR 289 LBS HORIZ. REACTION AT JOINT 1
 (SYMMETRIC PLATING TURNED ON)



LOADING (PSF)		MAX STRESSES		MINIMUM GRADE OF LUMBER	
TOP	30	TOP	1-2=0.702	TOP CHORD:2X4	No.2ND 19 SP
BOTT	0	BOTT	3-1=0.103	BOT CHORD:2X4	No.2ND 19 SP
		LL.DEFL.	< L/360	WEBS	:2X4 No.3 19 SP

STR.INC.: LUMB = 1.33 PLATE = 1.33 SPACING : 24.0 in. o. c.
 REPETITIVE STRESSES USED NO. OF MEMBERS = 1

TRUSS HAS BEEN CHECKED FOR 145 M.P.H. WIND LOAD, WALL HEIGHT 16 FT, ENCLOSED BLDG. TYPE, ASSUMING 10 PSF TOTAL D.L.(7+3) (SBC-97).
 DEFLECTION(IN.) L.L= 0.00,D.L=0.00,T.L=0.00
 PLATES ARE MITEK M20-249,200 MANUFACTURED FROM ASTM A 446 GRD A GALVANIZED STEEL(EXCEPT AS SHOWN)
 PLATE MUST BE INSTALLED ON EA. FACE OF JOINT,SYMMETRICALLY(EXCEPT AS SHOWN)DESIGN CONFORMS W/NDS DESIGN SPECS, SIBC,ANSI/TP1-95
 THIS DESIGN IS FOR TRUSS FABRICATION ONLY.FOR PERMANENT AND TEMPORARY BRACING(WHICH IS ALWAYS REQD)CONSULT BLDG ARCHITECT OR ENGINEER.

[Signature]
 MAR 01 2001

====<<<<ACES-32 Ver. 1.1>>>>=====[001811]=====<<<<FL.QUALITY-MMH>>>>=====

Customer : FOGLIA

Project #: 00469

Span : 7-0

Truss ID : J7

Quantity : 23

Thu Mar 1 08:53:11 2001

Family # : 205

Top Pitch : 6/12

==Bld(1/21/2000),v1.1=====

FORCES - GRAVITY LOADS

1-2= 0 3-1= 0
2-3= 0

REACTIONS - SIZE

1=-473 8.00
3=-66 1.50
2=-297 1.50

5

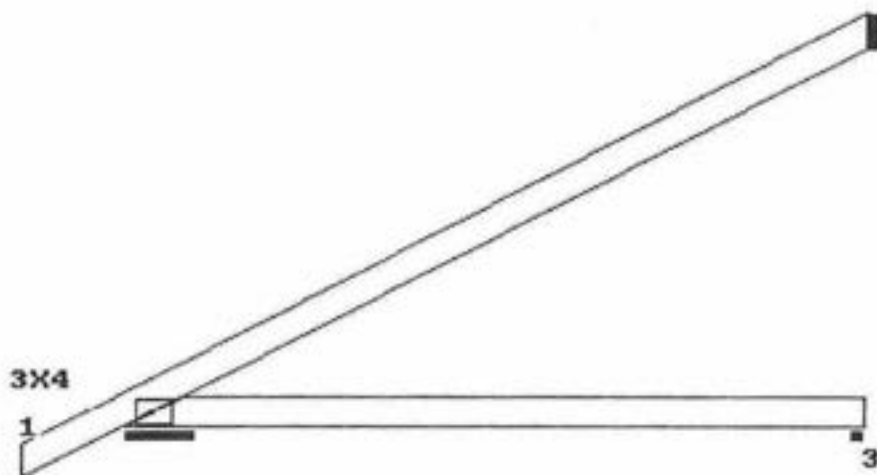
PROVIDE FOR 341 LBS UPLIFT AT JOINT 1 (0.72)

PROVIDE FOR 449 LBS UPLIFT AT JOINT 2 (1.51)

PROVIDE FOR 407 LBS HORIZ. REACTION AT JOINT 1

(SYMMETRIC PLATING TURNED ON)

1-0 7-0
7-0



7-0
7-0

L. HL TO PK:7-9-15

LEFT HEIGHT:0-4-3

SPAN:7-0

RISE:3-10-3

RIGHT HEIGHT:3-10-3

LOADING (PSF)

L D

TOP 30 15

BOTT 0 10

MAX STRESSES

TOP 1-2=0.993

BOTT 3-1=0.211

LL.DEFL. < L/360

MINIMUM GRADE OF LUMBER

TOP CHORD:2X4 No.1D 19 SP

BOT CHORD:2X4 No.2ND 19 SP

WEBS :2X4 No.3 19 SP

STR.INC.: LUMB = 1.33 PLATE = 1.33

REPETITIVE STRESSES USED

SPACING : 24.0 in. o. c.

NO. OF MEMBERS = 1

TRUSS HAS BEEN CHECKED FOR 145 M.P.H. WIND LOAD, WALL HEIGHT 16 FT, ENCLOSED BLDG. TYPE, ASSUMING 10 PSF TOTAL D.L.(7*3) (SBC-97).

DEFLECTION(IN.) L.L= 0.00,D.L=0.00,T.L=0.00

PLATES ARE MITEK M20-249,200 MANUFACTURED FROM ASTM A 446 GRD A GALVANIZED STEEL(EXCEPT AS SHOWN)

PLATE MUST BE INSTALLED ON EA. FACE OF JOINT,SYMMETRICALLY(EXCEPT AS SHOWN)DESIGN CONFORMS W/NDS DESIGN SPECS, SSSC,ANSI/TPI-95

THIS DESIGN IS FOR TRUSS FABRICATION ONLY.FOR PERMANENT AND TEMPORARY BRACING(WHICH IS ALWAYS REQD)CONSULT BLDG ARCHITECT OR ENGINEER.

MAR 01 2001

====<<<ACES-32 Ver. 1.1>>>===== [001812] =====<<<FL.QUALITY-MMH>>>=====

Customer : FOGLIA Truss ID : H8 Thu Mar 1 08:53:27 2001

Project #: 00469 Quantity : 1 Family #: 314

Span : 24-0 Top Pitch : 6/12

==Bld(1/21/2000),v1.1=====

FORCES - LOAD CASE #1			REACTIONS - SIZE	
1-2=5255	7-8=5255	8-9= 4576	2-13= 67	5-10=1080
2-3=5023		9-10= 4576	2-12=103	6-10= 1764
3-4=4484		10-11= 5228	3-12=1764	7-10=103
4-5=5279		11-12= 5228	4-12=1080	7-9= 67
5-6=4484		12-13= 4576	4-11= 124	
6-7=5023		13-1= 4576	5-11= 124	

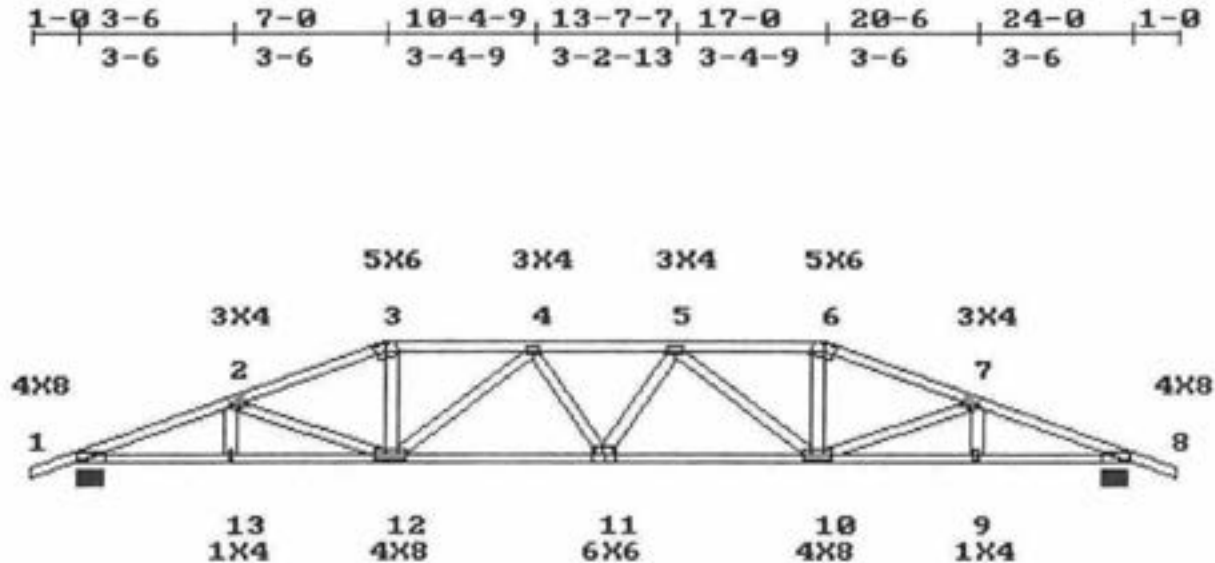
6

PROVIDE FOR 2744 LBS UPLIFT AT JOINT 1 (0.96)

PROVIDE FOR 2744 LBS UPLIFT AT JOINT 8 (0.96)

PROVIDE FOR 119 LBS HORIZ. REACTION AT JOINT 1

PLATE OFFSETS (X=LEFT,Y=TOP):[j3=2.5,2.5],[j6=3.5,2.5],[j11=3,2.5],(SYMMETRIC PLATING TURNED ON)



3-6		7-0		12-0		17-0		20-6		24-0	
3-6		3-6		5-0		5-0		3-6		3-6	

L. HL TO PK:7-9-15 LEFT HEIGHT:0-4-3 SPAN:24-0 RISE:3-10-3 R. HL TO PK:7-9-15 RIGHT HEIGHT:0-4-3

LOADING (PSF)		MAX STRESSES		MINIMUM GRADE OF LUMBER	
TOP	30 15	TOP	7-8=0.908	TOP CHORD:2X4	No.1D 19 SP
BOTT	0 10	BOTT	10-11=0.997	BOT CHORD:2X4	No.1D 19 SP
		LL.DEFL.@11=0.25 < L/360	WEBS	:2X4	No.3 19 SP

REPETITIVE STRESSES NOT USED

SPACING : 24.0 in. o. c.

NO. OF MEMBERS = 1

LOADING STRESS INCREASE	LOADING LUMBER PLATE	LOADING TYPE	PANEL(PLF) / JOINTS(LBS)
1	1.33 1.33	UNIFORM	1- 3= 90 3- 6= 203 6- 8= 90 8-10= 20 10-12= 45 12- 1= 20
		CONCENTRATED	10= 787 12= 787

TRUSS HAS BEEN CHECKED FOR 145 M.P.H. WIND LOAD, WALL HEIGHT 16 FT, ENCLOSED BLDG. TYPE, ASSUMING 10 PSF TOTAL D.L.(7+3) (SBC-97).

DEFLECTION(IN.) L.L= 0.25,D.L=0.21,T.L=0.45

PLATES ARE MITEK M20-249,200 MANUFACTURED FROM ASTM A 446 GRD A GALVANIZED STEEL(EXCEPT AS SHOWN)

PLATE MUST BE INSTALLED ON EA. FACE OF JOINT,SYMMETRICALLY(EXCEPT AS SHOWN)DESIGN CONFORMS W/NDIS DESIGN SPECS, SSSC,ANSI/TPI-95

THIS DESIGN IS FOR TRUSS FABRICATION ONLY.FOR PERMANENT AND TEMPORARY BRACING(WHICH IS ALWAYS REQD)CONSULT BLDG ARCHITECT OR ENGINEER.

[Signature]

March 1 2001

====<<<<ACES-32 Ver. 1.1>>>>===== [001813] =====<<<<FL.QUALITY-MMH>>>>=====

Customer : FOGLIA	Truss ID : H1	Thu Mar 1 08:53:39 2001
Project #: 00469	Quantity : 1	Family #: 314
Span : 24-0		Top Pitch : 6/12

==Bld(1/21/2000),v1.1=====

FORCES - GRAVITY LOADS

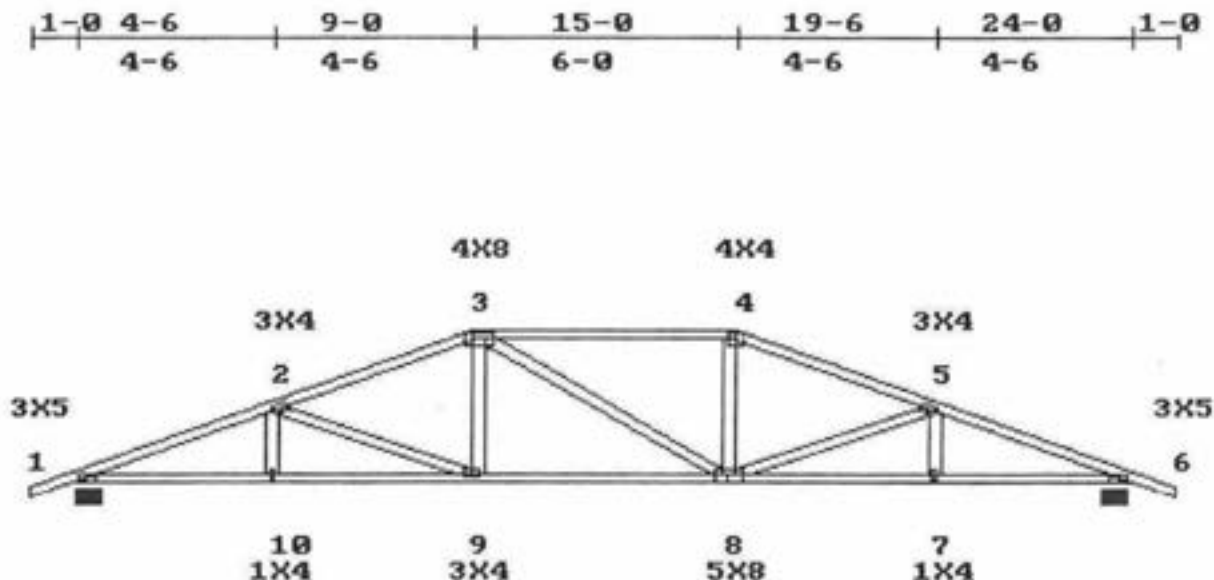
1-2=2190	6-7= 1920	2-10= 87	5-7= 87
2-3=1719	7-8= 1920	2-9=432	
3-4=1535	8-9= 1535	3-9= 301	
4-5=1719	9-10= 1920	3-8= 0	
5-6=2190	10-1= 1920	4-8= 301	
		5-8=432	

REACTIONS - SIZE

1=1393	8.00
6=1393	8.00

⑦

PROVIDE FOR 1196 LBS UPLIFT AT JOINT 1 (0.86)
 PROVIDE FOR 1196 LBS UPLIFT AT JOINT 6 (0.86)
 PROVIDE FOR 153 LBS HORIZ. REACTION AT JOINT 1
 PLATE OFFSETS (X=LEFT,Y=TOP):[JB=4,2],(SYMMETRIC PLATING TURNED ON)



4-6	9-0	15-0	19-6	24-0
4-6	4-6	6-0	4-6	4-6

L. HL TO PK:10-0-12
 LEFT HEIGHT:0-4-3

SPAN:24-0

RISE:4-10-3

R. HL TO PK:10-0-12
 RIGHT HEIGHT:0-4-3

LOADING (PSF)

	L	D
TOP	30	15
BOTT	0	10

MAX STRESSES

TOP	3-4=0.966
BOTT	9-10=0.493
LL.DEFL.@9	=0.08 < L/360

MINIMUM GRADE OF LUMBER

TOP CHORD:	2X4	No.2ND 19 SP
BOT CHORD:	2X4	No.2ND 19 SP
WEBS	2X4	No.3 19 SP

STR.INC.: LUMB = 1.33 PLATE = 1.33
 REPETITIVE STRESSES USED

SPACING : 24.0 in. o. c.
 NO. OF MEMBERS = 1

CHORD: 3-4 TO BE 2X4 No.1D 19 SP

TRUSS HAS BEEN CHECKED FOR 145 M.P.H. WIND LOAD, WALL HEIGHT 16 FT, ENCLOSED BLDG. TYPE, ASSUMING 10 PSF TOTAL D.L.(7+3) (SBC-97).
 DEFLECTION(IN.) L.L= 0.08,D.L=0.07,T.L=0.15

PLATES ARE MITEK M20-249,200 MANUFACTURED FROM ASTM A 446 GRD A GALVANIZED STEEL(EXCEPT AS SHOWN)

PLATE MUST BE INSTALLED ON EA. FACE OF JOINT,SYMMETRICALLY(EXCEPT AS SHOWN)DESIGN CONFORMS W/NDS DESIGN SPECS, SSSC,ANSI/TPI-95

THIS DESIGN IS FOR TRUSS FABRICATION ONLY.FOR PERMANENT AND TEMPORARY BRACING(WHICH IS ALWAYS REQD)CONSULT BLDG ARCHITECT OR ENGINEER.

[Signature]
 MAR 01 2001

====<<<<ACES-32 Ver. 1.1>>>>===== [002018] =====<<<<FL.QUALITY-MMH>>>>=====

Customer : FOGLIA Wed Mar 7 15:08:57 2001

Project #: 00469 Truss ID : H2 Family # : spec.

Span : 24-0 B.Span : 24-0 Quant. : 1 Top Pitch : 6/12

Bot pitch : 90/12

==Bld(1/21/2000),v1.1=====

FORCES - LOAD CASE #1										REACTIONS - SIZE	
1-2=3998	7-8=10061	10-11=3519	16-17=8648	2-20=57	5-16=2308	8-12=4402	1=2181	8.00			
2-3=3770	8-9=3770	11-12=3519	17-18=19	2-19=179	6-16=0	9-12=179	10=2181	8.00			
3-4=10061	9-10=3998	12-13=0	18-19=0	3-19=4402	6-15=2308	9-11=57					
4-5=5612		13-14=19	19-20=3519	3-17=5795	7-15=4046	14-12=5660					
5-6=5008		14-15=8648	20-1=3519	4-17=3527	7-14=3527	17-19=5660					
6-7=5612		15-16=5008		4-16=4046	8-14=5795						

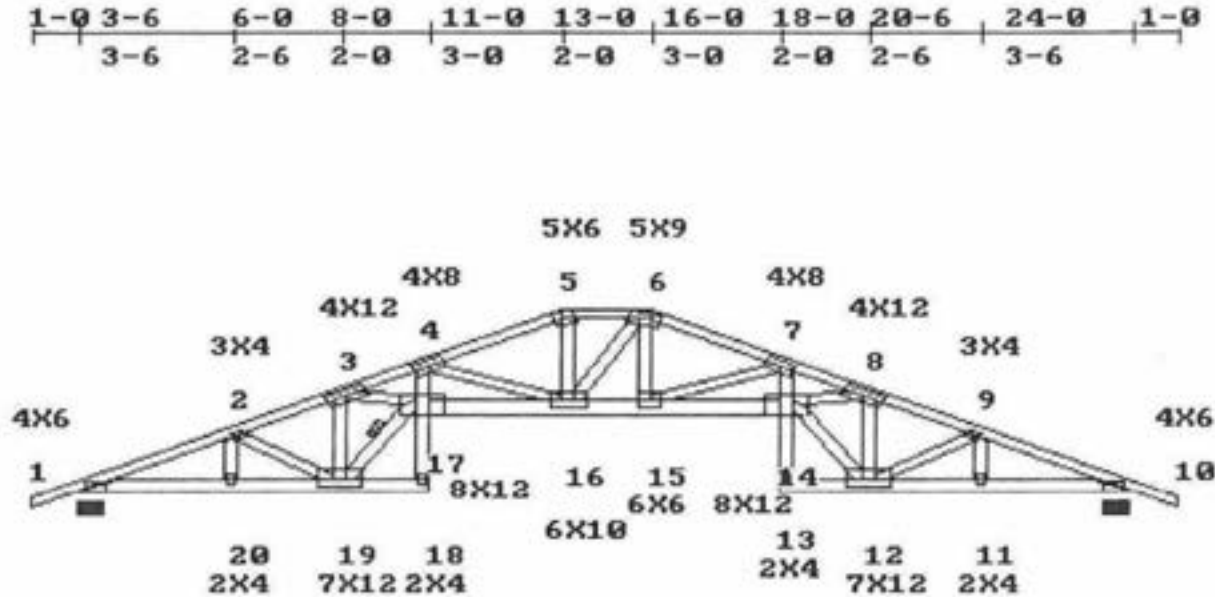
8

PROVIDE FOR 1870 LBS UPLIFT AT JOINT 1 (0.86)

PROVIDE FOR 1870 LBS UPLIFT AT JOINT 10 (0.86)

PROVIDE FOR 183 LBS HORIZ. REACTION AT JOINT 1

PLATE OFFSETS (X=LEFT,Y=TOP): [j5=2.5,2.5], [j14=6,2.5], [j17=6,2.5],



3-6	6-0	8-0	11-0	13-0	16-0	18-0	20-6	24-0
3-6	2-6	2-0	3-0	2-0	3-0	2-0	2-6	3-6

L. HL TO PK:12-3-9 INTERNAL RISE:2-6 R. HL TO PK:12-3-9

LEFT HEIGHT:0-4-3 SPAN:24-0 RISE:5-10-3 RIGHT HEIGHT:0-4-3

LOADING (PSF)		MAX STRESSES		MINIMUM GRADE OF LUMBER	
TOP	30	TOP	5-6=0.910	TOP CHORD:	2X4 SS 19 SP
BOTT	0	BOTT	16-17=0.652	BOT CHORD:	2X6 No.2D 19 SP
		LL.DEFL.@15=0.50	< L/360	WEBS:	2X4 No.3 19 SP

REPETITIVE STRESSES NOT USED

NOTE: PROVIDE FOR 0.69 IN. HORIZONTAL DISPLACEMENT AT ONE BEARING

LOADING	STRESS INCREASE	LOADING	PANEL(PLF) / JOINTS(LBS)
1	1.33	1.33	UNIFORM
			1-10= 90 10-13= 20 14-17= 210 18- 1= 20

CHORD: 5-6 TO BE 2X4 No.2ND 19 SP

CHORD: 13-14; 17-18 TO BE 2X4 No.3 19 SP

CHORD: 14-15; 15-16; 16-17 TO BE 2X6 SS 19 SP

WEB: 3-19; 4-17; 4-16; 7-15; 7-14; 8-12 TO BE 2X4 No.2ND 19 SP

WEB: 3-17; 8-14; 14-12; 17-19 TO BE 2X4 No.1D 19 SP

WEB 17-19 BRACED AT 1/2 POINTS AS SHOWN ABOVE

Note:Use 1x4 or 2x3 Cont.Bracing conn.w/2-8d nails,or T-brace of same size and grade as web conn. to narrow face w/10d nails 6 in. o.c

TRUSS HAS BEEN CHECKED FOR 145 M.P.H. WIND LOAD, WALL HEIGHT 16 FT, ENCLOSED BLDG. TYPE, ASSUMING 10 PSF TOTAL D.L.(7+3) (SBC-97).

DEFLECTION(IN.) L.L= 0.50,D.L=0.42,T.L=0.93

PLATES ARE MITEK M20-249,200 MANUFACTURED FROM ASTM A 446 GRD A GALVANIZED STEEL(EXCEPT AS SHOWN)

PLATE MUST BE INSTALLED ON EA. FACE OF JOINT,SYMMETRICALLY(EXCEPT AS SHOWN)DESIGN CONFORMS W/NDS DESIGN SPECS, SSB,ANSI/TPI-95

MAR 07 2001

====<<<<ACES-32 Ver. 1.1>>>>===== [002017] =====<<<<FL.QUALITY-MMH>>>>=====

Customer : FOGLIA Wed Mar 7 15:07:38 2001

Project # : 00469 Truss ID : S2 Family # : spec.

Span : 24-0 B.Span : 24-0 Quant. : 1 Top Pitch : 6/12

Bot pitch : 90/12

==Bld(1/21/2000),v1.1=====

FORCES - LOAD CASE #1								REACTIONS - SIZE	
1-2=-3998	7-8=-10082	11-12= 3519	17-18= 19	2-20= 57	5-16=-2128	9-13=-4402	1=-2181	8.00	
2-3=-3770	8-9=-10061	12-13= 3519	18-19= 0	2-19=-179	6-16= 4172	10-13=-179	11=-2181	8.00	
3-4=-10061	9-10=-3770	13-14= 0	19-20= 3519	3-19=-4402	7-16=-2128	10-12= 57			
4-5=-10082	10-11=-3998	14-15= 19	20-1= 3519	3-17= 5795	7-15= 5050	15-13= 5660			
5-6=-4866		15-16= 5696		4-17=-190	8-15=-190	17-19= 5660			
6-7=-4866		16-17= 5696		5-17= 5050	9-15= 5795				

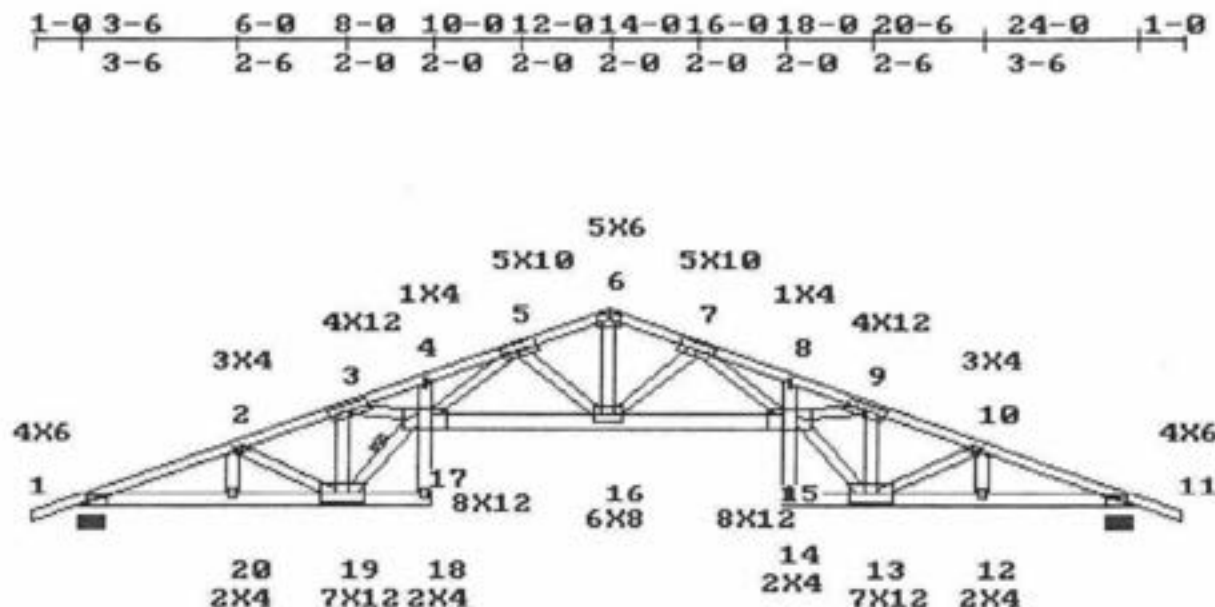
9

PROVIDE FOR 1850 LBS UPLIFT AT JOINT 1 (0.85)

PROVIDE FOR 1850 LBS UPLIFT AT JOINT 11 (0.85)

PROVIDE FOR 200 LBS HORIZ. REACTION AT JOINT 1

PLATE OFFSETS (X=LEFT,Y=TOP): (j6=3,2), (j15=6,2.5), (j17=6,2.5),



3-6	6-0	8-0	12-0	16-0	18-0	20-6	24-0
3-6	2-6	2-0	4-0	4-0	2-0	2-6	3-6

L. HL TO PK:13-5 INTERNAL RISE:2-6 R. HL TO PK:13-5

LEFT HEIGHT:0-4-3 SPAN:24-0 RISE:6-4-3 RIGHT HEIGHT:0-4-3

LOADING (PSF)		MAX STRESSES		MINIMUM GRADE OF LUMBER	
TOP	30	TOP	3-4=0.969	TOP CHORD:	2X4 SS 19 SP
BOTT	0	BOTT	15-16=0.567	BOT CHORD:	2X6 No.2D 19 SP
		LL.DEFL.@16	=0.57 < L/360	WEBS	:2X4 No.3 19 SP

REPETITIVE STRESSES NOT USED

NOTE: PROVIDE FOR 0.75 IN. HORIZONTAL DISPLACEMENT AT ONE BEARING

SPACING : 24.0 in. o. c.

NO. OF MEMBERS = 1

LOADING	STRESS INCREASE	LOADING	PANEL(PLF) / JOINTS(LBS)
LUMBER	1.33	PLATE	1.33
TYPE	UNIFORM		
		1-11= 90	11-14= 20 15-17= 210 18- 1= 20

CHORD: 14-15; 17-18 TO BE 2X4 No.3 19 SP

CHORD: 15-16; 16-17 TO BE 2X6 SS 19 SP

WEB: 3-19; 5-17; 6-16; 7-15; 9-13 TO BE 2X4 No.2ND 19 SP

WEB: 3-17; 9-15; 15-13; 17-19 TO BE 2X4 No.1D 19 SP

WEB 17-19 BRACED AT 1/2 POINTS AS SHOWN ABOVE

Note:Use 1x4 or 2x3 Cont.Bracing conn.w/2-8d nails,or T-brace of same size and grade as web conn. to narrow face w/10d nails 6 in. o.c

TRUSS HAS BEEN CHECKED FOR 145 M.P.H. WIND LOAD, WALL HEIGHT 16 FT, ENCLOSED BLDG. TYPE, ASSUMING 10 PSF TOTAL D.L.(7+3) (SBC-97).

DEFLECTION(IN.) L.L= 0.57,D.L=0.47,T.L=1.04

PLATES ARE MITEK M20-249,200 MANUFACTURED FROM ASTM A 446 GRD A GALVANIZED STEEL(EXCEPT AS SHOWN)

PLATE MUST BE INSTALLED ON EA. FACE OF JOINT,SYMMETRICALLY(EXCEPT AS SHOWN)DESIGN CONFORMS W/NDS DESIGN SPECS, S88C,ANSI/TPI-95

THIS DESIGN IS FOR TRUSS FABRICATION ONLY.FOR PERMANENT AND TEMPORARY BRACING(WHICH IS ALWAYS REQD)CONSULT BLDG ARCHITECT OR ENGINEER.

MAR 07 2001

====<<<<ACES-32 Ver. 1.1>>>>===== [002016] =====<<<<FL.QUALITY-MMH>>>>=====

Customer : FOGLIA Wed Mar 7 15:05:38 2001

Project #: 00469 Truss ID : S3 Family #: spec.

Span : 23-2-6 B.Span : 23-2-6 Quant. : 1 Top Pitch : 67/12

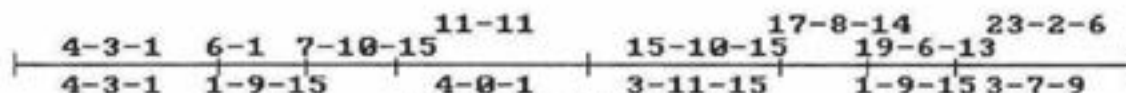
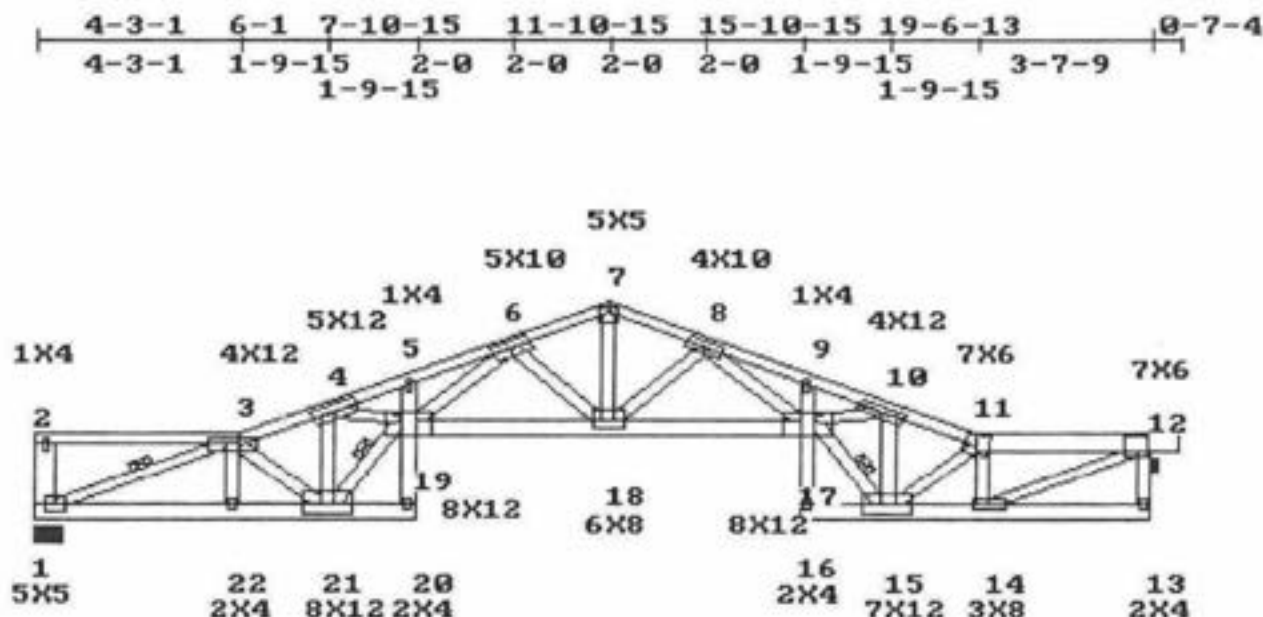
Bot pitch : 90/12

==Bld(1/21/2000),v1.1=====

FORCES - LOAD CASE #1

TOP CHORD: 1-2=-181, 2-3= 0, 3-4=-3683, 4-5=-9910, 5-6=-9931, 6-7=-4746, 7-8=-4753,
8-9=-9693, 9-10=-9672, 10-11=-3545, 11-12=-3189, 12-13= 36,
BOTTOM CHORD: 13-14= 0, 14-15= 3188, 15-16= 0, 16-17= 17, 17-18= 5524, 18-19= 5581, 19-20= 17,
20-21= 0, 21-22= 3415, 22-1= 3415,
WEBS: 3-1=-3868, 3-22= 59, 3-21=-168, 4-21=-4750, 4-19= 5834, 5-19=-182, 6-19= 5012,
6-18=-2110, 7-18= 4068, 8-18=-2017, 8-17= 4776, 9-17=-182, 10-17= 5754, 10-15=-4656,
11-15=-27, 11-14=-1755, 12-14= 3666, 17-15= 5679, 19-21= 5922,
REACTIONS,SIZE:1=-2037,8.00 12=-2131,2.90
PROVIDE FOR 1712 LBS UPLIFT AT JOINT 1 (0.84)
PROVIDE FOR 1807 LBS UPLIFT AT JOINT 12 (0.85)
PROVIDE FOR 137 LBS HORIZ. REACTION AT JOINT 1
PLATE OFFSETS (X=LEFT,Y=TOP):[j7=2.5,2],[j11=3,4],[j12=3,5],[j17=6,2.5],[j19=6,2.5],

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LEFT HEIGHT:2-6-3 SPAN:23-2-6 INTERNAL RISE:2-6 RISE:6-4-2 RIGHT HEIGHT:2-6-3

LOADING (PSF) MAX STRESSES MINIMUM GRADE OF LUMBER

L	D	TOP	BOTT	LL.DEFL.@20	TOP CHORD	NO.2ND	19 SP
30	15	5-6=0.954	18-19=0.560	< L/360	2X4	No.2D	19 SP
0	10				2X4	No.3	19 SP

REPETITIVE STRESSES NOT USED

NOTE: PROVIDE FOR 0.75 IN. HORIZONTAL DISPLACEMENT AT ONE BEARING

SPACING : 24.0 in. o. c.
NO. OF MEMBERS = 1

LOADING	STRESS INCREASE	LUMBER	PLATE	TYPE	PANEL(PLF) / JOINTS(LBS)
1	1.33	1.33	UNIFORM		2-12= 90 13-16= 20 17-19= 210 20- 1= 20

CHORD: 3-4; 4-5; 5-6; 6-7; 7-8; 8-9; 9-10; 10-11 TO BE 2X4 SS 19 SP
CHORD: 11-12 TO BE 2X6 No.2D 19 SP
CHORD: 16-17; 19-20 TO BE 2X4 No.3 19 SP
CHORD: 17-18; 18-19 TO BE 2X6 SS 19 SP
WEB: 1-2 TO BE 2X6 No.2D 19 SP
WEB: 4-21; 6-19; 7-18; 8-17; 10-15; 12-14 TO BE 2X4 No.2ND 19 SP
WEB: 4-19; 10-17; 17-15; 19-21 TO BE 2X4 No.1D 19 SP
BOTH END VERTICALS ARE NOT EXPOSED.

WEB 3-1; 17-15; 19-21 BRACED AT 1/2 POINTS AS SHOWN ABOVE

Note:Use 1x4 or 2x3 Cont.Bracing conn.w/2-Bd nails,or T-brace of same size and grade as web conn. to narrow face w/10d nails 6 in. o.c

MAR 07 2001

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TRUSS HAS BEEN CHECKED FOR 145 M.P.H. WIND LOAD, WALL HEIGHT 16 FT, ENCLOSED BLDG. TYPE, ASSUMING 10 PSF TOTAL D.L.(7+3) (SBC-97).
DEFLECTION(IN.) L.L= 0.54,D.L=0.45,T.L=0.99
PLATES ARE MITEK M20-249,200 MANUFACTURED FROM ASTM A 446 GRD A GALVANIZED STEEL(EXCEPT AS SHOWN)
PLATE MUST BE INSTALLED ON EA. FACE OF JOINT,SYMMETRICALLY(EXCEPT AS SHOWN)DESIGN CONFORMS W/NDS DESIGN SPECS, SSBC,ANSI/TPI-95
THIS DESIGN IS FOR TRUSS FABRICATION ONLY.FOR PERMANENT AND TEMPORARY BRACING(WHICH IS ALWAYS REQD)CONSULT BLDG ARCHITECT OR ENGINEER.



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MAR 07 2001

====<<<ACES-32 Ver. 1.1>>>===== [002015] =====<<<FL.QUALITY-MMH>>>=====

Customer : FOGLIA Wed Mar 7 15:03:46 2001

Project # : 00469 Truss ID : S4 Family # : spec.

Span : 24-6-6 B.Span : 24-6-6 Quant. : 1 Top Pitch : 6/12

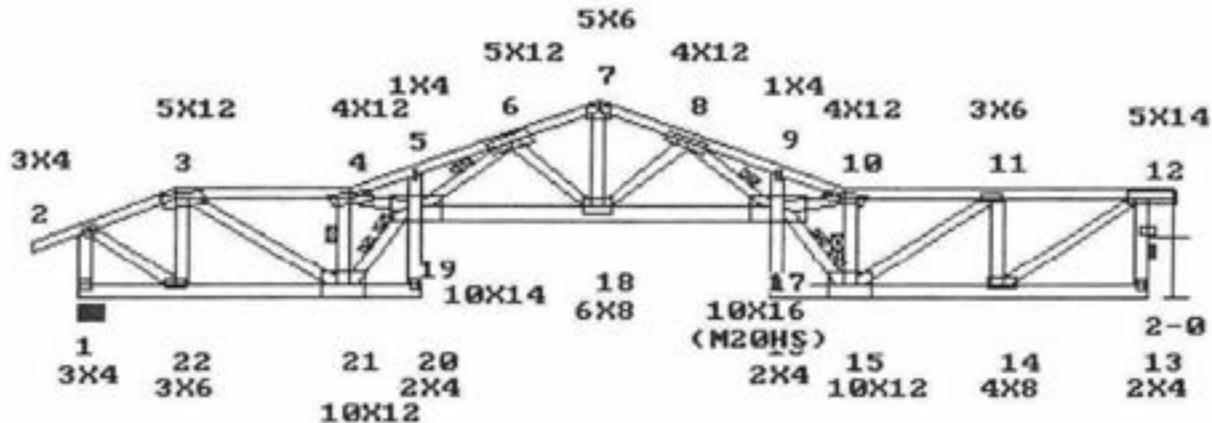
Bot pitch : 90/12

==Bld(1/21/2000),v1.1=====

FORCES - LOAD CASE #1

TOP CHORD: 1-2=-2256, 2-3=-1536, 3-4=-3541, 4-5=-10707, 5-6=-10784, 6-7=-5225, 7-8=-5217,
8-9=-11190, 9-10=-11113, 10-11=-3804, 11-12=-2104, 12-13= 35,
BOTTOM CHORD: 13-14= 0, 14-15= 2104, 15-16= 0, 16-17= 15, 17-18= 6198, 18-19= 6090, 19-20= 15,
20-21= 0, 21-22= 1371, 22-1= 0,
WEBS: 2-22= 1920, 3-22=-1285, 3-21= 2760, 4-21=-7552, 4-19= 6593, 5-19=-281, 6-19= 5384,
6-18=-2241, 7-18= 4490, 8-18=-2426, 8-17= 5801, 9-17=-283, 10-17= 6715, 10-15=-7752,
11-15= 2285, 11-14=-1841, 12-14= 2842, 17-15= 7337, 19-21= 6888,
REACTIONS,SIZE:1=-2277,8.00 12=-2168,3.03
PROVIDE FOR 1954 LBS UPLIFT AT JOINT 1 (0.86)
PROVIDE FOR 1869 LBS UPLIFT AT JOINT 12 (0.86)
PROVIDE FOR 221 LBS HORIZ. REACTION AT JOINT 1
PLATE OFFSETS (X=LEFT,Y=TOP): [J3=6,2], [J7=3,2], [J12=2.5,3], [J14=4,2.5], [J17=8,4], [J19=7,4.5],

1-1 2-3-1 6-3-1 7-10-14 11-11 15-10-14 21-0-13 24-6-6 0-7-4
2-3-1 4-0 1-7-13 2-0-1 1-11-15 3-6 3-5-9
2-0-1 1-11-15 1-7-15



2-3 6-3 7-10-13 11-10-15 15-10-14 17-6-13 21-0-13 24-6-6
2-3 4-0 1-7-13 3-11-15 3-6 3-5-9

L. HL TO PK:2-6-4 INTERNAL RISE:2-6
LEFT HEIGHT:2-4-11 SPAN:24-6-6 RISE:6-4-3 RIGHT HEIGHT:3-6-4

LOADING (PSF) MAX STRESSES MINIMUM GRADE OF LUMBER
L D TOP 10-11=0.937 TOP CHORD:2X4 No.2ND 19 SP
30 15 BOT 17-18=0.600 BOT CHORD:2X6 No.2D 19 SP
0 10 LL.DEFL.@16=0.71 < L/360 WEBS :2X4 No.3 19 SP

REPETITIVE STRESSES NOT USED

NOTE: PROVIDE FOR 0.86 IN. HORIZONTAL DISPLACEMENT AT ONE BEARING

LOADING	STRESS INCREASE	LOADING	PANEL(PLF) / JOINTS(LBS)
LUMBER	PLATE	TYPE	
1	1.33	1.33	UNIFORM 2-12= 90 13-16= 20 17-19= 210 20- 1= 20

CHORD: 3-4 TO BE 2X4 No.10 19 SP
CHORD: 4-5; 5-6; 6-7; 7-8; 8-9; 9-10 TO BE 2X4 SS 19 SP
CHORD: 16-17; 19-20 TO BE 2X4 No.3 19 SP
CHORD: 17-18; 18-19 TO BE 2X6 SS 19 SP
WEB: 4-21; 4-19; 8-17; 10-17; 10-15; 17-15; 19-21 TO BE 2X4 No.10 19 SP
WEB: 6-19; 7-18 TO BE 2X4 No.2ND 19 SP
BOTH END VERTICALS ARE NOT EXPOSED.
WEB 4-21; 6-19; 8-17; 10-15 BRACED at 1/2 POINTS AS SHOWN ABOVE
WEB 17-15; 19-21 BRACED at 1/3 POINTS AS SHOWN ABOVE

Note:Use 1x4 or 2x3 Cont.Bracing conn.w/2-8d nails,or T-brace of same size and grade as web conn. to narrow face w/10d nails 6 in. o.c

MAR 07 2001

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TRUSS HAS BEEN CHECKED FOR 145 M.P.H. WIND LOAD, WALL HEIGHT 16 FT, ENCLOSED BLDG. TYPE, ASSUMING 10 PSF TOTAL D.L.(7+3) (SBC-97).
DEFLECTION(IN.) L.L= 0.71,D.L=0.59,T.L=1.30
PLATES ARE MITEK M20-249,200 M20HS-187,150 MANUFACTURED FROM ASTM A 446 GRD A GALVANIZED STEEL(EXCEPT AS SHOWN)
PLATE MUST BE INSTALLED ON EA. FACE OF JOINT,SYMMETRICALLY(EXCEPT AS SHOWN)DESIGN CONFORMS W/ND5 DESIGN SPECS, SBC,ANSI/TP1-95
THIS DESIGN IS FOR TRUSS FABRICATION ONLY.FOR PERMANENT AND TEMPORARY BRACING(WHICH IS ALWAYS REQD)CONSULT BLDG ARCHITECT OR ENGINEER.

 (W)

MAR 07 2001

====<<<ACES-32 Ver. 1.1>>>===== [002014] =====<<<FL.QUALITY-MMH>>>=====

Customer : FOGLIA Wed Mar 7 15:01:33 2001

Project # : 00469 Truss ID : S5 Family # : spec.

Span : 24-6-6 B.Span : 24-6-6 Quant. : 1 Top Pitch : 6/12

Bot pitch : 90/12

==Bld(1/21/2000),v1.1=====

FORCES - LOAD CASE #1						REACTIONS - SIZE	
1-2=-2100	7-8=-8104	11-12= 0	17-18= 6800	2-20= 1996	5-16=-3784	9-14= 6024	1=-2142 8.00
2-3=-1997	8-9=-7123	12-13= 0	18-19= 25	3-20=-2610	6-16= 3970	9-12=-3482	10=-2043 3.09
3-4=-6677	9-10=-1982	13-14= 27	19-20= 0	3-18= 5850	7-16=-4126	10-12= 2620	
4-5=-7792	10-11= 48	14-15= 7235	20-1= 0	4-18=-1391	7-15=-685	14-12= 2707	
5-6=-4843		15-16= 8104		4-17= 1561	8-15= 1375	18-20= 2519	
6-7=-4843		16-17= 7792		5-17=-825	8-14=-1258		

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PROVIDE FOR 1842 LBS UPLIFT AT JOINT 1 (0.86)

PROVIDE FOR 1813 LBS UPLIFT AT JOINT 10 (0.89)

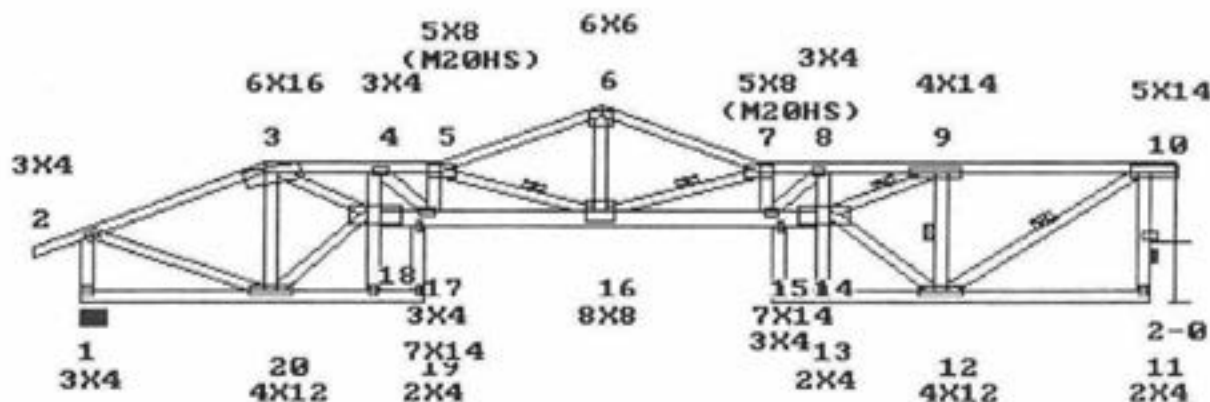
PROVIDE FOR 304 LBS HORIZ. REACTION AT JOINT 1

PLATE OFFSETS (X=LEFT,Y=TOP): [j3=8,2], [j6=3,2], [j10=2.5,3], [j14=7,2], [j18=7,2],

1-1 4-3-1 6-10-14 11-10-15 16-10-14 24-6-6 0-7-4

4-3-1 2-7-13 3-7-14 3-7-14 2-10 4-9-8

1-4-3 1-4-1



8-3-1 15-6-13 19-8-14

4-3-1 6-10-14 11-10-15 16-10-14 24-6-6

4-3-1 2-7-13 3-7-14 3-7-14 2-10 4-9-8

L. HL TO PK:4-9-1

LEFT HEIGHT:2-4-11

INTERNAL RISE:2-6

SPAN:24-6-6

RISE:6-4-2

RIGHT HEIGHT:4-6-3

LOADING (PSF)

L D

TOP 30 15

BOTT 0 10

MAX STRESSES

TOP 2-3=0.979

BOTT 15-16=0.684

LL.DEFL.@15=0.46 < L/360

MINIMUM GRADE OF LUMBER

TOP CHORD:2X4 No.2ND 19 SP

BOT CHORD:2X6 No.2D 19 SP

WEBS :2X4 No.3 19 SP

SPACING : 24.0 in. o. c.

NO. OF MEMBERS = 1

REPETITIVE STRESSES NOT USED

NOTE: PROVIDE FOR 0.62 IN. HORIZONTAL DISPLACEMENT AT ONE BEARING

LOADING STRESS INCREASE

LUMBER PLATE

1 1.33 1.33

LOADING

TYPE

UNIFORM

PANEL(PLF) / JOINTS(LBS)

2-10= 90 11-13= 20 14-15= 20 15-17= 200 17-18= 20 19- 1= 20

CHORD: 3-4; 4-5; 7-8; 8-9; 9-10 TO BE 2X4 SS 19 SP

CHORD: 5-6; 6-7 TO BE 2X4 No.1D 19 SP

CHORD: 13-14; 18-19 TO BE 2X4 No.3 19 SP

CHORD: 14-15; 15-16; 16-17; 17-18 TO BE 2X6 SS 19 SP

WEB: 3-18; 9-14 TO BE 2X4 No.1D 19 SP

WEB: 5-16; 6-16; 7-16 TO BE 2X4 No.2ND 19 SP

BOTH END VERTICALS ARE NOT EXPOSED.

WEB 5-16; 7-16; 9-14; 9-12; 10-12 BRACED AT 1/2 POINTS AS SHOWN ABOVE

Note:Use 1x4 or 2x3 Cont.Bracing conn.w/2-8d nails,or f-brace of same size and grade as web conn. to narrow face w/10d nails 6 in. o.c

TRUSS HAS BEEN CHECKED FOR 145 M.P.H. WIND LOAD, WALL HEIGHT 16 FT, ENCLOSED BLDG. TYPE, ASSUMING 10 PSF TOTAL D.L.(7+3) (SBC-97).

DEFLECTION(IN.) L.L= 0.46,D.L=0.38,T.L=0.84

MAR 07 2001

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PLATES ARE MITEK M20-249,200 M20HS-187,150 MANUFACTURED FROM ASTM A 446 GRD A GALVANIZED STEEL(EXCEPT AS SHOWN)
PLATE MUST BE INSTALLED ON EA. FACE OF JOINT,SYMMETRICALLY(EXCEPT AS SHOWN)DESIGN CONFORMS W/NDG DESIGN SPECS, SSBC,ANSI/TPI-95
THIS DESIGN IS FOR TRUSS FABRICATION ONLY.FOR PERMANENT AND TEMPORARY BRACING(WHICH IS ALWAYS REQD)CONSULT BLDG ARCHITECT OR ENGINEER.

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MAR 07 2001

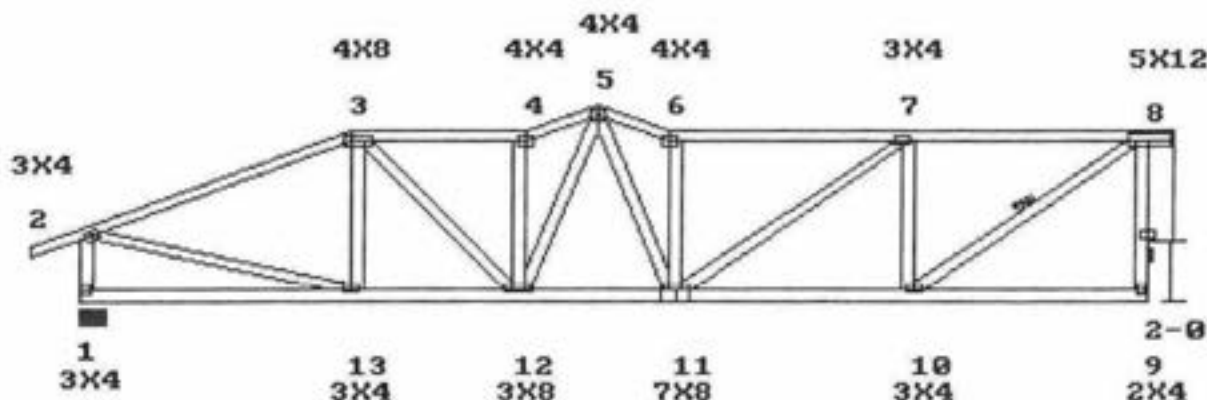
====<<<<ACES-32 Ver. 1.1>>>>=====[001819]====<<<<FL.QUALITY-MMH>>>>=====
 Customer : FOGLIA Thu Mar 1 09:01:59 2001
 Project #: 00469 Truss ID : S6 Family #: special
 Span : 24-6-14 Quantity : 1 Top Pitch : 6/12
 ==Bld(1/21/2000),v1.1=====

FORCES - GRAVITY LOADS			REACTIONS - SIZE	
1-2=1402	7-8=1115	9-10= 0	2-13= 1264	6-11=1096
2-3=1340	8-9= 55	10-11= 1114	3-13=304	7-11= 638
3-4=1549		11-12= 1372	3-12= 572	7-10=931
4-5=1724		12-13= 1197	4-12=1013	8-10= 1525
5-6=1760		13-1= 0	5-12= 659	
6-7=1580			5-11= 777	

(13)

PROVIDE FOR 1242 LBS UPLIFT AT JOINT 1 (0.85)
 PROVIDE FOR 1299 LBS UPLIFT AT JOINT 8 (0.92)
 PROVIDE FOR 388 LBS HORIZ. REACTION AT JOINT 1
 PLATE OFFSETS (X=LEFT,Y=TOP):[j8=2,3],[j11=4,2),(SYMMETRIC PLATING TURNED ON)

1-1 6-3-1 10-3-1 13-6-13 19-0-14 24-6-14 0-7-4
 6-3-1 4-0 1-7-14 5-6 5-6
 1-7-14



6-3-1 10-3-1 13-6-13 19-0-13 24-6-14
 6-3-1 4-0 3-3-12 5-6 5-6-1

L. HL TO PK:6-11-15
 LEFT HEIGHT:2-4-11 SPAN:24-6-14 RISE:6-4-2 RIGHT HEIGHT:5-6-3

LOADING (PSF)		MAX STRESSES		MINIMUM GRADE OF LUMBER	
TOP	30	TOP	6-7=0.996	TOP CHORD:	2X4 No.1D 19 SP
BOTT	0	BOTT	11-12=0.205	BOT CHORD:	2X6 No.2D 19 SP
		LL.DEFL.@11=0.06	< L/360	WEBS	:2X4 No.3 19 SP

STR.INC.: LUMB = 1.33 PLATE = 1.33
 REPETITIVE STRESSES USED SPACING : 24.0 in. o. c.
 NO. OF MEMBERS = 1

CHORD: 3-4; 4-5; 5-6 TO BE 2X4 No.2ND 19 SP
 BOTH END VERTICALS ARE NOT EXPOSED.

WEB 8-10 BRACED at 1/2 POINTS AS SHOWN ABOVE

Note:Use 1x4 or 2x3 Cont.Bracing conn.w/2-8d nails,or T-brace of same size and grade as web conn. to narrow face w/10d nails 6 in. o.c

TRUSS HAS BEEN CHECKED FOR 145 M.P.H. WIND LOAD, WALL HEIGHT 16 FT, ENCLOSED BLDG. TYPE, ASSUMING 10 PSF TOTAL D.L.(7+3) (SBC-97).

DEFLECTION(IN.) L.L= 0.06,D.L=0.05,T.L=0.12

PLATES ARE MITEK M20-249,200 MANUFACTURED FROM ASTM A 446 GRD A GALVANIZED STEEL(EXCEPT AS SHOWN)

PLATE MUST BE INSTALLED ON EA. FACE OF JOINT,SYMMETRICALLY(EXCEPT AS SHOWN)DESIGN CONFORMS W/NDG DESIGN SPECS, SSB,ANSI/TPI-95

THIS DESIGN IS FOR TRUSS FABRICATION ONLY.FOR PERMANENT AND TEMPORARY BRACING(WHICH IS ALWAYS REQD)CONSULT BLDG ARCHITECT OR ENGINEER.

[Signature]

MAR 01 2001

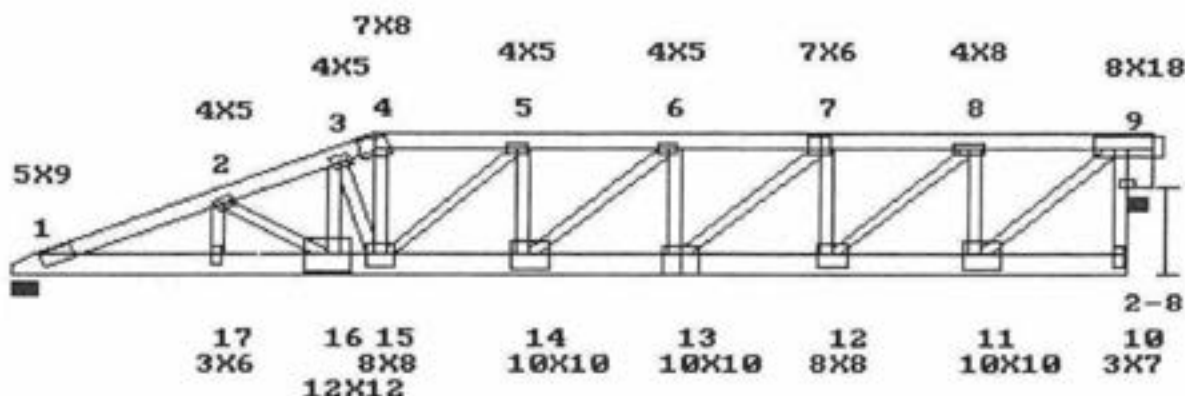
====<<<<ACES-32 Ver. 1.1>>>>=====[001820]====<<<<FL.QUALITY-MMH>>>>=====
 Customer : FOGLIA Thu Mar 1 09:02:58 2001
 Project # : 00469 Truss ID : GT25 Family # : 313
 Span : 24-7-6 B.Span : 25-3-14 Quant. : 3 Top Pitch : 6/12
 ==Bld(1/21/2000),v1.1=====

FORCES - LOAD CASE #1						REACTIONS - SIZE	
1-2=-22696	7-8=-16634	10-11= 0	16-17= 20386	2-17= 55	5-14= 2570	8-11=-7947	1=-10210 8.00
2-3=-22348	8-9=-9843	11-12= 9843	17-1= 20386	2-16=-510	6-14= 1042	9-11= 14988	9=-13122 6.00
3-4=-21157	9-10= 1678	12-13= 16634		3-16= 4983	6-13=-935		
4-5=-18687		13-14= 20371		3-15=-5154	7-13= 5691		
5-6=-21055		14-15= 21055		4-15= 9827	7-12=-4441		
6-7=-20371		15-16= 19989		5-15=-3607	8-12= 10340		

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PROVIDE FOR 10339 LBS UPLIFT AT JOINT 1 (1.01)
 PROVIDE FOR 13047 LBS UPLIFT AT JOINT 9 (0.99)
 PROVIDE FOR 448 LBS HORIZ. REACTION AT JOINT 1
 PLATE OFFSETS (X=LEFT,Y=TOP):[j4=4,4],[j7=3,5],[j9=5,5],[j13=5,3],

4-6-12 7-1-8 11-3-14 14-7-12 17-11-10 24-7-6 0-7-4
 4-6-12 2-6-12 3-3-14 3-3-14 3-3-14 3-3-14 3-3-14
 0-10-8



4-6-12 7-1-8 11-3-14 14-7-12 17-11-10 21-3-8 24-7-6
 4-6-12 2-6-12 3-3-14 3-3-14 3-3-14 3-3-14 3-3-14

L. HL TO PK:8-11-5
 LEFT HEIGHT:0-4-3 SPAN:24-7-6 RISE:4-4-3 RIGHT HEIGHT:4-4-3

LOADING (PSF)		MAX STRESSES		MINIMUM GRADE OF LUMBER	
TOP	30	15	TOP 1-2=0.906	TOP CHORD:2X6	No.2D 19 SP
BOTT	0	10	BOTT 14-15=0.512	BOT CHORD:2X8	SS 19 SP
			LL.DEFL.@14=0.18 < L/360	WEBS :2X4	No.3 19 SP

REPETITIVE STRESSES NOT USED

SPACING : 24.0 in. o. c.
 NO. OF MEMBERS = 3

LOADING	STRESS INCREASE	LOADING	PANEL(PLF) / JOINTS(LBS)
LUMBER	1.33	PLATE	1- 2= 90
		TYPE	2- 9= 45
		UNIFORM	10-16= 1010
		CONCENTRATED	16-17= 10
			17- 1= 20
			16= 4208

WEB: 8-12; 9-11 TO BE 2X4 No.2ND 19 SP
 RIGHT END VERTICAL IS NOT EXPOSED.
 3 MEMBERS NAILED TOG. W/1 ROW(S) OF .131x3 in. NAILS 12 in. o.c.(TOP CHS.),AND 3 ROW(S) OF .131x3 in. NAILS 5 in. o.c.(BOTT. CHS.)
 For Webs use 1 ROW of NAILS 12 in. o.c.
 TRUSS HAS BEEN CHECKED FOR 145 M.P.H. WIND LOAD, WALL HEIGHT 16 FT, ENCLOSED BLDG. TYPE, ASSUMING 10 PSF TOTAL D.L.(7+3) (SBC-97).
 DEFLECTION(IN.) L.L= 0.18,D.L=0.15,T.L=0.33
 PLATES ARE MITEK M20-249,200 MANUFACTURED FROM ASTM A 446 GRD A GALVANIZED STEEL(EXCEPT AS SHOWN)
 PLATE MUST BE INSTALLED ON EA. FACE OF JOINT,SYMMETRICALLY(EXCEPT AS SHOWN)DESIGN CONFORMS W/NDSD DESIGN SPECS, SSBC,ANSI/TPI-95
 THIS DESIGN IS FOR TRUSS FABRICATION ONLY.FOR PERMANENT AND TEMPORARY BRACING(WHICH IS ALWAYS REQD)CONSULT BLDG ARCHITECT OR ENGINEER.

MAR 01 2001

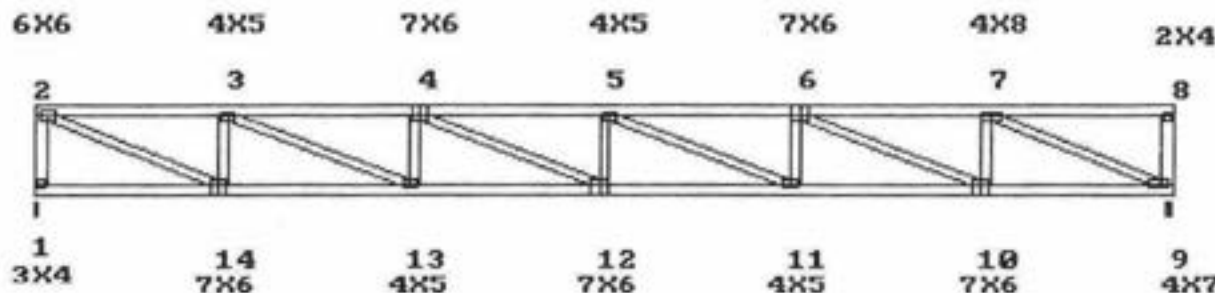
====<<<<ACES-32 Ver. 1.1>>>>=====[001821]====<<<<FL.QUALITY-MMH>>>>=====
 Customer : FOGLIA Thu Mar 1 09:03:20 2001
 Project #: 00469 Truss ID : FG3 Family # : 301
 Span : 35-2 B.Span : 35-3-8 Quant. : 2 Top Pitch : /12
 ==Bld(1/21/2000),v1.1=====

FORCES - LOAD CASE #1				REACTIONS - SIZE	
1-2=-4064	7-8= 0	9-10= 5940	2-14= 6897	5-11=-1405	1=-4193 2.77
2-3=-5940	8-9=-561	10-11= 9588	3-14=-3242	6-11= 967	9=-4193 2.77
3-4=-9588		11-12= 10804	3-13= 4214	6-10=-4214	
4-5=-10804		12-13= 9588	4-13=-1846	7-10= 2371	
5-6=-9588		13-14= 5940	4-12= 1405	7-9=-6897	
6-7=-5940		14-1= 0	5-12=-440		

(15)

PROVIDE FOR 5479 LBS UPLIFT AT JOINT 1 (1.31)
 PROVIDE FOR 5479 LBS UPLIFT AT JOINT 9 (1.31)
 PLATE OFFSETS (X=LEFT,Y=TOP):[j4=3,5],[j6=3,5],[j10=3,2],[j12=3,2],[j14=3,2],

5-10-5	11-8-11	17-7	23-5-5	29-3-11	35-2
5-10-5	5-10-5	5-10-5	5-10-5	5-10-5	5-10-5



5-10-5	11-8-11	17-7	23-5-5	29-3-11	35-2
5-10-5	5-10-5	5-10-5	5-10-5	5-10-5	5-10-5

LEFT HEIGHT:3-10-3 SPAN:35-2 RISE:3-10-3 RIGHT HEIGHT:3-10-3

LOADING (PSF)		MAX STRESSES		MINIMUM GRADE OF LUMBER	
TOP	30 15	TOP	4-5=0.908	TOP CHORD:	2X6 No.2D 19 SP
BOTT	0 10	BOTT	11-12=0.715	BOT CHORD:	2X6 No.2D 19 SP
		LL.DEFL.@12=0.36	< L/360	WEBS:	2X4 No.3 19 SP

REPETITIVE STRESSES NOT USED

SPACING : 24.0 in. o. c.
 NO. OF MEMBERS = 2

LOADING	STRESS INCREASE	LOADING	PANEL(PLF) / JOINTS(LBS)
LUMBER	PLATE	TYPE	
1	1.33	1.33	UNIFORM 2- 8= 195 9- 1= 45

WEB: 1-2; 8-9 TO BE 2X6 No.2D 19 SP
 WEB: 2-14; 7-9 TO BE 2X4 No.2D 19 SP
 BOTH END VERTICALS ARE NOT EXPOSED.
 2 MEMBERS NAILED TOG. W/1 ROW(S) OF .131x3 in. NAILS 12 in. o.c.(TOP CHS.),AND 1 ROW(S) OF .131x3 in. NAILS 12 in. o.c.(BOTT. CHS.)
 For Webs use 1 ROW of NAILS 12 in. o.c.
 TRUSS HAS BEEN CHECKED FOR 145 M.P.H. WIND LOAD, WALL HEIGHT 16 FT, ENCLOSED BLDG. TYPE, ASSUMING 10 PSF TOTAL D.L.(7+3) (SBC-97).
 DEFLECTION(IN.) L.L= 0.36,D.L=0.30,T.L=0.66
 PLATES ARE MITEK M20-249,200 MANUFACTURED FROM ASTM A 446 GRD A GALVANIZED STEEL(EXCEPT AS SHOWN)
 PLATE MUST BE INSTALLED ON EA. FACE OF JOINT,SYMMETRICALLY(EXCEPT AS SHOWN)DESIGN CONFORMS W/NDG DESIGN SPECS, SBC,ANSI/TPI-95
 THIS DESIGN IS FOR TRUSS FABRICATION ONLY.FOR PERMANENT AND TEMPORARY BRACING(WHICH IS ALWAYS REQD)CONSULT BLDG ARCHITECT OR ENGINEER.

[Signature]
 MAR 01 2001

====<<<<ACES-32 Ver. 1.1>>>>===== [001991] =====<<<<FL.QUALITY-MMH>>>>=====

Customer : FOGLIA Mon Mar 5 12:07:19 2001

Project #: 00469 Truss ID : GT36 Family #: spec.

Span : 36-10 B.Span : 36-10 Quant. : 3 Top Pitch : 6/12

Bot pitch : 90/12

==Bld(1/21/2000),v1.1=====

FORCES - LOAD CASE #1

TOP CHORD: 1-2=-6857, 2-3=-6492, 3-4=-1548, 4-5= 4349, 5-6=-5721, 6-7=-5118, 7-8=-10654,

8-9=-14038, 9-10=-15314, 10-11=-8638, 11-12=-13114,

BOTTOM CHORD: 12-13= 0, 13-14= 8638, 14-15= 15314, 15-16= 12556, 16-17= 10654, 17-18=-2920, 18-19=-21353,

19-20= 0, 20-21= 5807, 21-22= 6094, 22-1= 6094,

WEBS: 2-22= 68, 2-21=-317, 3-21= 6198, 3-20=-6118, 4-20= 6598, 4-18=-6695, 5-18=-14098,

5-17= 12365, 6-17= 2445, 7-17=-9306, 7-16= 7200, 8-16=-3199, 8-15= 8686, 9-15=-5732,

9-14=-1390, 10-14= 11654, 10-13=-9701, 11-13= 15643, 18-20= 1779,

REACTIONS,SIZE:1=-3430,8.00 12=-14739,8.00 19=-23020,12.74

PROVIDE FOR 4011 LBS UPLIFT AT JOINT 1 (1.17)

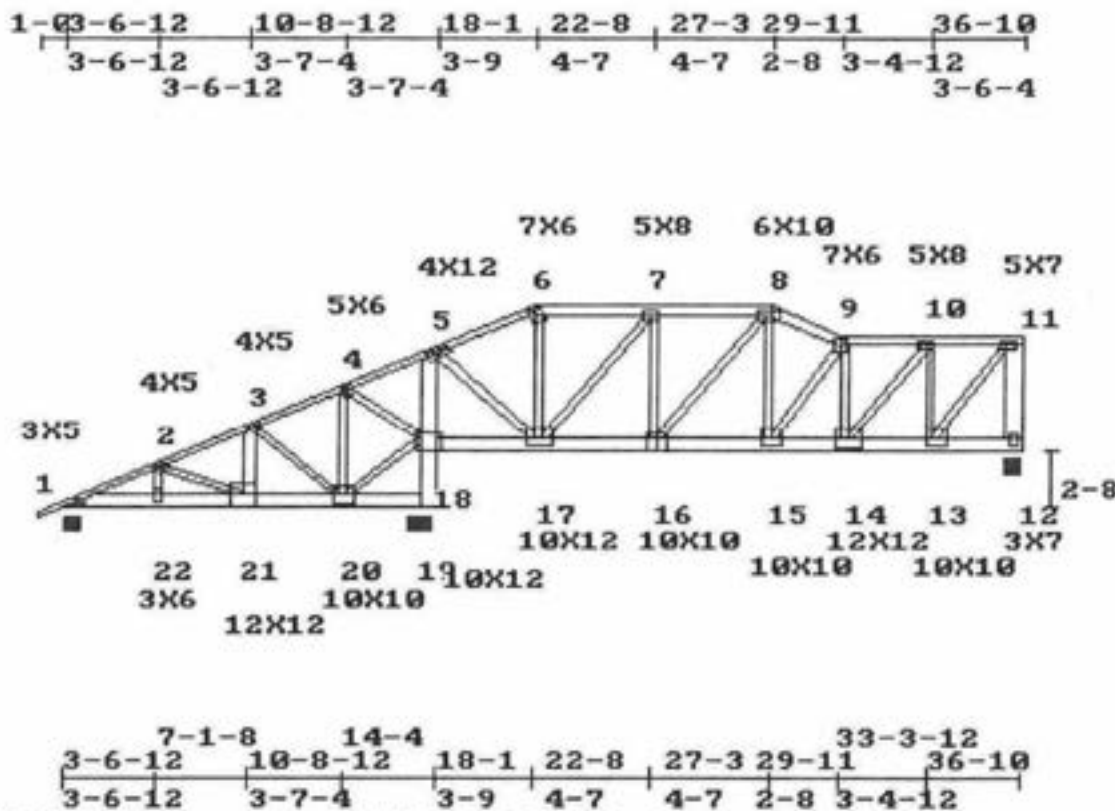
PROVIDE FOR 16480 LBS UPLIFT AT JOINT 12 (1.12)

PROVIDE FOR 26321 LBS UPLIFT AT JOINT 19 (1.14)

PROVIDE FOR 1129 LBS HORIZ. REACTION AT JOINT 1

PLATE OFFSETS (X=LEFT,Y=TOP):[j4=3,3],[j6=3,2.5],[j9=3,4],[j10=4.5,2.5],[j11=2.5,2.5],[j13=3.5,5],[j16=5,3],[j18=6,3],

*** WARNING: NO PLATE AT JOINT(S):j19, REVIEW REQUIRED ***



L. HL TO PK:20-2-10 INTERNAL RISE:2-8

LEFT HEIGHT:0-4-3 SPAN:36-10 RISE:9-4-11 RIGHT HEIGHT:5-4-11

LOADING (PSF) MAX STRESSES MINIMUM GRADE OF LUMBER

TOP 30 15 TOP 9-10=0.711 TOP CHORD:2X4 No.2ND 19 SP

BOTT 0 10 BOTT 15-16=0.487 BOT CHORD:2X8 SS 19 SP

LL.DEFL.@15=0.14 < L/360 WEBS :2X4 No.3 19 SP

SPACING : 24.0 in. o. c.

NO. OF MEMBERS = 3

REPETITIVE STRESSES NOT USED

LOADING	STRESS INCREASE	LOADING	PANEL(PLF) / JOINTS(LBS)
LUMBER	PLATE	TYPE	
1	1.33	1.33	UNIFORM 1- 3= 90 3-11= 45 12-18= 1010 19-21= 1010 21- 1= 20
			CONCENTRATED 14= 5101 21= 4208

CHORD: 6-7; 7-8; 8-9; 9-10; 10-11 TO BE 2X6 No.2D 19 SP

WEB: 11-12 TO BE 2X8 SS 19 SP

WEB: 3-21 TO BE 2X6 No.2D 19 SP

WEB: 4-20; 7-17; 10-14; 10-13 TO BE 2X4 No.2ND 19 SP

WEB: 5-18; 5-17; 11-13 TO BE 2X4 No.1D 19 SP

RIGHT END VERTICAL IS NOT EXPOSED.

MEMBERS NAILED TOG. W/1 ROW(S) OF .131x3 in. NAILS 12 in. o.c.(TOP CHS.),AND 3 ROW(S) OF .131x3 in. NAILS 5 in. o.c.(BOTT. CHS.)

For Webs use 1 ROW of NAILS 12 in. o.c.

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****NOTE:Special hanger is reqd. to dist. bottom chord concentrated load(s) to all plies.****

TRUSS HAS BEEN CHECKED FOR 145 M.P.H. WIND LOAD, WALL HEIGHT 16 FT, ENCLOSED BLDG. TYPE, ASSUMING 10 PSF TOTAL D.L.(7+3) (SBC-97).
DEFLECTION(IN.) L.L= 0.14,D.L=0.12,T.L=0.26

PLATES ARE MITEK M20-249,200 M16-174,122 MANUFACTURED FROM ASTM A 446 GRD A GALVANIZED STEEL(EXCEPT AS SHOWN)

PLATE MUST BE INSTALLED ON EA. FACE OF JOINT,SYMMETRICALLY(EXCEPT AS SHOWN)DESIGN CONFORMS W/NDG DESIGN SPECS, SBC,ANSI/TP1-95

THIS DESIGN IS FOR TRUSS FABRICATION ONLY.FOR PERMANENT AND TEMPORARY BRACING(WHICH IS ALWAYS REQD)CONSULT BLDG ARCHITECT OR ENGINEER.

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====<<<<ACES-32 Ver. 1.1>>>>=====[001822]=====<<<<FL.QUALITY-MMH>>>>=====

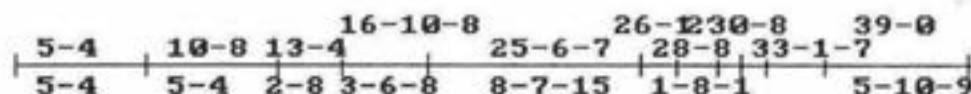
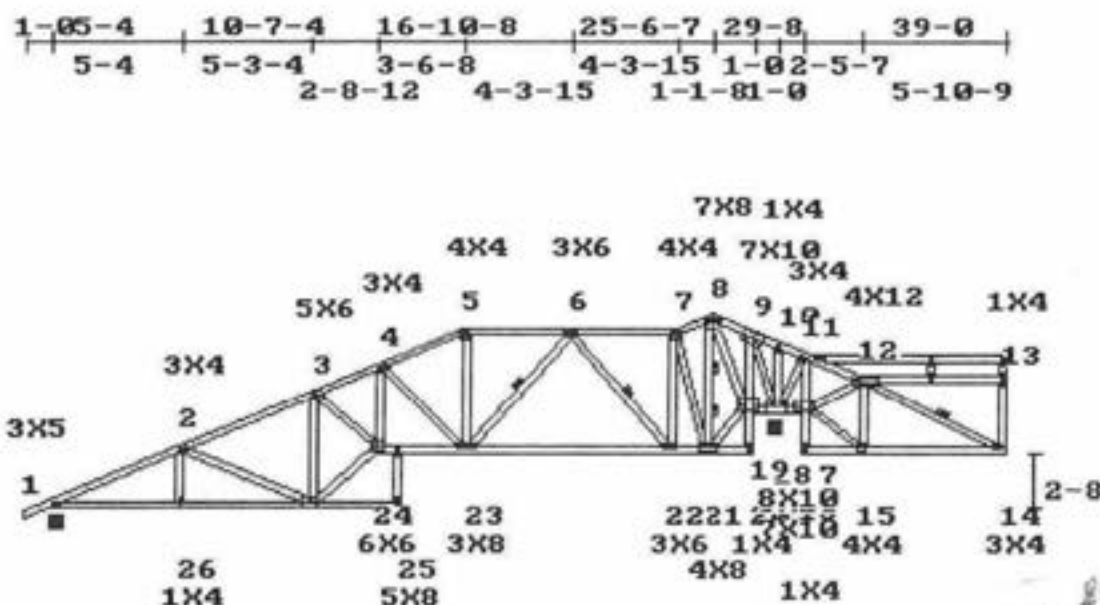
Customer : FOGLIA Thu Mar 1 09:12:57 2001
 Project #: 00469 Truss ID : S26 Family #: special
 Span : 39-0 Quantity : 1 Top Pitch : 6/12
 Bot pitch : 90/12

24-25 12/12,

==Bld(1/21/2000),v1.1=====

FORCES - GRAVITY LOADS

TOP CHORD: 1-2=-2479, 2-3=-1931, 3-4=-2703, 4-5=-1734, 5-6=-1548, 6-7=-298, 7-8= 51 ,
 8-9= 798, 9-10= 1620, 10-11= 1620, 11-12= 1513, 12-13= 0, 13-14=-258,
 BOTTOM CHORD: 14-15=-518, 15-16= 0, 16-17= 23, 17-18=-1364, 18-19=-790, 19-20= 15, 20-21= 0,
 21-22= 298, 22-23= 1068, 23-24= 2418, 24-25= 2420, 25-26= 2180, 26-1= 2180,
 WEBS: 2-26= 79, 2-25=-505, 3-25=-1382, 3-24= 978, 4-24= 1138, 4-23=-1320, 5-23= 427,
 6-23= 805, 6-22=-1290, 7-22= 1137, 7-21=-1417, 8-21= 1465, 8-19=-2095, 9-19= 1953,
 9-18=-2527, 10-18=-90, 11-18=-245, 11-17= 130, 12-17=-1000, 12-15= 529, 12-14= 606,
 17-15=-685, 19-21=-76,
 REACTIONS, SIZE: 1=-1565, 8.00 18=-2782, 8.00
 PROVIDE FOR 1474 LBS UPLIFT AT JOINT 1 (0.94)
 PROVIDE FOR 4565 LBS UPLIFT AT JOINT 18 (1.64)
 PROVIDE FOR 698 LBS HORIZ. REACTION AT JOINT 1
 PLATE OFFSETS (X=LEFT, Y=TOP): (J3=3,3), (J8=4,1.5), (J19=5,5), (J25=4,2), (SYMMETRIC PLATING TURNED ON)



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L. HL TO PK: 18-10-6
 LEFT HEIGHT: 0-4-3

INTERNAL RISE: 4-8
 SPAN: 39-0

RISE: 9-6-3

RIGHT HEIGHT: 3-9-7

LOADING (PSF) MAX STRESSES MINIMUM GRADE OF LUMBER
 TOP 30 15 TOP 1-2=0.959 TOP CHORD: 2X4 No.1D 19 SP
 BOTT 0 10 BOTT 14-15=0.756 BOT CHORD: 2X4 No.1D 19 SP
 LL.DEFL.@14=0.26 < L/360 WEBS : 2X4 No.3 19 SP
 STR.INC.: LUMB = 1.33 PLATE = 1.33 SPACING : 24.0 in. o. c.
 REPETITIVE STRESSES USED NO. OF MEMBERS = 1

CHORD: 5-6; 6-7; 7-8; 8-9; 9-10; 10-11; 11-12; 17-18; 18-19; 20-21; 21-22; 22-23; 23-24; 24-25; 25-26; 26-1 TO BE 2X4 No.2ND 19 SP
 CHORD: 16-17; 19-20 TO BE 2X4 No.3 19 SP
 WEB: 8-19; 9-18 TO BE 2X4 No.2ND 19 SP
 RIGHT END VERTICAL IS NOT EXPOSED.
 WEB 6-23; 6-22; 12-14 BRACED at 1/2 POINTS AS SHOWN ABOVE
 WEB 8-21 BRACED at 1/3 POINTS AS SHOWN ABOVE
 Note: Use 1x4 or 2x3 Cont. Bracing conn. w/2-8d nails, or T-brace of same size and grade as web conn. to narrow face w/10d nails 6 in. o.c.
 TRUSS HAS BEEN CHECKED FOR 145 M.P.H. WIND LOAD, WALL HEIGHT 16 FT, ENCLOSED BLDG. TYPE, ASSUMING 10 PSF TOTAL D.L.(7+3) (SBC-97).
 REFLECTION(IN.) L.L.= 0.26, D.L.=0.22, T.L.=0.48
 *LATES ARE MITEK M20-249, 200 MANUFACTURED FROM ASTM A 446 GRD A GALVANIZED STEEL(EXCEPT AS SHOWN)
 *LATE MUST BE INSTALLED ON EA. FACE OF JOINT, SYMMETRICALLY(EXCEPT AS SHOWN) DESIGN CONFORMS W/NDOS DESIGN SPECS, S5BC, ANSI/TP1-95
 THIS DESIGN IS FOR TRUSS FABRICATION ONLY. FOR PERMANENT AND TEMPORARY BRACING(WHICH IS ALWAYS REGO) CONSULT BLDG ARCHITECT OR ENGINEER.

====<<<<ACES-32 Ver. 1.1>>>>===== [001823] =====<<<<FL.QUALITY-MMH>>>>=====

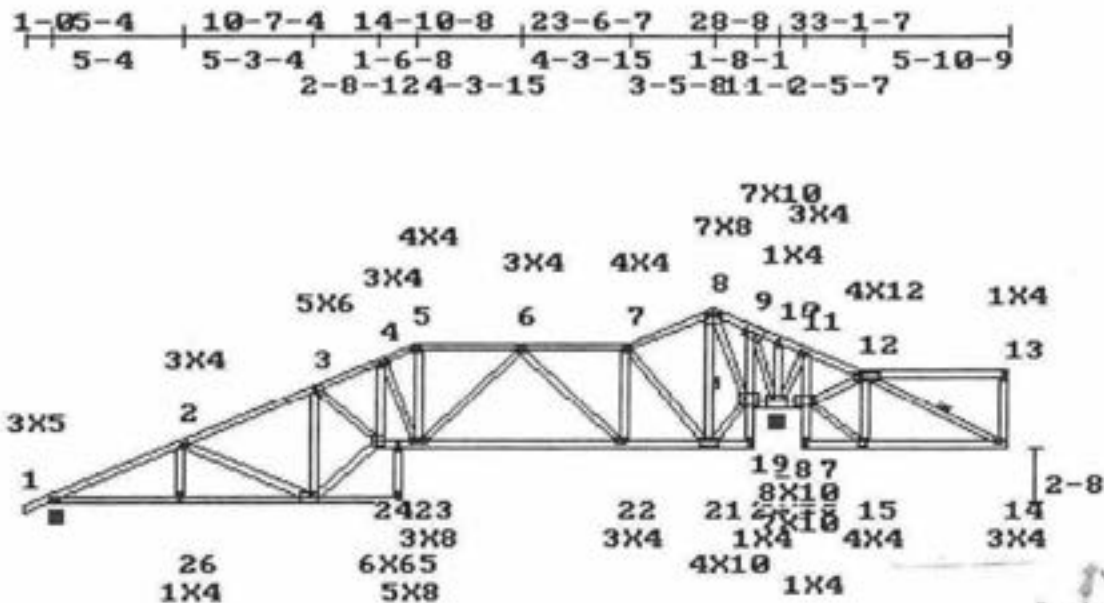
Customer : FOGLIA Truss ID : S25 Thu Mar 1 09:13:17 2001
 Project #: 00469 Quantity : 1 Family #: special
 Span : 39-0 Top Pitch : 6/12
 Bot pitch : 90/12

24-25 12/12,

==8ld(1/21/2000),v1.1=====

FORCES - GRAVITY LOADS

TOP CHORD: 1-2=-2479, 2-3=-1931, 3-4=-2703, 4-5=-2216, 5-6=-1974, 6-7=-860, 7-8= 51 ,
 8-9= 798, 9-10= 1620, 10-11= 1620, 11-12= 1513, 12-13= 0, 13-14=-258,
 BOTTOM CHORD: 14-15=-518, 15-16= 0, 16-17= 23, 17-18=-1364, 18-19=-790, 19-20= 15, 20-21= 0,
 21-22= 860, 22-23= 1591, 23-24= 2419, 24-25= 2420, 25-26= 2180, 26-1= 2180,
 WEBS: 2-26= 79, 2-25=-505, 3-25=-1382, 3-24= 978, 4-24= 1171, 4-23=-1245, 5-23= 745,
 6-23= 572, 6-22=-1096, 7-22= 937, 7-21=-1556, 8-21= 1375, 8-19=-2095, 9-19= 1953,
 9-18=-2527, 10-18=-90, 11-18=-245, 11-17= 130, 12-17=-1000, 12-15= 529, 12-14= 606,
 17-15=-685, 19-21=-76,
 REACTIONS, SIZE: 1=-1565, 8.00 18=-2782, 8.00
 PROVIDE FOR 1474 LBS UPLIFT AT JOINT 1 (0.94)
 PROVIDE FOR 4565 LBS UPLIFT AT JOINT 18 (1.64)
 PROVIDE FOR 698 LBS HORIZ. REACTION AT JOINT 1
 PLATE OFFSETS (X=LEFT, Y=TOP): [j3=3,3], [j8=4,1.5], [j19=5,5], [j25=4,2], (SYMMETRIC PLATING TURNED ON)



====<<<<ACES-32 Ver. 1.1>>>>=====[001824]=====<<<<FL.QUALITY-MMH>>>>=====

Customer : FOGLIA Thu Mar 1 09:13:49 2001

Project #: 00469 Truss ID : S24 Family # : spec.

Span : 39-0 B.Span : 39-0 Quant. : 1 Top Pitch : 6/12

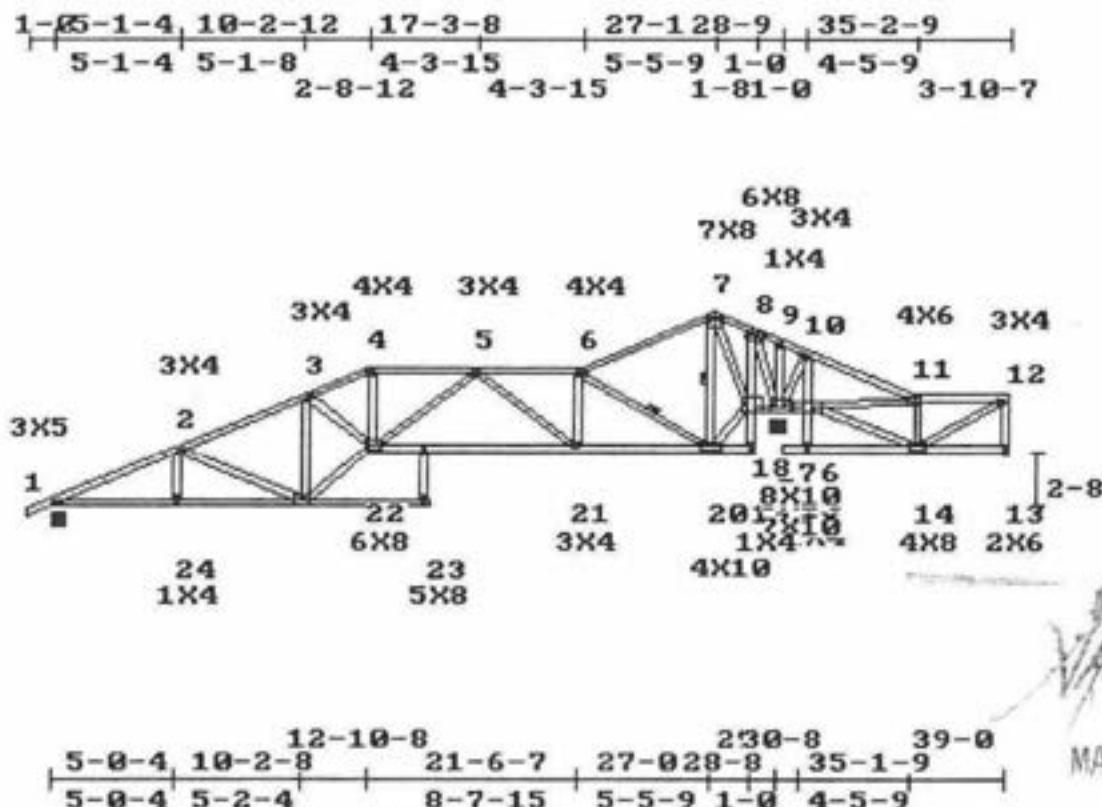
Bot pitch : 90/12

22-23 12/12,

==8ld(1/21/2000),v1.1=====

FORCES - GRAVITY LOADS

TOP CHORD: 1-2=-2509, 2-3=-1985, 3-4=-2844, 4-5=-2538, 5-6=-1592, 6-7= 51, 7-8= 787,
8-9= 1597, 9-10= 1597, 10-11= 1494, 11-12= 301, 12-13= 37,
BOTTOM CHORD: 13-14= 0, 14-15= 0, 15-16= 43, 16-17=-1341, 17-18=-779, 18-19= 15, 19-20= 0,
20-21= 1592, 21-22= 2281, 22-23= 2486, 23-24= 2204, 24-1= 2204,
WEBS: 2-24= 74, 2-23=-477, 3-23=-1444, 3-22= 1044, 4-22= 967, 5-22= 342, 5-21=-924,
6-21= 741, 6-20=-2006, 7-20= 1289, 7-18=-2090, 8-18= 1952, 8-17=-2518, 9-17=-90,
10-17=-256, 10-16= 44, 11-16=-1046, 11-14= 425, 12-14=-364, 16-14=-331, 18-20=-75,
REACTIONS, SIZE: 1=-1570, 8.00 17=-2786, 8.00
PROVIDE FOR 1511 LBS UPLIFT AT JOINT 1 (0.96)
PROVIDE FOR 4456 LBS UPLIFT AT JOINT 17 (1.60)
PROVIDE FOR 602 LBS HORIZ. REACTION AT JOINT 1
PLATE OFFSETS (X=LEFT, Y=TOP): [J7=4, 1.5], [J18=5, 5], [J23=4, 2],



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L. HL TO PK: 14-5-14 INTERNAL RISE: 4-8
LEFT HEIGHT: 0-4-3 SPAN: 39-0 RISE: 9-6-12 RIGHT HEIGHT: 2-9-15

LOADING (PSF)		MAX STRESSES		MINIMUM GRADE OF LUMBER	
L	D	TOP	BOTT	TOP CHORD	BOT CHORD
30	15	1-2=0.942	16-17=0.757	2X4 No.1D 19 SP	2X4 No.2ND 19 SP
0	10	LL.DEFL.@13=0.25 < L/360		WEBS: 2X4 No.3 19 SP	

STR.INC.: LUMB = 1.33 PLATE = 1.33 SPACING : 24.0 in. o. c.
REPETITIVE STRESSES USED NO. OF MEMBERS = 1

CHORD: 7-8; 8-9; 9-10; 10-11; 11-12 TO BE 2X4 No.2ND 19 SP

CHORD: 15-16; 18-19 TO BE 2X4 No.3 19 SP

WEB: 7-18; 8-17 TO BE 2X4 No.2ND 19 SP

RIGHT END VERTICAL IS NOT EXPOSED.

WEB 6-20; 7-20 BRACED AT 1/2 POINTS AS SHOWN ABOVE

Note: Use 1x4 or 2x3 Cont. Bracing conn. w/ 2-8d nails, or T-brace of same size and grade as web conn. to narrow face w/ 10d nails 6 in. o.c

TRUSS HAS BEEN CHECKED FOR 145 M.P.H. WIND LOAD, WALL HEIGHT 16 FT, ENCLOSED BLDG. TYPE, ASSUMING 10 PSF TOTAL D.L.(7+3) (SBC-97).

DEFLECTION(IN.) L.L= 0.25, D.L=0.21, T.L=0.46

PLATES ARE MITEK M20-249,200 MANUFACTURED FROM ASTM A 446 GRD A GALVANIZED STEEL(EXCEPT AS SHOWN)

PLATE MUST BE INSTALLED ON EA. FACE OF JOINT, SYMMETRICALLY(EXCEPT AS SHOWN) DESIGN CONFORMS W/ NDS DESIGN SPECS, SIBC, ANSI/TP1-95

THIS DESIGN IS FOR TRUSS FABRICATION ONLY. FOR PERMANENT AND TEMPORARY BRACING(WHICH IS ALWAYS REQD) CONSULT BLDG ARCHITECT OR ENGINEER.

====<<<ACES-32 Ver. 1.1>>>===== [001825] =====<<<FL.QUALITY-MMH>>>=====

Customer : FOGLIA Thu Mar 1 09:14:53 2001

Project #: 00469 Truss ID : S23 Family #: spec.

Span : 39-0 B.Span : 39-0 Quant. : 1 Top Pitch : 6/12

Bot pitch : 90/12

==8ld(1/21/2000),v1.1=====

FORCES - GRAVITY LOADS						REACTIONS - SIZE	
1-2=-2440	7-8= 1061	10-11=-315	16-17= 0	2-18= 105	5-14= 526	8-11= 97	1=-1512
2-3=-1766	8-9= 0	11-12=-315	17-18= 2138	2-17=-593	5-13=-2394	8-10= 378	12=-2835
3-4=-3132	9-10=-171	12-13= 234	18-1= 2138	3-17=-1010	6-13= 1217	15-17= 2046	
4-5=-2350		13-14= 2350		3-15= 2066	6-12=-1934		
5-6=-262		14-15= 3131		4-15= 28	7-12=-935		
6-7= 948		15-16= 32		4-14=-886	8-12=-687		

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PROVIDE FOR 1467 LBS UPLIFT AT JOINT 1 (0.97)

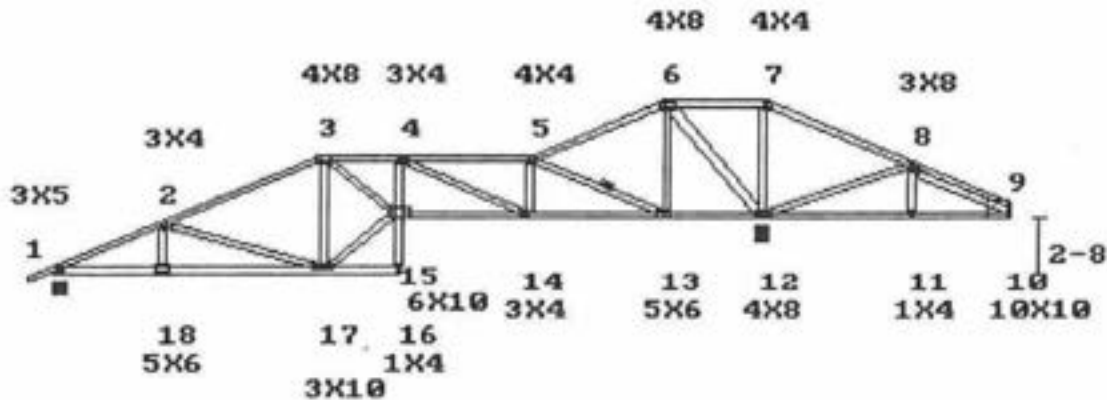
PROVIDE FOR 4641 LBS UPLIFT AT JOINT 12 (1.64)

PROVIDE FOR 667 LBS HORIZ. REACTION AT JOINT 1

PLATE OFFSETS (X=LEFT,Y=TOP):[j13=3,2],[j18=3,2],

1-64-5-4 10-10-8 19-6-7 24-10-8 35-0-12

4-5-4 6-5-4 3-4-8 5-3-7 5-4-1 4-2-15 5-11-5 3-11-4



14-3 29-1-7 39-0

4-5-4 10-10-8 19-6-7 24-10-8 35-0-11

4-5-4 6-5-4 3-4-8 5-3-7 5-4-1 4-2-15 5-11-4

L. HL TO PK:12-1-14 INTERNAL RISE:2-8 R. HL TO PK :11-0-9

LEFT HEIGHT:0-4-3 SPAN:39-0 RISE:8-5-8 RIGHT HEIGHT:0-10-3

LOADING (PSF)		MAX STRESSES		MINIMUM GRADE OF LUMBER	
L	D	TOP	BOTT	TOP CHORD	BOTT CHORD
30	15	7-8=0.983	10-11=0.788	2X4 SS 19 SP	2X4 No.1D 19 SP
0	10	LL.DEFL.@16=0.18 < L/360		WEBS :2X4 No.3 19 SP	

STR.INC.: LUMB = 1.33 PLATE = 1.33

REPETITIVE STRESSES USED

SPACING : 24.0 in. o. c.

NO. OF MEMBERS = 1

CHORD: 3-4; 4-5; 5-6; 7-8; 8-9 TO BE 2X4 No.1D 19 SP

CHORD: 6-7; 15-16; 16-17; 18-1 TO BE 2X4 No.2ND 19 SP

CHORD: 11-12; 12-13 TO BE 2X4 SS 19 SP

WEB: 9-10 TO BE 2X4 No.2ND 19 SP

WEB: 6-12 TO BE 2X6 No.2D 19 SP

RIGHT END VERTICAL IS NOT EXPOSED.

WEB 5-13 BRACED AT 1/2 POINTS AS SHOWN ABOVE

Note:Use 1x4 or 2x3 Cont.Bracing conn.w/2-8d nails,or T-brace of same size and grade as web conn. to narrow face w/10d nails 6 in. o.c

TRUSS HAS BEEN CHECKED FOR 145 M.P.H. WIND LOAD, WALL HEIGHT 16 FT, ENCLOSED BLDG. TYPE, ASSUMING 10 PSF TOTAL D.L.(7+3) (SBC-97).

DEFLECTION(IN.) L.L= 0.18,D.L=0.15,T.L=0.34

PLATES ARE MITEK M20-249,200 MANUFACTURED FROM ASTM A 446 GRD A GALVANIZED STEEL(EXCEPT AS SHOWN)

PLATE MUST BE INSTALLED ON EA. FACE OF JOINT,SYMMETRICALLY(EXCEPT AS SHOWN)DESIGN CONFORMS W/ND5 DESIGN SPECS, SSBC,ANSI/TPI-95

THIS DESIGN IS FOR TRUSS FABRICATION ONLY.FOR PERMANENT AND TEMPORARY BRACING(WHICH IS ALWAYS REQD)CONSULT BLDG ARCHITECT OR ENGINEER.

MAR 01 2001

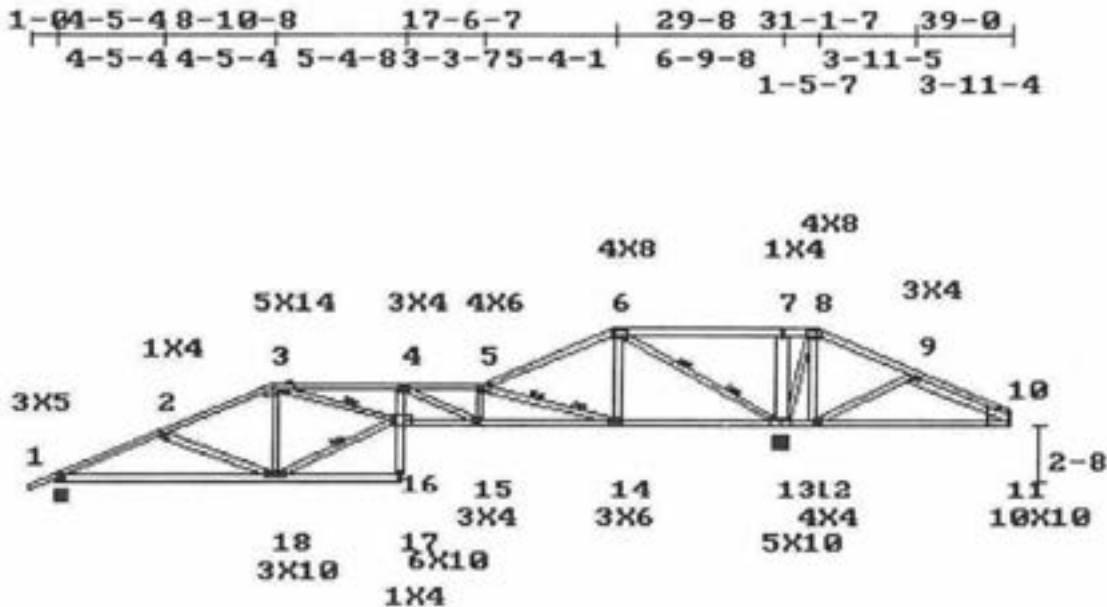
====<<<ACES-32 Ver. 1.1>>>=====[001826]====<<<FL.QUALITY-MMH>>>=====
 Customer : FOGLIA Thu Mar 1 09:15:25 2001
 Project #: 00469 Truss ID : S22 Family #: spec.
 Span : 39-0 B.Span : 39-0 Quant. : 1 Top Pitch : 6/12
 Bot pitch : 90/12

==Bld(1/21/2000),v1.1=====

FORCES - GRAVITY LOADS						REACTIONS - SIZE	
1-2=-2458	7-8= 1032	11-12=-375	17-18= 0	2-18=-315	5-14=-3921	9-12=-423	1=-1565 8.00
2-3=-2097	8-9= 819	12-13=-732	18-1= 2153	3-18=-674	6-14= 1392	9-11= 450	13=-2782 8.00
3-4=-5237	9-10= 0	13-14= 1079		3-16= 3564	6-13=-2532	16-18= 2102	
4-5=-4789	10-11=-171	14-15= 4789		4-16=-135	7-13=-371		
5-6=-1206		15-16= 5247		4-15=-525	8-13=-978		
6-7= 1032		16-17= 52		5-15= 343	8-12= 319		

(20)

PROVIDE FOR 1238 LBS UPLIFT AT JOINT 1 (0.79)
 PROVIDE FOR 3636 LBS UPLIFT AT JOINT 13 (1.31)
 PROVIDE FOR 516 LBS HORIZ. REACTION AT JOINT 1
 PLATE OFFSETS (X=LEFT,Y=TOP):(J3=5.5,2),(J13=5,2),



8-10-8		14-3		17-6-7		22-10-8		29-8		31-1-7		39-0	
8-10-8		5-4-83		3-3-75		4-1		6-9-8		7-10-9			

L. HL TO PK:9-11-1 INTERNAL RISE:2-8 R. HL TO PK :8-9-12
 LEFT HEIGHT:0-4-3 SPAN:39-0 RISE:7-5-8 RIGHT HEIGHT:0-10-3

LOADING (PSF)		MAX STRESSES		MINIMUM GRADE OF LUMBER	
TOP	30	15	TOP 6-7=0.988	TOP CHORD:2X4	No.2ND 19 SP
BOTT	0	10	BOTT 18-1=0.952	BOT CHORD:2X4	SS 19 SP
		LL.DEFL.@17=0.40 < L/360		WEBS	:2X4 No.3 19 SP

STR.INC.: LUMB = 1.33 PLATE = 1.33 SPACING : 24.0 in. o. c.
 REPETITIVE STRESSES USED NO. OF MEMBERS = 1

CHORD: 3-4; 4-5; 6-7; 7-8; 13-14; 14-15; 15-16 TO BE 2X4 No.1D 19 SP
 CHORD: 16-17 TO BE 2X4 No.3 19 SP
 CHORD: 17-18; 18-1 TO BE 2X4 No.2ND 19 SP
 WEB: 3-16 TO BE 2X4 No.2ND 19 SP
 WEB: 7-13 TO BE 2X6 No.2D 19 SP
 RIGHT END VERTICAL IS NOT EXPOSED.

WEB 3-16; 16-18 BRACED at 1/2 POINTS AS SHOWN ABOVE
 WEB 5-14; 6-13 BRACED at 1/3 POINTS AS SHOWN ABOVE

Note:Use 1x4 or 2x3 Cont.Bracing conn.w/2-8d nails,or T-brace of same size and grade as web conn. to narrow face w/10d nails 6 in. o.c
 TRUSS HAS BEEN CHECKED FOR 145 M.P.H. WIND LOAD, WALL HEIGHT 16 FT, ENCLOSED BLDG. TYPE, ASSUMING 10 PSF TOTAL D.L.(7+3) (SBC-97).
 DEFLECTION(IN.) L.L= 0.40,D.L=0.34,T.L=0.74

PLATES ARE MITEK M20-249,200 MANUFACTURED FROM ASTM A 446 GRD A GALVANIZED STEEL(EXCEPT AS SHOWN)

PLATE MUST BE INSTALLED ON EA. FACE OF JOINT,SYMMETRICALLY(EXCEPT AS SHOWN)DESIGN CONFORMS W/NDG DESIGN SPECS, SSBG,ANSI/TPI-95

THIS DESIGN IS FOR TRUSS FABRICATION ONLY,FOR PERMANENT AND TEMPORARY BRACING(WHICH IS ALWAYS REQD)CONSULT BLDG ARCHITECT OR ENGINEER.

[Signature]
 MAR 01 2001

====<<<<ACES-32 Ver. 1.1>>>>===== [001994] =====<<<<FL.QUALITY-MMH>>>>=====

Customer : FOGLIA

Mon Mar 5 12:16:00 2001

Project #: 00469

Truss ID : HH2

Family # : 313

Span : 14-0

B.Span : 14-4

Quant. : 2

Top Pitch : 6/12

==Bld(1/21/2000),v1.1=====

FORCES - LOAD CASE #1

REACTIONS - SIZE

1-2=2176 5-6=1381 2-7= 903
2-3=1381 6-7=1919 2-6=754
3-4= 0 7-1=1919 3-6=683
4-5=348 3-5=1957

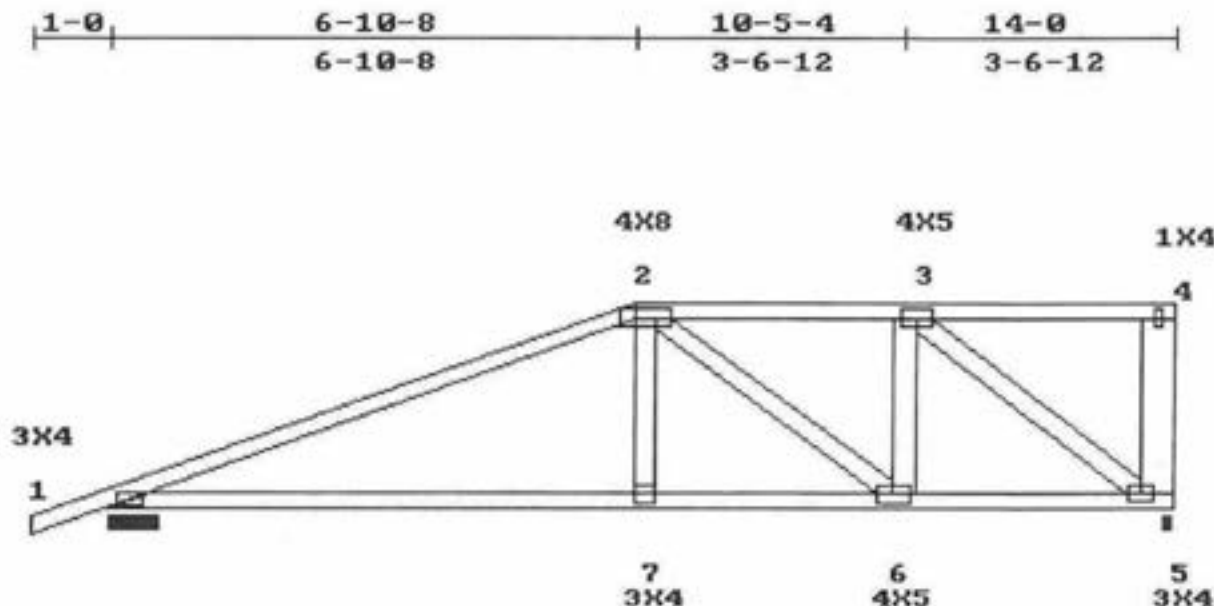
1=1385 8.00
5=1811 2.03

22

PROVIDE FOR 1982 LBS UPLIFT AT JOINT 1 (1.43)

PROVIDE FOR 2346 LBS UPLIFT AT JOINT 5 (1.30)

PROVIDE FOR 410 LBS HORIZ. REACTION AT JOINT 1



6'-10"-8 10'-5"-4 14'-0
6'-10"-8 3'-6"-12 3'-6"-12

L. HL TO PK:7-8-4

LEFT HEIGHT:0-4-3

SPAN:14-0

RISE:3-9-7

RIGHT HEIGHT:3-9-7

LOADING (PSF)

MAX STRESSES

MINIMUM GRADE OF LUMBER

TOP 30 15

TOP 1-2=0.676

TOP CHORD:2X4 No.2ND 19 SP

BOTT 0 10

BOTT 6-7=0.385

BOT CHORD:2X4 No.2ND 19 SP

LL.DEFL.@7=0.03 < L/360

WEBS :2X4 No.3 19 SP

REPETITIVE STRESSES NOT USED

SPACING : 24.0 in. o. c.

NO. OF MEMBERS = 2

LOADING STRESS INCREASE

LOADING

PANEL(PLF) / JOINTS(LBS)

LUMBER PLATE

TYPE

1 1.33 1.33

UNIFORM

1- 2= 90

2- 4= 200

5- 7= 44

7- 1= 20

CONCENTRATED

7= 759

WEB: 4-5 TO BE 2X6 No.2D 19 SP

RIGHT END VERTICAL IS NOT EXPOSED.

2 MEMBERS NAILED TOG. W/1 ROW(S) OF .131x3 in. NAILS 12 in. o.c.(TOP CHS.),AND 1 ROW(S) OF .131x3 in. NAILS 12 in. o.c.(BOTT. CHS.)

For Webs use 1 ROW OF NAILS 12 in. o.c.

TRUSS HAS BEEN CHECKED FOR 145 M.P.H. WIND LOAD, WALL HEIGHT 16 FT, ENCLOSED BLDG. TYPE, ASSUMING 10 PSF TOTAL D.L.(7+3) (SBC-97).

DEFLECTION(IN.) L.L= 0.03,D.L=0.02,T.L=0.05

PLATES ARE MITEK M20-249,200 MANUFACTURED FROM ASTM A 446 GRD A GALVANIZED STEEL(EXCEPT AS SHOWN)

PLATE MUST BE INSTALLED ON EA. FACE OF JOINT,SYMMETRICALLY(EXCEPT AS SHOWN)DESIGN CONFORMS W/NDS DESIGN SPECS, SSB,ANSI/TPI-95

THIS DESIGN IS FOR TRUSS FABRICATION ONLY.FOR PERMANENT AND TEMPORARY BRACING(WHICH IS ALWAYS REQD)CONSULT BLDG ARCHITECT OR ENGINEER.

MAR 06 2001

====<<<ACES-32 Ver. 1.1>>>===== [001828] =====<<<FL.QUALITY-MMH>>>=====

Customer : FOGLIA

Project # : 00469

Span : 9-8-11

Truss ID : CJ9

Quantity : 3

Thu Mar 1 09:17:12 2001

Family # : 204

Top Pitch : 4.243/12

==Bld(1/21/2000),v1.1=====

FORCES - LOAD CASE #1

REACTIONS - SIZE

1-2=749 4-1= 699 2-4=744

1=674 8.00

2-3= 0

4=763 1.66

3-4=392

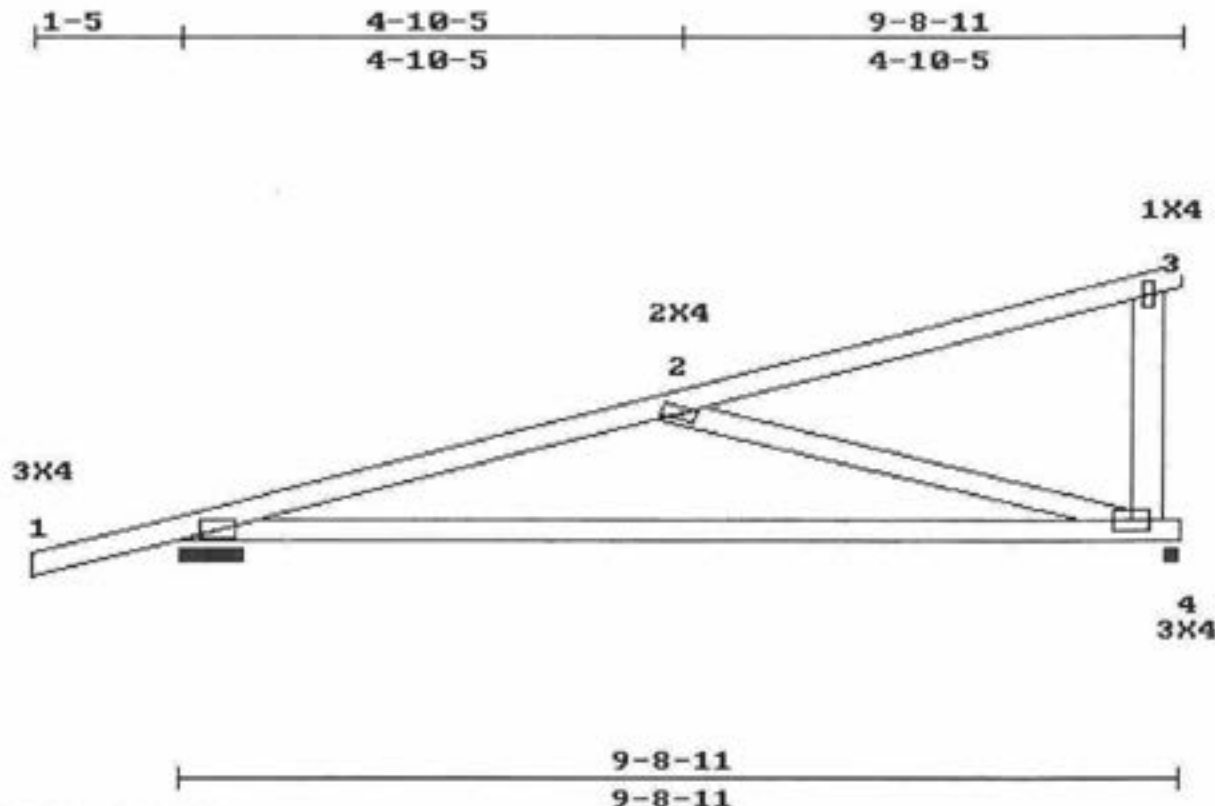
23

PROVIDE FOR 974 LBS UPLIFT AT JOINT 1 (1.45)

PROVIDE FOR 1195 LBS UPLIFT AT JOINT 4 (1.57)

PROVIDE FOR 401 LBS HORIZ. REACTION AT JOINT 1

(SYMMETRIC PLATING TURNED ON)



L. HL TO PK:10-3-12

LEFT HEIGHT:0-3-15

SPAN:9-8-11

RISE:3-9-3

RIGHT HEIGHT:3-9-3

LOADING (PSF)

MAX STRESSES

MINIMUM GRADE OF LUMBER

TOP 30 15

TOP 2-3=0.724

TOP CHORD:2X4 No.2ND 19 SP

BOTT 0 10

BOTT 4-1=0.878

BOT CHORD:2X4 No.2ND 19 SP

LL.DEFL.@0=0.00 < L/360

WEBS :2X4 No.3 19 SP

REPETITIVE STRESSES NOT USED

SPACING : 24.0 in. o. c.

NO. OF MEMBERS = 1

LOADING STRESS INCREASE LOADING PANEL(PLF) / JOINTS(LBS)

LUMBER PLATE TYPE

1 1.33 1.33 UNIFORM 1- 2= 58 2- 3= 163 4- 1= 25

RIGHT END VERTICAL IS NOT EXPOSED.

TRUSS HAS BEEN CHECKED FOR 145 M.P.H. WIND LOAD, WALL HEIGHT 16 FT, ENCLOSED BLDG. TYPE, ASSUMING 10 PSF TOTAL D.L.(7+3) (SBC-97).

DEFLECTION(IN.) L.L= 0.00,D.L=0.00,T.L=0.00

PLATES ARE MITEK M20-249,200 MANUFACTURED FROM ASTM A 446 GRD A GALVANIZED STEEL(EXCEPT AS SHOWN)

PLATE MUST BE INSTALLED ON EA. FACE OF JOINT,SYMMETRICALLY(EXCEPT AS SHOWN)DESIGN CONFORMS W/NDIS DESIGN SPECS, SSSC,ANSI/TP1-95

THIS DESIGN IS FOR TRUSS FABRICATION ONLY.FOR PERMANENT AND TEMPORARY BRACING(WHICH IS ALWAYS REQD)CONSULT BLDG ARCHITECT OR ENGINEER.

[Signature]
MAR 01 2001

====<<<ACES-32 Ver. 1.1>>>===== [001829] =====<<<FL.QUALITY-MMH>>>=====

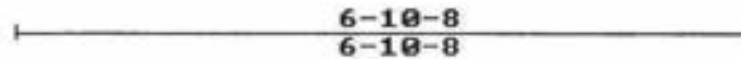
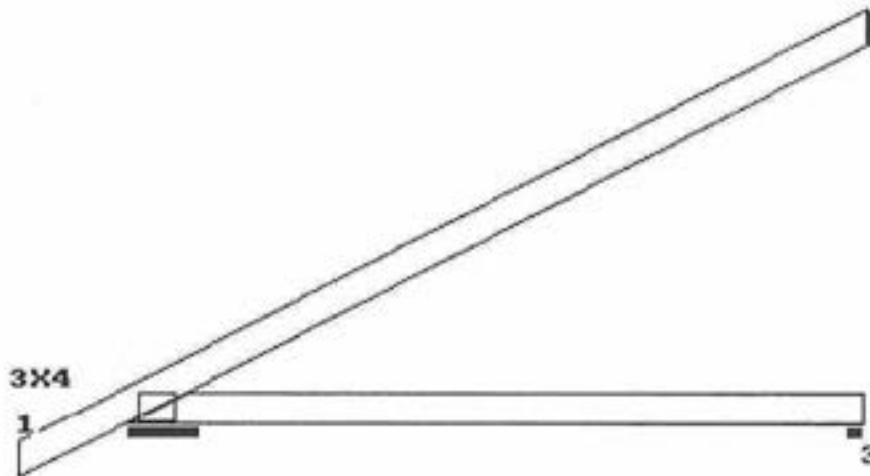
Customer : FOGLIA	Truss ID : J6	Thu Mar 1 09:17:22 2001
Project #: 00469	Quantity : 11	Family # : 205
Span : 6-10-8		Top Pitch : 6/12

==Bld(1/21/2000),v1.1=====

FORCES - GRAVITY LOADS		REACTIONS - SIZE
1-2= 0	3-1= 0	1--466 8.00
2-3= 0		3--65 1.50
		2--292 1.50

(24)

PROVIDE FOR 337 LBS UPLIFT AT JOINT 1 (0.72)
 PROVIDE FOR 441 LBS UPLIFT AT JOINT 2 (1.51)
 PROVIDE FOR 400 LBS HORIZ. REACTION AT JOINT 1
 (SYMMETRIC PLATING TURNED ON)



L. HL TO PK:7-8-4		SPAN:6-10-8		RISE:3-9-7		RIGHT HEIGHT:3-9-7	
LEFT HEIGHT:0-4-3							
LOADING (PSF)		MAX STRESSES		MINIMUM GRADE OF LUMBER			
	L D	TOP	1-2=0.961	TOP CHORD:2X4		No.1D 19 SP	
TOP	30 15	BOTT	3-1=0.203	BOT CHORD:2X4		No.2ND 19 SP	
BOTT	0 10	LL.DEFL.	< L/360	WEBS		:2X4 No.3 19 SP	
STR.INC.: LUMB = 1.33 PLATE = 1.33				SPACING : 24.0 in. o. c.			
REPETITIVE STRESSES USED				NO. OF MEMBERS = 1			

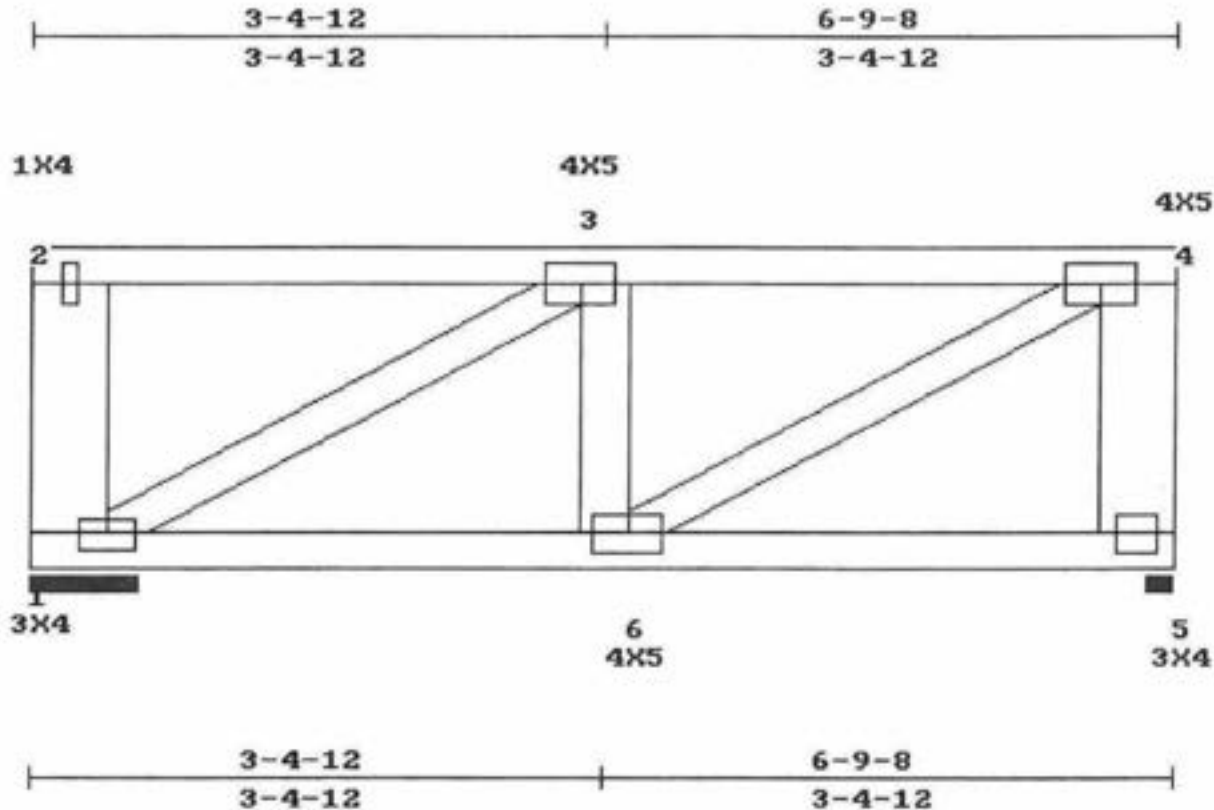
TRUSS HAS BEEN CHECKED FOR 145 M.P.H. WIND LOAD, WALL HEIGHT 16 FT, ENCLOSED BLDG. TYPE, ASSUMING 10 PSF TOTAL D.L.(7+3) (SBC-97).
 DEFLECTION(IN.) L.L= 0.00,D.L=0.00,T.L=0.00
 PLATES ARE MITEK M20-249,200 MANUFACTURED FROM ASTM A 446 GRD A GALVANIZED STEEL(EXCEPT AS SHOWN)
 PLATE MUST BE INSTALLED ON EA. FACE OF JOINT,SYMMETRICALLY(EXCEPT AS SHOWN)DESIGN CONFORMS W/NDG DESIGN SPECS, SBC,ANSI/TPI-95
 THIS DESIGN IS FOR TRUSS FABRICATION ONLY.FOR PERMANENT AND TEMPORARY BRACING(WHICH IS ALWAYS REQD)CONSULT BLDG ARCHITECT OR ENGINEER.

[Signature]
 MAR 01 2001

====<<<<ACES-32 Ver. 1.1>>>>=====[001996]====<<<<FL.QUALITY-MMH>>>>=====
 Customer : FOGLIA Mon Mar 5 12:20:24 2001
 Project #: 00469 Truss ID : FG2 Family #: 301
 Span : 6-9-8 B.Span : 6-9-8 Quant. : 2 Top Pitch : /12
 ==Bld(1/21/2000),v1.1=====

FORCES - LOAD CASE #1			REACTIONS - SIZE	
1-2=-459	5-6= 0	3-1=-850	1=-998	8.00
2-3= 0	6-1= 680	3-6=-429	5=-998	2.03
3-4=-680		4-6= 837		
4-5=-968				

PROVIDE FOR 1214 LBS UPLIFT AT JOINT 1 (1.22)
 PROVIDE FOR 1214 LBS UPLIFT AT JOINT 5 (1.22)



LEFT HEIGHT:2-8 SPAN:6-9-8 RISE:2-8 RIGHT HEIGHT:2-8

LOADING (PSF)		MAX STRESSES		MINIMUM GRADE OF LUMBER	
TOP	30 15	TOP	3-4=0.376	TOP CHORD:2X4	No.2ND 19 SP
BOTT	0 10	BOTT	6-1=0.086	BOT CHORD:2X4	No.2ND 19 SP
		LL.DEFL.@6=0.00	< L/360	WEBS	:2X4 No.3 19 SP

REPETITIVE STRESSES NOT USED

SPACING : 24.0 in. o. c.
 NO. OF MEMBERS = 2

LOADING STRESS INCREASE	LOADING LUMBER PLATE TYPE	PANEL(PLF) / JOINTS(LBS)
1	1.33 1.33 UNIFORM	2- 4= 290 5- 1= 18

WEB: 1-2; 4-5 TO BE 2X6 No.20 19 SP
 BOTH END VERTICALS ARE NOT EXPOSED.
 2 MEMBERS NAILED TOG. W/1 ROW(S) OF .131x3 in. NAILS 9 in. o.c.(TOP CHS.),AND 1 ROW(S) OF .131x3 in. NAILS 12 in. o.c.(BOTT. CHS.)
 For Webs use 1 ROW of NAILS 12 in. o.c.
 TRUSS HAS BEEN CHECKED FOR 145 M.P.H. WIND LOAD, WALL HEIGHT 16 FT, ENCLOSED BLDG. TYPE, ASSUMING 10 PSF TOTAL D.L.(7+3) (SBC-97).
 DEFLECTION(IN.) L.L= 0.00,D.L=0.00,T.L=0.01
 PLATES ARE MITEK M20-249,200 MANUFACTURED FROM ASTM A 446 GRD A GALVANIZED STEEL(EXCEPT AS SHOWN)
 PLATE MUST BE INSTALLED ON EA. FACE OF JOINT,SYMMETRICALLY(EXCEPT AS SHOWN)DESIGN CONFORMS W/NDS DESIGN SPECS, S58C,ANSI/TPI-95
 THIS DESIGN IS FOR TRUSS FABRICATION ONLY.FOR PERMANENT AND TEMPORARY BRACING(WHICH IS ALWAYS REQD)CONSULT BLDG ARCHITECT OR ENGINEER.

MAR 06 2001

====<<<ACES-32 Ver. 1.1>>>===== [001831] =====<<<FL.QUALITY-MMH>>>=====

Customer : FOGLIA Thu Mar 1 09:17:55 2001

Project #: 00469 Truss ID : C4 Family #: 205

Span : 5-0 Quantity : 1 Top Pitch : 6/12

==Bld(1/21/2000),v1.1=====

FORCES - GRAVITY LOADS

1-2= 0
2-3= 0

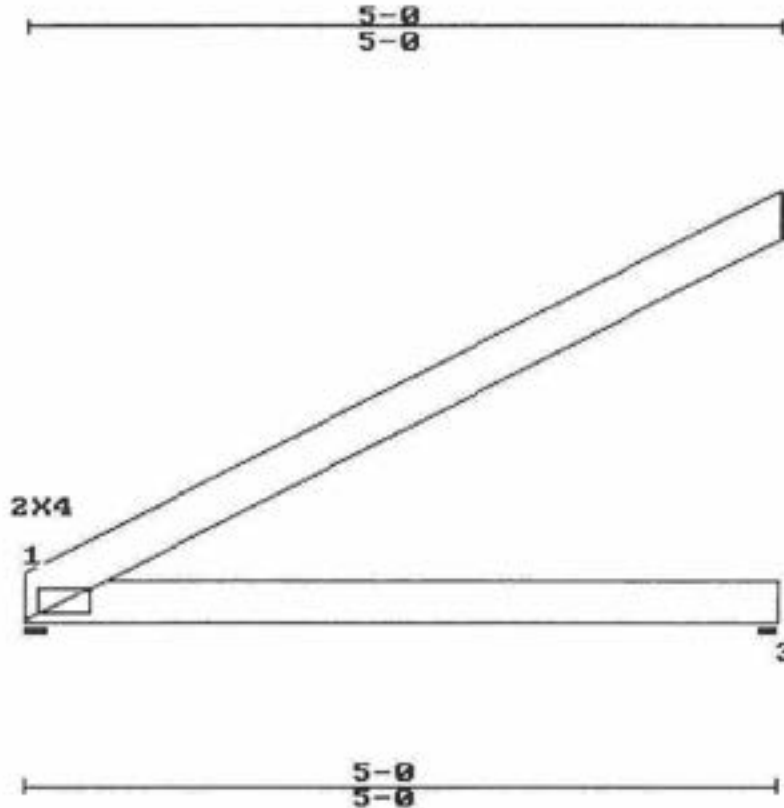
3-1= 0

REACTIONS - SIZE

1=-267 2.00
3=-49 1.50
2=-218 1.50

(24)

PROVIDE FOR 163 LBS UPLIFT AT JOINT 1 (0.61)
PROVIDE FOR 325 LBS UPLIFT AT JOINT 2 (1.49)
PROVIDE FOR 289 LBS HORIZ. REACTION AT JOINT 1
(SYMMETRIC PLATING TURNED ON)



L. HL TO PK:5-7-1
LEFT HEIGHT:0-4-3

SPAN:5-0

RISE:2-10-3

RIGHT HEIGHT:2-10-3

LOADING (PSF)

	L	D
TOP	30	15
BOTT	0	10

MAX STRESSES

TOP	1-2=0.702
BOTT	3-1=0.103
LL.DEFL.	< L/360

MINIMUM GRADE OF LUMBER

TOP CHORD:2X4	No.2ND 19 SP
BOT CHORD:2X4	No.2ND 19 SP
WEBS	:2X4 No.3 19 SP

STR.INC.: LUMB = 1.33 PLATE = 1.33
REPETITIVE STRESSES USED

SPACING : 24.0 in. o. c.
NO. OF MEMBERS = 1

TRUSS HAS BEEN CHECKED FOR 145 M.P.H. WIND LOAD, WALL HEIGHT 16 FT, ENCLOSED BLDG. TYPE, ASSUMING 10 PSF TOTAL D.L.(7+3) (SBC-97).
DEFLECTION(IN.) L.L= 0.00,D.L=0.00,T.L=0.00

PLATES ARE MITEK M20-249,200 MANUFACTURED FROM ASTM A 446 GRD A GALVANIZED STEEL(EXCEPT AS SHOWN)

PLATE MUST BE INSTALLED ON EA. FACE OF JOINT,SYMMETRICALLY(EXCEPT AS SHOWN)DESIGN CONFORMS W/NDS DESIGN SPECS, SSSC,ANSI/TP1-95

THIS DESIGN IS FOR TRUSS FABRICATION ONLY.FOR PERMANENT AND TEMPORARY BRACING(WHICH IS ALWAYS REQD)CONSULT BLDG ARCHITECT OR ENGINEER.

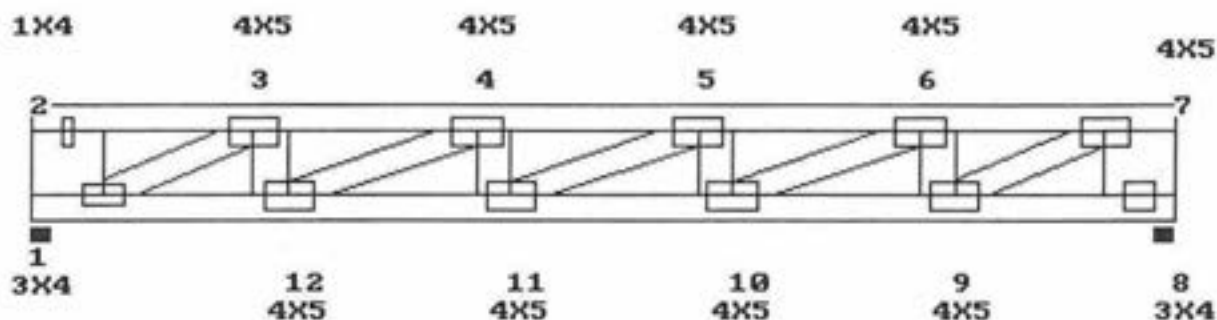
MAR 01 2001

====<<<ACES-32 Ver. 1.1>>>=====[001832]====<<<FL.QUALITY-MMH>>>=====
 Customer : FOGLIA Thu Mar 1 09:18:20 2001
 Project #: 00469 Truss ID : NG2 Family #: 301
 Span : 9-7 Quantity : 2 Top Pitch : /12
 ==Bld(1/21/2000),v1.1=====

FORCES - LOAD CASE #1				REACTIONS - SIZE	
1-2=-229	7-8=-1158	8-9= 0	3-1=-1950	6-10= 943	1=-1168 2.00
2-3= 0		9-10= 1714	3-12= 480	6-9=-911	8=-1168 2.00
3-4=-1714		10-11= 2536	4-12=-943	7-9= 1950	
4-5=-2536		11-12= 2536	4-11= 19		
5-6=-2536		12-1= 1714	5-11= 0		
6-7=-1714			5-10=-442		

PROVIDE FOR 1510 LBS UPLIFT AT JOINT 1 (1.29)
 PROVIDE FOR 1510 LBS UPLIFT AT JOINT 8 (1.29)
 (SYMMETRIC PLATING TURNED ON)

2-0-1	3-10-6	5-8-10	7-6-15	9-7
2-0-1	1-10-5	1-10-5	1-10-5	2-0-1



2-0-1	3-10-6	5-8-10	7-6-15	9-7
2-0-1	1-10-5	1-10-5	1-10-5	2-0-1

LEFT HEIGHT:1-4 SPAN:9-7 RISE:1-4 RIGHT HEIGHT:1-4

LOADING (PSF)		MAX STRESSES		MINIMUM GRADE OF LUMBER	
TOP	L 30 D 15	TOP	5-6=0.345	TOP CHORD:2X4	No.2ND 19 SP
BOTT	0 10	BOTT	11-12=0.239	BOT CHORD:2X4	No.2ND 19 SP
		LL.DEFL.@11=0.03 < L/360	WEBS	:2X4	No.3 19 SP

REPETITIVE STRESSES NOT USED

SPACING : 24.0 in. o. c.
 NO. OF MEMBERS = 2

LOADING	STRESS INCREASE	LOADING	PANEL(PLF) / JOINTS(LBS)
LUMBER	PLATE	TYPE	
1	1.33	1.33	UNIFORM 2- 7= 238 8- 1= 10

WEB: 1-2; 7-8 TO BE 2X8 SS 19 SP
 BOTH END VERTICALS ARE NOT EXPOSED.
 2 MEMBERS NAILED TOG. W/1 ROW(S) OF .131x3 in. NAILS 11 in. o.c.(TOP CHS.),AND 1 ROW(S) OF .131x3 in. NAILS 12 in. o.c.(BOTT. CHS.)
 For Webs use 1 ROW of NAILS 12 in. o.c.
 TRUSS HAS BEEN CHECKED FOR 145 M.P.H. WIND LOAD, WALL HEIGHT 16 FT, ENCLOSED BLDG. TYPE, ASSUMING 10 PSF TOTAL D.L.(7+3) (SBC-97).
 DEFLECTION(IN.) L.L= 0.03,D.L=0.03,T.L=0.06
 PLATES ARE MITEK M20-249,200 MANUFACTURED FROM ASTM A 446 GRD A GALVANIZED STEEL(EXCEPT AS SHOWN)
 PLATE MUST BE INSTALLED ON EA. FACE OF JOINT,SYMMETRICALLY(EXCEPT AS SHOWN)DESIGN CONFORMS W/ND5 DESIGN SPECS, 558C,ANSI/TPI-95
 THIS DESIGN IS FOR TRUSS FABRICATION ONLY.FOR PERMANENT AND TEMPORARY BRACING(WHICH IS ALWAYS REQD)CONSULT BLDG ARCHITECT OR ENGINEER.

MAR 01 2001
[Signature]

====<<<<ACES-32 Ver. 1.1>>>>===== [001833] =====<<<<FL.QUALITY-MMH>>>>=====

Customer : FOGLIA

Project # : 00469

Span : 1-0

Truss ID : C1A

Quantity : 2

Thu Mar 1 09:18:32 2001

Family # : 205

Top Pitch : 6/12

==Bld(1/21/2000),v1.1=====

FORCES - GRAVITY LOADS

REACTIONS - SIZE

1-2= 271 3-1= 0

3-10 1.50

2-3= 0

2-159 1.50

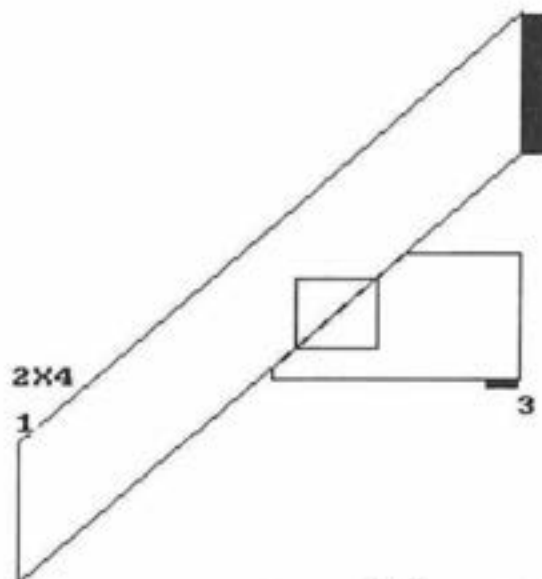
PROVIDE FOR 60 LBS UPLIFT AT JOINT 3 (6.00)

PROVIDE FOR 281 LBS UPLIFT AT JOINT 2 (1.76)

PROVIDE FOR 146 LBS HORIZ. REACTION AT JOINT 3

(SYMMETRIC PLATING TURNED ON)

1-0 1-0
1-0



1-0
1-0

L. HL TO PK:1-1-7

LEFT HEIGHT:0-4-3

SPAN:1-0

RISE:0-10-3

RIGHT HEIGHT:0-10-3

LOADING (PSF)

L D

TOP 30 15

BOTT 0 10

MAX STRESSES

TOP 1-2=0.055

BOTT 3-1=0.040

LL.DEFL. < L/360

MINIMUM GRADE OF LUMBER

TOP CHORD:2X4 No.2ND 19 SP

BOT CHORD:2X4 No.2ND 19 SP

WEBS :2X4 No.3 19 SP

STR.INC.: LUMB = 1.33 PLATE = 1.33

REPETITIVE STRESSES USED

SPACING : 24.0 in. o. c.

NO. OF MEMBERS = 1

TRUSS HAS BEEN CHECKED FOR 145 M.P.H. WIND LOAD, WALL HEIGHT 16 FT, ENCLOSED BLDG. TYPE, ASSUMING 10 PSF TOTAL D.L.(7+3) (SBC-97).

DEFLECTION(IN.) L.L= 0.00,D.L=0.00,T.L=0.00

PLATES ARE MITEK M20-249,200 MANUFACTURED FROM ASTM A 446 GRD A GALVANIZED STEEL(EXCEPT AS SHOWN)

PLATE MUST BE INSTALLED ON EA. FACE OF JOINT,SYMMETRICALLY(EXCEPT AS SHOWN)DESIGN CONFORMS W/NOS DESIGN SPECS, SSBC,ANSI/TP1-95

THIS DESIGN IS FOR TRUSS FABRICATION ONLY.FOR PERMANENT AND TEMPORARY BRACING(WHICH IS ALWAYS REQD)CONSULT BLDG ARCHITECT OR ENGINEER.

MAR 01 2001

====<<<ACES-32 Ver. 1.1>>>===== [001834] =====<<<FL.QUALITY-MMH>>>=====

Customer : FOGLIA

Project # : 00469

Span : 3-0

Truss ID : C3A

Quantity : 2

Thu Mar 1 09:18:43 2001

Family # : 205

Top Pitch : 6/12

==Bld(1/21/2000),v1.1=====

FORCES - GRAVITY LOADS

1-2= 358 3-1=148

2-3= 0

REACTIONS - SIZE

3=30 1.50

2=292 1.50

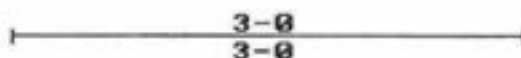
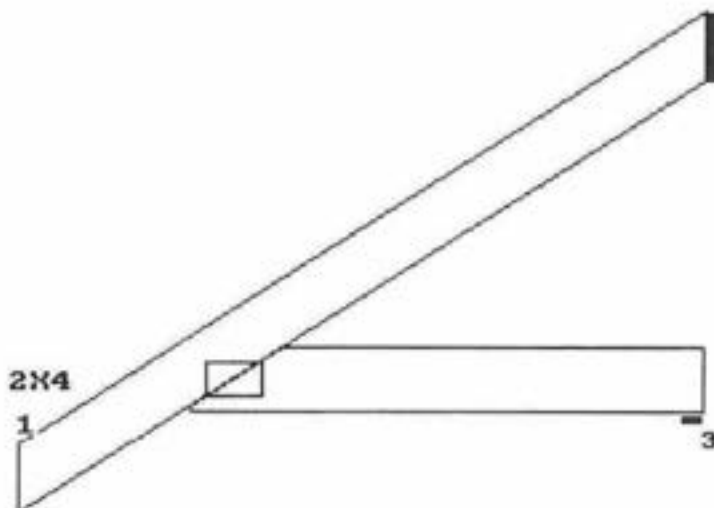
(2a)

PROVIDE FOR 180 LBS UPLIFT AT JOINT 3 (6.00)

PROVIDE FOR 470 LBS UPLIFT AT JOINT 2 (1.61)

PROVIDE FOR 593 LBS HORIZ. REACTION AT JOINT 3

(SYMMETRIC PLATING TURNED ON)



L. HL TO PK:3-4-4

LEFT HEIGHT:0-4-3

SPAN:3-0

RISE:1-10-3

RIGHT HEIGHT:1-10-3

LOADING (PSF)

L D

TOP 30 15

BOTT 0 10

MAX STRESSES

TOP 1-2=0.237

BOTT 3-1=0.312

LL.DEFL. < L/360

MINIMUM GRADE OF LUMBER

TOP CHORD:2X4 No.2ND 19 SP

BOT CHORD:2X4 No.2ND 19 SP

WEBS :2X4 No.3 19 SP

STR.INC.: LUMB = 1.33 PLATE = 1.33

REPETITIVE STRESSES USED

SPACING : 24.0 in. o. c.

NO. OF MEMBERS = 1

TRUSS HAS BEEN CHECKED FOR 145 M.P.H. WIND LOAD, WALL HEIGHT 16 FT, ENCLOSED BLDG. TYPE, ASSUMING 10 PSF TOTAL D.L.(7+3) (SBC-97).

DEFLECTION(IN.) L.L= 0.00,D.L=0.00,T.L=0.00

PLATES ARE MITEK M20-249,200 MANUFACTURED FROM ASTM A 446 GRD A GALVANIZED STEEL(EXCEPT AS SHOWN)

PLATE MUST BE INSTALLED ON EA. FACE OF JOINT,SYMMETRICALLY(EXCEPT AS SHOWN)DESIGN CONFORMS W/NDIS DESIGN SPECS, SSSC,ANSI/TPI-95

THIS DESIGN IS FOR TRUSS FABRICATION ONLY.FOR PERMANENT AND TEMPORARY BRACING(WHICH IS ALWAYS REQD)CONSULT BLDG ARCHITECT OR ENGINEER.

[Signature]

MAR 01 2001

====<<<ACES-32 Ver. 1.1>>>=====[001835]=====<<<FL.QUALITY-MMH>>>=====

Customer : FOGLIA

Project # : 00469

Span : 5-0

Truss ID : C5A

Quantity : 2

Thu Mar 1 09:18:53 2001

Family # : 205

Top Pitch : 6/12

==Bld(1/21/2000),v1.1=====

FORCES - GRAVITY LOADS

1-2=1717 3-1=1190

2-3= 0

REACTIONS - SIZE

3=50 1.50

2=984 1.50

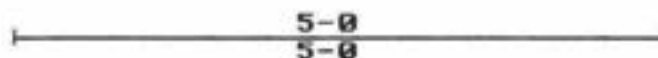
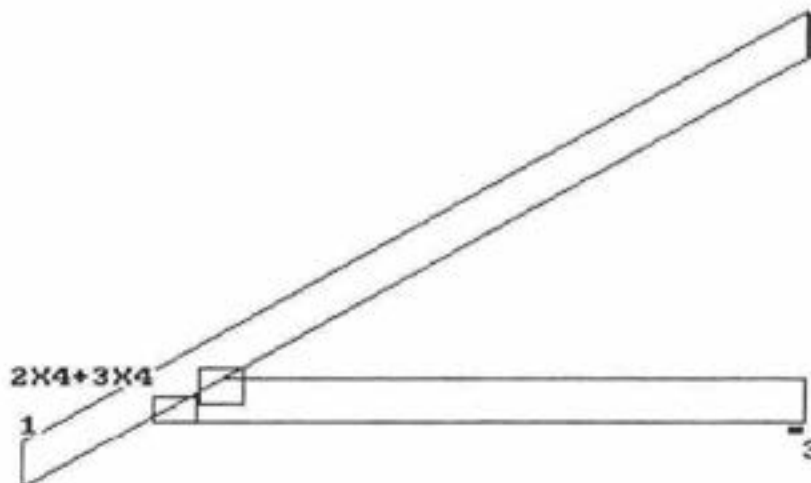
(20)

PROVIDE FOR 300 LBS UPLIFT AT JOINT 3 (6.00)

PROVIDE FOR 934 LBS UPLIFT AT JOINT 2 (0.95)

PROVIDE FOR 1189 LBS HORIZ. REACTION AT JOINT 3

(SYMMETRIC PLATING TURNED ON)



L. HL TO PK:5-7-1

LEFT HEIGHT:0-4-3

SPAN:5-0

RISE:2-10-3

RIGHT HEIGHT:2-10-3

LOADING (PSF)

L D

TOP 30 15

BOTT 0 10

MAX STRESSES

TOP 1-2=0.742

BOTT 3-1=0.620

LL.DEFL. < L/360

MINIMUM GRADE OF LUMBER

TOP CHORD:2X4 No.2ND 19 SP

BOT CHORD:2X4 No.2ND 19 SP

WEBS :2X4 No.3 19 SP

STR.INC.: LUMB = 1.33 PLATE = 1.33

REPETITIVE STRESSES USED

SPACING : 24.0 in. o. c.

NO. OF MEMBERS = 1

TRUSS HAS BEEN CHECKED FOR 145 M.P.H. WIND LOAD, WALL HEIGHT 16 FT, ENCLOSED BLDG. TYPE, ASSUMING 10 PSF TOTAL D.L.(7*3) (SBC-97).

DEFLECTION(IN.) L.L.= 0.00,D.L.=0.00,T.L.=0.00

PLATES ARE MITEK M20-249,200 MANUFACTURED FROM ASTM A 446 GRD A GALVANIZED STEEL(EXCEPT AS SHOWN)

PLATE MUST BE INSTALLED ON EA. FACE OF JOINT,SYMMETRICALLY(EXCEPT AS SHOWN)DESIGN CONFORMS W/NDS DESIGN SPECS, SSBC,ANSI/TPI-95

THIS DESIGN IS FOR TRUSS FABRICATION ONLY.FOR PERMANENT AND TEMPORARY BRACING(WHICH IS ALWAYS REQD)CONSULT BLDG ARCHITECT OR ENGINEER.

[Signature]
MAR 01 2001

====<<<<ACES-32 Ver. 1.1>>>>===== [001993] =====<<<<FL.QUALITY-MMH>>>>=====

Customer : FOGLIA Mon Mar 5 12:14:23 2001

Project #: 00469 Truss ID : H4 Family #: 314

Span : 25-11-8 B.Span : 25-11-8 Quant. : 2 Top Pitch : 6/12

==Bld(1/21/2000),v1.1=====

FORCES - LOAD CASE #1

1-2=1592	7-8=652	2-11=188	5-8=1050
2-3=1416	8-9=961	3-11=2313	6-8=346
3-4=592	9-10=2643	3-10=2074	
4-5=2643	10-11=592	4-10=3065	
5-6=1076	11-1=1416	4-9=2888	
6-7=733		5-9=2603	

REACTIONS - SIZE

1=1102	2.00
9=5008	5.66

31

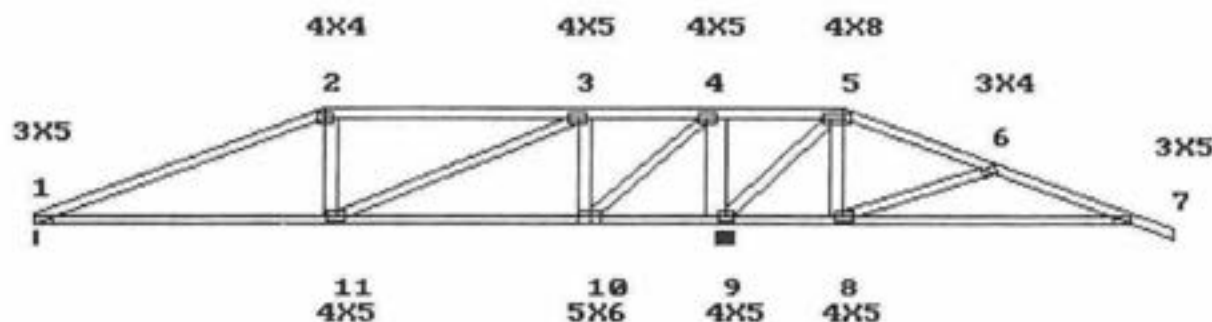
PROVIDE FOR 1163 LBS UPLIFT AT JOINT 1 (1.06)

PROVIDE FOR 8145 LBS UPLIFT AT JOINT 9 (1.63)

PROVIDE FOR 117 LBS HORIZ. REACTION AT JOINT 1

PLATE OFFSETS (X=LEFT,Y=TOP):(1)10=3,2),

6-10-8	12-11-12	16-1-8	19-1	22-6-4	25-11-8	1-0
6-10-8	6-1-4	3-1-12	2-11-8	3-5-4	3-5-4	



6-10-8	12-11-12	16-1-8	19-1	25-11-8
6-10-8	6-1-4	3-1-12	2-11-8	6-10-8

L. HL TO PK:7-8-4 LEFT HEIGHT:0-4-3 SPAN:25-11-8 RISE:3-9-7 R. HL TO PK:7-8-4 RIGHT HEIGHT:0-4-3

LOADING (PSF)	MAX STRESSES	MINIMUM GRADE OF LUMBER
L D	TOP 3-4=0.878	TOP CHORD:2X4 No.2ND 19 SP
TOP 30 15	BOTT 7-8=0.714	BOT CHORD:2X4 No.2ND 19 SP
BOTT 0 10	LL.DEFL.@7=0.14 < L/360	WEBS :2X4 No.3 19 SP

REPETITIVE STRESSES NOT USED

SPACING : 24.0 in. o. c.

NO. OF MEMBERS = 2

LOADING	STRESS INCREASE	LOADING	PANEL(PLF) / JOINTS(LBS)
LUMBER	PLATE	TYPE	
1	1.33	1.33	UNIFORM 1- 2= 90 2- 5= 200 5- 7= 90 7- 8= 20 8-11= 44 11- 1= 20
			CONCENTRATED 8= 759 11= 759

WEB: 4-9 TO BE 2X6 No.20 19 SP

2 MEMBERS NAILED TOG. W/1 ROW(S) OF .131x3 in. NAILS 12 in. o.c.(TOP CHS.),AND 1 ROW(S) OF .131x3 in. NAILS 12 in. o.c(BOTT. CHS.)

For Webs use 1 ROW of NAILS 12 in. o.c.

TRUSS HAS BEEN CHECKED FOR 145 M.P.H. WIND LOAD, WALL HEIGHT 16 FT, ENCLOSED BLDG. TYPE, ASSUMING 10 PSF TOTAL D.L.(7+3) (SBC-97).

DEFLECTION(IN.) L.L= 0.14,D.L=0.11,T.L=0.25

PLATES ARE MITEK M20-249,200 MANUFACTURED FROM ASTM A 446 GRD A GALVANIZED STEEL(EXCEPT AS SHOWN)

PLATE MUST BE INSTALLED ON EA. FACE OF JOINT,SYMMETRICALLY(EXCEPT AS SHOWN)DESIGN CONFORMS W/ND S DESIGN SPECS, SSBC,ANSI/TPI-95

THIS DESIGN IS FOR TRUSS FABRICATION ONLY.FOR PERMANENT AND TEMPORARY BRACING(WHICH IS ALWAYS REQD)CONSULT BLDG ARCHITECT OR ENGINEER.

MAR 06 2001

====<<<ACES-32 Ver. 1.1>>>===== [001837] =====<<<FL.QUALITY-MMH>>>=====

Customer : FOGLIA Thu Mar 1 09:19:58 2001

Project #: 00469 Family #: 301

Span : 7-0 B.Span : 7-0 Quant. : 2 Top Pitch : /12

==Bld(1/21/2000),v1.1=====

FORCES - LOAD CASE #1

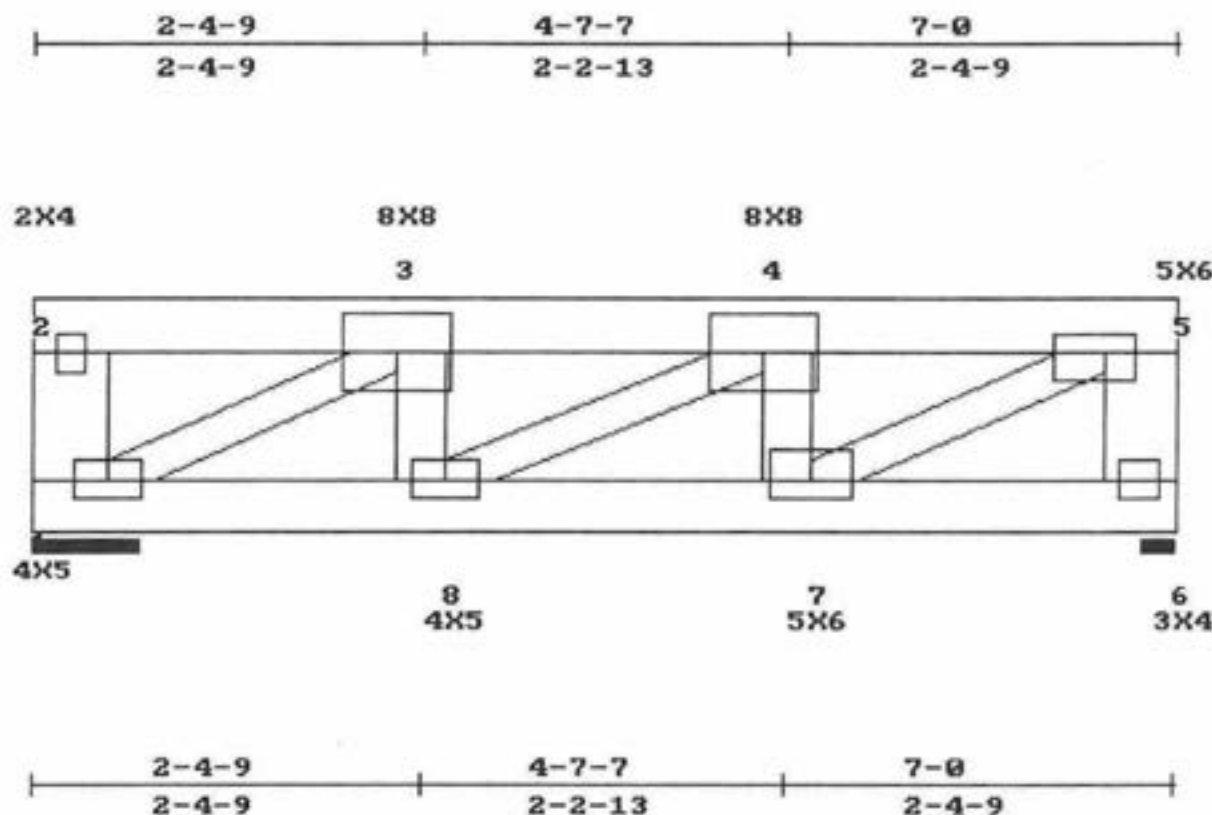
1-2=-1625 6-7= 0 3-1=-5930
2-3= 0 7-8= 4956 3-8= 139
3-4=-4821 8-1= 4821 4-8=-164
4-5=-4956 4-7=-3312
5-6=-5077 5-7= 5988

REACTIONS - SIZE

1=-5101 8.00
6=-5101 2.59

32

PROVIDE FOR 4916 LBS UPLIFT AT JOINT 1 (0.96)
PROVIDE FOR 4916 LBS UPLIFT AT JOINT 6 (0.96)
PLATE OFFSETS (X=LEFT,Y=TOP):[J5=3.5,2],[J7=3,3],



LEFT HEIGHT:2-0 SPAN:7-0 RISE:2-0 RIGHT HEIGHT:2-0

LOADING (PSF) MAX STRESSES MINIMUM GRADE OF LUMBER

L	D	TOP	4-5=0.617	TOP CHORD:2X6	No.2D 19 SP
30	15	BOTT	7-8=0.296	BOT CHORD:2X6	No.2D 19 SP
0	10	LL.DEFL.	@8=0.02 < L/360	WEBS	:2X4 No.3 19 SP

REPETITIVE STRESSES NOT USED

SPACING : 24.0 in. o. c.
NO. OF MEMBERS = 2

LOADING	STRESS INCREASE	LOADING	PANEL(PLF) / JOINTS(LBS)
LUMBER	PLATE	TYPE	
1	1.33	1.33	UNIFORM 2- 5= 1510 6- 1= 21

WEB: 1-2; 5-6 TO BE 2X6 No.2D 19 SP

BOTH END VERTICALS ARE NOT EXPOSED.

2 MEMBERS NAILED TOG. W/2 ROW(S) OF .131x3 in. NAILS 3 in. o.c.(TOP CHS.),AND 1 ROW(S) OF .131x3 in. NAILS 12 in. o.c(BOTT. CHS.)

For Webs use 1 ROW of NAILS 12 in. o.c.

TRUSS HAS BEEN CHECKED FOR 145 M.P.H. WIND LOAD, WALL HEIGHT 16 FT, ENCLOSED BLDG. TYPE, ASSUMING 10 PSF TOTAL D.L.(7+3) (SBC-97).

DEFLECTION(IN.) L.L= 0.02,D.L=0.02,T.L=0.04

PLATES ARE MITEK M20-249,200 MANUFACTURED FROM ASTM A 446 GRD A GALVANIZED STEEL(EXCEPT AS SHOWN)

PLATE MUST BE INSTALLED ON EA. FACE OF JOINT,SYMMETRICALLY(EXCEPT AS SHOWN)DESIGN CONFORMS W/NDG DESIGN SPECS, SSSC,ANSI/TPI-95

THIS DESIGN IS FOR TRUSS FABRICATION ONLY.FOR PERMANENT AND TEMPORARY BRACING(WHICH IS ALWAYS REQD)CONSULT BLDG ARCHITECT OR ENGINEER.

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MAR 01 2001

====<<<<ACES-32 Ver. 1.1>>>>=====[001999]=====<<<<FL.QUALITY-MMH>>>>=====

Customer : FOGLIA Mon Mar 5 12:28:54 2001

Project #: 00469 Truss ID : T2 Family #: 313

Span : 35-1-8 B.Span : 35-1-8 Quant. : 1 Top Pitch : 6/12

==Bld(1/21/2000),v1.1=====

FORCES - GRAVITY LOADS

1-2=-28	7-8=-2321	9-10= 0	3-1=-1833	6-11=-655
2-3= 0	8-9=-1851	10-11= 2321	3-13= 2691	7-11= 1424
3-4=-2505		11-12= 3505	4-13=-620	7-10=-1406
4-5=-2505		12-13= 3566	5-13=-1276	8-10= 2787
5-6=-3566		13-1= 252	5-12= 95	
6-7=-3505			6-12= 74	

REACTIONS - SIZE

1=-1920	2.68
9=-1920	2.68

33

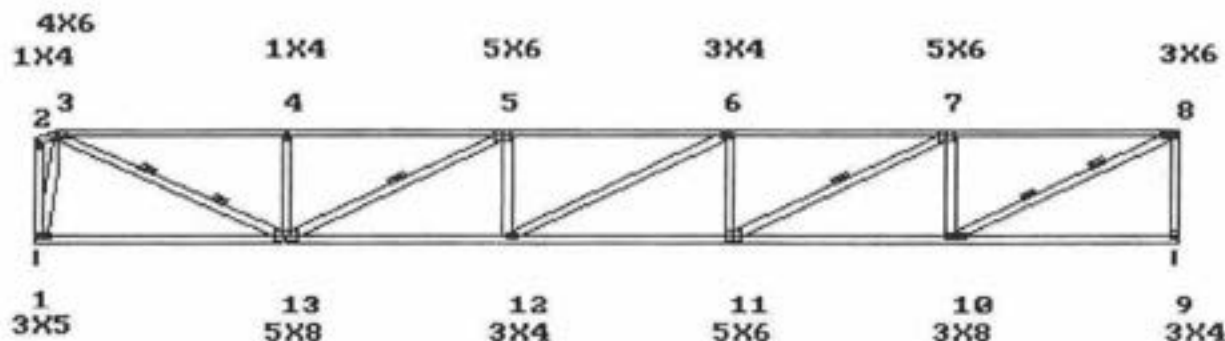
PROVIDE FOR 1699 LBS UPLIFT AT JOINT 1 (0.88)

PROVIDE FOR 1708 LBS UPLIFT AT JOINT 9 (0.89)

PROVIDE FOR 37 LBS HORIZ. REACTION AT JOINT 1

PLATE OFFSETS (X=LEFT,Y=TOP):[j5=3,3],[j7=3,3],[j8=4,1.5],[j11=3,2],[j13=4,2],

0-8-157-8-8	14-6-5	21-4-2	28-1-15	35-1-8
0-8-156-11-9	6-9-13	6-9-13	6-9-13	6-11-9



7-8-8	14-6-5	21-4-2	28-1-15	35-1-8
7-8-8	6-9-13	6-9-13	6-9-13	6-11-9

L. HL TO PK:0-10

LEFT HEIGHT:4-5-11

SPAN:35-1-8

RISE:4-10-3

RIGHT HEIGHT:4-10-3

LOADING (PSF)

	L	D
TOP	30	15
BOTT	0	10

MAX STRESSES

TOP	6-7=0.871
BOTT	12-13=0.898
LL.DEFL.@12	=0.25 < L/360

MINIMUM GRADE OF LUMBER

TOP CHORD:2X4	No.2ND 19 SP
BOT CHORD:2X4	No.1D 19 SP
WEBS	:2X4 No.3 19 SP

STR.INC.: LUMB = 1.33 PLATE = 1.33

REPETITIVE STRESSES USED

SPACING : 24.0 in. o. c.

NO. OF MEMBERS = 1

CHORD: 3-4; 4-5; 5-6; 6-7; 7-8 TO BE 2X4 SS 19 SP

WEB: 1-2 TO BE 2X4 No.2ND 19 SP

BOTH END VERTICALS ARE NOT EXPOSED.

WEB 5-13; 7-11 BRACED at 1/2 POINTS AS SHOWN ABOVE

WEB 3-13; 8-10 BRACED at 1/3 POINTS AS SHOWN ABOVE

Note:Use 1x4 or 2x3 Cont.Bracing conn.w/2-8d nails,or T-brace of same size and grade as web conn. to narrow face w/10d nails 6 in. o.c

TRUSS HAS BEEN CHECKED FOR 145 M.P.H. WIND LOAD, WALL HEIGHT 16 FT, ENCLOSED BLDG. TYPE, ASSUMING 10 PSF TOTAL D.L.(7+3) (SBC-97).

DEFLECTION(IN.) L.L= 0.25,D.L=0.21,T.L=0.45

PLATES ARE MITEK M20-249,200 MANUFACTURED FROM ASTM A 446 GRD A GALVANIZED STEEL(EXCEPT AS SHOWN)

PLATE MUST BE INSTALLED ON EA. FACE OF JOINT,SYMMETRICALLY(EXCEPT AS SHOWN)DESIGN CONFORMS W/NDS DESIGN SPECS, SIBC,ANSI/TPI-95

THIS DESIGN IS FOR TRUSS FABRICATION ONLY.FOR PERMANENT AND TEMPORARY BRACING(WHICH IS ALWAYS REQD)CONSULT BLDG ARCHITECT OR ENGINEER.

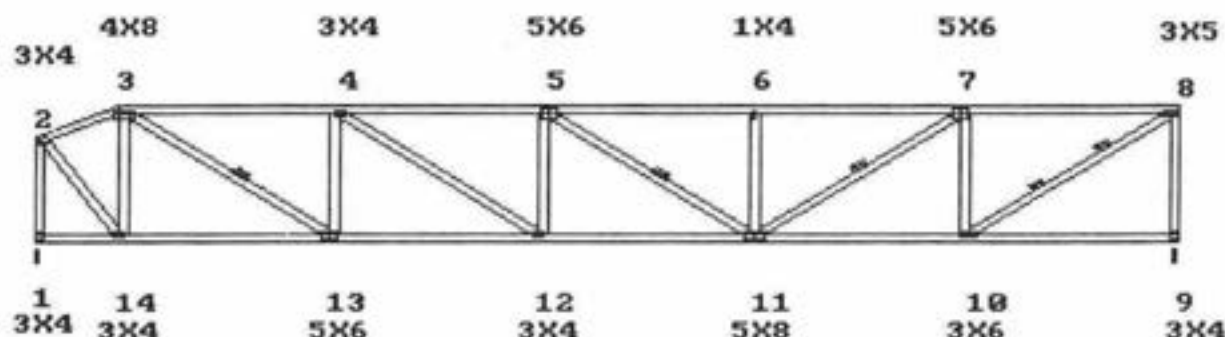
MAR 06 2001

====<<<<ACES-32 Ver. 1.1>>>>=====[001992]====<<<<FL.QUALITY-MMH>>>>=====
 Customer : FOGLIA Mon Mar 5 12:13:36 2001
 Project #: 00469 Truss ID : T3 Family #: 313
 Span : 35-1-8 B.Span : 35-1-8 Quant. : 1 Top Pitch : 6/12
 ==Bld(1/21/2000),v1.1=====

FORCES - GRAVITY LOADS				REACTIONS - SIZE	
1-2=1895	7-8=1853	9-10= 0	2-14= 1599	5-11=219	1=1920 2.68
2-3=899	8-9=1855	10-11= 1853	3-14=1291	6-11=581	9=1920 2.68
3-4=2354		11-12= 3024	3-13= 2014	7-11= 1316	
4-5=3024		12-13= 2354	4-13=1155	7-10=1434	
5-6=2856		13-14= 804	4-12= 878	8-10=2424	
6-7=2856		14-1= 0	5-12=439		

PROVIDE FOR 1683 LBS UPLIFT AT JOINT 1 (0.88)
 PROVIDE FOR 1723 LBS UPLIFT AT JOINT 9 (0.90)
 PROVIDE FOR 145 LBS HORIZ. REACTION AT JOINT 1
 PLATE OFFSETS (X=LEFT,Y=TOP): [j5=3,3], [j7=3,3], [j8=3,1.5], [j11=4,2], [j13=3,2],

2-6-15	9-2-2	15-7-8	22-0-15	28-6-5	35-1-8
2-6-15	6-7-3	6-5-7	6-5-7	6-5-7	6-7-3



2-6-15	9-2-2	15-7-8	22-0-15	28-6-5	35-1-8
2-6-15	6-7-3	6-5-7	6-5-7	6-5-7	6-7-3

L. HL TO PK:2-10-9
 LEFT HEIGHT:4-5-11 SPAN:35-1-8 RISE:5-9-3 RIGHT HEIGHT:5-9-3

LOADING (PSF)		MAX STRESSES		MINIMUM GRADE OF LUMBER	
TOP	30	D	TOP 4-5=0.838	TOP CHORD:2X4	No.2ND 19 SP
BOTT	0	10	BOTT 11-12=0.756	BOT CHORD:2X4	No.2ND 19 SP
			LL.DEFL.@12=0.19 < L/360	WEBS :2X4	No.3 19 SP

STR.INC.: LUMB = 1.33 PLATE = 1.33 SPACING : 24.0 in. o. c.
 REPETITIVE STRESSES USED NO. OF MEMBERS = 1

CHORD: 3-4; 4-5; 5-6; 6-7; 7-8 TO BE 2X4 SS 19 SP
 BOTH END VERTICALS ARE NOT EXPOSED.
 WEB 3-13; 5-11; 7-11 BRACED AT 1/2 POINTS AS SHOWN ABOVE
 WEB 8-10 BRACED AT 1/3 POINTS AS SHOWN ABOVE
 Note:Use 1x4 or 2x3 Cont.Bracing conn.w/2-8d nails,or T-brace of same size and grade as web conn. to narrow face w/10d nails 6 in. o.c
 TRUSS HAS BEEN CHECKED FOR 145 M.P.H. WIND LOAD, WALL HEIGHT 16 FT, ENCLOSED BLDG. TYPE, ASSUMING 10 PSF TOTAL D.L.(7+3) (SBC-97).
 DEFLECTION(IN.) L.L= 0.19,D.L=0.16,T.L=0.34
 PLATES ARE MITEK M20-249,200 MANUFACTURED FROM ASTM A 446 GRD A GALVANIZED STEEL(EXCEPT AS SHOWN)
 PLATE MUST BE INSTALLED ON EA. FACE OF JOINT,SYMMETRICALLY(EXCEPT AS SHOWN)DESIGN CONFORMS W/NDOS DESIGN SPECS, SSBC,ANSI/TP1-95
 THIS DESIGN IS FOR TRUSS FABRICATION ONLY.FOR PERMANENT AND TEMPORARY BRACING(WHICH IS ALWAYS REQD)CONSULT BLDG ARCHITECT OR ENGINEER.

MAR 06 2001

====<<<ACES-32 Ver. 1.1>>>===== [001840] =====<<<FL.QUALITY-MMH>>>=====

Customer : FOGLIA Truss ID : S27 Thu Mar 1 09:30:29 2001

Project #: 00469 Quantity : 1 Family # : special

Span : 35-1-8 Top Pitch : 6/12

====Bldg(1/21/2000),v1.1=====

FORCES - GRAVITY LOADS

1-2=-270	7-8=-1274	9-10= 0	3-1=-2154	6-12= 574
2-3= 0	8-9=-1875	10-11= 1140	3-14= 1045	6-11=-970
3-4=-1453		11-12= 2157	4-14=-1262	7-11= 1493
4-5=-2314		12-13= 2548	4-13= 374	7-10=-998
5-6=-2548		13-14= 2314	5-13=-344	8-10= 1587
6-7=-2157		14-1= 1453	5-12=-298	

REACTIONS - SIZE

1=-1920 2.68

9=-1920 2.68

35

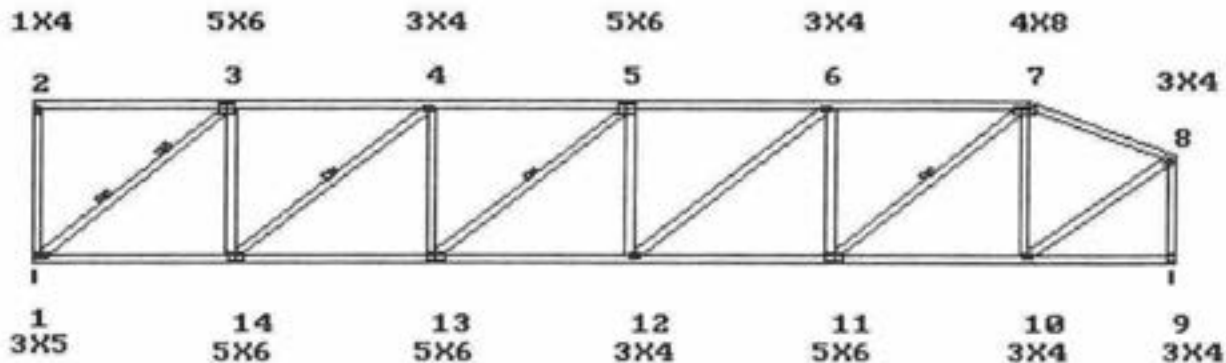
PROVIDE FOR 1744 LBS UPLIFT AT JOINT 1 (0.91)

PROVIDE FOR 1662 LBS UPLIFT AT JOINT 9 (0.87)

PROVIDE FOR 262 LBS HORIZ. REACTION AT JOINT 1

PLATE OFFSETS (X=LEFT,Y=TOP): [j3=3,3], [j5=3,3], [j11=3,2], [j13=3,2], [j14=3,2], (SYMMETRIC PLATING TURNED ON)

6-1-5	12-2-10	18-3-15	24-5-4	30-6-9	35-1-8
6-1-5	6-1-5	6-1-5	6-1-5	6-1-5	4-6-15



6-1-5	12-2-10	18-3-15	24-5-4	30-6-9	35-1-8
6-1-5	6-1-5	6-1-5	6-1-5	6-1-5	4-6-15

LEFT HEIGHT:6-10-3 SPAN:35-1-8 RISE:6-10-4 R. HL TO PK :5-1-7

RIGHT HEIGHT:4-6-12

LOADING (PSF)	MAX STRESSES	MINIMUM GRADE OF LUMBER
L D	TOP 5-6=0.994	TOP CHORD:2X4 No.2ND 19 SP
TOP 30 15	BOTT 12-13=0.648	BOT CHORD:2X4 No.2ND 19 SP
BOTT 0 10	LL.DEFL.@12=0.14 < L/360	WEBS :2X4 No.3 19 SP

STR.INC.: LUMB = 1.33 PLATE = 1.33 SPACING : 24.0 in. o. c.

REPETITIVE STRESSES USED NO. OF MEMBERS = 1

CHORD: 3-4; 4-5; 5-6; 6-7 TO BE 2X4 No.1D 19 SP

BOTH END VERTICALS ARE NOT EXPOSED.

WEB 4-14; 5-13; 7-11 BRACED at 1/2 POINTS AS SHOWN ABOVE

WEB 3-1 BRACED at 1/3 POINTS AS SHOWN ABOVE

Note:Use 1x4 or 2x3 Cont.Bracing conn.w/2-8d nails,or T-brace of same size and grade as web conn. to narrow face w/10d nails 6 in. o.c

TRUSS HAS BEEN CHECKED FOR 145 M.P.H. WIND LOAD, WALL HEIGHT 16 FT, ENCLOSED BLDG. TYPE, ASSUMING 10 PSF TOTAL D.L.(7+3) (SBC-97).

DEFLECTION(IN.) L.L= 0.14,D.L=0.11,T.L=0.25

PLATES ARE MITEK M20-249,200 MANUFACTURED FROM ASTM A 446 GRD A GALVANIZED STEEL(EXCEPT AS SHOWN)

PLATE MUST BE INSTALLED ON EA. FACE OF JOINT,SYMMETRICALLY(EXCEPT AS SHOWN)DESIGN CONFORMS W/NDS DESIGN SPECS, SBC,ANSI/TP1-95

THIS DESIGN IS FOR TRUSS FABRICATION ONLY.FOR PERMANENT AND TEMPORARY BRACING(WHICH IS ALWAYS REQD)CONSULT BLDG ARCHITECT OR ENGINEER.

[Signature]

MAR 01 2001

====<<<<ACES-32 Ver. 1.1>>>>===== [001841] =====<<<<FL.QUALITY-MMH>>>>=====

Customer : FOGLIA Truss ID : S28 Thu Mar 1 09:30:50 2001

Project #: 00469 Quantity : 1 Family #: special

Span : 35-2 Bot pitch : 6/12

Bot pitch : 90/12

==81d(1/21/2000),v1.1=====

FORCES - GRAVITY LOADS						REACTIONS - SIZE
1-2=-177	7-8=-1506	9-10= 0	15-1= 1370	3-1=-2187	5-11=-251	1=-1922 2.68
2-3= 0	8-9=-1857	10-11= 2052	3-15= 1252	6-11= 318		9=-1922 2.68
3-4=-1408		11-12= 2210	4-15=-535	6-10=-1119		
4-5=-2210		12-13= 1822	4-13=-691	7-10= 105		
5-6=-2052		13-14= 0	4-12= 616	8-10= 1615		
6-7=-1347		14-15= 60	5-12=-356	15-13= 1995		

(36)

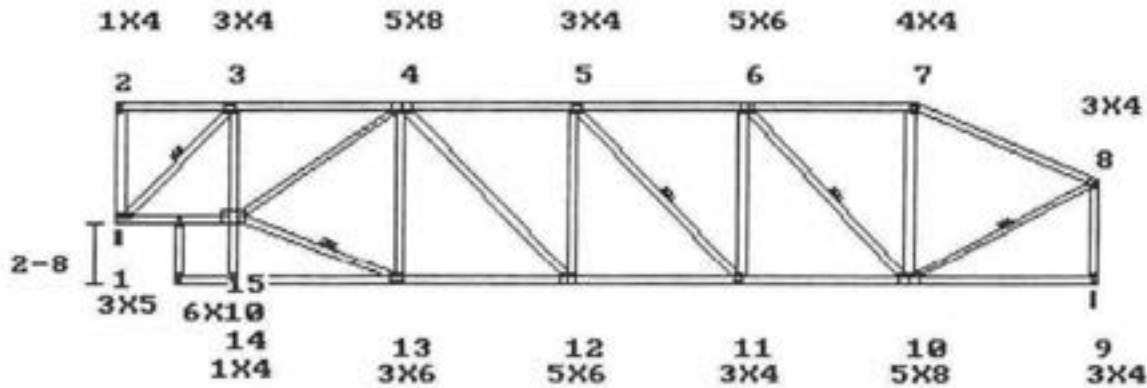
PROVIDE FOR 1741 LBS UPLIFT AT JOINT 1 (0.91)

PROVIDE FOR 1670 LBS UPLIFT AT JOINT 9 (0.87)

PROVIDE FOR 382 LBS HORIZ. REACTION AT JOINT 1

PLATE OFFSETS (X=LEFT,Y=TOP): [j4=4,3], [j6=3,3], [j10=4,2], [j12=3,2], (SYMMETRIC PLATING TURNED ON)

4-0-8	10-2	16-3-9	22-5-1	28-6-9	35-2
4-0-8	6-1-8	6-1-8	6-1-8	6-1-8	6-7-7



4-0-8	10-2	16-3-9	22-5-1	28-6-9	35-2
4-0-8	6-1-8	6-1-8	6-1-8	6-1-8	6-7-7

LEFT HEIGHT:5-2-3 SPAN:35-2 RISE:7-10-3 R. HL TO PK :7-4-13

RIGHT HEIGHT:4-6-7

LOADING (PSF)		MAX STRESSES		MINIMUM GRADE OF LUMBER	
TOP	L D	TOP	4-5=0.914	TOP CHORD:	2X4 No.1D 19 SP
BOTT	0 10	BOTT	11-12=0.587	BOT CHORD:	2X4 No.2ND 19 SP
		LL.DEFL.@12=0.11 < L/360	WEBS	:2X4	No.3 19 SP

STR.INC.: LUMB = 1.33 PLATE = 1.33 SPACING : 24.0 in. o. c.

REPETITIVE STRESSES USED NO. OF MEMBERS = 1

CHORD: 7-8 TO BE 2X4 SS 19 SP

CHORD: 14-15 TO BE 2X4 No.3 19 SP

BOTH END VERTICALS ARE NOT EXPOSED.

WEB 3-1; 5-11; 6-10; 8-10; 15-13 BRACED AT 1/2 POINTS AS SHOWN ABOVE

Note:Use 1x4 or 2x3 Cont.Bracing conn.w/2-8d nails,or T-brace of same size and grade as web conn. to narrow face w/10d nails 6 in. o.c

TRUSS HAS BEEN CHECKED FOR 145 M.P.H. WIND LOAD, WALL HEIGHT 16 FT, ENCLOSED BLDG. TYPE, ASSUMING 10 PSF TOTAL D.L.(7+3) (SBC-97).

DEFLECTION(IN.) L.L= 0.11,D.L=0.09,T.L=0.20

PLATES ARE MITEK M20-249,200 MANUFACTURED FROM ASTM A 446 GRD A GALVANIZED STEEL(EXCEPT AS SHOWN)

PLATE MUST BE INSTALLED ON EA. FACE OF JOINT,SYMMETRICALLY(EXCEPT AS SHOWN)DESIGN CONFORMS W/NDS DESIGN SPECS, S58C,ANSI/TPI-95

THIS DESIGN IS FOR TRUSS FABRICATION ONLY.FOR PERMANENT AND TEMPORARY BRACING(WHICH IS ALWAYS REQD)CONSULT BLDG ARCHITECT OR ENGINEER.

[Signature]

MAR 01 2001

====<<<ACES-32 Ver. 1.1>>>===== [001842] =====<<<FL.QUALITY-MMH>>>=====

Customer : FOGLIA Thu Mar 1 09:31:24 2001

Project #: 00469 Truss ID : T4 Family # : spec.

Span : 35-1-8 B.Span : 35-3-8 Quant. : 1 Top Pitch : 6/12

Bot pitch : 90/12

==8ld(1/21/2000),v1.1=====

FORCES - GRAVITY LOADS

1-2=-269	7-8=-1611	10-11= 1064	3-1=-2241	6-12=-1928
2-3= 0	8-9= 0	11-12= 1439	3-15= 1148	7-12= 845
3-4=-2142	9-10=-184	12-13= 0	4-15=-941	7-11=-419
4-5=-2885		13-14= 40	4-14= 417	8-11= 661
5-6=-2874		14-15= 2580	5-14=-460	8-10=-1964
6-7=-1806		15-1= 1609	6-14= 1887	14-12= 2195

REACTIONS - SIZE

1=-1920	3.06
10=-1920	2.89

(37)

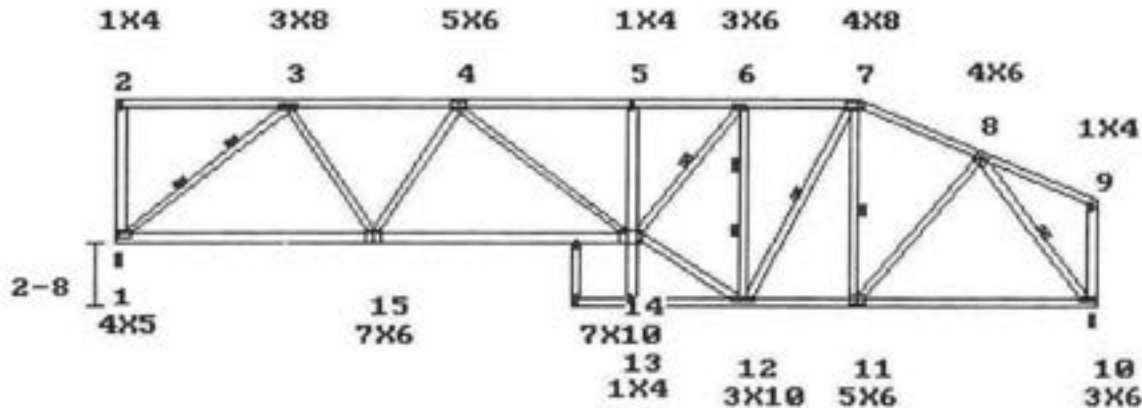
PROVIDE FOR 2206 LBS UPLIFT AT JOINT 1 (1.15)

PROVIDE FOR 2078 LBS UPLIFT AT JOINT 10 (1.08)

PROVIDE FOR 604 LBS HORIZ. REACTION AT JOINT 1

PLATE OFFSETS (X=LEFT,Y=TOP): [J4=3,3], [J11=3,2], [J14=5,2], [J15=3,2],

6-1-3	12-2-5	18-3-8	22-5	26-6-9	35-1-8
6-1-3	6-1-3	6-1-3	4-1-8	4-1-8	4-4-8 4-2-8



9-1-12	18-3-8	22-5-1	26-6-9	35-1-8
9-1-12	9-1-12	4-1-8	4-1-8	8-6-15

LEFT HEIGHT:6-2-3 SPAN:35-1-8 RISE:8-10-3 R. HL TO PK :9-7-1

RIGHT HEIGHT:4-6-11

LOADING (PSF)	MAX STRESSES	MINIMUM GRADE OF LUMBER
L D	TOP 4-5=0.844	TOP CHORD:2X4 SS 19 SP
TOP 30 15	BOTT 11-12=0.527	BOT CHORD:2X4 No.2ND 19 SP
BOTT 0 10	LL.DEFL.@14=0.17 < L/360	WEBS :2X4 No.3 19 SP

STR.INC.: LUMB = 1.33 PLATE = 1.33 SPACING : 24.0 in. o. c.

REPETITIVE STRESSES USED NO. OF MEMBERS = 1

CHORD: 7-8; 8-9 TO BE 2X4 No.10 19 SP

CHORD: 13-14 TO BE 2X4 No.3 19 SP

CHORD: 14-15; 15-1 TO BE 2X6 No.2D 19 SP

BOTH END VERTICALS ARE NOT EXPOSED.

WEB 6-14; 7-12; 7-11; 8-10 BRACED at 1/2 POINTS AS SHOWN ABOVE

WEB 3-1; 6-12 BRACED at 1/3 POINTS AS SHOWN ABOVE

Note:Use 1x4 or 2x3 Cont.Bracing conn.w/2-8d nails,or T-brace of same size and grade as web conn. to narrow face w/10d nails 6 in. o.c

TRUSS HAS BEEN CHECKED FOR 145 M.P.H. WIND LOAD, WALL HEIGHT 16 FT, ENCLOSED BLDG. TYPE, ASSUMING 10 PSF TOTAL D.L.(7*3) (SBC-97).

DEFLECTION(IN.) L.L= 0.17,D.L=0.14,T.L=0.30

PLATES ARE MITEK M20-249,200 MANUFACTURED FROM ASTM A 446 GRD A GALVANIZED STEEL(EXCEPT AS SHOWN)

PLATE MUST BE INSTALLED ON EA. FACE OF JOINT,SYMMETRICALLY(EXCEPT AS SHOWN)DESIGN CONFORMS W/MDS DESIGN SPECS, SSSC,ANSI/TPI-95

THIS DESIGN IS FOR TRUSS FABRICATION ONLY.FOR PERMANENT AND TEMPORARY BRACING(WHICH IS ALWAYS REQD)CONSULT BLDG ARCHITECT OR ENGINEER.

[Signature]
MAR 01 2001

====<<<<ACES-32 Ver. 1.1>>>>===== [001843] =====<<<<FL.QUALITY-MMH>>>>=====

Customer : FOGLIA Thu Mar 1 09:31:53 2001

Project #: 00469 Truss ID : H5 Family # : spec.

Span : 35-1-8 B.Span : 35-3-8 Quant. : 1 Top Pitch : 6/12

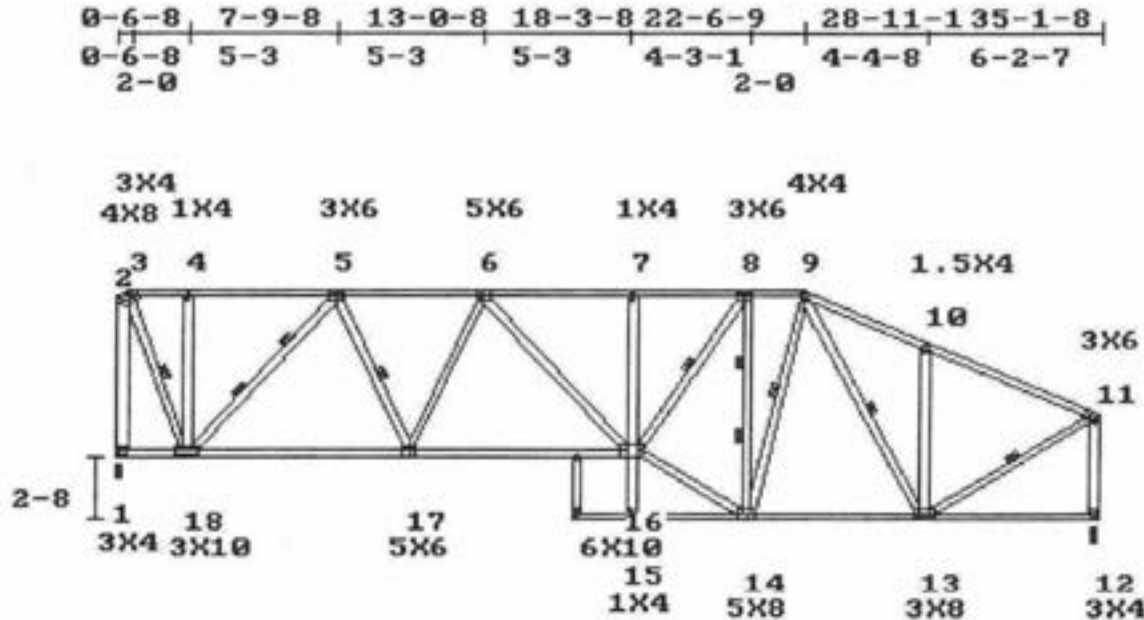
Bot pitch : 90/12

==Bld(1/21/2000),v1.1=====

REACTIONS - SIZE
1=-1920 3.07
12=-1920 2.85

(38)

PROVIDE FOR 2212 LBS UPLIFT AT JOINT 1 (1.15)
PROVIDE FOR 2055 LBS UPLIFT AT JOINT 12 (1.07)
PROVIDE FOR 725 LBS HORIZ. REACTION AT JOINT 1
PLATE OFFSETS (X=LEFT,Y=TOP): [j6=3,3], [j14=4,2], [j17=3,2],



2-6-8 10-5 18-3-8 22-6-9 28-11-1 35-1-8
2-6-8 7-10-8 7-10-8 4-3-1 6-4-8 6-2-7

L. HL TO PK:0-7-4 LEFT HEIGHT:6-10-14 SPAN:35-1-8 RISE:9-10-3 R. HL TO PK:11-9-15
RIGHT HEIGHT:4-6-11

LOADING (PSF) MAX STRESSES MINIMUM GRADE OF LUMBER
L D TOP 5-6=0.916 TOP CHORD:2X4 No.2ND 19 SP
TOP 30 15 BOTT 16-17=0.701 BOT CHORD:2X4 No.2ND 19 SP
BOTT 0 10 LL.DEFL.@16=0.22 < L/360 WEBS :2X4 No.3 19 SP

STR.INC.: LUMB = 1.33 PLATE = 1.33 SPACING : 24.0 in. o. c.
REPETITIVE STRESSES USED NO. OF MEMBERS = 1

CHORD: 3-4; 4-5; 5-6; 6-7; 7-8; 8-9 TO BE 2X4 No.1D 19 SP

CHORD: 9-10; 10-11 TO BE 2X4 SS 19 SP

CHORD: 15-16 TO BE 2X4 No.3 19 SP

WEB: 1-2 TO BE 2X6 No.2D 19 SP

BOTH END VERTICALS ARE NOT EXPOSED.

WEB 2-18; 5-17; 8-16; 9-14; 9-13; 11-13 BRACED at 1/2 POINTS AS SHOWN ABOVE

WEB 5-18; 8-14 BRACED at 1/3 POINTS AS SHOWN ABOVE

Note:Use 1x4 or 2x3 Cont.Bracing conn.w/2-8d nails,or T-brace of same size and grade as web conn. to narrow face w/10d nails 6 in. o.c
TRUSS HAS BEEN CHECKED FOR 145 M.P.H. WIND LOAD, WALL HEIGHT 16 FT, ENCLOSED BLDG. TYPE, ASSUMING 10 PSF TOTAL D.L.(7+3) (SBC-97).

DEFLECTION(IN.) L.L= 0.22,D.L=0.18,T.L=0.40

PLATES ARE MITEK M20-249,200 MANUFACTURED FROM ASTM A 446 GRD A GALVANIZED STEEL(EXCEPT AS SHOWN)

PLATE MUST BE INSTALLED ON EA. FACE OF JOINT,SYMMETRICALLY(EXCEPT AS SHOWN)DESIGN CONFORMS W/NDS DESIGN SPECS, SSBC,NDS,ANSI/TPI-95

THIS DESIGN IS FOR TRUSS FABRICATION ONLY.FOR PERMANENT AND TEMPORARY BRACING(WHICH IS ALWAYS REQD)CONSULT BLDG ARCHITECT OR ENGINEER.

MAR 01 2001

====<<<<ACES-32 Ver. 1.1>>>>=====[001844]=====<<<<FL.QUALITY-MMH>>>>=====

Customer : FOGLIA
Project #: 00469
Span : 35-1-8

Truss ID : H6
Quantity : 1

Thu Mar 1 09:32:28 2001
Family # : special
Top Pitch : 6/12
Bot pitch : 90/12

==Bld(1/21/2000),v1.1=====

FORCES - GRAVITY LOADS

FORCES - GRAVITY LOADS			REACTIONS - SIZE					
1-2=-1895	7-8=-1631	10-11= 0	16-1= 0	2-16= 1608	6-14=-428	14-12= 1737	1=-1920	2.98
2-3=-615	8-9=-1467	11-12= 1313	3-16=-70	7-14= 1422	7-12=-1007		10=-1920	2.84
3-4=-550	9-10=-1859	12-13= 0	4-16=-1606	8-12= 220	8-11=-807			
4-5=-1728		13-14= 41	4-15= 911	5-15=-744	5-14= 283	9-11= 1610		
5-6=-2123		14-15= 1963						
6-7=-2115		15-16= 1440						

(34)

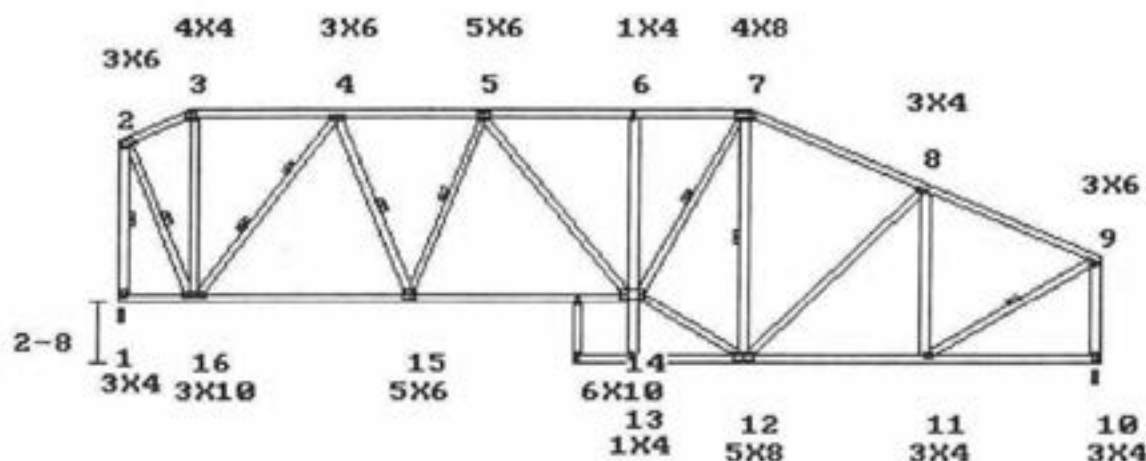
PROVIDE FOR 2143 LBS UPLIFT AT JOINT 1 (1.12)

PROVIDE FOR 2043 LBS UPLIFT AT JOINT 10 (1.06)

PROVIDE FOR 766 LBS HORIZ. REACTION AT JOINT 1

PLATE OFFSETS (X=LEFT,Y=TOP): [j5=3,3], [j12=4,2], [j15=3,2], (SYMMETRIC PLATING TURNED ON)

2-6-8 7-9-8 13-0-8 18-3-8 22-6-9 28-11-1 35-1-8
2-6-8 5-3 5-3 4-3-1 6-4-8 6-2-7



2-6-8 10-5 18-3-8 22-6-9 28-11-1 35-1-8
2-6-8 7-10-8 7-10-8 4-3-1 6-4-8 6-2-7

L. HL TO PK:2-10-2

LEFT HEIGHT:6-10-14

SPAN:35-1-8

RISE:10-10-3

R. HL TO PK :14-0-12

RIGHT HEIGHT:4-6-11

LOADING (PSF)

L D

TOP 30 15

BOTT 0 10

MAX STRESSES

TOP 7-8=0.961

BOTT 14-15=0.657

LL.DEFL.@14=0.12 < L/360

MINIMUM GRADE OF LUMBER

TOP CHORD:2X4 No.2ND 19 SP

BOT CHORD:2X4 No.2ND 19 SP

WEBS :2X4 No.3 19 SP

STR.INC.: LUMB = 1.33 PLATE = 1.33

REPETITIVE STRESSES USED

SPACING : 24.0 in. o. c.

NO. OF MEMBERS = 1

CHORD: 3-4; 4-5; 5-6; 6-7 TO BE 2X4 No.10 19 SP

CHORD: 7-8; 8-9 TO BE 2X4 SS 19 SP

CHORD: 13-14 TO BE 2X4 No.3 19 SP

BOTH END VERTICALS ARE NOT EXPOSED.

WEB 1-2; 2-16; 4-15; 5-15; 7-14; 7-12; 9-11 BRACED at 1/2 POINTS AS SHOWN ABOVE

WEB 4-16 BRACED at 1/3 POINTS AS SHOWN ABOVE

Note:Use 1x4 or 2x3 Cont.Bracing conn.w/2-8d nails,or T-brace of same size and grade as web conn. to narrow face w/10d nails 6 in. o.c

TRUSS HAS BEEN CHECKED FOR 145 M.P.H. WIND LOAD, WALL HEIGHT 16 FT, ENCLOSED BLDG. TYPE, ASSUMING 10 PSF TOTAL D.L.(7+3) (SBC-97).

DEFLECTION(IN.) L.L= 0.12,D.L=0.10,T.L=0.22

PLATES ARE MITEK M20-249,200 MANUFACTURED FROM ASTM A 446 GRD A GALVANIZED STEEL(EXCEPT AS SHOWN)

PLATE MUST BE INSTALLED ON EA. FACE OF JOINT,SYMMETRICALLY(EXCEPT AS SHOWN)DESIGN CONFORMS W/ND S DESIGN SPECS, SSBC,ANSI/TPI-95

THIS DESIGN IS FOR TRUSS FABRICATION ONLY.FOR PERMANENT AND TEMPORARY BRACING(WHICH IS ALWAYS REQD)CONSULT BLDG ARCHITECT OR ENGINEER.

====<<<ACES-32 Ver. 1.1>>>===== [001845] =====<<<FL.QUALITY-MMH>>>=====

Customer : FOGLIA Thu Mar 1 09:36:58 2001

Project #: 00469 Truss ID : T5 Family #: spec.

Span : 35-1-8 B.Span : 35-3-8 Quant. : 1 Top Pitch : 6/12

==Bld(1/21/2000),v1.1=====

FORCES - GRAVITY LOADS

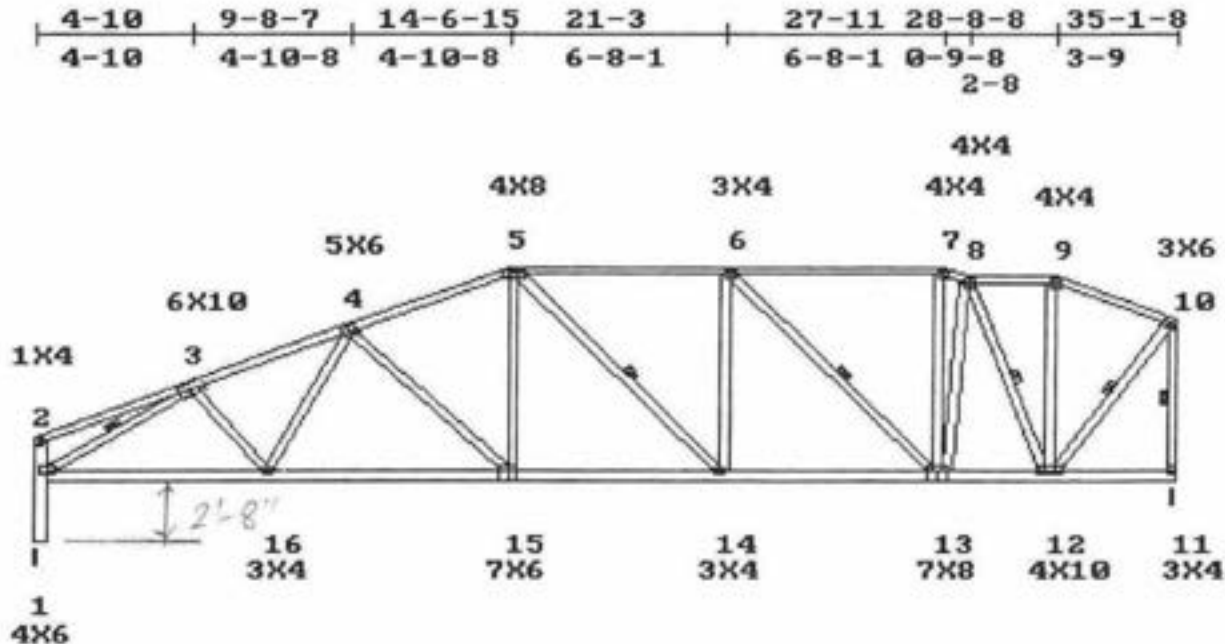
FORCES - GRAVITY LOADS			REACTIONS - SIZE	
1-2=-214	-7-8=-1385	11-12= 0	3-1=-2574	6-14= 172
2-3= 0	8-9=-750	12-13= 1183	3-16= 190	6-13=-969
3-4=-2330	9-10=-837	13-14= 1823	4-16=-19	7-13= 283
4-5=-2074	10-11=-1886	14-15= 1852	4-15=-392	8-13= 593
5-6=-1823	15-16= 2091	5-15= 452	8-12=-1432	9-12= 87
6-7=-1238	16-1= 1986	5-14=-48		

PROVIDE FOR 2029 LBS UPLIFT AT JOINT 1 (1.06)

PROVIDE FOR 2082 LBS UPLIFT AT JOINT 11 (1.08)

PROVIDE FOR 801 LBS HORIZ. REACTION AT JOINT 1

PLATE OFFSETS (X=LEFT,Y=TOP):[j4=3,3],[j13=4,2],[j15=3,2],



7-2-8		14-6-15		21-3		27-11		35-1-8	
7-2-8	7-4-7	6-8-1	6-8	3-5-8	3-9				

L. HL TO PK:16-3-9 LEFT HEIGHT:1-10-11 SPAN:35-1-8 RISE:9-2-3 R. HL TO PK:4-2-5 RIGHT HEIGHT:6-10-15

LOADING (PSF)		MAX STRESSES		MINIMUM GRADE OF LUMBER	
TOP	30	TOP	3-4=0.983	TOP CHORD:2X4	No.1D 19 SP
BOTT	0	BOTT	15-16=0.337	BOT CHORD:2X6	No.2D 19 SP
		LL.DEFL.@15=0.10	< L/360	WEBS	:2X4 No.3 19 SP

STR.INC.: LUMB = 1.33 PLATE = 1.33 SPACING : 24.0 in. o. c.

REPETITIVE STRESSES USED NO. OF MEMBERS = 1

CHORD: 5-6; 6-7 TO BE 2X4 SS 19 SP

CHORD: 7-8; 8-9; 9-10 TO BE 2X4 No.2ND 19 SP

WEB: 1-2 TO BE 2X6 No.2D 19 SP

BOTH END VERTICALS ARE NOT EXPOSED.

WEB 10-11; 3-1; 5-14; 6-13; 8-12; 10-12 BRACED at 1/2 POINTS AS SHOWN ABOVE

Note:Use 1x4 or 2x3 Cont.Bracing conn.w/2-8d nails,or T-brace of same size and grade as web conn. to narrow face w/10d nails 6 in. o.c

TRUSS HAS BEEN CHECKED FOR 145 M.P.H. WIND LOAD, WALL HEIGHT 16 FT, ENCLOSED BLDG. TYPE, ASSUMING 10 PSF TOTAL D.L.(7+3) (SBC-97).

DEFLECTION(IN.) L.L= 0.10,D.L=0.09,T.L=0.19

PLATES ARE MITEK M20-249,200 MANUFACTURED FROM ASTM A 446 GRD A GALVANIZED STEEL(EXCEPT AS SHOWN)

PLATE MUST BE INSTALLED ON EA. FACE OF JOINT,SYMMETRICALLY(EXCEPT AS SHOWN)DESIGN CONFORMS W/NDS DESIGN SPECS, SSBC,ANSI/TP1-95

THIS DESIGN IS FOR TRUSS FABRICATION ONLY.FOR PERMANENT AND TEMPORARY BRACING(WHICH IS ALWAYS REQD)CONSULT BLDG ARCHITECT OR ENGINEER.

====<<<<ACES-32 Ver. 1.1>>>>===== [001846] =====<<<<FL.QUALITY-MMH>>>>=====

Customer : FOGLIA Thu Mar 1 09:46:18 2001

Project #: 00469 Truss ID : T6 Family # : spec.

Span : 35-1-8 B.Span : 35-3-8 Quant. : 1 Top Pitch : 6/12

==81d(1/21/2000),v1.1=====

FORCES - GRAVITY LOADS

1-2=-207	7-8=-1470	11-12= 404	3-1=-2514	6-13=-673
2-3= 0	8-9=-981	12-13= 981	3-15= 183	7-13= 232
3-4=-2275	9-10= 0	13-14= 1605	4-15= 26	8-13= 616
4-5=-1896	10-11=-75	14-15= 2026	4-14=-511	8-12=-1510
5-6=-1694		15-1= 1916	5-14= 377	9-12= 1702
6-7=-1315			6-14= 207	9-11=-1841

REACTIONS - SIZE

1=-1915	8.00
11=-1915	2.05

(4)

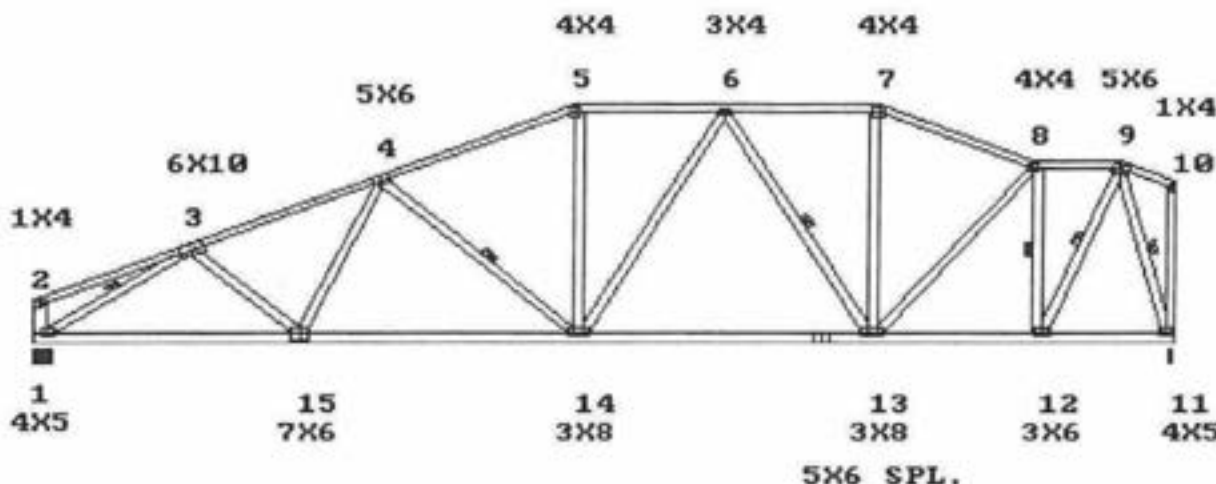
PROVIDE FOR 1983 LBS UPLIFT AT JOINT 1 (1.04)

PROVIDE FOR 2028 LBS UPLIFT AT JOINT 11 (1.06)

PROVIDE FOR 841 LBS HORIZ. REACTION AT JOINT 1

PLATE OFFSETS (X=LEFT,Y=TOP): [j4=3,3], [j9=3,2], [j15=3,2],

4-10	10-8-7	16-6-15	21-3	25-11	30-8-8	35-1-8
4-10	5-10-8	5-10-8	4-8-1	4-8-1	4-9-8	2-8 1-9



8-2-8	16-6-15	25-11	30-8-8	35-1-8
8-2-8	8-4-7	9-4-1	4-9-8	4-5

L. HL TO PK:18-6-7 R. HL TO PK:1-11-8

LEFT HEIGHT:1-10-11 SPAN:35-1-8 RISE:10-2-2 RIGHT HEIGHT:6-10-14

LOADING (PSF)	MAX STRESSES	MINIMUM GRADE OF LUMBER
TOP 30 15	TOP 7-8=0.954	TOP CHORD:2X4 SS 19 SP
BOTT 0 10	BOTT 14-15=0.379	BOT CHORD:2X6 No.2D 19 SP
	LL.DEFL.@14=0.10 < L/360	WEBS :2X4 No.3 19 SP

STR.INC.: LUMB = 1.33 PLATE = 1.33

REPETITIVE STRESSES USED

SPACING : 24.0 in. o. c.

NO. OF MEMBERS = 1

CHORD: 5-6; 6-7; 7-8 TO BE 2X4 No.1D 19 SP

CHORD: 8-9; 9-10 TO BE 2X4 No.2ND 19 SP

WEB: 1-2 TO BE 2X6 No.2D 19 SP

BOTH END VERTICALS ARE NOT EXPOSED.

WEB 3-1; 4-14; 6-13; 8-12; 9-12; 9-11 BRACED at 1/2 POINTS AS SHOWN ABOVE

Note:Use 1x4 or 2x3 Cont.Bracing conn.w/2-8d nails,or T-brace of same size and grade as web conn. to narrow face w/10d nails 6 in. o.c

TRUSS HAS BEEN CHECKED FOR 145 M.P.H. WIND LOAD, WALL HEIGHT 16 FT, ENCLOSED BLDG. TYPE, ASSUMING 10 PSF TOTAL D.L.(7+3) (SBC-97).

DEFLECTION(IN.) L.L= 0.10,D.L=0.08,T.L=0.18

PLATES ARE MITEK M20-249,200 MANUFACTURED FROM ASTM A 446 GRD A GALVANIZED STEEL(EXCEPT AS SHOWN)

PLATE MUST BE INSTALLED ON EA. FACE OF JOINT,SYMMETRICALLY(EXCEPT AS SHOWN)DESIGN CONFORMS W/NDS DESIGN SPECS, SSBC,ANSI/TP1-95

THIS DESIGN IS FOR TRUSS FABRICATION ONLY.FOR PERMANENT AND TEMPORARY BRACING(WHICH IS ALWAYS REQD)CONSULT BLDG ARCHITECT OR ENGINEER.

MAR 01 2001

[Signature]

====<<<<ACES-32 Ver. 1.1>>>>=====[001997]=====<<<<FL.QUALITY-MMH>>>>=====

Customer : FOGLIA Mon Mar 5 12:23:09 2001

Project #: 00469 Truss ID : S30 Family # : spec.

Span : 43-6-8 B.Span : 43-6-8 Quant. : 1 Top Pitch : 6/12

Bot pitch : 90/12

==Bld(1/21/2000),v1.1=====

FORCES - GRAVITY LOADS								REACTIONS - SIZE	
1-2=98	7-8=1565	11-12=0	17-18=474	2-20=-4	5-16=-142	8-13=-224	18=-2529	8.00	
2-3=838	8-9=1523	12-13=1126	18-19=29	2-18=-840	5-15=-274	9-13=407	11=-1933	2.00	
3-4=-2136	9-10=-1256	13-14=1360	19-20=0	3-18=-2456	6-15=297	9-12=-962	1=-428	4.00	
4-5=-2454	10-11=-1878	14-15=1992	20-1=88	3-17=2516	6-14=-735	10-12=1555			
5-6=-2227		15-16=2195		4-17=-724	7-14=293	18-20=120			
6-7=-1752		16-17=1911		4-16=370	8-14=464				

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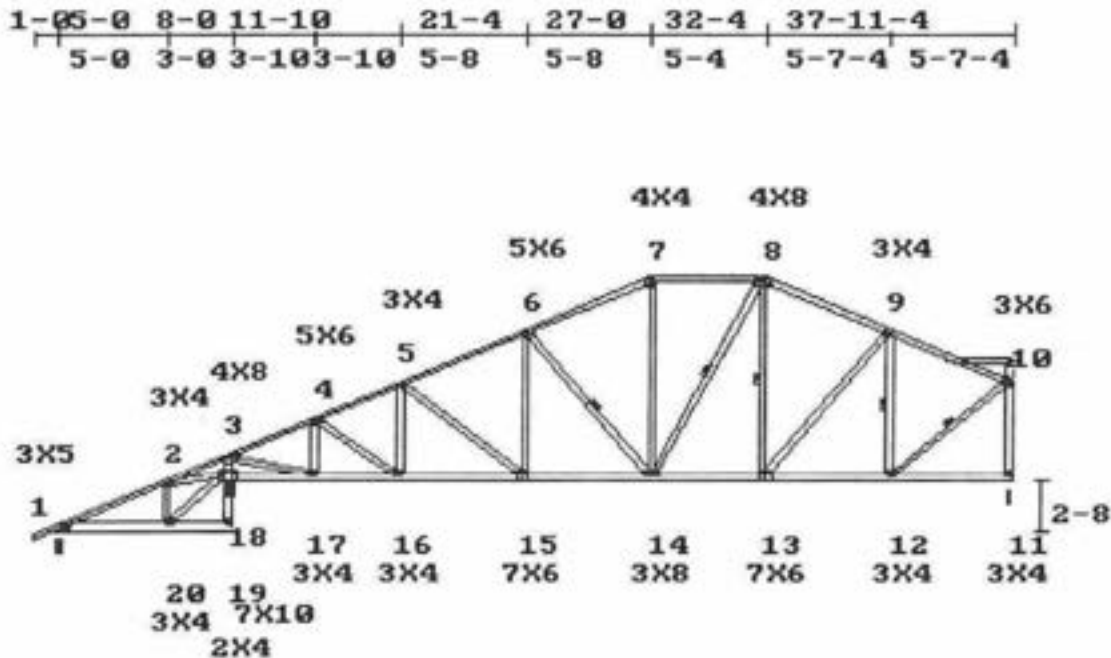
PROVIDE FOR 2630 LBS UPLIFT AT JOINT 18 (1.04)

PROVIDE FOR 1939 LBS UPLIFT AT JOINT 11 (1.00)

PROVIDE FOR 533 LBS UPLIFT AT JOINT 1 (1.24)

PROVIDE FOR 1358 LBS HORIZ. REACTION AT JOINT 18

PLATE OFFSETS (X=LEFT,Y=TOP): [j4=3,3], [j6=3,3], [j13=3,2], [j15=3,2], [j18=5,2],



15-8-1								43-6-8	
5-0	8-0	11-10	21-4	27-0	32-4	37-11-4			
5-0	3-0	3-10	3-10-1	5-8	5-4	5-7-4	5-7-4		

L. HL TO PK:30-2-4 INTERNAL RISE:2-8 R. HL TO PK:12-6-6

LEFT HEIGHT:0-4-3 SPAN:43-6-8 RISE:13-10-3 RIGHT HEIGHT:5-6-15

LOADING (PSF)		MAX STRESSES		MINIMUM GRADE OF LUMBER	
TOP	L D	TOP	5-6=0.876	TOP CHORD:2X4	SS 19 SP
BOTT	0 10	BOTT	15-16=0.315	BOT CHORD:2X6	No.2D 19 SP
		LL.DEFL.@15=0.11 < L/360	WEBS	:2X4	No.3 19 SP

STR.INC.: LUMB = 1.33 PLATE = 1.33 SPACING : 24.0 in. o. c.

REPETITIVE STRESSES USED NO. OF MEMBERS = 1

CHORD: 7-8 TO BE 2X4 No.1D 19 SP

CHORD: 18-19 TO BE 2X4 No.3 19 SP

RIGHT END VERTICAL IS NOT EXPOSED.

WEB 6-14; 8-14; 8-13; 9-12; 10-12 BRACED at 1/2 POINTS AS SHOWN ABOVE

Note:Use 1x4 or 2x3 Cont.Bracing conn.w/2-8d nails,or T-brace of same size and grade as web conn. to narrow face w/10d nails 6 in. o.c

TRUSS HAS BEEN CHECKED FOR 145 M.P.H. WIND LOAD, WALL HEIGHT 16 FT, ENCLOSED BLDG. TYPE, ASSUMING 10 PSF TOTAL D.L.(7*3) (SBC-97).

DEFLECTION(IN.) L.L= 0.11,D.L=0.09,T.L=0.20

PLATES ARE MITEK M20-249,200 MANUFACTURED FROM ASTM A 446 GRD A GALVANIZED STEEL(EXCEPT AS SHOWN)

PLATE MUST BE INSTALLED ON EA. FACE OF JOINT,SYMMETRICALLY(EXCEPT AS SHOWN)DESIGN CONFORMS W/NDOS DESIGN SPECS, SSBC,ANSI/TPI-95

THIS DESIGN IS FOR TRUSS FABRICATION ONLY.FOR PERMANENT AND TEMPORARY BRACING(WHICH IS ALWAYS REGO)CONSULT BLDG ARCHITECT OR ENGINEER.

MAR 06 2001

====<<<ACES-32 Ver. 1.1>>>===== [001998] =====<<<FL.QUALITY-MMH>>>=====

Customer : FOGLIA Mon Mar 5 12:24:51 2001

Project #: 00469 Truss ID : S31 Family # : spec.

Span : 43-6-8 B.Span : 43-6-8 Quant. : 1 Top Pitch : 6/12

Bot pitch : 90/12

==Bld(1/21/2000),v1.1=====

FORCES - GRAVITY LOADS				REACTIONS - SIZE			
1-2=98	7-8=1565	11-12=0	17-18=474	2-20=4	5-16=142	8-13=224	18=2529 8.00
2-3=838	8-9=1523	12-13=1126	18-19=29	2-18=840	5-15=274	9-13=407	11=1933 2.00
3-4=2136	9-10=1256	13-14=1360	19-20=0	3-18=2456	6-15=297	9-12=962	1=428 4.00
4-5=2454	10-11=1878	14-15=1992	20-1=88	3-17=2516	6-14=735	10-12=1555	
5-6=2227		15-16=2195		4-17=724	7-14=293	18-20=120	
6-7=1752		16-17=1911		4-16=370	8-14=464		

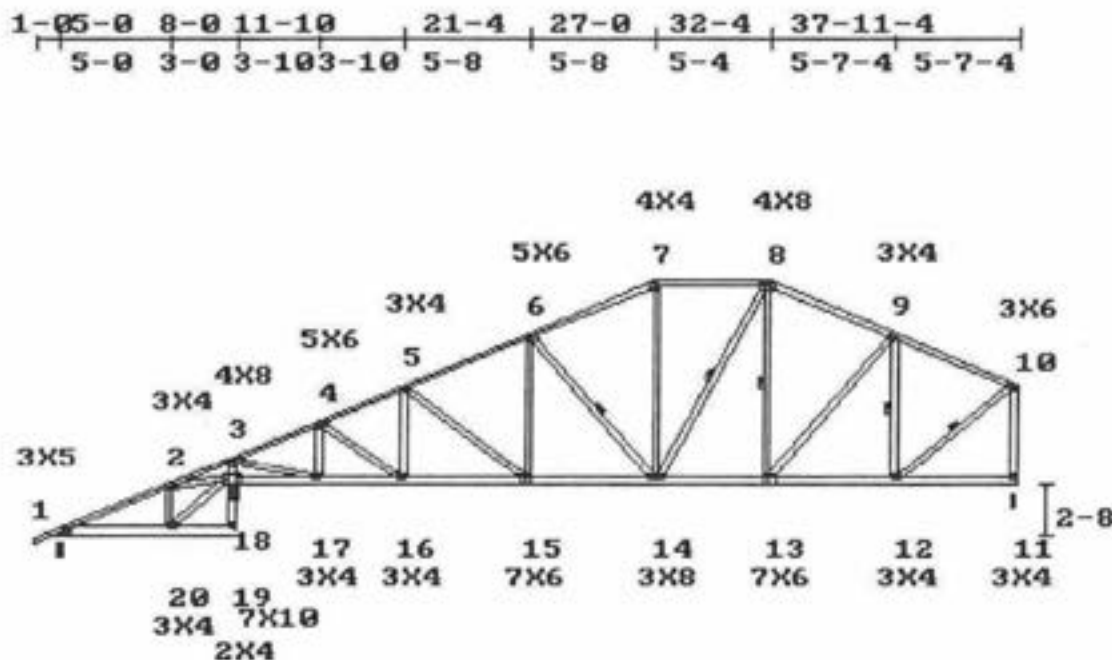
PROVIDE FOR 2630 LBS UPLIFT AT JOINT 18 (1.04)

PROVIDE FOR 1939 LBS UPLIFT AT JOINT 11 (1.00)

PROVIDE FOR 533 LBS UPLIFT AT JOINT 1 (1.24)

PROVIDE FOR 1358 LBS HORIZ. REACTION AT JOINT 18

PLATE OFFSETS (X=LEFT,Y=TOP): [J4=3,3], [J6=3,3], [J13=3,2], [J15=3,2], [J18=5,2],



15-8-1 43-6-8

5-0 8-0 11-10 21-4 27-0 32-4 37-11-4

5-0 3-0 3-10 3-10-1 5-8 5-4 5-7-4 5-7-4

L. HL TO PK:30-2-4 INTERNAL RISE:2-8 R. HL TO PK:12-6-6

LEFT HEIGHT:0-4-3 SPAN:43-6-8 RISE:13-10-3 RIGHT HEIGHT:5-6-15

LOADING (PSF)		MAX STRESSES		MINIMUM GRADE OF LUMBER	
TOP	30	TOP	3-4=0.970	TOP CHORD:2X4	No.2ND 19 SP
BOTT	0	BOTT	15-16=0.315	BOT CHORD:2X6	No.2D 19 SP
		LL.DEFL.@15=0.11	< L/360	WEBS	:2X4 No.3 19 SP

STR.INC.: LUMB = 1.33 PLATE = 1.33

REPETITIVE STRESSES USED

SPACING : 24.0 in. o. c.

NO. OF MEMBERS = 1

CHORD: 4-5; 5-6; 6-7; 8-9; 9-10 TO BE 2X4 SS 19 SP

CHORD: 7-8 TO BE 2X4 No.10 19 SP

CHORD: 18-19 TO BE 2X4 No.3 19 SP

RIGHT END VERTICAL IS NOT EXPOSED.

WEB 6-14; 8-14; 8-13; 9-12; 10-12 BRACED AT 1/2 POINTS AS SHOWN ABOVE

Note:Use 1x4 or 2x3 Cont.Bracing conn.w/2-8d nails,or T-brace of same size and grade as web conn. to narrow face w/10d nails 6 in. o.c

TRUSS HAS BEEN CHECKED FOR 145 M.P.H. WIND LOAD, WALL HEIGHT 16 FT, ENCLOSED BLDG. TYPE, ASSUMING 10 PSF TOTAL D.L.(7+3) (SBC-97).

DEFLECTION(IN.) L.L= 0.11,D.L=0.09,T.L=0.20

PLATES ARE MITEK M20-249,200 MANUFACTURED FROM ASTM A 446 GRD A GALVANIZED STEEL(EXCEPT AS SHOWN)

PLATE MUST BE INSTALLED ON EA. FACE OF JOINT,SYMMETRICALLY(EXCEPT AS SHOWN)DESIGN CONFORMS W/NDI DESIGN SPECS, SIBC,ANSI/TPI-95

THIS DESIGN IS FOR TRUSS FABRICATION ONLY.FOR PERMANENT AND TEMPORARY BRACING(WHICH IS ALWAYS REQD)CONSULT BLDG ARCHITECT OR ENGINEER.

MAR 06 2001

====<<<<ACES-32 Ver. 1.1>>>>=====[001849]=====<<<<FL.QUALITY-MMH>>>>=====

Customer : FOGLIA Thu Mar 1 09:48:14 2001

Project #: 00469 Truss ID : S32 Family #: spec.

Span : 43-6-8 B.Span : 43-6-8 Quant. : 1 Top Pitch : 6/12

Bot pitch : 90/12

==81d(1/21/2000),v1.1=====

FORCES - GRAVITY LOADS				REACTIONS - SIZE			
1-2=-100	7-8=-1502	10-11= 0	16-17= 29	2-18=-6	5-14=-66	8-11=-807	16=-2528 8.00
2-3= 830	8-9=-1386	11-12= 1242	17-18= 0	2-16=-835	5-13=-459	9-11= 1560	10=-1933 2.00
3-4=-2139	9-10=-1865	12-13= 1833	18-1= 90	3-16=-2452	6-13= 418	16-18= 123	1=-429 4.00
4-5=-2449		13-14= 2191		3-15= 2511	6-12=-903		
5-6=-2049		14-15= 1914		4-15=-713	7-12= 769		
6-7=-1494		15-16=-467		4-14= 333	8-12= 149		

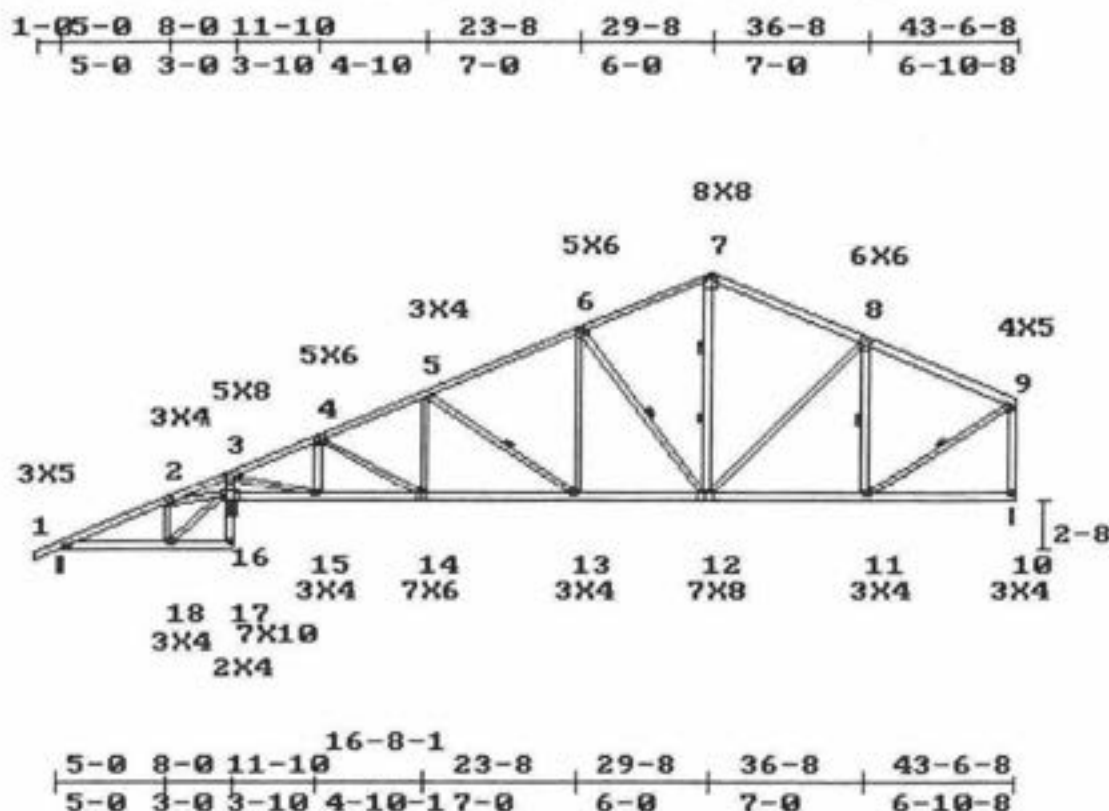
PROVIDE FOR 2575 LBS UPLIFT AT JOINT 16 (1.02)

PROVIDE FOR 1859 LBS UPLIFT AT JOINT 10 (0.96)

PROVIDE FOR 534 LBS UPLIFT AT JOINT 1 (1.24)

PROVIDE FOR 1402 LBS HORIZ. REACTION AT JOINT 16

PLATE OFFSETS (X=LEFT,Y=TOP): [j4=3,3], [j6=3,3], [j7=4,1.5], [j8=3,4], [j12=4,2], [j14=3,2], [j16=5,2],



L. HL TO PK:33-2 INTERNAL RISE:2-8 R. HL TO PK:15-6-2

LEFT HEIGHT:0-4-3 SPAN:43-6-8 RISE:15-2-3 RIGHT HEIGHT:5-6-15

LOADING (PSF) MAX STRESSES MINIMUM GRADE OF LUMBER

TOP 30 15 TOP 5-6=0.983 TOP CHORD:2X4 SS 19 SP

BOTT 0 10 BOTT 13-14=0.333 BOT CHORD:2X6 No.2D 19 SP

LL.DEFL.@13=0.11 < L/360 WEBS :2X4 No.3 19 SP

STR.INC.: LUMB = 1.33 PLATE = 1.33 SPACING : 24.0 in. o. c.

REPETITIVE STRESSES USED NO. OF MEMBERS = 1

CHORD: 7-8; 8-9 TO BE 2X6 No.2D 19 SP

CHORD: 16-17 TO BE 2X4 No.3 19 SP

RIGHT END VERTICAL IS NOT EXPOSED.

WEB 5-13; 6-12; 8-11; 9-11 BRACED AT 1/2 POINTS AS SHOWN ABOVE

WEB 7-12 BRACED AT 1/3 POINTS AS SHOWN ABOVE

Note:Use 1x4 or 2x3 Cont.Bracing conn.w/2-8d nails,or T-brace of same size and grade as web conn. to narrow face w/10d nails 6 in. o.c

TRUSS HAS BEEN CHECKED FOR 145 M.P.H. WIND LOAD, WALL HEIGHT 16 FT, ENCLOSED BLDG. TYPE, ASSUMING 10 PSF TOTAL D.L.(7*3) (SBC-97).

DEFLECTION(IN.) L.L= 0.11,D.L=0.09,T.L=0.19

PLATES ARE MITEK M20-249,200 MANUFACTURED FROM ASTM A 446 GRD A GALVANIZED STEEL(EXCEPT AS SHOWN)

PLATE MUST BE INSTALLED ON EA. FACE OF JOINT,SYMMETRICALLY(EXCEPT AS SHOWN)DESIGN CONFORMS W/ND5 DESIGN SPECS, SSBC,ANSI/TPI-95

THIS DESIGN IS FOR TRUSS FABRICATION ONLY.FOR PERMANENT AND TEMPORARY BRACING(WHICH IS ALWAYS REGO)CONSULT BLDG ARCHITECT OR ENGINEER.

====<<<ACES-32 Ver. 1.1>>>===== [001850] =====<<<FL.QUALITY-MMH>>>=====

Customer : FOGLIA Thu Mar 1 09:49:12 2001

Project # : 00469 Truss ID : T7 Family # : spec.

Span : 34-5-14 B.Span : 34-6-14 Quant. : 1 Top Pitch : 6/12

==Bld(1/21/2000),v1.1=====

FORCES - GRAVITY LOADS

1-2=-1820	7-8=-1822	8-9= 0	2-12= 1515	6-10=-210
2-3=-1345		9-10= 1985	3-12=-779	6-9=-410
3-4=-1447		10-11= 1814	3-11= 140	7-9= 2058
4-5=-1447		11-12= 1206	4-11= 665	
5-6=-2028		12-1= 0	5-11=-819	
6-7=-2215			5-10= 263	

REACTIONS - SIZE

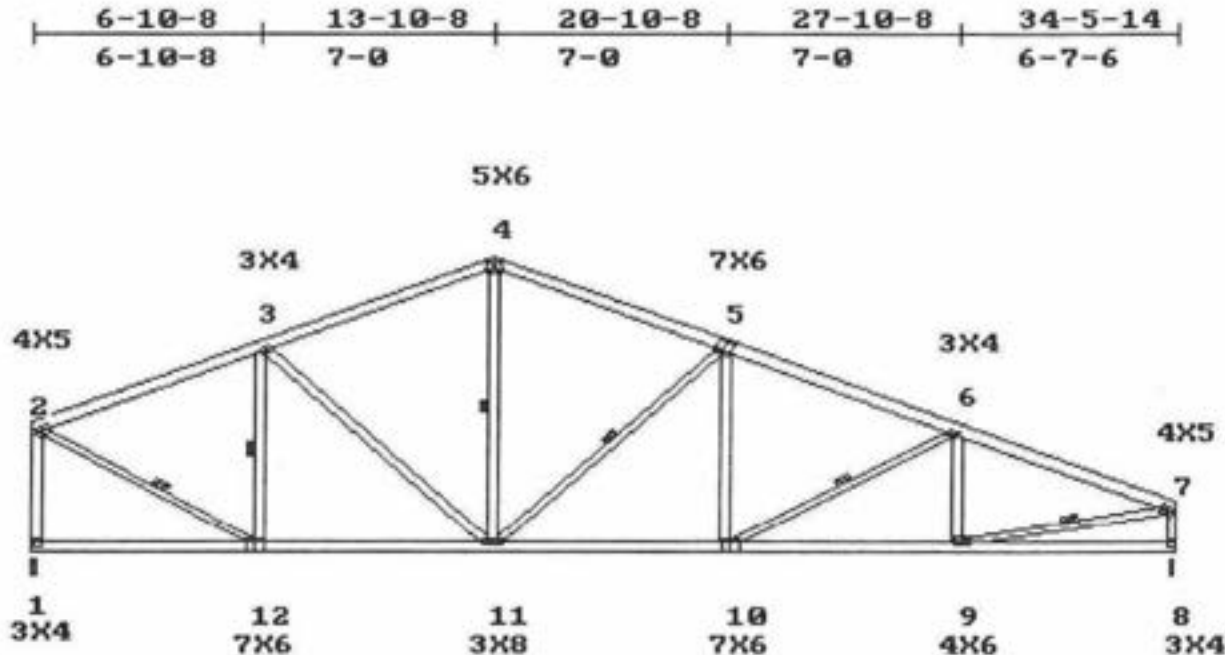
1=-1888	2.00
8=-1888	2.00

PROVIDE FOR 1822 LBS UPLIFT AT JOINT 1 (0.96)

PROVIDE FOR 1875 LBS UPLIFT AT JOINT 8 (0.99)

PROVIDE FOR 761 LBS HORIZ. REACTION AT JOINT 1

PLATE OFFSETS (X=LEFT,Y=TOP): [j5=3,5], [j9=3,2.5], [j10=3,2], [j12=3,2],



6-10-8	13-10-8	20-10-8	27-10-8	34-5-14
6-10-8	7-0	7-0	7-0	6-7-6

L. HL TO PK:15-6-2 LEFT HEIGHT:5-6-15 SPAN:34-5-14 RISE:12-6-3 R. HL TO PK:23-0-9 RIGHT HEIGHT:2-2-8

LOADING (PSF)	MAX STRESSES	MINIMUM GRADE OF LUMBER
TOP 30 15	TOP 5-6=0.982	TOP CHORD:2X6 No.2D 19 SP
BOTT 0 10	BOTT 9-10=0.321	BOT CHORD:2X6 No.2D 19 SP
	LL.DEFL.@10=0.07 < L/360	WEBS :2X4 No.3 19 SP

STR.INC.: LUMB = 1.33 PLATE = 1.33 SPACING : 24.0 in. o. c.

REPETITIVE STRESSES USED NO. OF MEMBERS = 1

BOTH END VERTICALS ARE NOT EXPOSED.

WEB 2-12; 3-12; 4-11; 5-11; 6-10; 7-9 BRACED AT 1/2 POINTS AS SHOWN ABOVE

Note:Use 1x4 or 2x3 Cont.Bracing conn.w/2-8d nails,or T-brace of same size and grade as web conn. to narrow face w/10d nails 6 in. o.c

TRUSS HAS BEEN CHECKED FOR 145 M.P.H. WIND LOAD, WALL HEIGHT 16 FT, ENCLOSED BLDG. TYPE, ASSUMING 10 PSF TOTAL D.L.(7+3) (SBC-97).

DEFLECTION(IN.) L.L= 0.07,D.L=0.06,T.L=0.13

PLATES ARE MITEK M20-249,200 MANUFACTURED FROM ASTM A 446 GRD A GALVANIZED STEEL(EXCEPT AS SHOWN)

PLATE MUST BE INSTALLED ON EA. FACE OF JOINT,SYMMETRICALLY(EXCEPT AS SHOWN)DESIGN CONFORMS W/WDS DESIGN SPECS, SIBC,ANSI/TPI-95

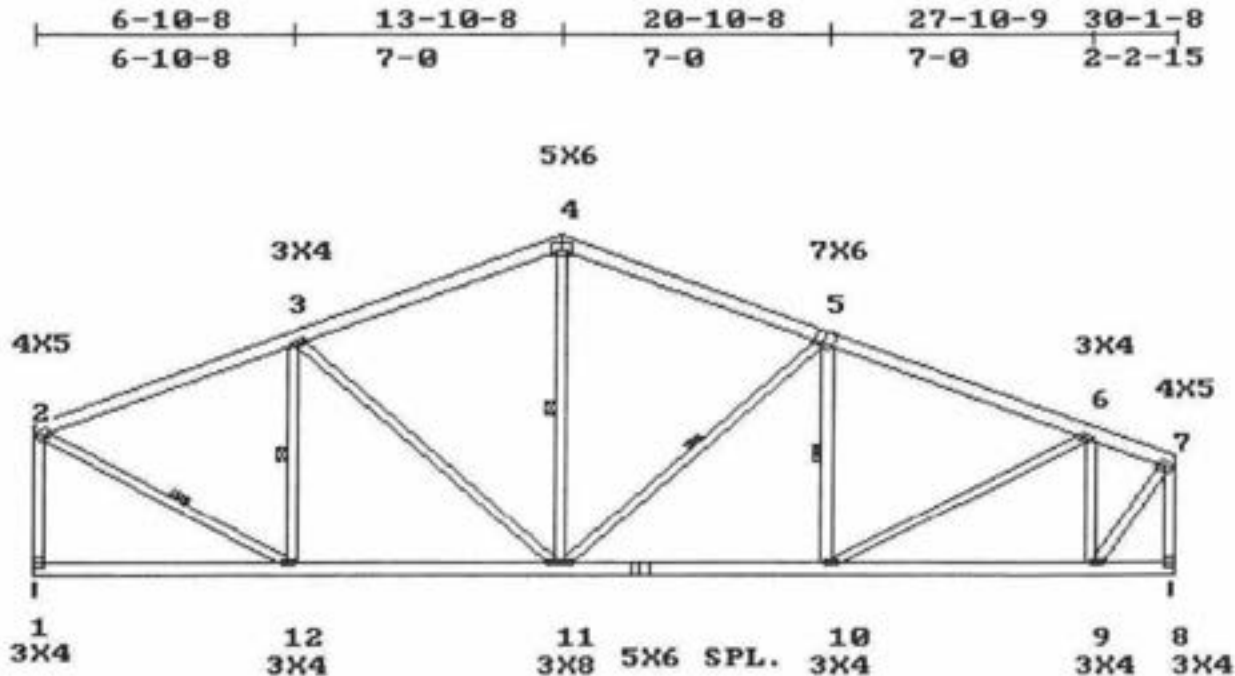
THIS DESIGN IS FOR TRUSS FABRICATION ONLY.FOR PERMANENT AND TEMPORARY BRACING(WHICH IS ALWAYS REGD)CONSULT BLDG ARCHITECT OR ENGINEER.

MAR 01 2001

====<<<ACES-32 Ver. 1.1>>>=====[001851]====<<<FL.QUALITY-MMH>>>=====
 Customer : FOGLIA Thu Mar 1 09:49:44 2001
 Project #: 00469 Truss ID : T7A Family #: spec.
 Span : 30-1-8 B.Span : 30-2-8 Quant. : 1 Top Pitch : 6/12
 ==8ld(1/21/2000),v1.1=====

FORCES - GRAVITY LOADS			REACTIONS - SIZE	
1-2=-1580	7-8=-1626	8-9= 0	2-12= 1275	6-10= 703
2-3=-1132		9-10= 658	3-12=-634	6-9=-1118
3-4=-1140		10-11= 1229	3-11= 8	7-9= 1377
4-5=-1140		11-12= 1014	4-11= 389	
5-6=-1374		12-1= 0	5-11=-330	
6-7=-731			5-10=-270	

PROVIDE FOR 1600 LBS UPLIFT AT JOINT 1 (0.97)
 PROVIDE FOR 1604 LBS UPLIFT AT JOINT 8 (0.97)
 PROVIDE FOR 449 LBS HORIZ. REACTION AT JOINT 1
 PLATE OFFSETS (X=LEFT,Y=TOP):[J5=3,5],



L. HL TO PK:15-6-2 LEFT HEIGHT:5-6-15 SPAN:30-1-8 RISE:12-6-3 R. HL TO PK:18-2 RIGHT HEIGHT:4-4-11

LOADING (PSF)		MAX STRESSES		MINIMUM GRADE OF LUMBER	
TOP	30	BOTT	10-11=0.232	TOP CHORD:	2X6 No.2D 19 SP
BOTT	0	LL.DEFL.@10=0.04	< L/360	BOT CHORD:	2X6 No.2D 19 SP
				WEBS	:2X4 No.3 19 SP

STR.INC.: LUMB = 1.33 PLATE = 1.33 SPACING : 24.0 in. o. c.
 REPETITIVE STRESSES USED NO. OF MEMBERS = 1

BOTH END VERTICALS ARE NOT EXPOSED.
 WEB 2-12; 3-12; 4-11; 5-11; 5-10 BRACED AT 1/2 POINTS AS SHOWN ABOVE
 Note:Use 1x4 or 2x3 Cont.Bracing conn.w/2-8d nails,or T-brace of same size and grade as web conn. to narrow face w/10d nails 6 in. o.c
 TRUSS HAS BEEN CHECKED FOR 145 M.P.H. WIND LOAD, WALL HEIGHT 16 FT, ENCLOSED BLDG. TYPE, ASSUMING 10 PSF TOTAL D.L.(7+3) (SBC-97).
 DEFLECTION(IN.) L.L= 0.04,D.L=0.03,T.L=0.07
 PLATES ARE MITEK M20-249,200 MANUFACTURED FROM ASTM A 446 GRD A GALVANIZED STEEL(EXCEPT AS SHOWN)
 PLATE MUST BE INSTALLED ON EA. FACE OF JOINT,SYMMETRICALLY(EXCEPT AS SHOWN)DESIGN CONFORMS W/ND S DESIGN SPECS, SIBC,ANSI/TPI-95
 THIS DESIGN IS FOR TRUSS FABRICATION ONLY.FOR PERMANENT AND TEMPORARY BRACING(WHICH IS ALWAYS REQD)CONSULT BLDG ARCHITECT OR ENGINEER.

MAR 01 2001

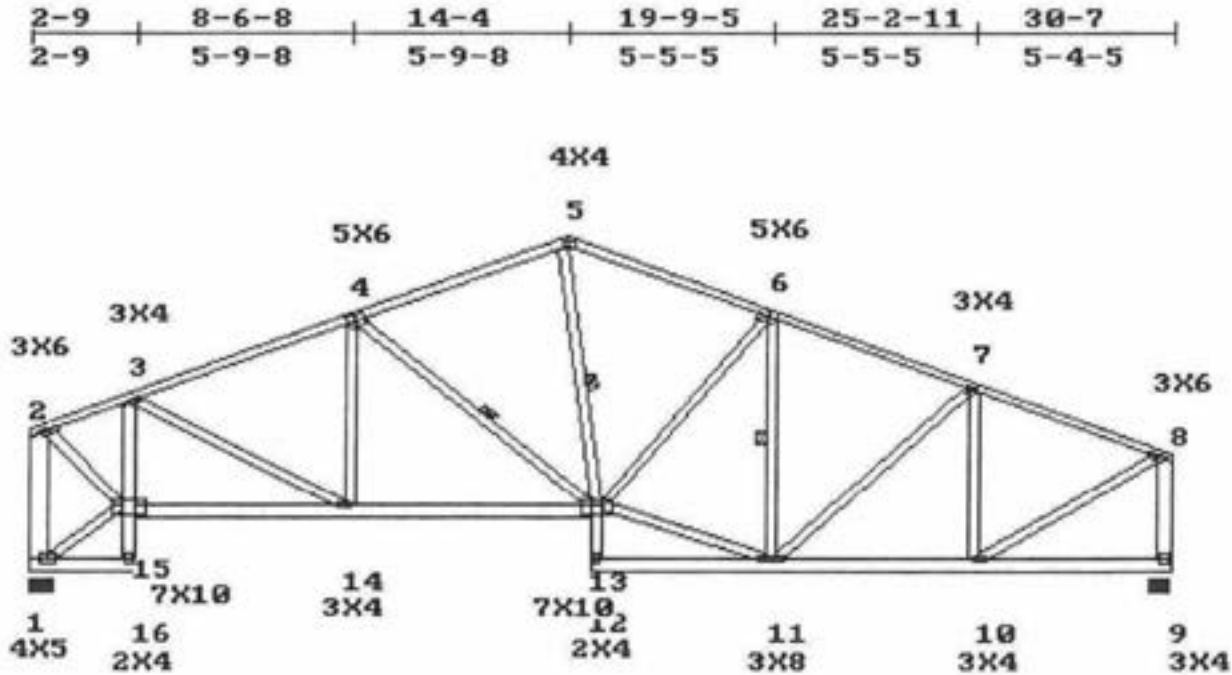
====<<<<ACES-32 Ver. 1.1>>>>=====[001852]====<<<<FL.QUALITY-MMH>>>>=====
 Customer : FOGLIA Thu Mar 1 09:50:03 2001
 Project #: 00469 Truss ID : S33 Family #: spec.
 Span : 30-7 B.Span : 40-8 Quant. : 1 Top Pitch : 6/12
 Bot pitch : 90/12

==Bld(1/21/2000),v1.1==

FORCES - GRAVITY LOADS						REACTIONS - SIZE	
1-2=1633	7-8=1181	9-10=0	15-16=24	2-15=1385	6-13=108	15-1=0	1=1657 8.00
2-3=948	8-9=1606	10-11=1056	16-1=0	3-15=1013	6-11=610		9=1657 8.00
3-4=1543		11-12=0		3-14=622	7-11=243		
4-5=1350		12-13=46		4-14=247	7-10=741		
5-6=1416		13-14=1380		4-13=255	8-10=1353		
6-7=1352		14-15=882		5-13=734	13-11=1317		

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PROVIDE FOR 1609 LBS UPLIFT AT JOINT 1 (0.97)
 PROVIDE FOR 1612 LBS UPLIFT AT JOINT 9 (0.97)
 PROVIDE FOR 424 LBS HORIZ. REACTION AT JOINT 1
 PLATE OFFSETS (X=LEFT,Y=TOP): [j4=3,3], [j6=3,3], [j13=5,2], [j15=5,2],



2-9		8-6-8		15-0		19-9-5		25-2-11		30-7	
2-9	5-9-8	6-5-8	4-9-5	5-5-6	5-4-5						

L. HL TO PK:16-0-5 INTERNAL RISE:2-0 R. HL TO PK:18-2
 LEFT HEIGHT:5-4-3 SPAN:30-7 RISE:12-6-3 RIGHT HEIGHT:4-4-11

LOADING (PSF)		MAX STRESSES		MINIMUM GRADE OF LUMBER	
TOP	30 15	TOP	6-7=0.989	TOP CHORD:2X4	SS 19 SP
BOTT	0 10	BOTT	13-14=0.229	BOT CHORD:2X6	No.2D 19 SP
		LL.DEFL.@13=0.06	< L/360	WEBS	:2X4 No.3 19 SP

STR.INC.: LUMB = 1.33 PLATE = 1.33 SPACING : 24.0 in. o. c.
 REPETITIVE STRESSES USED NO. OF MEMBERS = 1

CHORD: 6-7; 7-8 TO BE 2X4 No.1D 19 SP
 CHORD: 12-13; 15-16 TO BE 2X4 No.3 19 SP
 WEB: 1-2; 8-9 TO BE 2X6 No.2D 19 SP
 BOTH END VERTICALS ARE NOT EXPOSED.
 Note:Use 1x4 or 2x3 Cont.Bracing conn.w/2-8d nails,or T-brace of same size and grade as web conn. to narrow face w/10d nails 6 in. o.c
 TRUSS HAS BEEN CHECKED FOR 145 M.P.H. WIND LOAD, WALL HEIGHT 16 FT, ENCLOSED BLDG. TYPE, ASSUMING 10 PSF TOTAL D.L.(7+3) (SBC-97).
 DEFLECTION(IN.) L.L= 0.06,D.L=0.05,T.L=0.10
 PLATES ARE MITEK M20-249,200 MANUFACTURED FROM ASTM A 446 GRD A GALVANIZED STEEL(EXCEPT AS SHOWN)
 PLATE MUST BE INSTALLED ON EA. FACE OF JOINT,SYMMETRICALLY(EXCEPT AS SHOWN)DESIGN CONFORMS W/NDS DESIGN SPECS, SBC,ANSI/TP1-95
 THIS DESIGN IS FOR TRUSS FABRICATION ONLY.FOR PERMANENT AND TEMPORARY BRACING(WHICH IS ALWAYS REQD)CONSULT BLDG ARCHITECT OR ENGINEER.

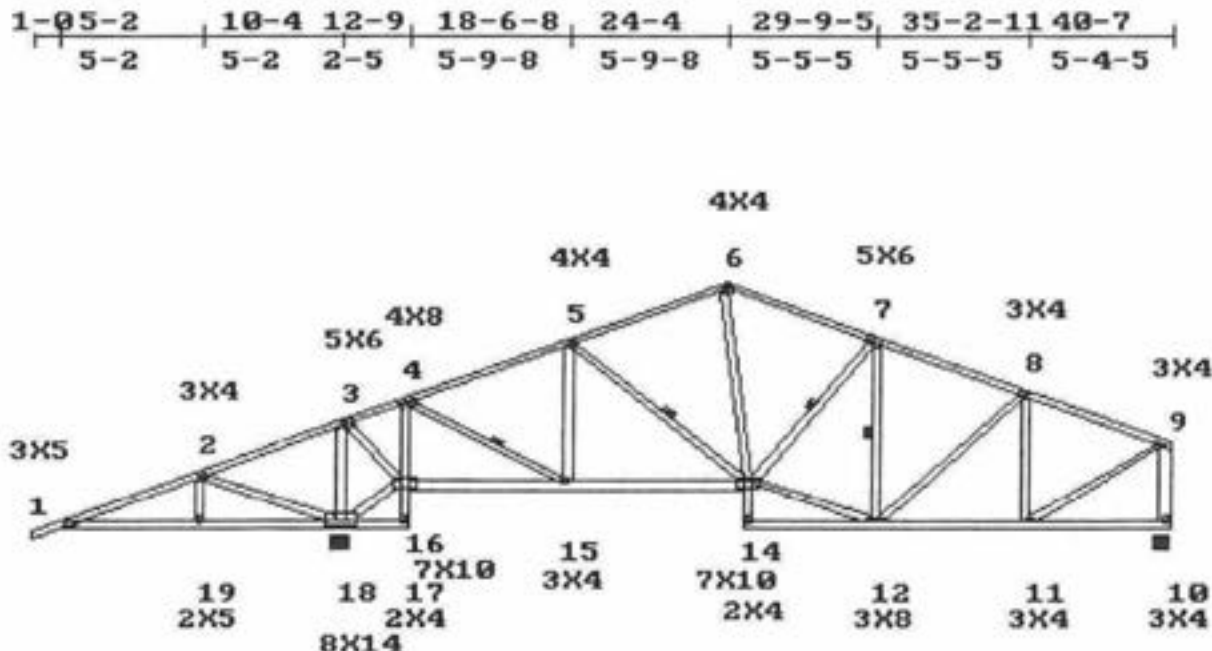
[Signature]
 MAR 0 1 2001

====<<<<ACES-32 Ver. 1.1>>>>=====[001853]====<<<<FL.QUALITY-MMH>>>>=====
 Customer : FOGLIA Thu Mar 1 09:50:37 2001
 Project #: 00469 Truss ID : S34 Family #: spec.
 Span : 40-7 B.Span : 40-8 Quant. : 1 Top Pitch : 6/12
 Bot pitch : 90/12

==8ld(1/21/2000),v1.1=====

FORCES - GRAVITY LOADS								REACTIONS - SIZE	
1-2= 893	7-8=-1051	10-11= 0	16-17= 23	2-19= 103	5-15=-847	8-11=-594	18=-3131	8.00	
2-3= 1530	8-9=-975	11-12= 872	17-18= 0	2-18=-631	5-14= 303	9-11= 1118	10=-1418	8.00	
3-4= 795	9-10=-1366	12-13= 0	18-19=-802	3-18=-1574	6-14= 373	14-12= 1024			
4-5=-735		13-14= 46	19-1=-802	3-16= 1117	7-14=-91	16-18=-1823			
5-6=-964		14-15= 658		4-16=-2028	7-12=-389				
6-7=-998		15-16=-643		4-15= 1624	8-12= 108				

PROVIDE FOR 5086 LBS UPLIFT AT JOINT 18 (1.62)
 PROVIDE FOR 1180 LBS UPLIFT AT JOINT 10 (0.83)
 PROVIDE FOR 914 LBS HORIZ. REACTION AT JOINT 18
 PLATE OFFSETS (X=LEFT,Y=TOP):[j3=3,3],[j7=3,3],[j14=5,2],[j16=5,2],[j19=3,1],



5-2	10-4	12-9	18-6-8	25-0	29-9-5	35-2-11	40-7
5-2	5-2	2-5	5-9-8	6-5-8	4-9-5	5-5-6	5-4-5

L. HL TO PK:27-2-7 INTERNAL RISE:2-0 R. HL TO PK:18-2
 LEFT HEIGHT:0-4-3 SPAN:40-7 RISE:12-6-3 RIGHT HEIGHT:4-4-11

LOADING (PSF)		MAX STRESSES		MINIMUM GRADE OF LUMBER	
TOP	30	TOP	5-6=0.997	TOP CHORD:2X4	No.2ND 19 SP
BOTT	0	BOTT	18-19=0.557	BOT CHORD:2X6	No.2D 19 SP
		LL.DEFL.@19=0.08	< L/360	WEBS	:2X4 No.3 19 SP

STR.INC.: LUMB = 1.33 PLATE = 1.33 SPACING : 24.0 in. o. c.
 REPETITIVE STRESSES USED NO. OF MEMBERS = 1

CHORD: 3-4; 4-5; 5-6; 6-7; 7-8; 8-9 TO BE 2X4 No.10 19 SP
 CHORD: 13-14; 16-17 TO BE 2X4 No.3 19 SP
 WEB: 9-10; 3-18 TO BE 2X6 No.2D 19 SP
 WEB: 16-18 TO BE 2X4 No.2ND 19 SP
 RIGHT END VERTICAL IS NOT EXPOSED.

WEB 4-15; 5-14; 7-14; 7-12 BRACED AT 1/2 POINTS AS SHOWN ABOVE

Note:Use 1x4 or 2x3 Cont.Bracing conn.w/2-8d nails,or T-brace of same size and grade as web conn. to narrow face w/10d nails 6 in. o.c
 TRUSS HAS BEEN CHECKED FOR 145 M.P.H. WIND LOAD, WALL HEIGHT 16 FT, ENCLOSED BLDG. TYPE, ASSUMING 10 PSF TOTAL D.L.(7+3) (SBC-97).

DEFLECTION(IN.) L.L= 0.08,D.L=0.06,T.L=0.14

PLATES ARE MITEK M20-249,200 MANUFACTURED FROM ASTM A 446 GRD A GALVANIZED STEEL(EXCEPT AS SHOWN)

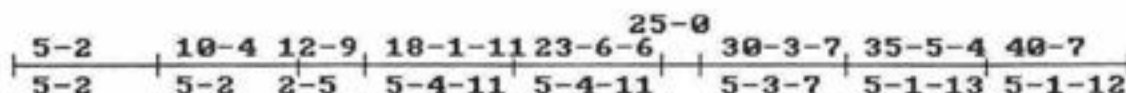
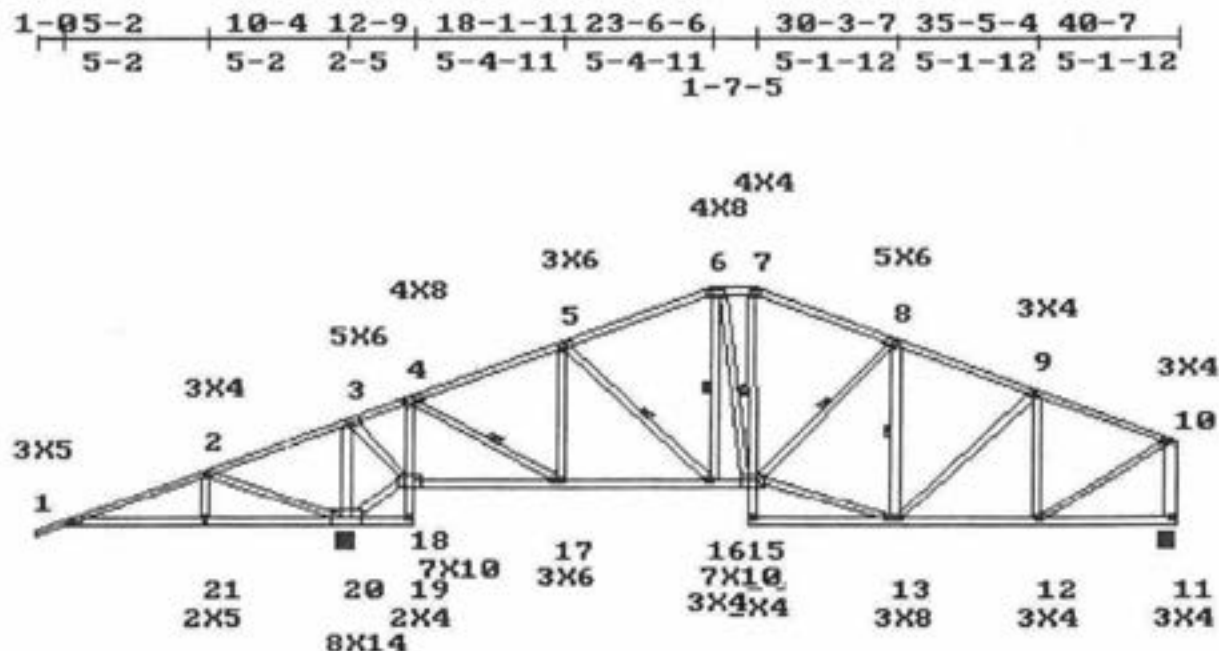
PLATE MUST BE INSTALLED ON EA. FACE OF JOINT,SYMMETRICALLY(EXCEPT AS SHOWN)DESIGN CONFORMS W/NDS DESIGN SPECS, S58C,ANSI/TPI-95

THIS DESIGN IS FOR TRUSS FABRICATION ONLY.FOR PERMANENT AND TEMPORARY BRACING(WHICH IS ALWAYS REQD)CONSULT BLDG ARCHITECT OR ENGINEER.

MAR 01 2001

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PROVIDE FOR 5110 LBS UPLIFT AT JOINT 20 (1.63)
 PROVIDE FOR 1201 LBS UPLIFT AT JOINT 11 (0.85)
 PROVIDE FOR 899 LBS HORIZ. REACTION AT JOINT 20
 PLATE OFFSETS (X=LEFT,Y=TOP): [3=3,3], [8=3,3], [15=5,2], [18=5,2], [21=3,1]



LOADING (PSF)		MAX STRESSES		MINIMUM GRADE OF LUMBER	
L	D	TOP	BOTT	TOP CHORD	NO. 2ND
30	15	5-6=0.951	20-21=0.557	2X4	19 SP
0	10	LL.DEFL.@21=0.08 < L/360		2X6	NO.2D 19 SP
				WEBS	2X4 NO.3 19 SP
STR.INC.: LUMB = 1.33 PLATE = 1.33				SPACING : 24.0 in. o. c.	
REPETITIVE STRESSES USED				NO. OF MEMBERS = 1	

CHORD: 3-4; 4-5; 5-6; 7-8; 8-9; 9-10 TO BE 2X4 No.1D 19 SP
CHORD: 14-15; 18-19 TO BE 2X4 No.3 19 SP
WEB: 10-11; 3-20 TO BE 2X6 No.2D 19 SP
WEB: 18-20 TO BE 2X4 No.2ND 19 SP
RIGHT END VERTICAL IS NOT EXPOSED.
WEB 4-17; 5-16; 6-16; 6-15; 8-15; 8-13 BRACED AT 1/2 POINTS AS SHOWN ABOVE
Note: Use 1x4 or 2x3 Cont. Bracing conn. w/2-8d nails, or T-brace of same size and grade as web conn. to narrow face w/10d nails 6 in. o.c.
TRUSS HAS BEEN CHECKED FOR 145 M.P.H. WIND LOAD, WALL HEIGHT 16 FT, ENCLOSED BLDG. TYPE, ASSUMING 10 PSF TOTAL D.L.(7+3) (SBC-97).
DEFLECTION (IN.) L.L.= 0.08, D.L.= 0.07, T.L.= 0.14
PLATES ARE MITEK M20-249,200 MANUFACTURED FROM ASTM A 446 GRD A GALVANIZED STEEL (EXCEPT AS SHOWN)
PLATE MUST BE INSTALLED ON EA. FACE OF JOINT, SYMMETRICALLY (EXCEPT AS SHOWN) DESIGN CONFORMS W/WDG DESIGN SPECS, SSBC, ANSI/TPI-95
THIS DESIGN IS FOR TRUSS FABRICATION ONLY, FOR PERMANENT AND TEMPORARY BRACING (WHICH IS ALWAYS REQD) CONSULT BLDG ARCHITECT OR ENGINEER.

====<<<<ACES-32 Ver. 1.1>>>>===== [001855] =====<<<<FL.QUALITY-MMH>>>>=====

Customer : FOGLIA Thu Mar 1 09:51:21 2001

Project #: 00469 Truss ID : S41 Family #: spec.

Span : 40-7 B.Span : 40-8 Quant. : 1 Top Pitch : 6/12

Bot pitch : 90/12

==81d(1/21/2000),v1.1=====

FORCES - GRAVITY LOADS						REACTIONS - SIZE	
1-2= 893	7-8= 997	11-12= 0	17-18= 27	2-20= 103	5-16= 606	9-13= 126	19= 3131 8.00
2-3= 1529	8-9= 951	12-13= 935	18-19= 0	2-19= 631	6-16= 88	9-12= 461	11= 1418 8.00
3-4= 600	9-10= 1045	13-14= 0	19-20= 801	3-19= 1749	7-16= 511	10-12= 1107	
4-5= 588	10-11= 1353	14-15= 24	20-1= 801	3-17= 1280	7-15= 230	15-13= 1108	
5-6= 922		15-16= 993		4-17= 321	8-15= 557	17-19= 1709	
6-7= 823		16-17= 500		5-17= 1873	8-13= 527		

50

PROVIDE FOR 5163 LBS UPLIFT AT JOINT 19 (1.65)

PROVIDE FOR 1249 LBS UPLIFT AT JOINT 11 (0.88)

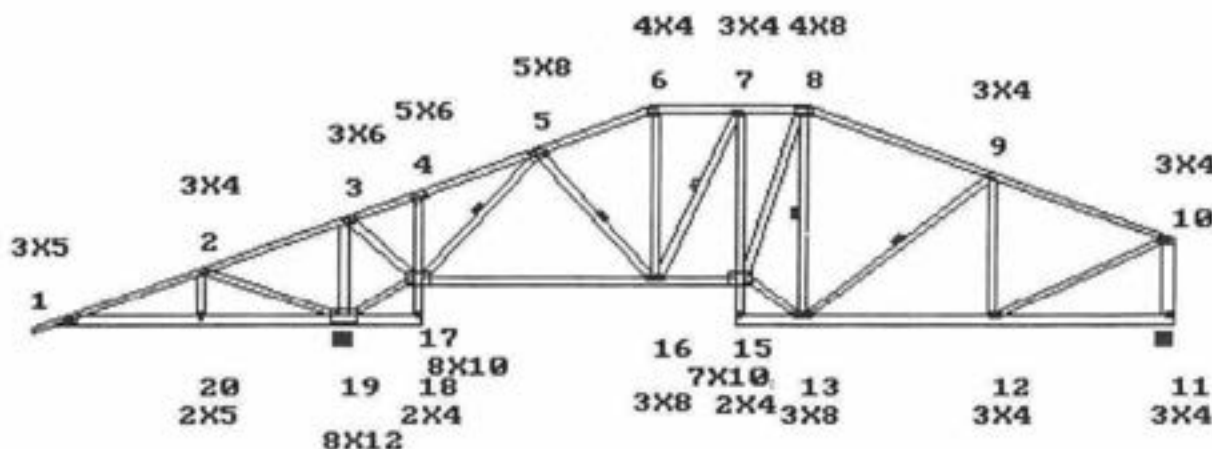
PROVIDE FOR 858 LBS HORIZ. REACTION AT JOINT 19

PLATE OFFSETS (X=LEFT,Y=TOP): [j4=3,3], [j15=5,2], [j17=5,3], [j20=3,1],

1-05-2 | 10-4 | 13-1-12 | 21-6-6 | 27-1-11 | 40-7

5-2 | 5-2 | 2-9-12 | 4-2-5 | 2-6-7 | 6-8-11 | 6-8-11

4-2-5 | 3-0-14



5-2 | 10-4 | 13-1-12 | 21-6-6 | 27-1-11 | 33-10-6 | 40-7

5-2 | 5-2 | 2-9-12 | 8-4-10 | 3-0-14 | 6-8-11 | 6-8-11

L. HL TO PK:24-0-14

LEFT HEIGHT:0-4-3

INTERNAL RISE:1-11-15

SPAN:40-7

RISE:11-1-7

R. HL TO PK:15-0-6

RIGHT HEIGHT:4-4-12

LOADING (PSF)

L D

TOP 30 15

BOTT 0 10

MAX STRESSES

TOP 8-9=0.944

BOTT 19-20=0.557

LL.DEFL.@20=0.08 < L/360

MINIMUM GRADE OF LUMBER

TOP CHORD:2X4 No.2ND 19 SP

BOT CHORD:2X6 No.2D 19 SP

WEBS :2X4 No.3 19 SP

STR.INC.: LUMB = 1.33 PLATE = 1.33

REPETITIVE STRESSES USED

SPACING : 24.0 in. o. c.

NO. OF MEMBERS = 1

CHORD: 8-9; 9-10 TO BE 2X4 SS 19 SP

CHORD: 14-15; 17-18 TO BE 2X4 No.3 19 SP

WEB: 10-11; 3-19 TO BE 2X6 No.2D 19 SP

WEB: 17-19 TO BE 2X4 No.2ND 19 SP

RIGHT END VERTICAL IS NOT EXPOSED.

WEB 5-17; 5-16; 7-16; 8-13; 9-13 BRACED AT 1/2 POINTS AS SHOWN ABOVE

Note:Use 1x4 or 2x3 Cont.Bracing conn.w/2-8d nails,or T-brace of same size and grade as web conn. to narrow face w/10d nails 6 in. o.c

TRUSS HAS BEEN CHECKED FOR 145 M.P.H. WIND LOAD, WALL HEIGHT 16 FT, ENCLOSED BLDG. TYPE, ASSUMING 10 PSF TOTAL D.L.(7+3) (SBC-97).

DEFLECTION(IN.) L.L= 0.08,D.L=0.06,T.L=0.14

PLATES ARE MITEK M20-249,200 MANUFACTURED FROM ASTM A 446 GRD A GALVANIZED STEEL(EXCEPT AS SHOWN)

PLATE MUST BE INSTALLED ON EA. FACE OF JOINT,SYMMETRICALLY(EXCEPT AS SHOWN)DESIGN CONFORMS W/NDIS DESIGN SPECS, SSSC,ANSI/TP1-95

THIS DESIGN IS FOR TRUSS FABRICATION ONLY.FOR PERMANENT AND TEMPORARY BRACING(WHICH IS ALWAYS REQD)CONSULT BLDG ARCHITECT OR ENGINEER.

MAR 01 2001

==<<<<ACES-32 Ver. 1.1>>>>===== [001856] =====<<<<FL.QUALITY-MMH>>>>=====

Customer : FOGLIA Thu Mar 1 09:51:42 2001

Project #: 00469 Truss ID : H40A Family #: spec.

Span : 44-5-2 B.Span : 44-6-2 Quant. : 1 Top Pitch : 6/12

==Bld(1/21/2000),v1.1=====

FORCES - GRAVITY LOADS				REACTIONS - SIZE			
1-2=893	7-8=1432	11-12=0	17-1=-801	2-17=103	6-15=-831	9-12=-565	16=-3316 8.00
2-3=1529	8-9=1720	12-13=1454		2-16=-631	6-14=349	10-12=1599	11=-1656 8.00
3-4=1529	9-10=-1626	13-14=1538		3-16=-439	7-14=197		
4-5=-847	10-11=-1607	14-15=1124		4-16=-2887	8-14=-446		
5-6=-757		15-16=153		4-15=1147	8-13=27		
6-7=-1279		16-17=-801		5-15=-42	9-13=113		

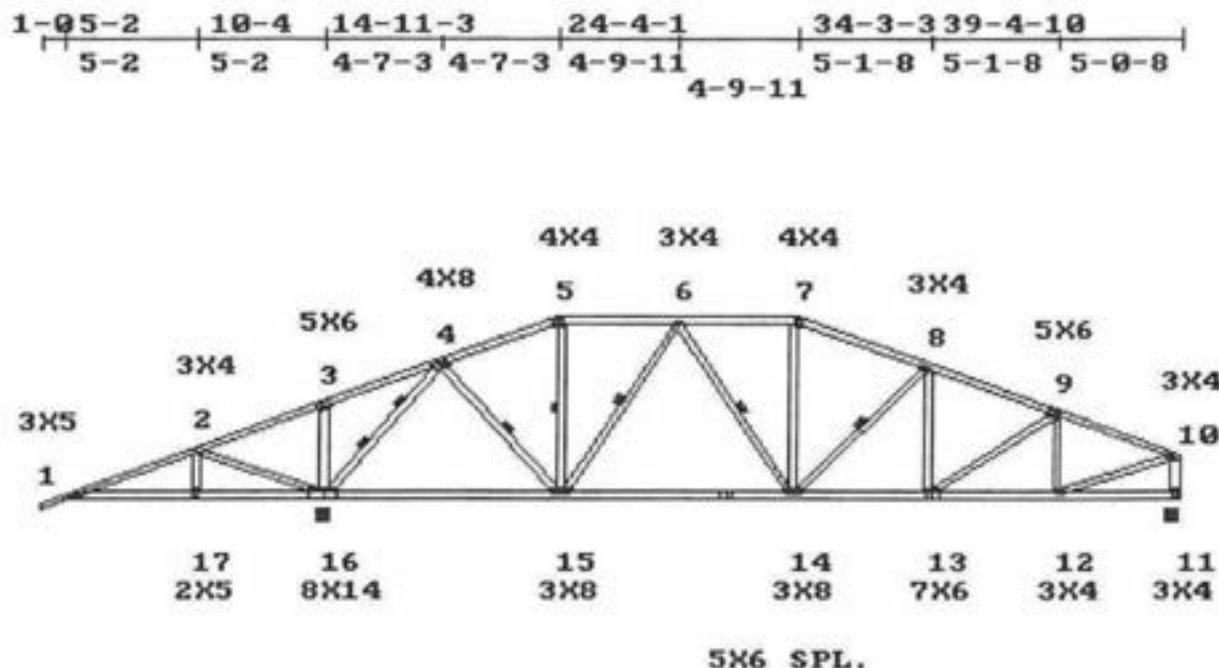
(51)

PROVIDE FOR 5363 LBS UPLIFT AT JOINT 16 (1.62)

PROVIDE FOR 1564 LBS UPLIFT AT JOINT 11 (0.94)

PROVIDE FOR 621 LBS HORIZ. REACTION AT JOINT 16

PLATE OFFSETS (X=LEFT,Y=TOP): [J3=3,3], [J9=3,3], [J13=3,2], [J16=7,3], [J17=3,1],



5-2 10-4 19-6-6 29-1-11 34-3-3 39-4-10 44-5-2

5-2 5-2 9-2-6 9-7-5 5-1-8 5-1-8 5-0-8

L. HL TO PK:21-10-1 R. HL TO PK:17-1-1

LEFT HEIGHT:0-4-3 SPAN:44-5-2 RISE:10-1-7 RIGHT HEIGHT:2-5-11

LOADING (PSF)		MAX STRESSES		MINIMUM GRADE OF LUMBER	
TOP	30	TOP	8-9=0.971	TOP CHORD:	2X4 No.2ND 19 SP
BOTT	0	BOTT	16-17=0.639	BOT CHORD:	2X6 No.2D 19 SP
		LL.DEFL.@17=0.07 < L/360		WEBS:	2X4 No.3 19 SP

STR.INC.: LUMB = 1.33 PLATE = 1.33 SPACING : 24.0 in. o. c.

REPETITIVE STRESSES USED NO. OF MEMBERS = 1

CHORD: 5-6; 6-7; 7-8; 8-9; 9-10 TO BE 2X4 No.1D 19 SP

CHORD: 16-17; 17-1 TO BE 2X6 SS 19 SP

WEB: 10-11 TO BE 2X6 No.2D 19 SP

WEB: 3-16 TO BE 2X6 SS 19 SP

WEB: 4-16 TO BE 2X4 No.2ND 19 SP

RIGHT END VERTICAL IS NOT EXPOSED.

WEB 4-15; 5-15; 6-15; 6-14; 8-14 BRACED at 1/2 POINTS AS SHOWN ABOVE

WEB 4-16 BRACED at 1/3 POINTS AS SHOWN ABOVE

Note:Use 1x4 or 2x3 Cont.Bracing conn.w/2-8d nails,or T-brace of same size and grade as web conn. to narrow face w/10d nails 6 in. o.c

TRUSS HAS BEEN CHECKED FOR 145 M.P.H. WIND LOAD, WALL HEIGHT 16 FT, ENCLOSED BLDG. TYPE, ASSUMING 10 PSF TOTAL D.L.(7+3) (SBC-97).

DEFLECTION(IN.) L.L= 0.07,D.L=0.06,T.L=0.13

PLATES ARE MITEK M20-249,200 MANUFACTURED FROM ASTM A 446 GRD A GALVANIZED STEEL(EXCEPT AS SHOWN)

PLATE MUST BE INSTALLED ON EA. FACE OF JOINT,SYMMETRICALLY(EXCEPT AS SHOWN)DESIGN CONFORMS W/NDS DESIGN SPECS, SSB,ANSI/TPI-95

THIS DESIGN IS FOR TRUSS FABRICATION ONLY.FOR PERMANENT AND TEMPORARY BRACING(WHICH IS ALWAYS REQD)CONSULT BLDG ARCHITECT OR ENGINEER.

MAR 01 2001

====<<<<ACES-32 Ver. 1.1>>>>===== [001857] =====<<<<FL.QUALITY-MMH>>>>=====

Customer : FOGLIA Thu Mar 1 09:52:17 2001

Project #: 00469 Truss ID : S40 Family #: spec.

Span : 45-8-4 B.Span : 45-9-4 Quant. : 1 Top Pitch : 6/12

==81d(1/21/2000),v1.1=====

FORCES - LOAD CASE #1				REACTIONS - SIZE				
1-2=.644	7-8=-2851	11-12= 0	17-18=-577	2-18= 103	5-15= 1616	9-13= 272	17=-4678	10.36
2-3= 1281	8-9=-3097	12-13= 2556	18-1=-577	2-17=-633	6-15=-1132	9-12=-723	11=-2368	8.00
3-4=-2141	9-10=-2858	13-14= 2770		3-17=-4275	7-15= 451	10-12= 2685		
4-5=-2141	10-11=-2320	14-15= 2550		3-16= 3763	7-14= 403			
5-6=-2823		15-16= 2017		4-16=-357	8-14=-358			
6-7=-2823		16-17=-1146		5-16=-1205	8-13=-66			

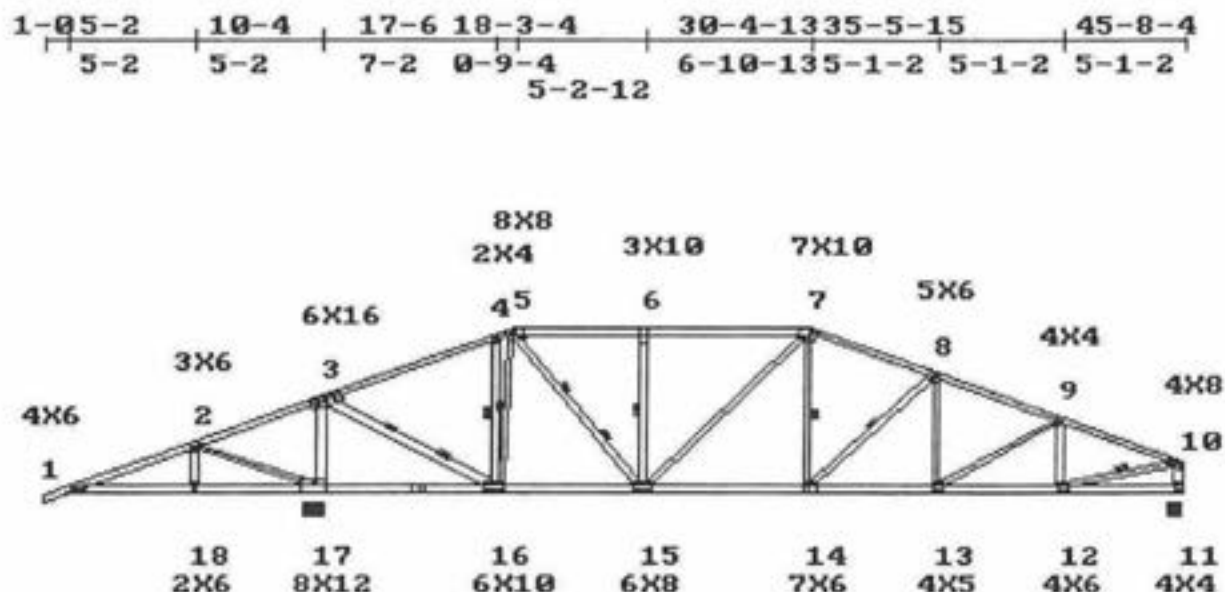
(52)

PROVIDE FOR 10255 LBS UPLIFT AT JOINT 17 (2.19)

PROVIDE FOR 4112 LBS UPLIFT AT JOINT 11 (1.74)

PROVIDE FOR 530 LBS HORIZ. REACTION AT JOINT 17

PLATE OFFSETS (X=LEFT,Y=TOP): [J5=4,2.5], [J7=5,4], [J8=3,3], [J10=3.5,2], [J14=3,2], [J17=6.5,4],



5X6 SPL.

5-2 10-4 17-6 23-6 30-4-1335-5-15 40-7-2 45-8-4

5-2 5-2 7-2 6-0 6-10-135-1-2 5-1-2 5-1-2

L. HL TO PK:20-5-2 R. HL TO PK:17-1-1

LEFT HEIGHT:0-4-3 SPAN:45-8-4 RISE:9-5-14 RIGHT HEIGHT:1-10-2

LOADING (PSF)		MAX STRESSES		MINIMUM GRADE OF LUMBER	
TOP	30	15	TOP 2-3=0.925	TOP CHORD:2X4	SS 19 SP
BOTT	0	10	BOTT 17-18=0.890	BOT CHORD:2X6	No.2D 19 SP
			LL.DEFL.@14=0.18 < L/360	WEBS	:2X4 No.3 19 SP

REPETITIVE STRESSES NOT USED

SPACING : 24.0 in. o. c.

NO. OF MEMBERS = 1

LOADING	STRESS INCREASE	LOADING	PANEL(PLF) / JOINTS(LBS)
LUMBER	1.33	1.33	UNIFORM
PLATE			CONCENTRATED
			1- 5= 90 5- 6= 290 6-10= 90 11- 1= 20
			15= 500 16= 500

CHORD: 5-6; 6-7 TO BE 2X6 SS 19 SP

WEB: 10-11; 3-16 TO BE 2X6 No.20 19 SP

WEB: 3-17 TO BE 2X6 SS 19 SP

WEB: 5-15 TO BE 2X4 No.10 19 SP

RIGHT END VERTICAL IS NOT EXPOSED.

WEB 4-16; 5-16; 6-15; 7-14; 8-14; 10-12 BRACED at 1/2 POINTS AS SHOWN ABOVE

WEB 3-16; 5-15 BRACED at 1/3 POINTS AS SHOWN ABOVE

Note:Use 1x4 or 2x3 Cont.Bracing conn.w/2-8d nails,or T-brace of same size and grade as web conn. to narrow face w/10d nails 6 in. o.c.

TRUSS HAS BEEN CHECKED FOR 145 M.P.H. WIND LOAD, WALL HEIGHT 16 FT, ENCLOSED BLDG. TYPE, ASSUMING 10 PSF TOTAL D.L.(7+3) (SBC-97).

DEFLECTION(IN.) L.L= 0.18,D.L=0.15,T.L=0.33

PLATES ARE MITEK M20-249,200 MANUFACTURED FROM ASTM A 446 GRD A GALVANIZED STEEL(EXCEPT AS SHOWN)

PLATE MUST BE INSTALLED ON EA. FACE OF JOINT,SYMMETRICALLY(EXCEPT AS SHOWN)DESIGN CONFORMS W/NDS DESIGN SPECS, SSSC,ANSI/TP1-95

[Signature]

MAR 01 2001

THIS DESIGN IS FOR TRUSS FABRICATION ONLY, FOR PERMANENT AND TEMPORARY BRACING (WHICH IS ALWAYS REQD) CONSULT BLDG ARCHITECT OR ENGINEER.

52

V. K. Kishore

2001. 11. 1

====<<<<ACES-32 Ver. 1.1>>>>===== [001858] =====<<<<FL.QUALITY-MMH>>>>=====

Customer : FOGLIA Thu Mar 1 09:52:41 2001

Project #: 00469 Truss ID : G1 Family #: spec.

Span : 1-10-8 B.Span : 1-10-8 Quant. : 2 Top Pitch : 6/12

==Bld(1/21/2000),v1.1=====

REACTIONS - SIZE

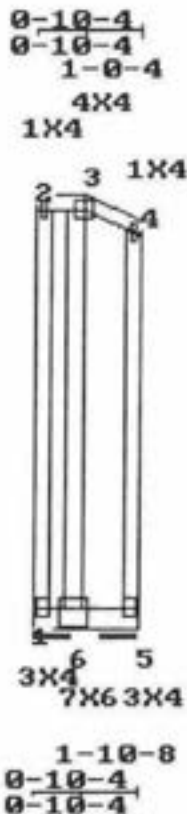
1=-483	8.00
5=-483	8.00

(53)

PROVIDE FOR 629 LBS UPLIFT AT JOINT 1 (1.30)

PROVIDE FOR 555 LBS UPLIFT AT JOINT 5 (1.15)

PLATE OFFSETS (X=LEFT,Y=TOP):[J6=3,2],



LEFT HEIGHT:9-0-11 SPAN:1-10-8 RISE:9-0-11 R. HL TO PK :1-1-11

RIGHT HEIGHT:8-6-9

LOADING (PSF)	MAX STRESSES	MINIMUM GRADE OF LUMBER
L D	TOP 2-3=0.052	TOP CHORD:2X4 No.2ND 19 SP
TOP 30 15	BOTT 6-1=0.106	BOT CHORD:2X6 No.2D 19 SP
BOTT 0 10	LL.DEFL.@5=0.00 < L/360	WEBS :2X4 No.3 19 SP

REPETITIVE STRESSES NOT USED

SPACING : 24.0 in. o. c.

NO. OF MEMBERS = 2

LOADING	STRESS INCREASE	LOADING	PANEL(PLF) / JOINTS(LBS)
LUMBER	PLATE	TYPE	
1	1.33	1.33	UNIFORM 2- 4= 340 5- 1= 270

BOTH END VERTICALS ARE NOT EXPOSED.

2 MEMBERS NAILED TOG. W/1 ROW(S) OF .131x3 in. NAILS 7 in. o.c.(TOP CHS.),AND 1 ROW(S) OF .131x3 in. NAILS 9 in. o.c.(BOTT. CHS.)

For Webs use 1 ROW of NAILS 12 in. o.c.

TRUSS HAS BEEN CHECKED FOR 145 M.P.H. WIND LOAD, WALL HEIGHT 16 FT, ENCLOSED BLDG. TYPE, ASSUMING 10 PSF TOTAL D.L.(7+3) (SBC-97).

DEFLECTION(IN.) L.L= 0.00,D.L=0.00,T.L=0.00

PLATES ARE MITEK M20-249,200 MANUFACTURED FROM ASTM A 446 GRD A GALVANIZED STEEL(EXCEPT AS SHOWN)

PLATE MUST BE INSTALLED ON EA. FACE OF JOINT,SYMMETRICALLY(EXCEPT AS SHOWN)DESIGN CONFORMS W/NDS DESIGN SPECS, S58C,NDS,ANSI/TP1-95

THIS DESIGN IS FOR TRUSS FABRICATION ONLY.FOR PERMANENT AND TEMPORARY BRACING(WHICH IS ALWAYS REQD)CONSULT BLDG ARCHITECT OR ENGINEER.

[Signature]

MAR 01 2001

====<<<ACES-32 Ver. 1.1>>>===== [001859] =====<<<FL.QUALITY-MMH>>>=====

Customer : FOGLIA Thu Mar 1 09:53:01 2001

Project #: 00469 Truss ID : G1A Family #: 201

Span : 1-10-8 B.Span : 1-10-8 Quant. : 2 Top Pitch : 6/12

==8(d(1/21/2000),v1.1=====

FORCES - LOAD CASE #1

1-2=-290 4-1= 0 2-4= 0

2-3= 0

3-4=-290

REACTIONS - SIZE

1=-500 2.00

4=-500 2.00

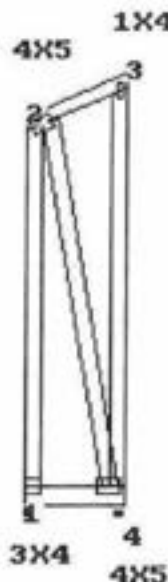
PROVIDE FOR 177 LBS UPLIFT AT JOINT 1 (0.35)

PROVIDE FOR 1190 LBS UPLIFT AT JOINT 4 (2.38)

PROVIDE FOR 113 LBS HORIZ. REACTION AT JOINT 1

1-10-8

1-10-8



1-10-8

1-10-8

L. HL TO PK:2-1-2

LEFT HEIGHT:8-6-9

SPAN:1-10-8

RISE:9-5-13

RIGHT HEIGHT:9-5-13

LOADING (PSF)

TOP 30 15

BOTT 0 10

MAX STRESSES

TOP 2-3=0.147

BOTT 4-1=0.024

LL.DEFL.@0=0.00 < L/360

MINIMUM GRADE OF LUMBER

TOP CHORD:2X4 No.2ND 19 SP

BOT CHORD:2X6 No.2D 19 SP

WEBS :2X4 No.3 19 SP

REPETITIVE STRESSES NOT USED

SPACING : 24.0 in. o. c.

NO. OF MEMBERS = 2

LOADING	STRESS INCREASE	LOADING	PANEL(PLF) / JOINTS(LBS)
LUMBER	PLATE	TYPE	
1	1.33	1.33	UNIFORM 2- 3= 340 4- 1= 245

BOTH END VERTICALS ARE NOT EXPOSED.

2 MEMBERS NAILED TOG. W/1 ROW(S) OF .131x3 in. NAILS 7 in. o.c.(TOP CHS.),AND 1 ROW(S) OF .131x3 in. NAILS 11 in. o.c(BOTT. CHS.)

For Webs use 1 ROW of NAILS 12 in. o.c.

TRUSS HAS BEEN CHECKED FOR 145 M.P.H. WIND LOAD, WALL HEIGHT 16 FT, ENCLOSED BLDG. TYPE, ASSUMING 10 PSF TOTAL D.L.(7+3) (SBC-97).

DEFLECTION(IN.) L.L= 0.00,D.L=0.00,T.L=0.00

PLATES ARE MITEK M20-249,200 MANUFACTURED FROM ASTM A 446 GRD A GALVANIZED STEEL(EXCEPT AS SHOWN)

PLATE MUST BE INSTALLED ON EA. FACE OF JOINT,SYMMETRICALLY(EXCEPT AS SHOWN)DESIGN CONFORMS W/NDG DESIGN SPECS, SSSC,ANSI/TPI-95

THIS DESIGN IS FOR TRUSS FABRICATION ONLY.FOR PERMANENT AND TEMPORARY BRACING(WHICH IS ALWAYS REQD)CONSULT BLDG ARCHITECT OR ENGINEER.

MAR 01 2001

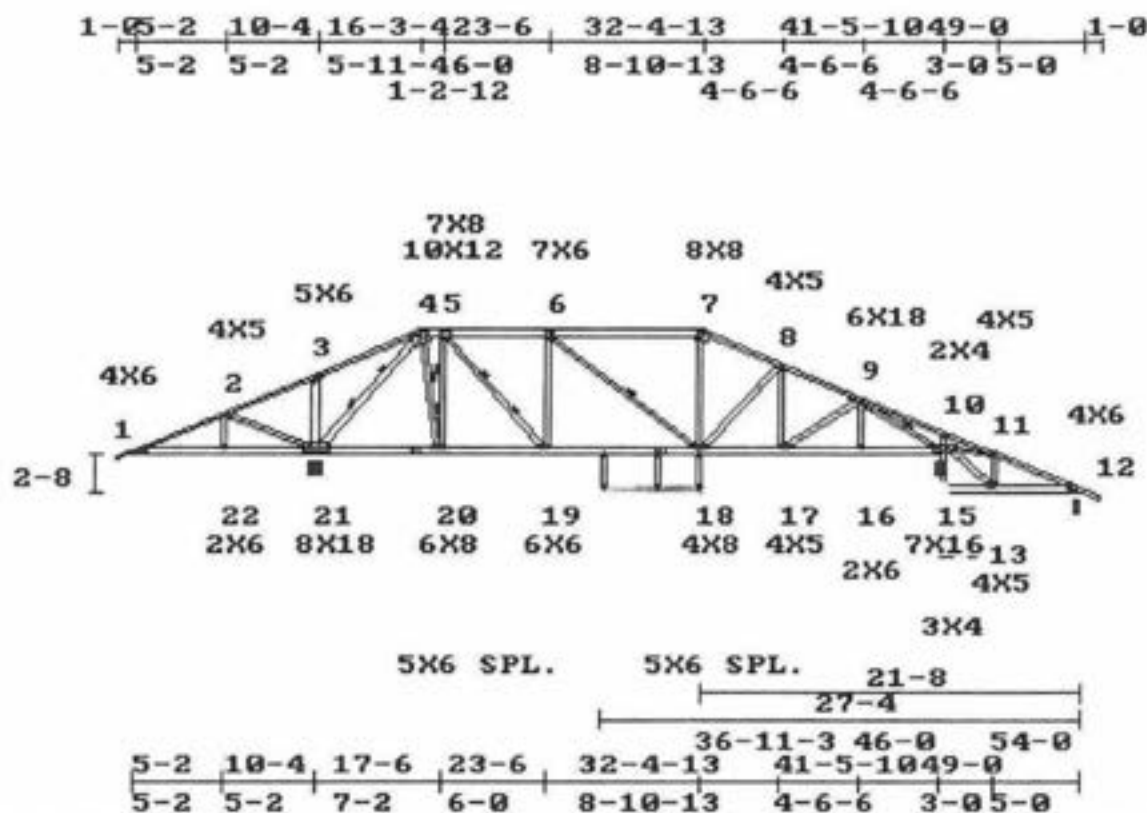
==<<<<ACES-32 Ver. 1.1>>>>===== [001860] =====<<<<FL.QUALITY-MMH>>>>=====

Customer : FOGLIA	Truss ID : S39	Thu Mar 1 09:53:40 2001
Project # : 00469	Quant. : 1	Family # : spec.
Span : 54-0	B.Span : 54-0	Top Pitch : 6/12
		Bot pitch : 90/12

==B(d(1/21/2000),v1.1)=====

FORCES - LOAD CASE #1

TOP CHORD: 1-2= 645 , 2-3= 1283, 3-4= 1283, 4-5=2042, 5-6=3106, 6-7=2704, 7-8=3023,
 8-9=3259, 9-10=67, 10-11=21, 11-12=368,
 BOTTOM CHORD: 12-13= 331, 13-14= 0, 14-15= 29, 15-16= 2758, 16-17= 2758, 17-18= 2915, 18-19= 3106,
 19-20= 2039, 20-21= 1591, 21-22=579, 22-1=579,
 WEBS: 2-22= 103, 2-21=633, 3-21=500, 4-21=4628, 4-20= 2978, 5-20=2312, 5-19= 1782,
 6-19=778, 6-18=542, 7-18= 768, 8-18=343, 8-17=32, 9-17= 199, 9-16= 92,
 9-15=3389, 10-15=362, 11-15=312, 11-13=230, 15-13= 452,
 REACTIONS, SIZE: 21=4632, 10.38 15=2771, 8.00 12=437, 4.00
 PROVIDE FOR 10276 LBS UPLIFT AT JOINT 21 (2.22)
 PROVIDE FOR 5039 LBS UPLIFT AT JOINT 15 (1.82)
 PROVIDE FOR 1132 LBS UPLIFT AT JOINT 12 (2.59)
 PROVIDE FOR 710 LBS HORIZ. REACTION AT JOINT 21
 PLATE OFFSETS (X=LEFT, Y=TOP): [j3=3,3], [j4=6.5,2], [j5=3.5,4], [j6=3,5], [j7=4,2.5], [j15=8,2], [j20=4.5,3],



L. HL TO PK: 18-2-5
 LEFT HEIGHT: 0-4-3
 SPAN: 54-0
 RISE: 11-1-12
 R. HL TO PK: 24-1-12
 RIGHT HEIGHT: 0-4-3

LOADING (PSF)		MAX STRESSES		MINIMUM GRADE OF LUMBER	
TOP	30 15	TOP	8-9=0.864	TOP CHORD:	2X4 No.1D 19 SP
BOTT	0 10	BOTT	21-22=0.891	BOT CHORD:	2X6 No.2D 19 SP
		LL.DEFL.@18=0.22 < L/360		WEBS:	2X4 No.3 19 SP

REPETITIVE STRESSES NOT USED

SPACING : 24.0 in. o. c.
 NO. OF MEMBERS = 1

LOADING	STRESS INCREASE	LOADING	PANEL (PLF) / JOINTS (LBS)
LUMBER	PLATE	TYPE	
1	1.33	1.33	UNIFORM 1- 5= 90 5- 6= 240 6-12= 90 12-14= 20 15- 1= 20
			CONCENTRATED 19= 500 20= 500

CHORD: 4-5; 5-6; 6-7 TO BE 2X6 SS 19 SP
 CHORD: 7-8; 8-9 TO BE 2X4 SS 19 SP
 CHORD: 9-10; 10-11; 11-12 TO BE 2X4 No.2ND 19 SP
 CHORD: 14-15 TO BE 2X4 No.3 19 SP
 WEB: 3-21 TO BE 2X6 No.2D 19 SP
 WEB: 4-21 TO BE 2X6 SS 19 SP
 WEB: 4-20; 5-19; 9-15 TO BE 2X4 No.1D 19 SP
 WEB: 5-20 TO BE 2X4 No.2ND 19 SP
 WEB 6-18; 9-15 BRACED at 1/2 POINTS AS SHOWN ABOVE

MAR 01 2001

WEB 4-21; 4-20; 5-20; 5-19 BRACED at 1/3 POINTS AS SHOWN ABOVE

PROVIDE 2*4 CONTINUOUS BRACING AT BOTTOM CHORD NOT EXCEEDING 4' O.C. OVER DROPPED BOTTOM CHORD AREAS WHERE CEILING IS NOT APPLIED.

Note: Use 1x4 or 2x3 Cont. Bracing conn. w/2-8d nails, or T-brace of same size and grade as web conn. to narrow face w/10d nails 6 in. o.c.
TRUSS HAS BEEN CHECKED FOR 145 M.P.H. WIND LOAD, WALL HEIGHT 16 FT, ENCLOSED BLDG. TYPE, ASSUMING 10 PSF TOTAL D.L. (7+3) (SBC-97).
DEFLECTION (IN.) L.L.= 0.22, D.L.=0.18, T.L.=0.40

PLATES ARE MITEK M20-249,200 MANUFACTURED FROM ASTM A 446 GRD A GALVANIZED STEEL (EXCEPT AS SHOWN)

PLATE MUST BE INSTALLED ON EA. FACE OF JOINT, SYMMETRICALLY (EXCEPT AS SHOWN) DESIGN CONFORMS W/NDS DESIGN SPECS, SBC, ANSI/TPI-95

THIS DESIGN IS FOR TRUSS FABRICATION ONLY. FOR PERMANENT AND TEMPORARY BRACING (WHICH IS ALWAYS REQD) CONSULT BLDG ARCHITECT OR ENGINEER.

55

[Signature]
MAR 6 1 2007

====<<<<ACES-32 Ver. 1.1>>>>===== [001861] =====<<<<FL.QUALITY-MMH>>>>=====

Customer : FOGLIA Thu Mar 1 09:54:04 2001
 Project #: 00469 Truss ID : S38A Family #: spec.
 Span : 54-0 B.Span : 54-0 Quant. : 1 Top Pitch : 6/12
 Bot pitch : 90/12

23-24 12/12,

==8ld(1/21/2000),v1.1=====

FORCES - GRAVITY LOADS

TOP CHORD: 1-2= 920, 2-3= 1611, 3-4= 298, 4-5= 129, 5-6= 1223, 6-7= 1704, 7-8= 1710,
 8-9= 2116, 9-10= 2113, 10-11= 480, 11-12= 517, 12-13= 199,
 BOTTOM CHORD: 13-14= 179, 14-15= 0, 15-16= 29, 16-17= 1637, 17-18= 1637, 18-19= 1667, 19-20= 55,
 20-21= 0, 21-22= 813, 22-23= 129, 23-24= 2038, 24-25= 826, 25-1= 826,
 WEBS: 2-25= 122, 2-24= 680, 3-24= 4521, 3-23= 3673, 4-23= 2025, 4-22= 1388, 5-22= 1435,
 5-21= 859, 6-21= 1182, 6-19= 830, 7-19= 293, 8-19= 322, 8-18= 459, 9-18= 436,
 10-18= 289, 10-17= 87, 10-16= 2720, 11-16= 301, 12-16= 643, 12-14= 88, 16-14= 245,
 19-21= 1360,

REACTIONS, SIZE: 24= 3437, 8.00 16= 2250, 8.00 13= 473, 4.00

PROVIDE FOR 5640 LBS UPLIFT AT JOINT 24 (1.64)

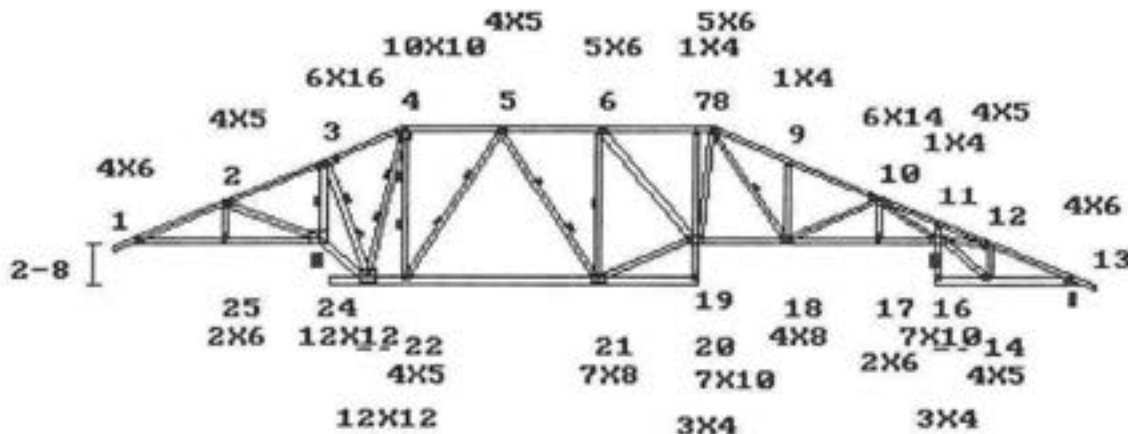
PROVIDE FOR 2229 LBS UPLIFT AT JOINT 16 (0.99)

PROVIDE FOR 607 LBS UPLIFT AT JOINT 13 (1.28)

PROVIDE FOR 680 LBS HORIZ. REACTION AT JOINT 24

PLATE OFFSETS (X=LEFT, Y=TOP): [j4=5,2], [j6=3,3], [j16=5,2], [j19=5,2], [j21=4,2], [j23=6,7], [j24=6,7],

1-05-4-8 15-5-8 26-8-8 33-2-9 42-8-14 54-01-0
 5-4-8 4-8-8 5-7-8 0-10-9 5-3-2 3-05-0
 5-4-8 5-7-8 5-7-8 4-3-2 3-3-2



10-9 15-5-8 42-8-1 49-0
 5-4-8 13-5 26-8-8 32-4 37-5-11 46-0 54-0
 5-4-8 2-8 11-3 5-7-8 5-3-3 3-05-0

L. HL TO PK: 17-3-6

LEFT HEIGHT: 0-4-3

SPAN: 54-0

RISE: 10-8-14

R. HL TO PK: 23-2-14

RIGHT HEIGHT: 0-4-3

LOADING (PSF)

L D

TOP 30 15

BOTT 0 10

MAX STRESSES

TOP 8-9=0.973

BOTT 23-24=0.649

LL.DEFL.@25=0.15 < L/360

MINIMUM GRADE OF LUMBER

TOP CHORD: 2X4 No.2ND 19 SP

BOT CHORD: 2X6 No.2D 19 SP

WEBS : 2X4 No.3 19 SP

STR.INC.: LUMB = 1.33 PLATE = 1.33

REPETITIVE STRESSES USED

SPACING : 24.0 in. o. c.

NO. OF MEMBERS = 1

CHORD: 4-5; 5-6; 6-7; 7-8; 8-9; 9-10 TO BE 2X4 No.1D 19 SP

CHORD: 15-16; 19-20 TO BE 2X4 No.3 19 SP

WEB: 3-24; 3-23 TO BE 2X6 No.2D 19 SP

WEB 3-24; 6-21; 8-18 BRACED at 1/2 POINTS AS SHOWN ABOVE

WEB 3-23; 4-23; 4-22; 5-22; 5-21 BRACED at 1/3 POINTS AS SHOWN ABOVE

Note: Use 1x4 or 2x3 Cent. Bracing conn. w/ 2-8d nails, or T-brace of same size and grade as web conn. to narrow face w/ 10d nails o.c.

TRUSS HAS BEEN CHECKED FOR 145 M.P.H. WIND LOAD, WALL HEIGHT 16 FT, ENCLOSED BLDG. TYPE, ASSUMING 10 PSF TOTAL D.L. (7+3) (SBC-97).

DEFLECTION (IN.) L.L.= 0.15, D.L.= 0.13, T.L.= 0.28

PLATES ARE MITEK M20-249, 200 MANUFACTURED FROM ASTM A 446 GRD A GALVANIZED STEEL (EXCEPT AS SHOWN)

PLATE MUST BE INSTALLED ON EA. FACE OF JOINT, SYMMETRICALLY (EXCEPT AS SHOWN) DESIGN CONFORMS W/ NDS DESIGN SPECS, SBC, ANSI/TPI-95

THIS DESIGN IS FOR TRUSS FABRICATION ONLY. FOR PERMANENT AND TEMPORARY BRACING (WHICH IS ALWAYS REQD) CONSULT BLDG ARCHITECT OR ENGINEER.

MAR 07 2001

====<<<<ACES-32 Ver. 1.1>>>>===== [001862] =====<<<<FL.QUALITY-MMH>>>>=====

Customer : FOGLIA

Project # : 00469

Span : 54-0

B.Span : 54-0

Truss ID : S38

Quant. : 1

Thu Mar 1 09:54:24 2001

Family # : spec.

Top Pitch : 6/12

Bot pitch : 90/12

24-26 12/12,

==Bld(1/21/2000),v1.1=====

FORCES - GRAVITY LOADS

TOP CHORD: 1-2= 964, 2-3= 1582, 3-4= 1598, 4-5= 495, 5-6= 715, 6-7= 2219, 7-8= 2232,
8-9= 2004, 9-10= 1972, 10-11= 854, 11-12= 885, 12-13= 1,
BOTTOM CHORD: 13-14= 1, 14-15= 0, 15-16= 24, 16-17= 1370, 17-18= 1370, 18-19= 1764, 19-20= 1789,
20-21= 68, 21-22= 0, 22-23= 1525, 23-24= 715, 24-25= 1332, 25-26= 1354, 26-27= 865,
27-1= 865,
WEBS: 2-27= 108, 2-26= 631, 3-26= 223, 4-26= 3836, 4-25= 50, 4-24= 2706, 5-24= 2047,
5-23= 1024, 6-23= 1214, 6-22= 310, 6-20= 910, 7-20= 533, 8-20= 676, 8-19= 43,
9-19= 44, 9-18= 322, 10-18= 548, 10-17= 60, 10-16= 2904, 11-16= 259, 12-16= 794,
12-14= 77, 16-14= 2, 20-22= 1589,

REACTIONS, SIZE: 26= 3424, 8.00 16= 2323, 3.93 13= 413, 2.00

PROVIDE FOR 5698 LBS UPLIFT AT JOINT 26 (1.66)

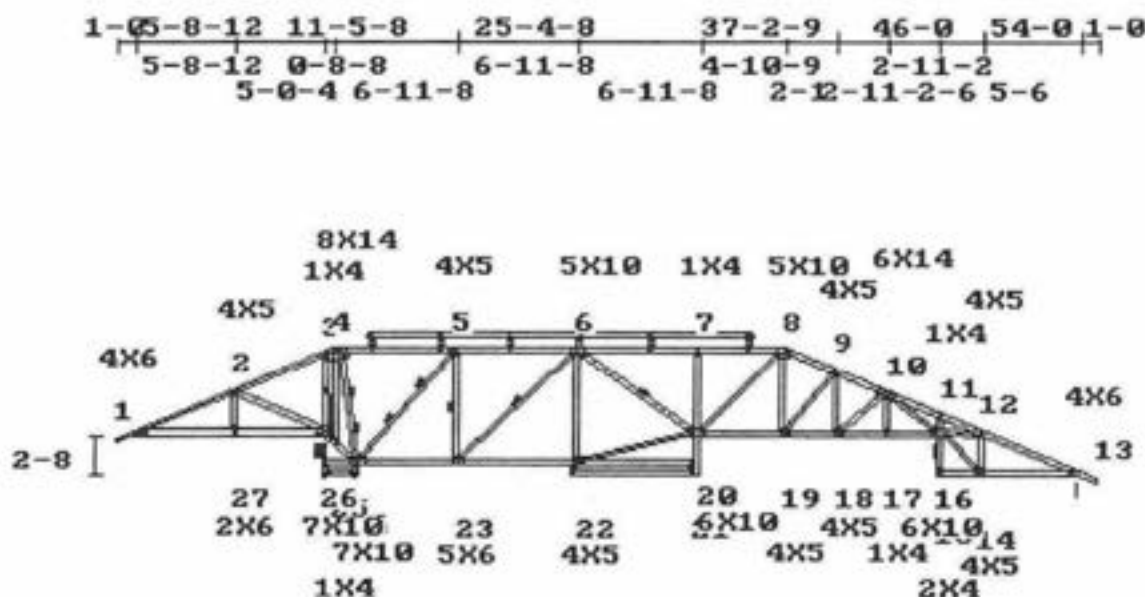
PROVIDE FOR 2296 LBS UPLIFT AT JOINT 16 (0.99)

PROVIDE FOR 574 LBS UPLIFT AT JOINT 13 (1.39)

PROVIDE FOR 625 LBS HORIZ. REACTION AT JOINT 26

PLATE OFFSETS (X=LEFT, Y=TOP): [j4=7,2], [j6=5,3], [j8=5,2], [j23=3,2], [j24=5,4], [j26=5,4],

*** WARNING: NO PLATE AT JOINT(S): j21, REVIEW REQUIRED ***



L. HL TO PK: 12-9-12

LEFT HEIGHT: 0-4-3

SPAN: 54-0

RISE: 8-8-15

R. HL TO PK: 18-9-3

RIGHT HEIGHT: 0-4-3

LOADING (PSF)

L D

TOP 30 15

BOTT 0 10

MAX STRESSES

TOP 6-7=0.915

BOTT 25-26=0.772

LL.DEFL.@27=0.12 < L/360

MINIMUM GRADE OF LUMBER

TOP CHORD: 2X4 No.2ND 19 SP

BOT CHORD: 2X4 No.2ND 19 SP

WEBS : 2X4 No.3 19 SP

STR.INC.: LUMB = 1.33 PLATE = 1.33

REPETITIVE STRESSES USED

SPACING : 24.0 in. o. c.

NO. OF MEMBERS = 1

CHORD: 4-5; 5-6; 6-7; 7-8 TO BE 2X4 SS 19 SP

CHORD: 15-16; 20-21 TO BE 2X4 No.3 19 SP

CHORD: 26-27; 27-1 TO BE 2X6 No.2D 19 SP

WEB: 4-26; 4-24 TO BE 2X4 No.1D 19 SP

WEB 4-26; 5-23; 6-23; 6-20 BRACED at 1/2 POINTS AS SHOWN ABOVE

WEB 4-24; 5-24 BRACED at 1/3 POINTS AS SHOWN ABOVE

PROVIDE 2"X4 CONTINUOUS BRACING AT BOTTOM CHORD NOT EXCEEDING 4' O.C. OVER DROPPED BOTTOM CHORD AREAS WHERE CEILING IS NOT APPLIED.

Note: Use 1x4 or 2x3 Cont. Bracing conn. w/ 2-8d nails, or T-brace of same size and grade as web conn. to narrow face w/ 10d nails 6 in. o.c.

TRUSS HAS BEEN CHECKED FOR 145 M.P.H. WIND LOAD, WALL HEIGHT 16 FT, ENCLOSED BLDG. TYPE, ASSUMING 10 PSF TOTAL D.L. (7+3) (SBC-97).

DEFLECTION (IN.) L.L.= 0.12, D.L.= 0.10, T.L.= 0.23

MAR 01 2001

PLATES ARE MITEK M20-249,200 MANUFACTURED FROM ASTM A 446 GRD A GALVANIZED STEEL(EXCEPT AS SHOWN)
PLATE MUST BE INSTALLED ON EA. FACE OF JOINT, SYMMETRICALLY(EXCEPT AS SHOWN) DESIGN CONFORMS W/NDG DESIGN SPECS, SSBC, ANSI/TPI-95
THIS DESIGN IS FOR TRUSS FABRICATION ONLY. FOR PERMANENT AND TEMPORARY BRACING(WHICH IS ALWAYS REQD)CONSULT BLDG ARCHITECT OR ENGINEER.

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MAR 22 2003

Bot pitch : 90712

PLATES ARE MITEK M20-249,200 MANUFACTURED FROM ASTM A 446 GRD A GALVANIZED STEEL(EXCEPT AS SHOWN)
PLATE MUST BE INSTALLED ON EA. FACE OF JOINT, SYMMETRICALLY(EXCEPT AS SHOWN)DESIGN CONFORMS W/ND S DESIGN SPECS, S58C,ANSI/TPI-95
THIS DESIGN IS FOR TRUSS FABRICATION ONLY.FOR PERMANENT AND TEMPORARY BRACING(WHICH IS ALWAYS REQD)CONSULT BLDG ARCHITECT OR ENGINEER.

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MAR 01 2001

====<<<<ACES-32 Ver. 1.1>>>>===== [001864] =====<<<<FL.QUALITY-MMH>>>>=====

Customer : FOGLIA

Project # : 00469

Span : 45-6-8

B.Span : 45-6-8

Truss ID : S36

Quant. : 1

Thu Mar 1 09:55:40 2001

Family # : spec.

Top Pitch : 67/12

Bot pitch : 90/12

==Bld(1/21/2000),v1.1=====

FORCES - GRAVITY LOADS

FORCES - GRAVITY LOADS				REACTIONS - SIZE			
1-2= 817	7-8=-2618	11-12= 0	17-18= 0	2-21= 95	5-17= 1249	8-13=-553	19=-3432 8.00
2-3= 1395	8-9=-2635	12-13= 1737	18-19= 53	2-20=-580	6-17=-1597	9-13= 1086	11=-1676 2.33
3-4= 1613	9-10=-1941	13-14= 53	19-20=-1246	3-20= 324	6-16= 108	9-12=-335	
4-5= 1623	10-11=-1614	14-15= 0	20-21=-729	3-19=-1277	6-15= 838	10-12= 1799	
5-6=-281		15-16= 1275	21-1=-729	4-19=-301	7-15=-1234	13-15= 1923	
6-7=-1797		16-17= 1275		5-19=-2575	7-13= 1111	19-17= 301	

PROVIDE FOR 4639 LBS UPLIFT AT JOINT 19 (1.35)

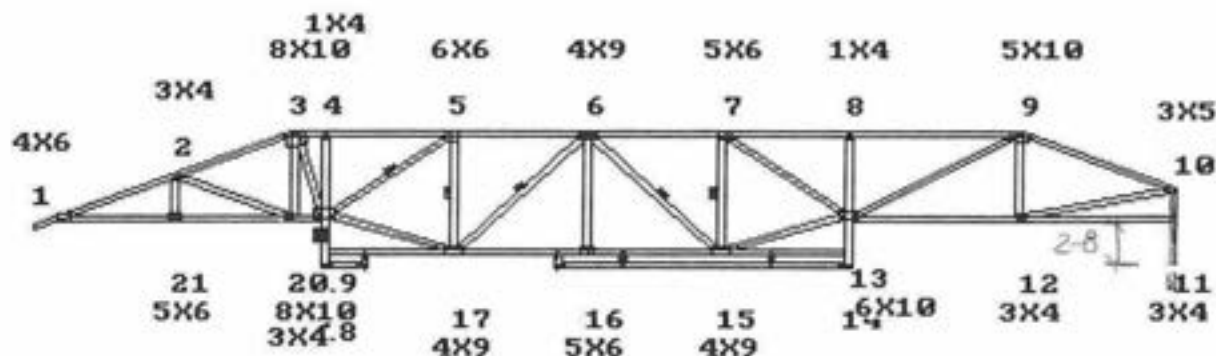
PROVIDE FOR 1312 LBS UPLIFT AT JOINT 11 (0.78)

PROVIDE FOR 301 LBS HORIZ. REACTION AT JOINT 19

PLATE OFFSETS (X=LEFT,Y=TOP):{j3=5,2},{j7=3,3},{j9=5,2},{j16=3,2},{j19=5,5},{j21=3,2},

*** WARNING: NO PLATE AT JOINT(S):j14,j18, REVIEW REQUIRED ***

1-4-8-12 10-9 21-6-8 26-11-4 39-2-9 45-6-8
4-8-12 1-3-8 5-4-12 5-4-12 5-4-12 6-10-9 6-3-15
4-8-12 5-4-12



9-5-8 16-1-12 32-4
4-8-12 10-9 21-6-8 26-11-4 39-2-9 45-6-8
4-8-12 1-3-8 5-4-12 5-4-12 5-4-12 6-10-9 6-3-15

L. HL TO PK:10-6-14

LEFT HEIGHT:0-4-3

SPAN:45-6-8

RISE:7-8-15

R. HL TO PK :7-0-14

RIGHT HEIGHT:1-10-15

LOADING (PSF)

L D

TOP 30 15

BOTT 0 10

MAX STRESSES

TOP 8-9=0.910

BOTT 19-20=0.927

LL.DEFL.@14=0.14 < L/360

MINIMUM GRADE OF LUMBER

TOP CHORD:2X4 No.2ND 19 SP

BOT CHORD:2X4 No.2ND 19 SP

WEBS :2X4 No.3 19 SP

STR.INC.: LUMB = 1.33 PLATE = 1.33

REPETITIVE STRESSES USED

SPACING : 24.0 in. o. c.

NO. OF MEMBERS = 1

CHORD: 3-4; 4-5; 5-6; 6-7; 7-8; 8-9; 19-20; 20-21; 21-1 TO BE 2X4 No.1D 19 SP

CHORD: 9-10 TO BE 2X4 SS 19 SP

CHORD: 13-14; 18-19 TO BE 2X4 No.3 19 SP

RIGHT END VERTICAL IS NOT EXPOSED.

WEB 5-19; 5-17; 6-17; 6-15; 7-15 BRACED AT 1/2 POINTS AS SHOWN ABOVE

Note:Use 1x4 or 2x3 Cont.Bracing conn.w/2-8d nails,or T-brace of same size and grade as web conn. to narrow face w/10d nails 6 in. o.c

TRUSS HAS BEEN CHECKED FOR 145 M.P.H. WIND LOAD, WALL HEIGHT 16 FT, ENCLOSED BLDG. TYPE, ASSUMING 10 PSF TOTAL D.L.(7+3) (SBC-97).

DEFLECTION(IN.) L.L= 0.14,D.L=0.12,T.L=0.26

PLATES ARE MITEK M20-249,200 MANUFACTURED FROM ASTM A 446 GRD A GALVANIZED STEEL(EXCEPT AS SHOWN)

PLATE MUST BE INSTALLED ON EA. FACE OF JOINT,SYMMETRICALLY(EXCEPT AS SHOWN)DESIGN CONFORMS W/ND S DESIGN SPECS, SSBC,ANSI/TPI-95

THIS DESIGN IS FOR TRUSS FABRICATION ONLY.FOR PERMANENT AND TEMPORARY BRACING(WHICH IS ALWAYS REQD)CONSULT BLDG ARCHITECT OR ENGINEER.

MAR 01 2001

====<<<<ACES-32 Ver. 1.1>>>>===== [001865] =====<<<<FL.QUALITY-MMH>>>>=====

Customer : FOGLIA Thu Mar 1 09:55:58 2001

Project #: 00469 Truss ID : S35 Family # : spec.

Span : 45-6 B.Span : 45-6-8 Quant. : 1 Top Pitch : 6/12

Bot pitch : 90/12

==Bld(1/21/2000),v1.1=====

FORCES - GRAVITY LOADS

FORCES - GRAVITY LOADS				REACTIONS - SIZE				
1-2= 804	7-8=3262	11-12= 0	17-18= 0	2-20=363	6-17=1633	8-12=1829	19=3329	8.00
2-3= 1174	8-9=1708	12-13= 3245	18-19= 63	3-20= 257	6-16= 128	9-12= 382	11=1775	2.47
3-4= 1730	9-10=1901	13-14= 63	19-20=1048	3-19=1256	6-15= 582	10-12= 1849		
4-5= 1746	10-11=1733	14-15= 0	20-1=723	4-19=390	7-15=1053	13-15= 2597		
5-6=830		15-16= 2042		5-19=2991	7-13= 921	19-17= 874		
6-7=2474		16-17= 2042		5-17= 945	8-13= 437			

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PROVIDE FOR 4385 LBS UPLIFT AT JOINT 19 (1.32)

PROVIDE FOR 1436 LBS UPLIFT AT JOINT 11 (0.81)

PROVIDE FOR 263 LBS HORIZ. REACTION AT JOINT 19

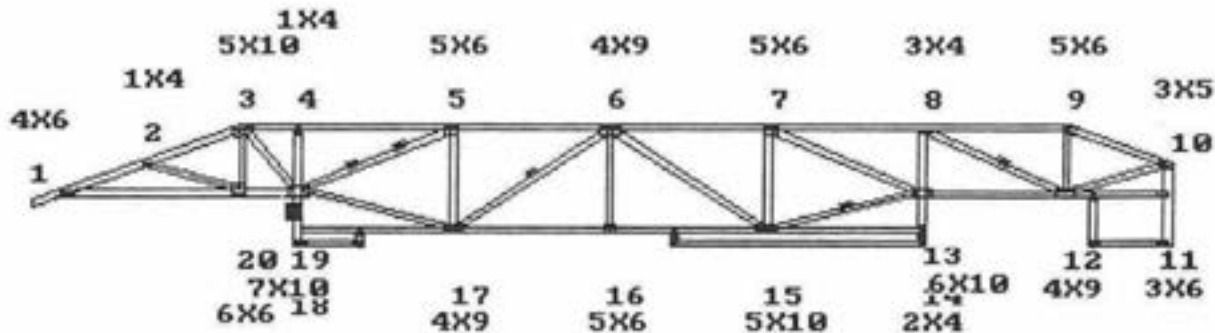
PLATE OFFSETS (X=LEFT,Y=TOP):[j3=5,2],[j5=3,3],[j7=3,3],[j16=3,2],[j19=5,2],

*** WARNING: NO PLATE AT JOINT(S):j18, REVIEW REQUIRED ***

1-03-8-12 9-8-716-1-5 22-6-4 28-11-2 35-4 41-2-9

3-8-12 2-2-15 6-4-14 6-4-14 6-4-14 5-10-9

3-8-12 6-4-14 4-3-7



7-5-8 9-8-716-1-5 22-6-3 28-11-2 35-4 41-2-9

7-5-8 2-2-15 6-4-14 6-4-14 6-4-14 5-10-9

10-2

20-6

45-6

L. HL TO PK:8-4-1 LEFT HEIGHT:0-4-3 SPAN:45-6 RISE:6-8-15 R. HL TO PK:4-9-8 RIGHT HEIGHT:1-11-3

LOADING (PSF)		MAX STRESSES		MINIMUM GRADE OF LUMBER	
L	D	TOP	BOTT	TOP CHORD	BOT CHORD
30	15	6-7=0.941	12-13=0.733	2X4 No.2ND 19 SP	2X4 No.2ND 19 SP
0	10	LL.DEFL.@15=0.17 < L/360		2X4 No.3 19 SP	2X4 No.3 19 SP

STR.INC.: LUMB = 1.33 PLATE = 1.33 SPACING : 24.0 in. o. c.

REPETITIVE STRESSES USED NO. OF MEMBERS = 1

CHORD: 3-4; 4-5; 5-6; 6-7 TO BE 2X4 No.1D 19 SP

CHORD: 7-8; 8-9 TO BE 2X4 SS 19 SP

CHORD: 13-14; 18-19 TO BE 2X4 No.3 19 SP

CHORD: 19-20; 20-1 TO BE 2X6 No.2D 19 SP

WEB: 10-11 TO BE 2X6 No.2D 19 SP

WEB: 5-19 TO BE 2X4 No.2ND 19 SP

RIGHT END VERTICAL IS NOT EXPOSED.

WEB 6-17; 8-12; 13-15 BRACED at 1/2 POINTS AS SHOWN ABOVE

WEB 5-19 BRACED at 1/3 POINTS AS SHOWN ABOVE

PROVIDE 2*4 CONTINUOUS BRACING AT BOTTOM CHORD NOT EXCEEDING 4' O.C. OVER DROPPED BOTTOM CHORD AREAS WHERE CEILING IS NOT APPLIED.

Note:Use 1x4 or 2x3 Cont.Bracing conn.w/2-8d nails,or T-brace of same size and grade as web conn. to narrow face w/10d nails 6 in. o.c

TRUSS HAS BEEN CHECKED FOR 145 M.P.H. WIND LOAD, WALL HEIGHT 16 FT, ENCLOSED BLDG. TYPE, ASSUMING 10 PSF TOTAL D.L.(7*3) (SBC-97).

DEFLECTION(IN.) L.L= 0.17,D.L=0.14,T.L=0.30

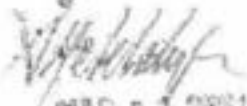
PLATES ARE MITEK M20-249,200 MANUFACTURED FROM ASTM A 446 GRD A GALVANIZED STEEL(EXCEPT AS SHOWN)

PLATE MUST BE INSTALLED ON EA. FACE OF JOINT,SYMMETRICALLY(EXCEPT AS SHOWN)DESIGN CONFORMS W/NOS DESIGN SPECS, SSBC,ANSI/TPI-95

MAR 01 2001

THIS DESIGN IS FOR TRUSS FABRICATION ONLY.FOR PERMANENT AND TEMPORARY BRACING(WHICH IS ALWAYS REQD)CONSULT BLDG ARCHITECT OR ENGINEER.

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MAR 2 1 2001

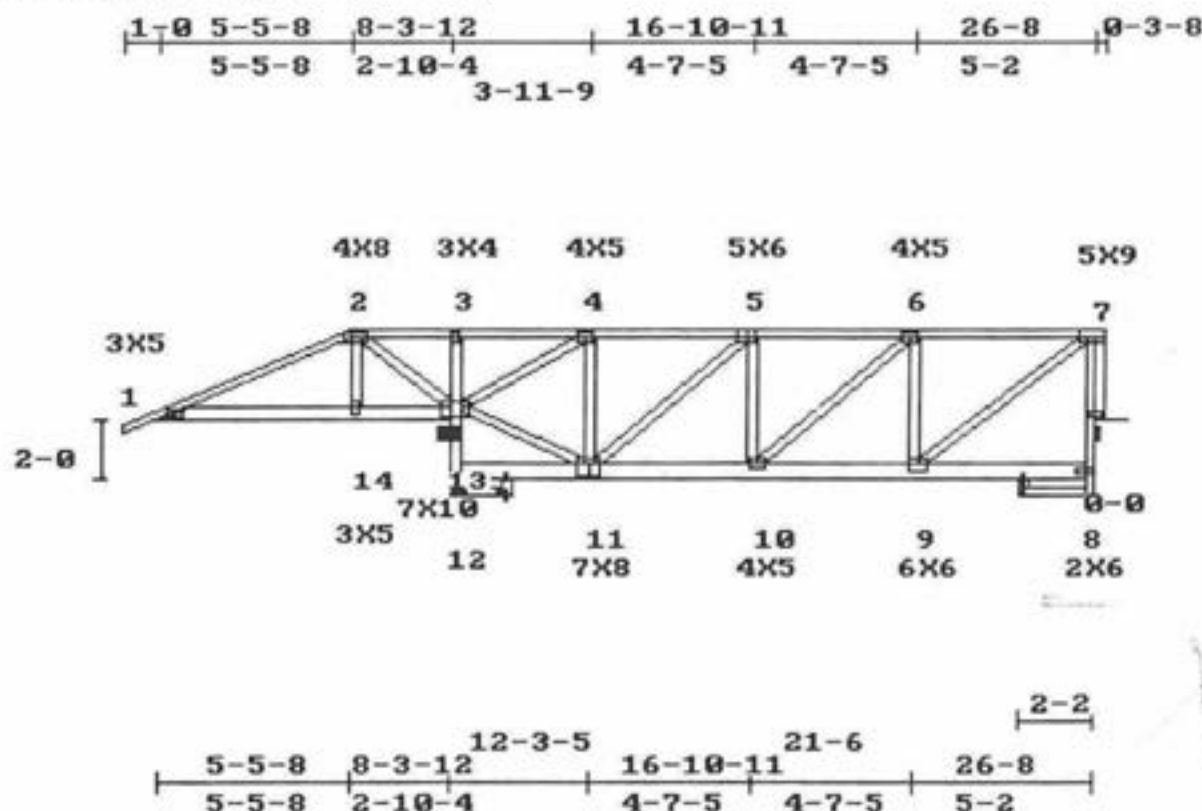
====<<<ACES-32 Ver. 1.1>>>=====[002010]====<<<FL.QUALITY-MMH>>>=====
 Customer : FOGLIA Wed Mar 7 07:20:30 2001
 Project #: 00469 Truss ID : S14 Family # : spec.
 Span : 26-8 B.Span : 27-0 Quant. : 2 Top Pitch : 6/12
 Bot pitch : 90/12

==81d(1/21/2000),v1.1==

FORCES - LOAD CASE #1				REACTIONS - SIZE	
1-2= 924	7-8= 801	8-9= 0	14-1=-828	2-14= 662	5-10= 870
2-3= 2457	9-10= 2135	2-13=-2192	6-10=-702	13=-4240	8.00
3-4= 2476	10-11= 1644	3-13=-359	6-9= 143	7=-2893	2.00
4-5=-316	11-12= 0	4-13=-3421	7-9= 2887		
5-6=-1644	12-13= 153	4-11= 1526	13-11= 357		
6-7=-2135	13-14=-828	5-11=-1896			

(61)

PROVIDE FOR 7051 LBS UPLIFT AT JOINT 13 (1.66)
 PROVIDE FOR 3123 LBS UPLIFT AT JOINT 7 (1.08)
 PROVIDE FOR 317 LBS HORIZ. REACTION AT JOINT 13
 PLATE OFFSETS (X=LEFT,Y=TOP):[J5=3,3],[J7=2.5,3],[J11=4,2],[J13=5,2],[J14=3,1.5],
 *** WARNING: NO PLATE AT JOINT(S):J12, REVIEW REQUIRED ***



L. HL TO PK:6-1-4
 LEFT HEIGHT:0-4-3 SPAN:26-8 RISE:5-8-15 RIGHT HEIGHT:5-0-15

LOADING (PSF)			MAX STRESSES		MINIMUM GRADE OF LUMBER	
TOP	L	D	TOP	6-7=0.501	TOP CHORD:2X4	No.2ND 19 SP
BOTT	0	10	BOTT	8-9=0.355	BOT CHORD:2X6	No.2D 19 SP
			LL.DEFL.@14=0.04 < L/360	WEBS	:2X4	No.3 19 SP

SPACING : 24.0 in. o. c.
 NO. OF MEMBERS = 2

REPETITIVE STRESSES NOT USED

LOADING	STRESS INCREASE	LOADING TYPE	PANEL(PLF) / JOINTS(LBS)
1	1.33	1.33	1- 2= 90 2- 6= 105 6- 7= 45 8- 9= 310 9-12= 80 13-14= 80 14- 1= 20
		UNIFORM	9= 1101 14= 487
		CONCENTRATED	

CHORD: 12-13 TO BE 2X4 No.3 19 SP

RIGHT END VERTICAL IS NOT EXPOSED.

PROVIDE 2"4 CONTINUOUS BRACING AT BOTTOM CHORD NOT EXCEEDING 4' O.C. OVER DROPPED BOTTOM CHORD AREAS WHERE CEILING IS NOT APPLIED.
 2 MEMBERS NAILED TOG. W/1 ROW(S) OF .131x3 in. NAILS 12 in. o.c.(TOP CHS.),AND 1 ROW(S) OF .131x3 in. NAILS 8 in. o.c.(BOTT. CHS.)
 For Webs use 1 ROW OF NAILS 12 in. o.c.

TRUSS HAS BEEN CHECKED FOR 145 M.P.H. WIND LOAD, WALL HEIGHT 16 FT, ENCLOSED BLDG. TYPE, ASSUMING 10 PSF TOTAL D.L.(7+3) (SBC-97).

DEFLECTION(IN.) L.L= 0.04,O.L=0.03,T.L=0.07

PLATES ARE MITEK M20-249,200 MANUFACTURED FROM ASTM A 446 GRD A GALVANIZED STEEL(EXCEPT AS SHOWN)

PLATE MUST BE INSTALLED ON EA. FACE OF JOINT,SYMMETRICALLY(EXCEPT AS SHOWN)DESIGN CONFORMS W/NDS DESIGN SPECS, SSSBC,ANSI/TPI-95

THIS DESIGN IS FOR TRUSS FABRICATION ONLY.FOR PERMANENT AND TEMPORARY BRACING(WHICH IS ALWAYS REQD)CONSULT BLDG ARCHITECT OR ENGINEER.

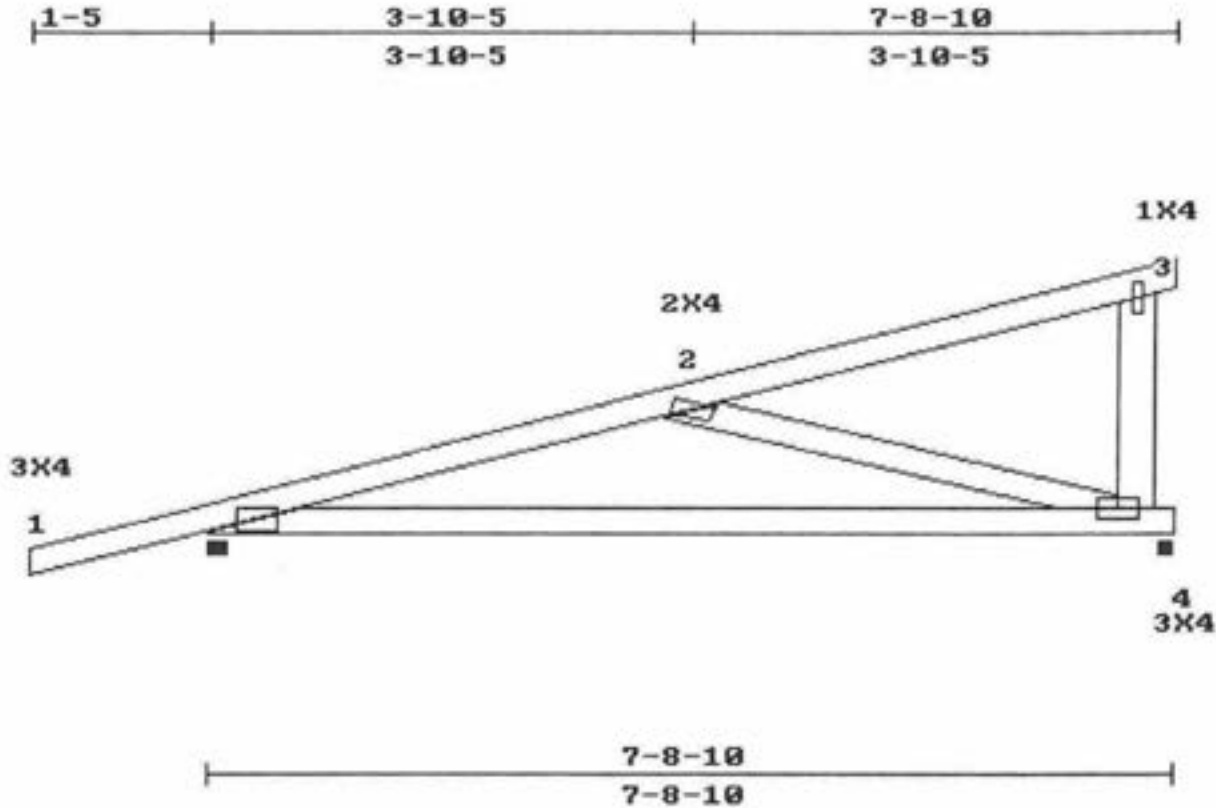
====<<<ACES-32 Ver. 1.1>>>=====[001867]====<<<FL.QUALITY-MMH>>>=====
 Customer : FOGLIA Thu Mar 1 09:57:29 2001
 Project #: 00469 Truss ID : CJ7A Family #: 204
 Span : 7-8-10 B.Span : 5-5-8 Quant. : 1 Top Pitch : 4.243
 ==Bld(1/21/2000),v1.1=====

FORCES - LOAD CASE #1
 1-2=486 4-1=456 2-4=485
 2-3=0
 3-4=247

REACTIONS - SIZE
 1=333 2.00
 4=490 1.50

(62)

PROVIDE FOR 790 LBS UPLIFT AT JOINT 1 (2.38)
 PROVIDE FOR 867 LBS UPLIFT AT JOINT 4 (1.77)
 PROVIDE FOR 318 LBS HORIZ. REACTION AT JOINT 1



L. HL TO PK:8-2-4
 LEFT HEIGHT:0-3-15 SPAN:7-8-10 RISE:3-0-11 RIGHT HEIGHT:3-0-11
 =====
 LOADING (PSF) MAX STRESSES MINIMUM GRADE OF LUMBER
 TOP 30 15 TOP 1-2=0.498 TOP CHORD:2X4 No.2ND 19 SP
 BOTT 0 10 BOTT 4-1=0.382 BOT CHORD:2X4 No.2ND 19 SP
 LL.DEFL.@0=0.00 < L/360 WEBS :2X4 No.3 19 SP
 =====

REPETITIVE STRESSES NOT USED

SPACING : 24.0 in. o. c.
 NO. OF MEMBERS = 1

LOADING	STRESS INCREASE	LOADING	PANEL(PLF) / JOINTS(LBS)
LUMBER	PLATE	TYPE	
1	1.33	1.33	UNIFORM 1- 2= 47 2- 3= 130 4- 1= 20

RIGHT END VERTICAL IS NOT EXPOSED.
 TRUSS HAS BEEN CHECKED FOR 145 M.P.H. WIND LOAD, WALL HEIGHT 16 FT, ENCLOSED BLDG. TYPE, ASSUMING 10 PSF TOTAL D.L.(7+3) (SBC-97).
 DEFLECTION(IN.) L.L= 0.00,D.L=0.00,T.L=0.00
 PLATES ARE MITEK M20-249,200 MANUFACTURED FROM ASTM A 446 GRD A GALVANIZED STEEL(EXCEPT AS SHOWN)
 PLATE MUST BE INSTALLED ON EA. FACE OF JOINT,SYMMETRICALLY(EXCEPT AS SHOWN)DESIGN CONFORMS W/NDS DESIGN SPECS, SBC,ANSI/TPI-95
 THIS DESIGN IS FOR TRUSS FABRICATION ONLY.FOR PERMANENT AND TEMPORARY BRACING(WHICH IS ALWAYS REQD)CONSULT BLDG ARCHITECT OR ENGINEER.

MAR 01 2001

====<<<<ACES-32 Ver. 1.1>>>>===== [001868] =====<<<<FL.QUALITY-MMH>>>>=====

Customer : FOGLIA

Project #: 00469

Span : 9-0

B.Span : 9-0

Truss ID : NG1

Quant. : 2

Thu Mar 1 09:57:50 2001

Family # : 301

Top Pitch : /12

==Bld(1/21/2000),v1.1=====

FORCES - LOAD CASE #1

REACTIONS - SIZE

1-2=246	7-8= 0	3-1=1954	6-8= 1954
2-3= 0	8-9= 1772	3-10= 338	
3-4=1772	9-10= 2341	4-10=631	
4-5=2341	10-1= 1772	4-9=207	
5-6=1772		5-9= 631	
6-7=1071		5-8=758	

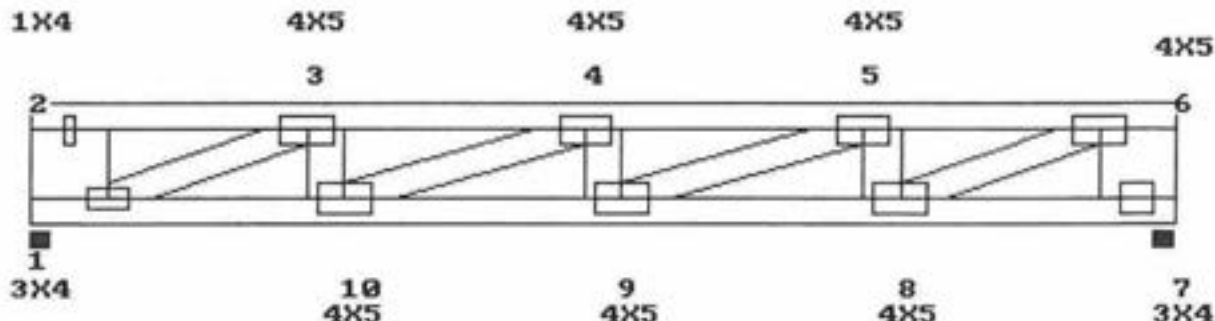
1=1104	2.00
7=1104	2.00

(3)

PROVIDE FOR 1425 LBS UPLIFT AT JOINT 1 (1.29)

PROVIDE FOR 1425 LBS UPLIFT AT JOINT 7 (1.29)

2-3-14	4-6	6-8-2	9-0
2-3-14	2-2-2	2-2-2	2-3-14



2-3-14	4-6	6-8-2	9-0
2-3-14	2-2-2	2-2-2	2-3-14

LEFT HEIGHT:1-4

SPAN:9-0

RISE:1-4

RIGHT HEIGHT:1-4

LOADING (PSF)

MAX STRESSES

MINIMUM GRADE OF LUMBER

TOP 30 15

TOP 4-5=0.325

TOP CHORD:2X4 No.2ND 19 SP

BOTT 0 10

BOTT 9-10=0.236

BOT CHORD:2X4 No.2ND 19 SP

LL.DEFL.@9=0.03 < L/360

WEBS :2X4 No.3 19 SP

REPETITIVE STRESSES NOT USED

SPACING : 24.0 in. o. c.
NO. OF MEMBERS = 2

LOADING	STRESS INCREASE	LOADING	PANEL(PLF) / JOINTS(LBS)
LUMBER	PLATE	TYPE	
1	1.33	1.33	UNIFORM 2- 6= 220 7- 1= 30

WEB: 1-2; 6-7 TO BE 2X8 SS 19 SP

BOTH END VERTICALS ARE NOT EXPOSED.

2 MEMBERS NAILED TOG. W/1 ROW(S) OF .131x3 in. NAILS 12 in. o.c.(TOP CHS.),AND 1 ROW(S) OF .131x3 in. NAILS 12 in. o.c.(BOTT. CHS.)

For Webs use 1 ROW of NAILS 12 in. o.c.

TRUSS HAS BEEN CHECKED FOR 145 M.P.H. WIND LOAD, WALL HEIGHT 16 FT, ENCLOSED BLDG. TYPE, ASSUMING 10 PSF TOTAL D.L.(7+3) (SBC-97).

DEFLECTION(IN.) L.L= 0.03,D.L=0.03,T.L=0.06

PLATES ARE MITEK M20-249,200 MANUFACTURED FROM ASTM A 446 GRD A GALVANIZED STEEL(EXCEPT AS SHOWN)

PLATE MUST BE INSTALLED ON EA. FACE OF JOINT,SYMMETRICALLY(EXCEPT AS SHOWN)DESIGN CONFORMS W/NDIS DESIGN SPECS, SSB,ANSI/TP1-95

THIS DESIGN IS FOR TRUSS FABRICATION ONLY.FOR PERMANENT AND TEMPORARY BRACING(WHICH IS ALWAYS REQD)CONSULT BLDG ARCHITECT OR ENGINEER.

MAR 01 2001

====<<<<ACES-32 Ver. 1.1>>>>=====[001869]====<<<<FL.QUALITY-MMH>>>>=====

Customer : FOGLIA

Project #: 00469

Span : 5-5-8

Truss ID : CE5

Quantity : 1

Thu Mar 1 09:58:08 2001

Family #: 205

Top Pitch : 6/12

==Bld(1/21/2000),v1.1=====

FORCES - GRAVITY LOADS

1-2= 0 3-1= 0
2-3= 0

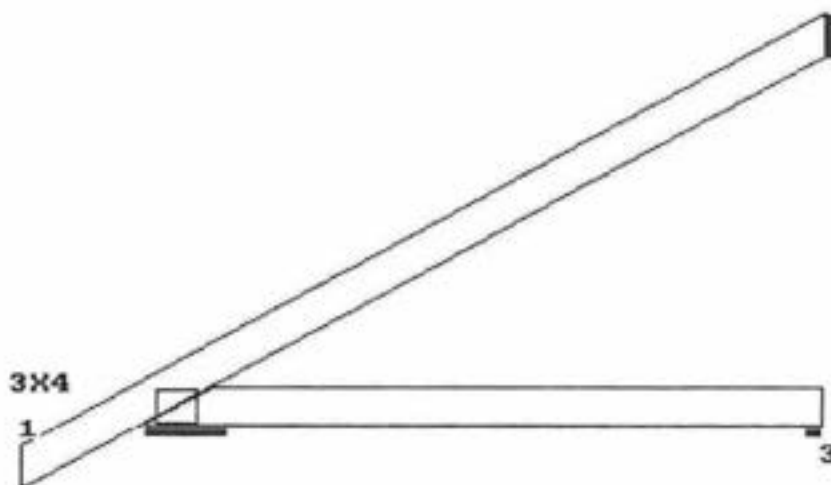
REACTIONS - SIZE

1--388 8.00
3--51 1.50
2--228 1.50

(64)

PROVIDE FOR 289 LBS UPLIFT AT JOINT 1 (0.74)
PROVIDE FOR 347 LBS UPLIFT AT JOINT 2 (1.52)
PROVIDE FOR 316 LBS HORIZ. REACTION AT JOINT 1
(SYMMETRIC PLATING TURNED ON)

1-0 5-5-8
5-5-8



5-5-8
5-5-8

L. HL TO PK:6-1-4
LEFT HEIGHT:0-4-3

SPAN:5-5-8

RISE:3-0-15

RIGHT HEIGHT:3-0-15

LOADING (PSF)			MAX STRESSES		MINIMUM GRADE OF LUMBER	
TOP	30	15	TOP	1-2=0.852	TOP CHORD:2X4	No.2ND 19 SP
BOTT	0	10	BOTT	3-1=0.125	BOT CHORD:2X4	No.2ND 19 SP
			LL.DEFL. < L/360		WEBS	:2X4 No.3 19 SP

STR.INC.: LUMB = 1.33 PLATE = 1.33
REPETITIVE STRESSES USED

SPACING : 24.0 in. o. c.
NO. OF MEMBERS = 1

TRUSS HAS BEEN CHECKED FOR 145 M.P.H. WIND LOAD, WALL HEIGHT 16 FT, ENCLOSED BLDG. TYPE, ASSUMING 10 PSF TOTAL D.L.(7+3) (SBC-97).
DEFLECTION(IN.) L.L= 0.00,D.L=0.00,T.L=0.00

PLATES ARE MITEK M20-249,200 MANUFACTURED FROM ASTM A 446 GRD A GALVANIZED STEEL(EXCEPT AS SHOWN)

PLATE MUST BE INSTALLED ON EA. FACE OF JOINT,SYMMETRICALLY(EXCEPT AS SHOWN)DESIGN CONFORMS W/NDS DESIGN SPECS, SBC,ANSI/TPI-95

THIS DESIGN IS FOR TRUSS FABRICATION ONLY.FOR PERMANENT AND TEMPORARY BRACING(WHICH IS ALWAYS REQD)CONSULT BLDG ARCHITECT OR ENGINEER.

MAR 01 2001

====<<<ACES-32 Ver. 1.1>>>===== [001870] =====<<<FL.QUALITY-MMH>>>=====

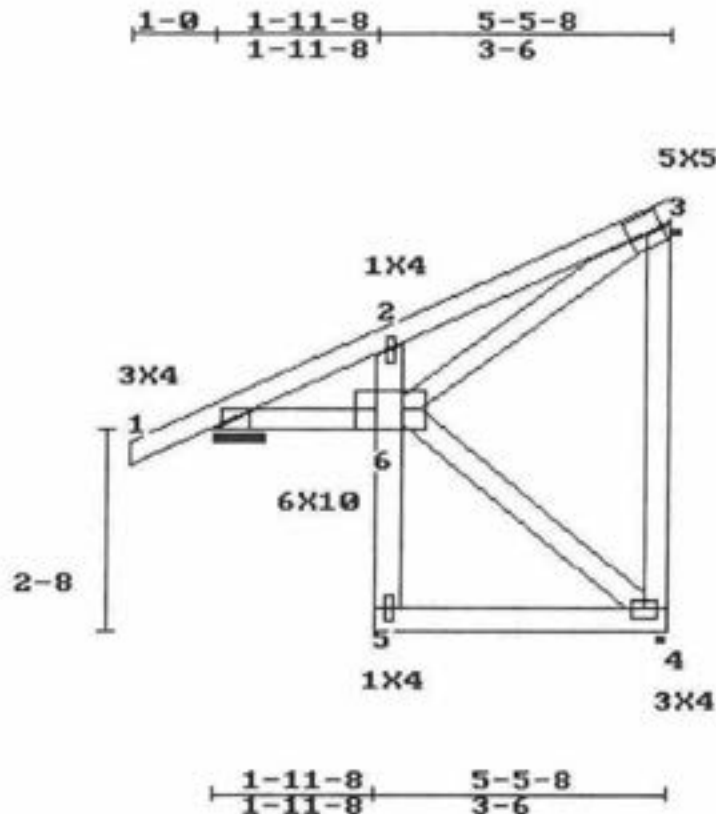
Customer : FOGLIA Truss ID : CF5 Thu Mar 1 09:58:21 2001
 Project #: 00469 Quantity : 1 Family #: special
 Span : 5-5-8 Top Pitch : 6/12
 Bot pitch : 90/12

==Bld(1/21/2000),v1.1=====

FORCES - GRAVITY LOADS			REACTIONS - SIZE	
1-2=353	4-5= 0	2-6=176	1=388	8.00
2-3=305	5-6= 33	3-6= 354	4=33	1.50
3-4= 0	6-1= 299	6-4= 0	3=246	1.50

(5)

PROVIDE FOR 289 LBS UPLIFT AT JOINT 1 (0.74)
 PROVIDE FOR 341 LBS UPLIFT AT JOINT 3 (1.39)
 PROVIDE FOR 317 LBS HORIZ. REACTION AT JOINT 1
 PLATE OFFSETS (X=LEFT,Y=TOP):[j3=2.5,3],(SYMMETRIC PLATING TURNED ON)



L. HL TO PK:6-1-4		SPAN:5-5-8		RISE:5-8-15		RIGHT HEIGHT:5-8-15	
LEFT HEIGHT:0-4-3							
LOADING (PSF)		MAX STRESSES		MINIMUM GRADE OF LUMBER			
TOP	30 15	TOP	2-3=0.364	TOP CHORD:	2X4	No.2ND	19 SP
BOTT	0 10	BOTT	6-1=0.068	BOT CHORD:	2X4	No.2ND	19 SP
		LL.DEFL.@6=0.00	< L/360	WEBS	:2X4	No.3	19 SP
STR.INC.: LUMB = 1.33 PLATE = 1.33				SPACING : 24.0 in. o. c.			
REPETITIVE STRESSES USED				NO. OF MEMBERS = 1			

CHORD: 5-6 TO BE 2X4 No.3 19 SP
 RIGHT END VERTICAL IS NOT EXPOSED.
 TRUSS HAS BEEN CHECKED FOR 145 M.P.H. WIND LOAD, WALL HEIGHT 16 FT, ENCLOSED BLDG. TYPE, ASSUMING 10 PSF TOTAL D.L.(7+3) (SBC-97).
 DEFLECTION(IN.) L.L= 0.00,D.L=0.00,T.L=0.01
 PLATES ARE MITEK M20-249,200 MANUFACTURED FROM ASTM A 446 GRD A GALVANIZED STEEL(EXCEPT AS SHOWN)
 PLATE MUST BE INSTALLED ON EA. FACE OF JOINT,SYMMETRICALLY(EXCEPT AS SHOWN)DESIGN CONFORMS W/NDS DESIGN SPECS, S58C,ANSI/TPI-95
 THIS DESIGN IS FOR TRUSS FABRICATION ONLY.FOR PERMANENT AND TEMPORARY BRACING(WHICH IS ALWAYS REQD)CONSULT BLDG ARCHITECT OR ENGINEER.

MAR 01 2001

====<<<ACES-32 Ver. 1.1>>>===== [001871] =====<<<FL.QUALITY-MMH>>>=====

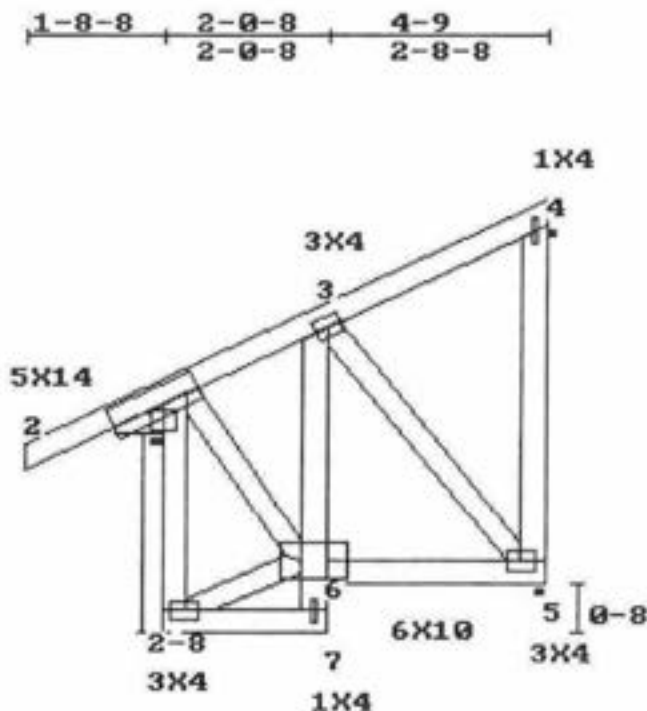
Customer : FOGLIA Truss ID : S21 Thu Mar 1 09:58:41 2001
 Project # : 00469 Quantity : 3 Family # : special
 Span : 4-9 Top Pitch : 6/12
 Bot pitch : 90/12

====Bld(1/21/2000),v1.1=====

FORCES - GRAVITY LOADS			REACTIONS - SIZE	
1-2= 19	5-6= 86	2-6= 137	2=-446	2.00
2-3=-92	6-7= 19	3-6=-62	5=-139	1.50
3-4= 0	7-1= 0	3-5=-140	4=-119	1.50
4-5= 0		6-1= 0		

(60)

PROVIDE FOR 382 LBS UPLIFT AT JOINT 2 (0.86)
 PROVIDE FOR 118 LBS UPLIFT AT JOINT 5 (0.85)
 PROVIDE FOR 174 LBS UPLIFT AT JOINT 4 (1.46)
 PROVIDE FOR 262 LBS HORIZ. REACTION AT JOINT 2
 PLATE OFFSETS (X=LEFT,Y=TOP): [2=12,3], (SYMMETRIC PLATING TURNED ON)



2-0-8 4-9
2-0-8 2-8-8

L. HL TO PK:5-3-12 INTERNAL RISE:0-8
 LEFT HEIGHT:3-4-7 SPAN:4-9 RISE:5-8-15 RIGHT HEIGHT:5-0-15

LOADING (PSF)		MAX STRESSES		MINIMUM GRADE OF LUMBER	
L	D	TOP	BOTT	TOP CHORD:	No.2ND 19 SP
30	15	2-3=0.153	5-6=0.051	2X4	No.2ND 19 SP
0	10	LL.DEFL.@7=0.00 < L/360		WEBS	:2X4 No.3 19 SP

STR.INC.: LUMB = 1.33 PLATE = 1.33 SPACING : 24.0 in. o. c.
 REPETITIVE STRESSES USED NO. OF MEMBERS = 1

CHORD: 6-7 TO BE 2X4 No.3 19 SP
 BOTH END VERTICALS ARE NOT EXPOSED.
 TRUSS HAS BEEN CHECKED FOR 145 M.P.H. WIND LOAD, WALL HEIGHT 16 FT, ENCLOSED BLDG. TYPE, ASSUMING 10 PSF TOTAL D.L.(7+3) (SBC-97).
 DEFLECTION(IN.) L.L= 0.00,D.L=0.00,T.L=0.00
 PLATES ARE MITEK M20-249,200 MANUFACTURED FROM ASTM A 446 GRD A GALVANIZED STEEL(EXCEPT AS SHOWN)
 PLATE MUST BE INSTALLED ON EA. FACE OF JOINT,SYMMETRICALLY(EXCEPT AS SHOWN)DESIGN CONFORMS W/NDS DESIGN SPECS, SBC,ANSI/TP1-95
 THIS DESIGN IS FOR TRUSS FABRICATION ONLY.FOR PERMANENT AND TEMPORARY BRACING(WHICH IS ALWAYS REQD)CONSULT BLDG ARCHITECT OR ENGINEER.

[Signature]
 MAR 01 2001

====<<<<ACES-32 Ver. 1.1>>>>=====[001872]====<<<<FL.QUALITY-MMH>>>>=====
 Customer : FOGLIA Thu Mar 1 09:58:51 2001
 Project #: 00469 Truss ID : S21A Family # : special
 Span : 5-2 Top Pitch : 6/12
 Bot pitch : 90/12

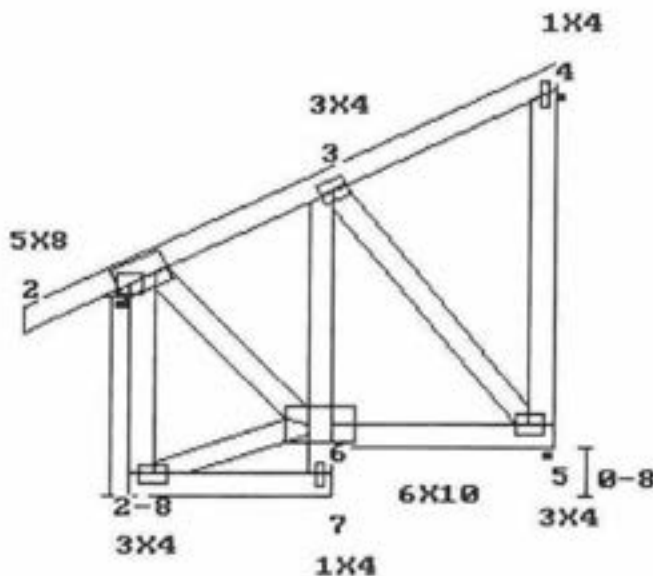
==Bld(1/21/2000),v1.1=====

FORCES - GRAVITY LOADS			REACTIONS - SIZE	
1-2= 23	5-6= 104	2-6= 142	2=-423	2.00
2-3=-112	6-7= 23	3-6=-48	5=-162	1.50
3-4= 0	7-1= 0	3-5=-169	4=-119	1.50
4-5= 0		6-1= 0		

(2)

PROVIDE FOR 343 LBS UPLIFT AT JOINT 2 (0.81)
 PROVIDE FOR 144 LBS UPLIFT AT JOINT 5 (0.89)
 PROVIDE FOR 174 LBS UPLIFT AT JOINT 4 (1.46)
 PROVIDE FOR 286 LBS HORIZ. REACTION AT JOINT 2
 PLATE OFFSETS (X=LEFT,Y=TOP):[J2=6,3],(SYMMETRIC PLATING TURNED ON)

1-3-8 2-5-8 5-2
 2-5-8 2-8-8



2-5-8 5-2
 2-5-8 2-8-8

L. HL TO PK:5-9-5 INTERNAL RISE:0-8
 LEFT HEIGHT:3-1-15 SPAN:5-2 RISE:5-8-15 RIGHT HEIGHT:5-0-15

LOADING (PSF)		MAX STRESSES		MINIMUM GRADE OF LUMBER	
TOP	30	TOP	2-3=0.171	TOP CHORD:2X4	No.2ND 19 SP
BOTT	0	BOTT	5-6=0.054	BOT CHORD:2X4	No.2ND 19 SP
		LL.DEFL.@7=0.00	< L/360	WEBS	:2X4 No.3 19 SP

STR.INC.: LUMB = 1.33 PLATE = 1.33
 REPETITIVE STRESSES USED SPACING : 24.0 in. o. c.
 NO. OF MEMBERS = 1

CHORD: 6-7 TO BE 2X4 No.3 19 SP
 BOTH END VERTICALS ARE NOT EXPOSED.
 TRUSS HAS BEEN CHECKED FOR 145 M.P.H. WIND LOAD, WALL HEIGHT 16 FT, ENCLOSED BLDG. TYPE, ASSUMING 10 PSF TOTAL D.L.(7+3) (SBC-97).
 DEFLECTION(IN.) L.L= 0.00,D.L=0.00,T.L=0.00
 PLATES ARE MITEK M20-249,200 MANUFACTURED FROM ASTM A 446 GRD A GALVANIZED STEEL(EXCEPT AS SHOWN)
 PLATE MUST BE INSTALLED ON EA. FACE OF JOINT,SYMMETRICALLY(EXCEPT AS SHOWN)DESIGN CONFORMS W/NDS DESIGN SPECS, SBC,ANSI/TPI-95
 THIS DESIGN IS FOR TRUSS FABRICATION ONLY.FOR PERMANENT AND TEMPORARY BRACING(WHICH IS ALWAYS REQD)CONSULT BLDG ARCHITECT OR ENGINEER.

[Signature]
 MAR 01 2001


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Thu Mar 1 09:59:02 2001
Family # : special
Top Pitch : 6/12
Bot pitch : 90/12

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FORCES - GRAVITY LOADS

REACTIONS - SIZE

2#-313	2.00
5#-162	1.50
4#-119	1.50

PLATE OFFSETS (X=LEFT,Y=TOP): [J2=6.3], (SYMMETRIC PLATING TURNED ON)



LOADING (PSF)		MAX STRESSES		MINIMUM GRADE OF LUMBER	
		TOP	2-3=0.171	TOP CHORD:2X4	No.2ND 19 SP
TOP	30 15	BOTT	5-6=0.054	BOT CHORD:2X4	No.2ND 19 SP
BOTT	0 10	LL.DEFL.@7=0.00 < L/360		WEBS :2X4	No.3 19 SP
STR.INC.: LUMB = 1.33 PLATE = 1.33				SPACING : 24.0 in. o. c.	
REPETITIVE STRESSES USED				NO. OF MEMBERS = 1	

CHORD: 6-7 TO BE 2X4 No.3 19 SP
BOTH END VERTICALS ARE NOT EXPOSED.
TRUSS HAS BEEN CHECKED FOR 145 M.P.H. WIND LOAD, WALL HEIGHT 16 FT, ENCLOSED BLDG. TYPE, ASSUMING 10 PSF TOTAL D.L.(7+3) (SBC-97).
DEFLECTION(IN.) L.L= 0.00,D.L=0.00,T.L=0.00
PLATES ARE MITEK M20-249,200 MANUFACTURED FROM ASTM A 446 GRD A GALVANIZED STEEL(EXCEPT AS SHOWN)
PLATE MUST BE INSTALLED ON EA. FACE OF JOINT,SYMMETRICALLY(EXCEPT AS SHOWN)DESIGN CONFORMS W/NDIS DESIGN SPECS, S58C,ANSI/TP1-95
THIS DESIGN IS FOR TRUSS FABRICATION ONLY.FOR PERMANENT AND TEMPORARY BRACING(WHICH IS ALWAYS REQ)CONSULT BLDG ARCHITECT OR ENGINEER.

MAR 01 2001

====<<<<ACES-32 Ver. 1.1>>>>===== [002013] =====<<<<FL.QUALITY-MMH>>>>=====

Customer : FOGLIA Wed Mar 7 14:53:43 2001

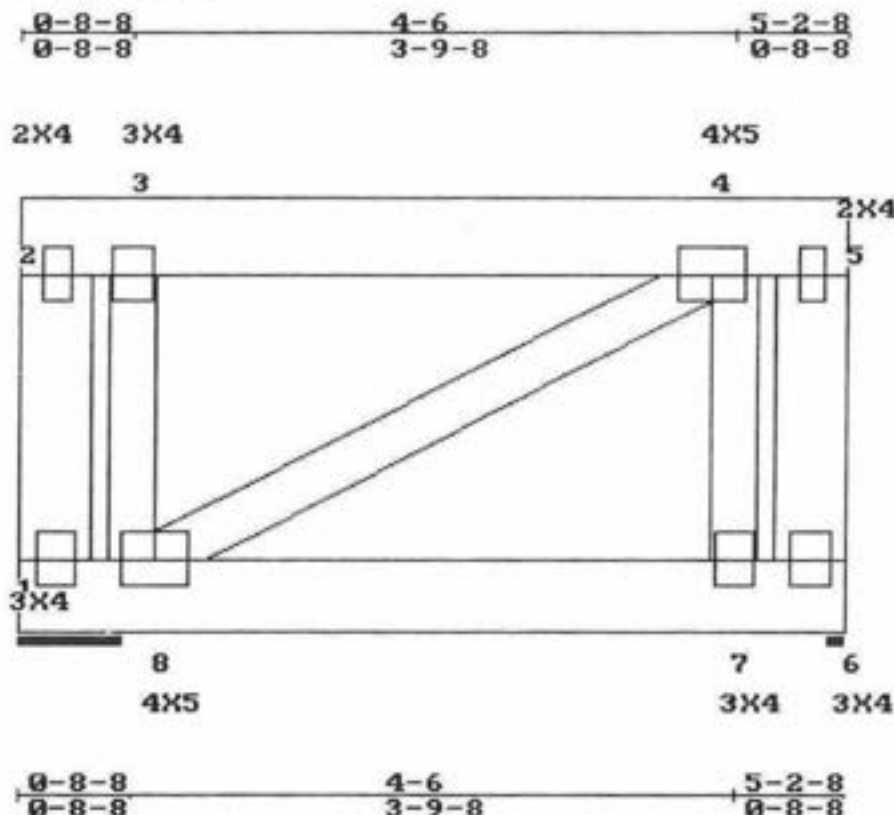
Project #: 00469 Truss ID : FG1 Family #: 301

Span : 5-2-8 B.Span : 5-10-8 Quant. : 2 Top Pitch : /12

==pld(1/21/2000),v1.1=====

REACTIONS - SIZE
1--270 8.00
6--270 1.50

PROVIDE FOR 240 LBS UPLIFT AT JOINT 1 (0.89)
PROVIDE FOR 240 LBS UPLIFT AT JOINT 6 (0.89)



LEFT HEIGHT:2-8 SPAN:5-2-8 RISE:2-8 RIGHT HEIGHT:2-8

LOADING (PSF)		MAX STRESSES		MINIMUM GRADE OF LUMBER	
L	D	TOP	BOTT	TOP CHORD	BOT CHORD
30	15	3-4=0.080	7-8=0.057	2X6 No.2D 19 SP	2X6 No.2D 19 SP
0	10	LL.DEFL.@7=0.01 < L/360		WEBS :2X4	No.3 19 SP

REPETITIVE STRESSES NOT USED

SPACING : 24.0 in. o. c.
NO. OF MEMBERS = 2

LOADING STRESS INCREASE	LOADING LUMBER PLATE TYPE	PANEL(PLF) / JOINTS(LBS)
1 1.33 1.33	UNIFORM	2- 5= 90 6- 1= 20

WEB: 1-2; 5-6 TO BE 2X6 No.2D 19 SP
BOTH END VERTICALS ARE NOT EXPOSED.
2 MEMBERS NAILED TOG. W/1 ROW(S) OF .131x3 in. NAILS 12 in. o.c.(TOP CHS.),AND 1 ROW(S) OF .131x3 in. NAILS 12 in. o.c(BOTT. CHS.)
For Webs use 1 ROW of NAILS 12 in. o.c.
TRUSS HAS BEEN CHECKED FOR 145 M.P.H. WIND LOAD, WALL HEIGHT 16 FT, ENCLOSED BLDG. TYPE, ASSUMING 10 PSF TOTAL D.L.(7+3) (SBC-97).
DEFLECTION(IN.) L.L= 0.01,D.L=0.00,T.L=0.01
PLATES ARE MITEK M20-249,200 MANUFACTURED FROM ASTM A 446 GRD A GALVANIZED STEEL(EXCEPT AS SHOWN)
PLATE MUST BE INSTALLED ON EA. FACE OF JOINT,SYMMETRICALLY(EXCEPT AS SHOWN)DESIGN CONFORMS W/NDS DESIGN SPECS, SIBC,NDS,ANSI/TPI-95
THIS DESIGN IS FOR TRUSS FABRICATION ONLY.FOR PERMANENT AND TEMPORARY BRACING(WHICH IS ALWAYS REQD)CONSULT BLDG ARCHITECT OR ENGINEER.

MAR 07 2001

====<<<<ACES-32 Ver. 1.1>>>>=====[001877]=====<<<<FL.QUALITY-MMH>>>>=====

Customer : FOGLIA

Thu Mar 1 10:00:34 2001

Project #: 00469

Truss ID : CJ7

Family # : 204

Span : 7-6-8

B.Span : 5-4

Quant. : 1

Top Pitch : 4.243

==8(d(1/21/2000),v1.1=====

FORCES - LOAD CASE #1

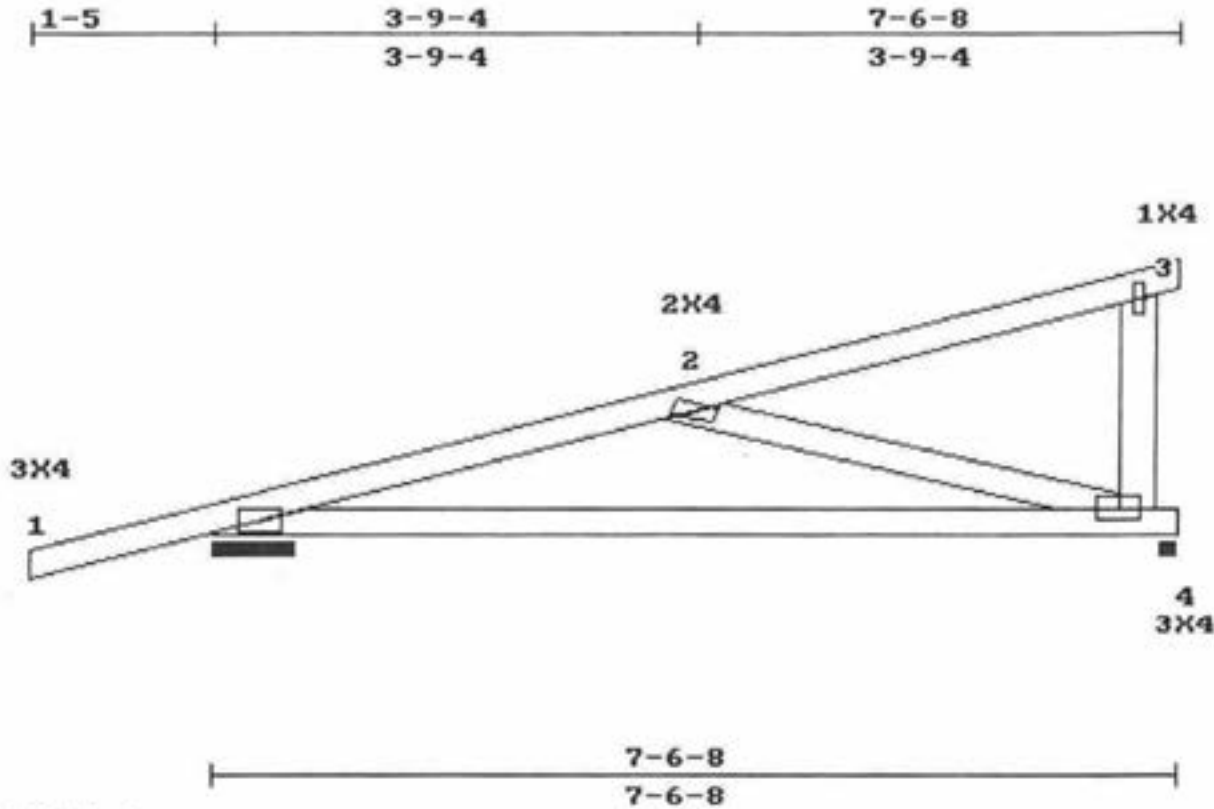
REACTIONS - SIZE

1-2=442 4-1= 411 2-4=438
2-3= 0
3-4=-235

1=468 8.00
4=456 1.50

70

PROVIDE FOR 721 LBS UPLIFT AT JOINT 1 (1.54)
PROVIDE FOR 806 LBS UPLIFT AT JOINT 4 (1.77)
PROVIDE FOR 311 LBS HORIZ. REACTION AT JOINT 1



L. HL TO PK:8-0
LEFT HEIGHT:0-3-15 SPAN:7-6-8 RISE:2-11-15 RIGHT HEIGHT:2-11-15

LOADING (PSF)		MAX STRESSES		MINIMUM GRADE OF LUMBER	
TOP	30	TOP	1-2=0.462	TOP CHORD:2X4	No.2ND 19 SP
BOTT	0	BOTT	4-1=0.353	BOT CHORD:2X4	No.2ND 19 SP
		LL.DEFL.@0=0.00	< L/360	WEBS	:2X4 No.3 19 SP

REPETITIVE STRESSES NOT USED

SPACING : 24.0 in. o. c.
NO. OF MEMBERS = 1

LOADING	STRESS INCREASE	LOADING	PANEL(PLF) / JOINTS(LBS)
LUMBER	1.33	PLATE	1- 2= 46 2- 3= 127 4- 1= 20
TYPE	UNIFORM		

RIGHT END VERTICAL IS NOT EXPOSED.

TRUSS HAS BEEN CHECKED FOR 145 M.P.H. WIND LOAD, WALL HEIGHT 16 FT, ENCLOSED BLDG. TYPE, ASSUMING 10 PSF TOTAL D.L.(7+3) (SBC-97).
DEFLECTION(IN.) L.L= 0.00,D.L=0.00,T.L=0.00

PLATES ARE MITEK M20-249,200 MANUFACTURED FROM ASTM A 446 GRD A GALVANIZED STEEL(EXCEPT AS SHOWN)

PLATE MUST BE INSTALLED ON EA. FACE OF JOINT,SYMMETRICALLY(EXCEPT AS SHOWN)DESIGN CONFORMS W/NDG DESIGN SPECS, SIBC,ANSI/TP1-95

THIS DESIGN IS FOR TRUSS FABRICATION ONLY.FOR PERMANENT AND TEMPORARY BRACING(WHICH IS ALWAYS REQD)CONSULT BLDG ARCHITECT OR ENGINEER.

[Handwritten Signature]

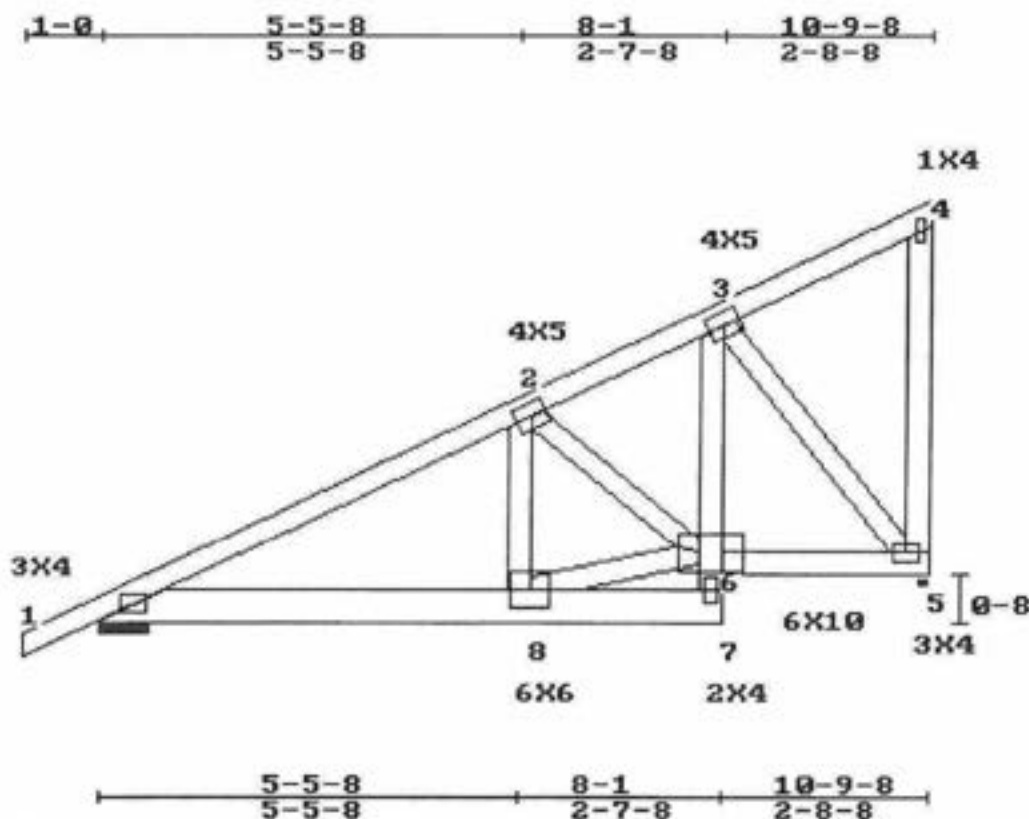
MAR 01 2001

====<<<ACES-32 Ver. 1.1>>>=====[001878]====<<<FL.QUALITY-MMH>>>=====
 Customer : FOGLIA Thu Mar 1 10:00:58 2001
 Project #: 00469 Truss ID : GT10 Family # : special
 Span : 10-9-8 Quantity : 2 Top Pitch : 6/12
 Bot pitch : 90/12

==Bld(1/21/2000),v1.1=====

FORCES - LOAD CASE #1			REACTIONS - SIZE	
1-2=1798	5-6= 739	2-8= 775	1=1116	8.00
2-3=880	6-7= 25	2-6=1056	5=1101	1.50
3-4= 0	7-8= 0	3-6= 1111		
4-5=119	8-1= 1592	3-5=1207		
		6-8= 1636		

PROVIDE FOR 1480 LBS UPLIFT AT JOINT 1 (1.33)
 PROVIDE FOR 1656 LBS UPLIFT AT JOINT 5 (1.50)
 PROVIDE FOR 620 LBS HORIZ. REACTION AT JOINT 1
 (SYMMETRIC PLATING TURNED ON)



L. HL TO PK:12-0-13 INTERNAL RISE:0-8
 LEFT HEIGHT:0-4-3 SPAN:10-9-8 RISE:5-8-15 RIGHT HEIGHT:5-0-15

LOADING (PSF)		MAX STRESSES		MINIMUM GRADE OF LUMBER	
TOP	30 15	TOP	1-2=0.355	TOP CHORD:2X4	No.2ND 19 SP
BOTT	0 10	BOTT	8-1=0.115	BOT CHORD:2X4	No.2ND 19 SP
		LL.DEFL.@8=0.02 < L/360		WEBS	:2X4 No.3 19 SP

REPETITIVE STRESSES NOT USED

SPACING : 24.0 in. o. c.
 NO. OF MEMBERS = 2

LOADING	STRESS INCREASE	LOADING	PANEL(PLF) / JOINTS(LBS)
LUMBER	1.33	TYPE	
PLATE	1.33	UNIFORM	1- 4= 90 5- 6= 20 7- 1= 20
		CONCENTRATED	8= 1074

CHORD: 6-7 TO BE 2X4 No.3 19 SP
 CHORD: 7-8; 8-1 TO BE 2X6 No.2D 19 SP
 RIGHT END VERTICAL IS NOT EXPOSED.
 2 MEMBERS NAILED TOG. W/1 ROW(S) OF .131x3 in. NAILS 12 in. o.c.(TOP CHS.),AND 1 ROW(S) OF .131x3 in. NAILS 10 in. o.c(BOTT. CHS.)
 For Webs use 1 ROW of NAILS 12 in. o.c.
 TRUSS HAS BEEN CHECKED FOR 145 M.P.H. WIND LOAD, WALL HEIGHT 16 FT, ENCLOSED BLDG. TYPE, ASSUMING 10 PSF TOTAL D.L.(7+3) (SBC-97).
 DEFLECTION(IN.) L.L= 0.02,D.L=0.01,T.L=0.03
 PLATES ARE MITEK M20-249,200 MANUFACTURED FROM ASTM A 446 GRD A GALVANIZED STEEL(EXCEPT AS SHOWN)
 PLATE MUST BE INSTALLED ON EA. FACE OF JOINT,SYMMETRICALLY(EXCEPT AS SHOWN)DESIGN CONFORMS W/NDS DESIGN SPECS, SBC,ANSI/TPI-95
 THIS DESIGN IS FOR TRUSS FABRICATION ONLY.FOR PERMANENT AND TEMPORARY BRACING(WHICH IS ALWAYS REQD)CONSULT BLDG ARCHITECT OR ENGINEER.

MAR 01 2001

====<<<<ACES-32 Ver. 1.1>>>>===== [001879] =====<<<<FL.QUALITY-MMH>>>>=====

Customer : FOGLIA Thu Mar 1 10:01:10 2001

Project #: 00469 Truss ID : S17 Family #: special

Span : 10-9-8 Top Pitch : 6/12

Bot pitch : 90/12

==Bld(1/21/2000),v1.1=====

FORCES - GRAVITY LOADS			REACTIONS - SIZE	
1-2=-659	5-6= 207	2-8=-13	1=-682	8.00
2-3=-245	6-7= 39	2-6=-399	5=-572	1.50
3-4= 0	7-8= 0	3-6= 330		
4-5=-76	8-1= 579	3-5=-520		
		6-8= 587		

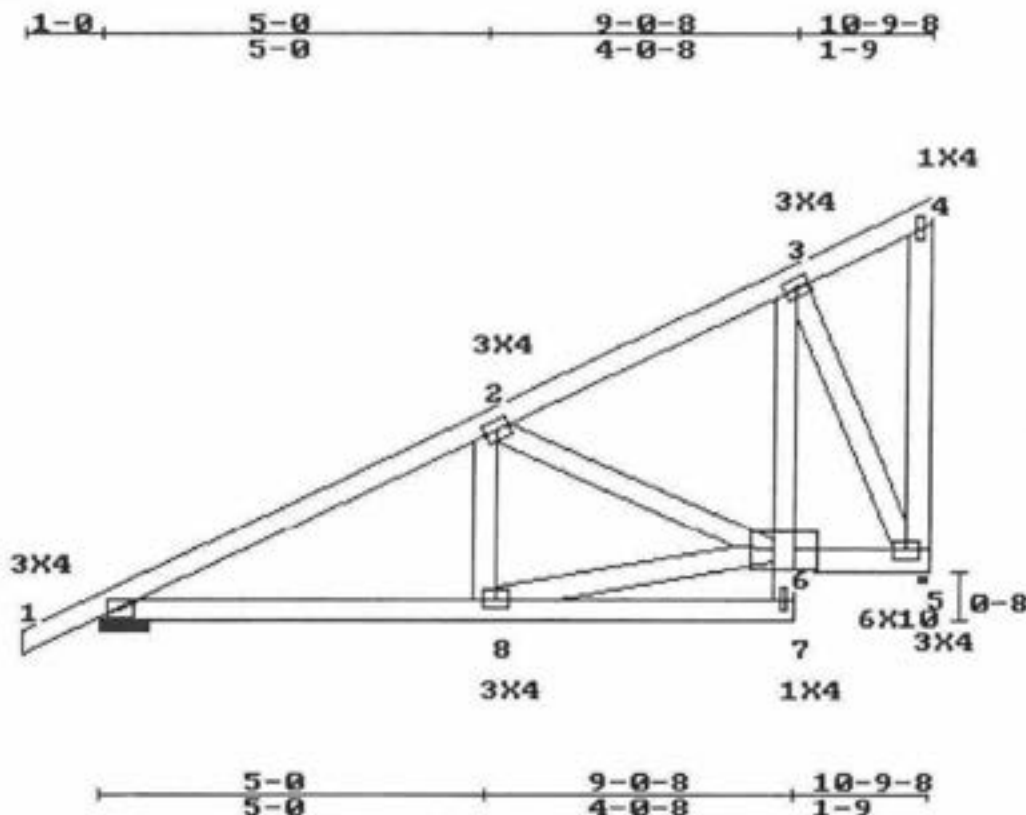
(12)

PROVIDE FOR 471 LBS UPLIFT AT JOINT 1 (0.69)

PROVIDE FOR 670 LBS UPLIFT AT JOINT 5 (1.17)

PROVIDE FOR 630 LBS HORIZ. REACTION AT JOINT 1

(SYMMETRIC PLATING TURNED ON)



LOADING (PSF)		MAX STRESSES		MINIMUM GRADE OF LUMBER	
TOP	30	TOP	1-2=0.615	TOP CHORD:2X4	No.2ND 19 SP
BOTT	0	BOTT	8-1=0.197	BOT CHORD:2X4	No.2ND 19 SP
		LL.DEFL.@8=0.01	< L/360	WEBS	:2X4 No.3 19 SP

STR.INC.: LUMB = 1.33 PLATE = 1.33

REPETITIVE STRESSES USED

SPACING : 24.0 in. o. c.

NO. OF MEMBERS = 1

CHORD: 6-7 TO BE 2X4 No.3 19 SP

RIGHT END VERTICAL IS NOT EXPOSED.

TRUSS HAS BEEN CHECKED FOR 145 M.P.H. WIND LOAD, WALL HEIGHT 16 FT, ENCLOSED BLDG. TYPE, ASSUMING 10 PSF TOTAL D.L.(7+3) (SBC-97).

DEFLECTION(IN.) L.L= 0.01,D.L=0.01,T.L=0.02

PLATES ARE MITEK M20-249,200 MANUFACTURED FROM ASTM A 446 GRD A GALVANIZED STEEL(EXCEPT AS SHOWN)

PLATE MUST BE INSTALLED ON EA. FACE OF JOINT,SYMMETRICALLY(EXCEPT AS SHOWN)DESIGN CONFORMS W/ND5 DESIGN SPECS, SSBC,ANSI/TP1-95

THIS DESIGN IS FOR TRUSS FABRICATION ONLY.FOR PERMANENT AND TEMPORARY BRACING(WHICH IS ALWAYS REQD)CONSULT BLDG ARCHITECT OR ENGINEER.

[Signature]

MAR 01 2001

====<<<<ACES-32 Ver. 1.1>>>>=====[001880]====<<<<FL.QUALITY-MMH>>>>=====

Customer : FOGLIA

Project # : 00469

Span : 10-9-8

Truss ID : J10

Quantity : 5

Thu Mar 1 10:01:20 2001

Family # : 201

Top Pitch : 6/12

==Bld(1/21/2000),v1.1=====

FORCES - GRAVITY LOADS

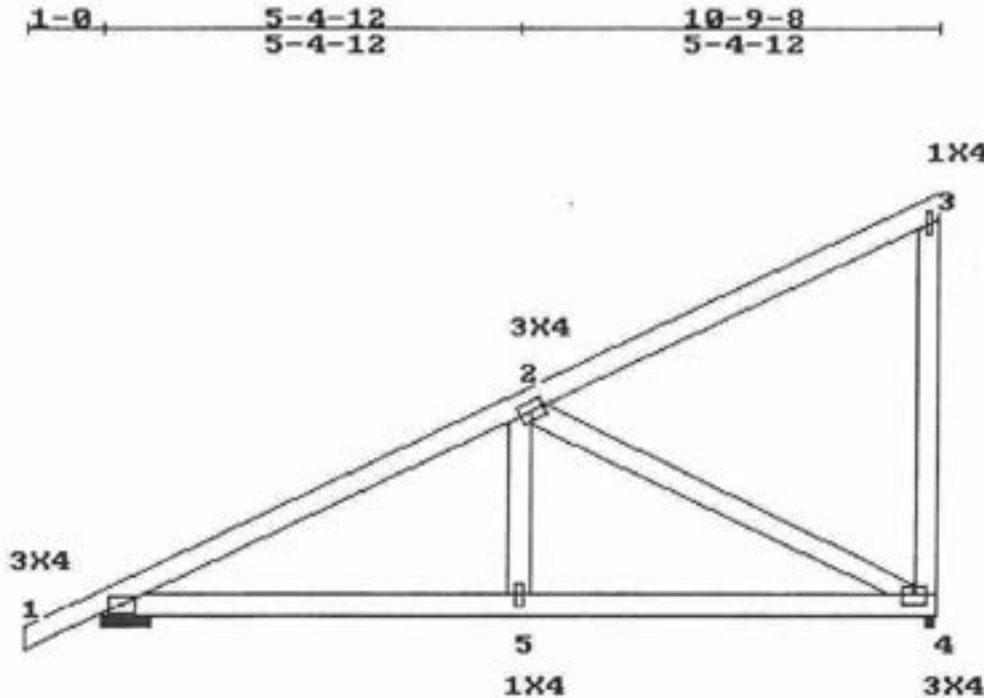
1-2=617 4-5= 543 2-5= 104
2-3= 0 5-1= 543 2-4=610
3-4=240

REACTIONS - SIZE

1=-682 8.00
4=-572 1.50

(13)

PROVIDE FOR 471 LBS UPLIFT AT JOINT 1 (0.69)
PROVIDE FOR 670 LBS UPLIFT AT JOINT 4 (1.17)
PROVIDE FOR 630 LBS HORIZ. REACTION AT JOINT 1
(SYMMETRIC PLATING TURNED ON)



L. HL TO PK:12-0-13
 LEFT HEIGHT:0-4-3 SPAN:10-9-8 RISE:5-8-15 RIGHT HEIGHT:5-8-15
 =====
 LOADING (PSF) MAX STRESSES MINIMUM GRADE OF LUMBER
 TOP 30 15 TOP 1-2=0.867 TOP CHORD:2X4 No.2ND 19 SP
 BOTT 0 10 BOTT 4-5=0.222 BOT CHORD:2X4 No.2ND 19 SP
 LL.DEFL.@5=0.01 < L/360 WEBS :2X4 No.3 19 SP
 =====
 STR.INC.: LUMB = 1.33 PLATE = 1.33 SPACING : 24.0 in. o. c.
 REPETITIVE STRESSES USED NO. OF MEMBERS = 1

RIGHT END VERTICAL IS NOT EXPOSED.
 TRUSS HAS BEEN CHECKED FOR 145 M.P.H. WIND LOAD, WALL HEIGHT 16 FT, ENCLOSED BLDG. TYPE, ASSUMING 10 PSF TOTAL D.L.(7+3) (SBC-97).
 DEFLECTION(IN.) L.L= 0.01,D.L=0.01,T.L=0.02
 PLATES ARE MITEK M20-249,200 MANUFACTURED FROM ASTM A 446 GRD A GALVANIZED STEEL(EXCEPT AS SHOWN)
 PLATE MUST BE INSTALLED ON EA. FACE OF JOINT,SYMMETRICALLY(EXCEPT AS SHOWN)DESIGN CONFORMS W/NDS DESIGN SPECS, SSBC,ANSI/TP1-95
 THIS DESIGN IS FOR TRUSS FABRICATION ONLY.FOR PERMANENT AND TEMPORARY BRACING(WHICH IS ALWAYS REQD)CONSULT BLDG ARCHITECT OR ENGINEER.

MAR 01 2001

====<<<<ACES-32 Ver. 1.1>>>>===== [001881] =====<<<<FL.QUALITY-MMH>>>>=====

Customer : FOGLIA
Project #: 00469
Span : 18-0

Truss ID : S15
Quantity : 2

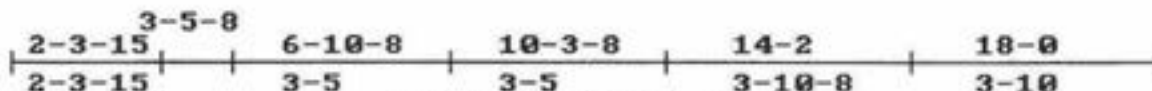
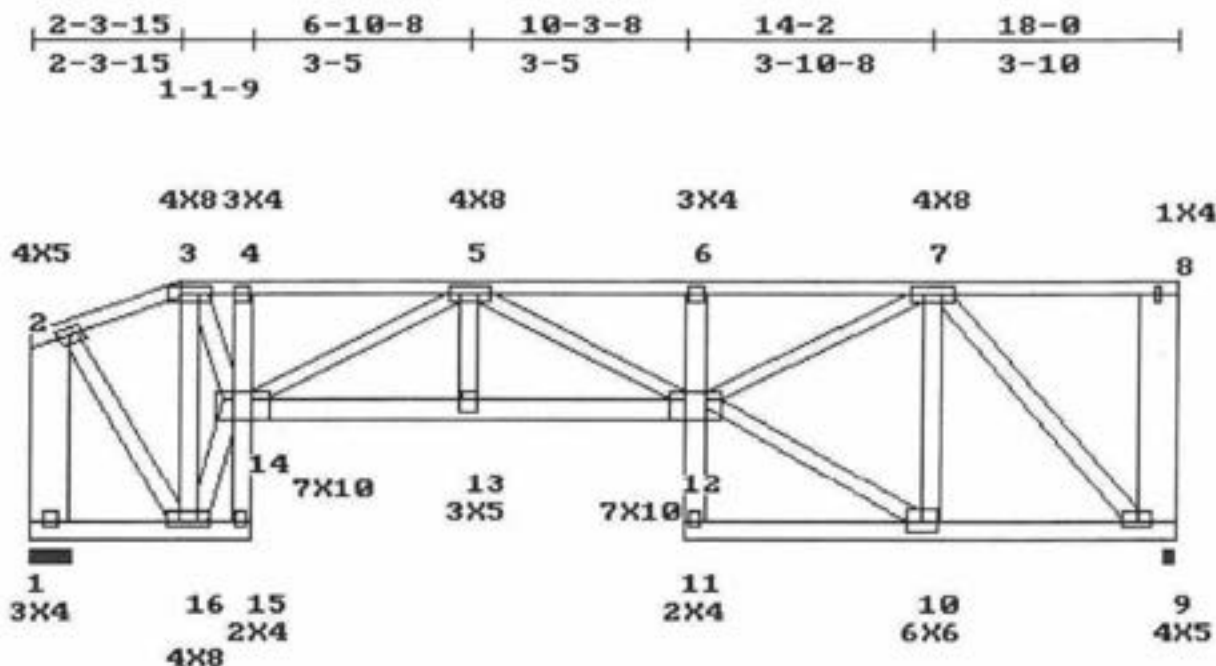
Thu Mar 1 10:06:23 2001
Family #: special
Top Pitch : 6/12
Bot pitch : 90/12

==Bld(1/21/2000),v1.1=====

FORCES - LOAD CASE #1				REACTIONS - SIZE				
1-2=-3209	7-8= 0	9-10= 1936	15-16= 0	2-16= 2781	5-12= 311	14-16= 3397	1=-3523	8.00
2-3=-1320	8-9=-169	10-11= 0	16-1= 0	3-16=-5240	6-12=-329		9=-3523	2.05
3-4=-3250		11-12= 578		3-14= 6062	7-12= 4524			
4-5=-3262		12-13= 5370		4-14=-205	7-10=-225			
5-6=-5617		13-14= 5370		5-14=-2646	7-9=-3382			
6-7=-5600		14-15= 153		5-13= 1104	12-10= 2381			

74

PROVIDE FOR 3997 LBS UPLIFT AT JOINT 1 (1.13)
PROVIDE FOR 4064 LBS UPLIFT AT JOINT 9 (1.15)
PROVIDE FOR 121 LBS HORIZ. REACTION AT JOINT 1
PLATE OFFSETS (X=LEFT,Y=TOP): [j12=5,2], [j13=3,1.5], [j14=5,2], (SYMMETRIC PLATING TURNED ON)



L. HL TO PK:2-7-4 INTERNAL RISE:2-8
LEFT HEIGHT:4-6-15 SPAN:18-0 RISE:5-8-15 RIGHT HEIGHT:5-8-15

LOADING (PSF)		MAX STRESSES		MINIMUM GRADE OF LUMBER	
TOP	30 15	TOP	6-7=0.702	TOP CHORD:2X4	No.2ND 19 SP
BOTT	0 10	BOTT	12-13=0.502	BOT CHORD:2X6	No.2D 19 SP
		LL.DEFL.@11=0.11 < L/360	WEBS	:2X4	No.3 19 SP

REPETITIVE STRESSES NOT USED

SPACING : 24.0 in. o. c.
NO. OF MEMBERS = 2

NOTE: PROVIDE FOR -0.28 IN. HORIZONTAL DISPLACEMENT AT ONE BEARING

LOADING	STRESS INCREASE	LOADING	PANEL(PLF) / JOINTS(LBS)
LUMBER	1.33	1.33	2- 8= 90 9-11= 310 12-14= 310 15- 1= 310
PLATE			
TYPE			
UNIFORM			

CHORD: 11-12; 14-15 TO BE 2X4 No.3 19 SP

WEB: 1-2; 8-9 TO BE 2X8 SS 19 SP

BOTH END VERTICALS ARE NOT EXPOSED.

2 MEMBERS NAILED TOG. W/1 ROW(S) OF .131x3 in. NAILS 12 in. o.c.(TOP CHS.),AND 1 ROW(S) OF .131x3 in. NAILS 8 in. o.c.(BOTT. CHS.)

For Webs use 1 ROW of NAILS 12 in. o.c.

TRUSS HAS BEEN CHECKED FOR 145 M.P.H. WIND LOAD, WALL HEIGHT 16 FT, ENCLOSED BLDG. TYPE, ASSUMING 10 PSF TOTAL D.L.(7+3) (SBC-97).

DEFLECTION(IN.) L.L= 0.11,D.L=0.09,T.L=0.20

PLATES ARE MITEK M20-249,200 MANUFACTURED FROM ASTM A 446 GRD A GALVANIZED STEEL(EXCEPT AS SHOWN)

PLATE MUST BE INSTALLED ON EA. FACE OF JOINT,SYMMETRICALLY(EXCEPT AS SHOWN)DESIGN CONFORMS W/MDS DESIGN SPECS, SSSBC,ANSI/TP1-95

THIS DESIGN IS FOR TRUSS FABRICATION ONLY.FOR PERMANENT AND TEMPORARY BRACING(WHICH IS ALWAYS REQD)CONSULT BLDG ARCHITECT OR ENGINEER.

MAR 01 2001

====<<<ACES-32 Ver. 1.1>>>===== [001882] =====<<<FL.QUALITY-MMH>>>=====

Customer : FOGLIA

Project # : 00469

Span : 10-6

Truss ID : S18

Quantity : 4

Thu Mar 1 10:06:33 2001

Family # : 201

Top Pitch : 6/12

==Bld(1/21/2000),v1.1=====

FORCES - GRAVITY LOADS

1-2=590 4-5= 0 2-5=-429
2-3=-577 5-1= 519 3-5= 743
3-4= 51

REACTIONS - SIZE

1=-669 8.00
3=-591 1.50

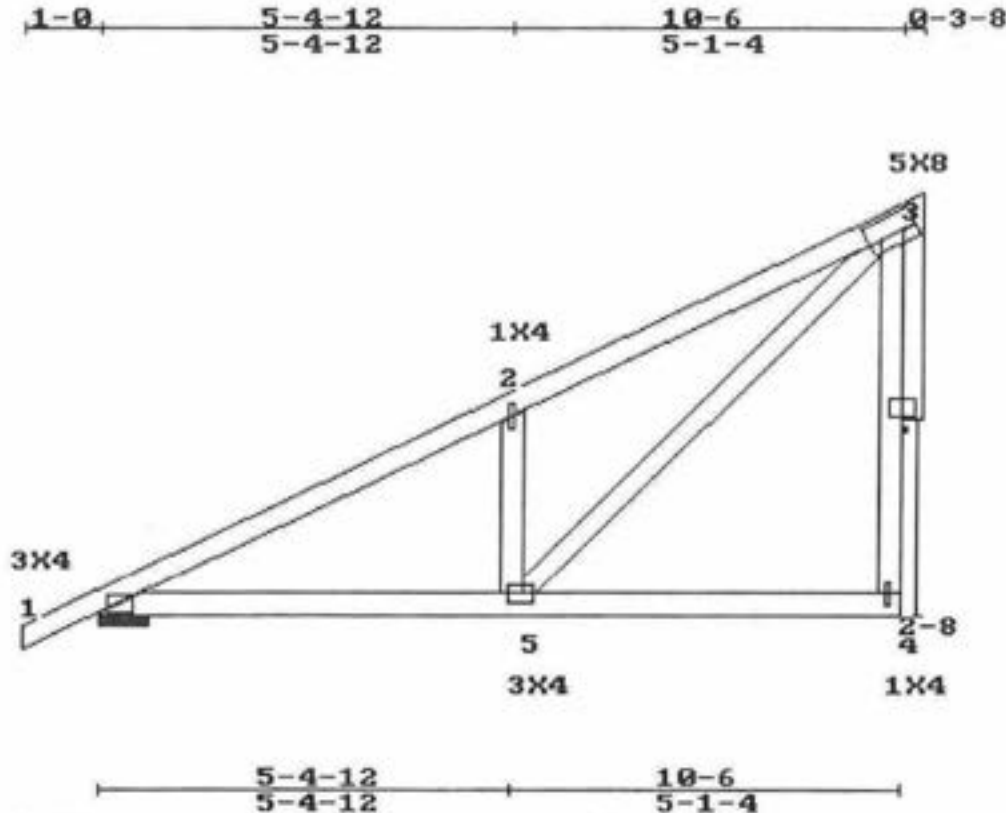
(15)

PROVIDE FOR 465 LBS UPLIFT AT JOINT 1 (0.69)

PROVIDE FOR 690 LBS UPLIFT AT JOINT 3 (1.17)

PROVIDE FOR 613 LBS HORIZ. REACTION AT JOINT 1

PLATE OFFSETS (X=LEFT,Y=TOP):[J3=2,3],(SYMMETRIC PLATING TURNED ON)



L. HL TO PK:11-8-14

LEFT HEIGHT:0-4-3

SPAN:10-6

RISE:5-8-15

RIGHT HEIGHT:5-7-3

LOADING (PSF)

TOP 30 15
BOTT 0 10

MAX STRESSES

TOP 2-3=0.880
BOTT 5-1=0.211
LL.DEFL.@5=0.01 < L/360

MINIMUM GRADE OF LUMBER

TOP CHORD:2X4 No.2ND 19 SP
BOT CHORD:2X4 No.2ND 19 SP
WEBS :2X4 No.3 19 SP

STR.INC.: LUMB = 1.33 PLATE = 1.33
REPETITIVE STRESSES USED

SPACING : 24.0 in. o. c.
NO. OF MEMBERS = 1

RIGHT END VERTICAL IS NOT EXPOSED.

TRUSS HAS BEEN CHECKED FOR 145 M.P.H. WIND LOAD, WALL HEIGHT 16 FT, ENCLOSED BLDG. TYPE, ASSUMING 10 PSF TOTAL D.L.(7+3) (SBC-97).

DEFLECTION(IN.) L.L= 0.01,D.L=0.01,T.L=0.03

PLATES ARE MITEK M20-249,200 MANUFACTURED FROM ASTM A 446 GRD A GALVANIZED STEEL(EXCEPT AS SHOWN)

PLATE MUST BE INSTALLED ON EA. FACE OF JOINT,SYMMETRICALLY(EXCEPT AS SHOWN)DESIGN CONFORMS W/NDS DESIGN SPECS, SIBC,ANSI/TPI-95

THIS DESIGN IS FOR TRUSS FABRICATION ONLY.FOR PERMANENT AND TEMPORARY BRACING(WHICH IS ALWAYS REQD)CONSULT BLDG ARCHITECT OR ENGINEER.

MAR 01 2001

====<<<<ACES-32 Ver. 1.1>>>>=====[001883]=====<<<<FL.QUALITY-MMH>>>>=====

Customer : FOGLIA
 Project # : 00469
 Span : 10-6

Truss ID : S19
 Quantity : 1

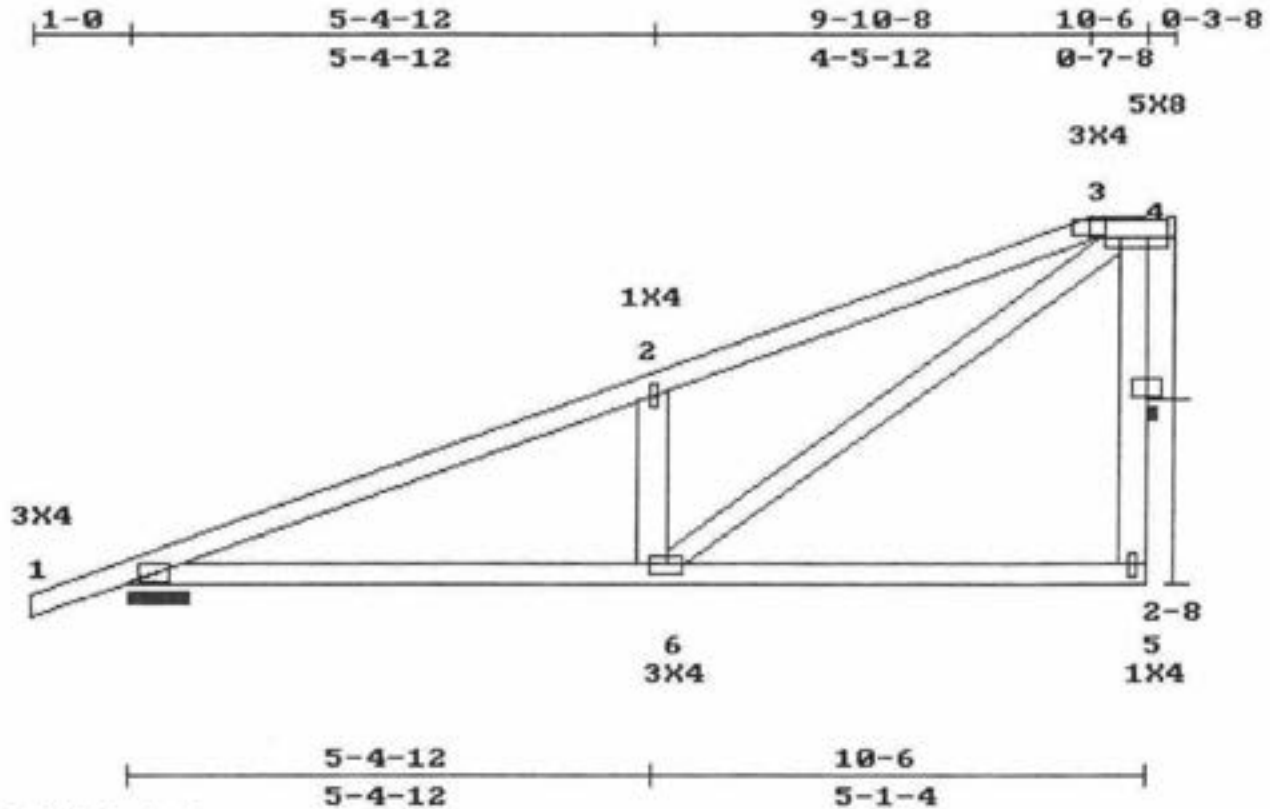
Thu Mar 1 10:06:42 2001
 Family # : 201
 Top Pitch : 6/12

==Bld(1/21/2000),v1.1=====

REACTIONS - SIZE
 1=669 8.00
 4=591 1.50

(76)

PROVIDE FOR 478 LBS UPLIFT AT JOINT 1 (0.71)
 PROVIDE FOR 677 LBS UPLIFT AT JOINT 4 (1.15)
 PROVIDE FOR 587 LBS HORIZ. REACTION AT JOINT 1
 PLATE OFFSETS (X=LEFT,Y=TOP):[J4=2,3],(SYMMETRIC PLATING TURNED ON)



L. HL TO PK:11-0-8
 LEFT HEIGHT:0-4-3
 SPAN:10-6
 RISE:5-3-7
 RIGHT HEIGHT:5-3-7

LOADING (PSF)		MAX STRESSES		MINIMUM GRADE OF LUMBER	
L	D	TOP	BOTT	TOP CHORD	BOT CHORD
30	15	1-2=0.770	6-1=0.303	2X4 No.2ND 19 SP	2X4 No.2ND 19 SP
0	10	LL.DEFL.@5=0.16 < L/360		WEBS :2X4 No.3 19 SP	

STR.INC.: LUMB = 1.33 PLATE = 1.33
 REPETITIVE STRESSES USED
 SPACING : 24.0 in. o. c.
 NO. OF MEMBERS = 1

RIGHT END VERTICAL IS NOT EXPOSED.
 TRUSS HAS BEEN CHECKED FOR 145 M.P.H. WIND LOAD, WALL HEIGHT 16 FT, ENCLOSED BLDG. TYPE, ASSUMING 10 PSF TOTAL D.L.(7+3) (SBC-97).
 DEFLECTION(IN.) L.L= 0.16,D.L=0.13,T.L=0.30
 PLATES ARE MITEK M20-249,200 MANUFACTURED FROM ASTM A 446 GRD A GALVANIZED STEEL(EXCEPT AS SHOWN)
 PLATE MUST BE INSTALLED ON EA. FACE OF JOINT,SYMMETRICALLY(EXCEPT AS SHOWN)DESIGN CONFORMS W/ND S DESIGN SPECS, SSBC,ND S,ANSI/TP1-95
 THIS DESIGN IS FOR TRUSS FABRICATION ONLY.FOR PERMANENT AND TEMPORARY BRACING(WHICH IS ALWAYS REQD)CONSULT BLDG ARCHITECT OR ENGINEER.

MAR 01 2001

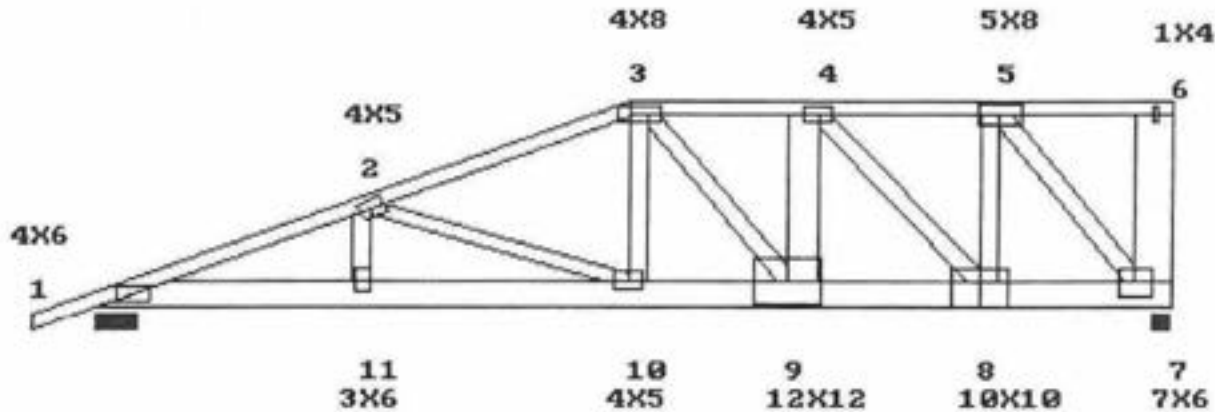
====<<<<ACES-32 Ver. 1.1>>>>====[001884]====<<<<FL.QUALITY-MMH>>>>====
 Customer : FOGLIA Thu Mar 1 10:07:34 2001
 Project #: 00469 Truss ID : S9 Family # : 313
 Span : 16-7-8 B.Span : 16-8 Quant. : 2 Top Pitch : 6/12
 ==8ld(1/21/2000),v1.1=====

FORCES - LOAD CASE #1				REACTIONS - SIZE	
1-2=5636	7-8= 3676	2-11= 79	5-8= 5399	1=2782	8.00
2-3=5171	8-9= 5859	2-10=439	5-7=6635	7=6717	4.07
3-4=5859	9-10= 4617	3-10= 242			
4-5=3676	10-11= 5014	3-9= 2220			
5-6= 0	11-1= 5014	4-9= 2918			
6-7=60		4-8=3745			

(77)

PROVIDE FOR 3358 LBS UPLIFT AT JOINT 1 (1.21)
 PROVIDE FOR 6891 LBS UPLIFT AT JOINT 7 (1.03)
 PROVIDE FOR 471 LBS HORIZ. REACTION AT JOINT 1
 PLATE OFFSETS (X=LEFT,Y=TOP):[j8=5,3],

1-0	4-1-5	8-2-9	10-11	13-9-8	16-7-8
	4-1-5	4-1-5	2-8-7	2-10-8	2-10



4-1-5	8-2-9	10-11	13-9-8	16-7-8
4-1-5	4-1-5	2-8-7	2-10-8	2-10

L. HL TO PK:9-2-3
 LEFT HEIGHT:0-4-3 SPAN:16-7-8 RISE:4-5-8 RIGHT HEIGHT:4-5-7

LOADING (PSF)		MAX STRESSES		MINIMUM GRADE OF LUMBER	
TOP	30 15	TOP	1-2=0.728	TOP CHORD:2X4	No.2ND 19 SP
BOTT	0 10	BOTT	8-9=0.279	BOT CHORD:2X8	SS 19 SP
		LL.DEFL.@9=0.06 < L/360		WEBS	:2X4 No.3 19 SP

REPETITIVE STRESSES NOT USED

SPACING : 24.0 in. o. c.
 NO. OF MEMBERS = 2

LOADING	STRESS INCREASE	LOADING	PANEL(PLF) / JOINTS(LBS)
LUMBER	1.33	PLATE	1.33
TYPE	UNIFORM	TYPE	CONCENTRATED
1	3- 90	3- 6= 45	7- 9= 850
	9= 3523	9-10= 10	10- 1= 20

WEB: 6-7 TO BE 2X8 SS 19 SP
 WEB: 4-9 TO BE 2X6 No.2D 19 SP
 WEB: 5-7 TO BE 2X4 No.2ND 19 SP
 RIGHT END VERTICAL IS NOT EXPOSED.
 2 MEMBERS NAILED TOG. W/1 ROW(S) OF .131x3 in. NAILS 12 in. o.c.(TOP CHS.),AND 1 ROW(S) OF .131x3 in. NAILS 3 in. o.c.(BOTT. CHS.)
 For Webs use 1 ROW of NAILS 12 in. o.c.
 TRUSS HAS BEEN CHECKED FOR 145 M.P.H. WIND LOAD, WALL HEIGHT 16 FT, ENCLOSED BLDG. TYPE, ASSUMING 10 PSF TOTAL D.L.(7+3) (SBC-97).
 DEFLECTION(IN.) L.L= 0.06,D.L=0.05,T.L=0.12
 PLATES ARE MITEK M20-249,200 MANUFACTURED FROM ASTM A 446 GRD A GALVANIZED STEEL(EXCEPT AS SHOWN)
 PLATE MUST BE INSTALLED ON EA. FACE OF JOINT,SYMMETRICALLY(EXCEPT AS SHOWN)DESIGN CONFORMS W/NDS DESIGN SPECS, 55SBC,ANSI/TPI-95
 THIS DESIGN IS FOR TRUSS FABRICATION ONLY.FOR PERMANENT AND TEMPORARY BRACING(WHICH IS ALWAYS REQD)CONSULT BLDG ARCHITECT OR ENGINEER.

MAR 01 2001

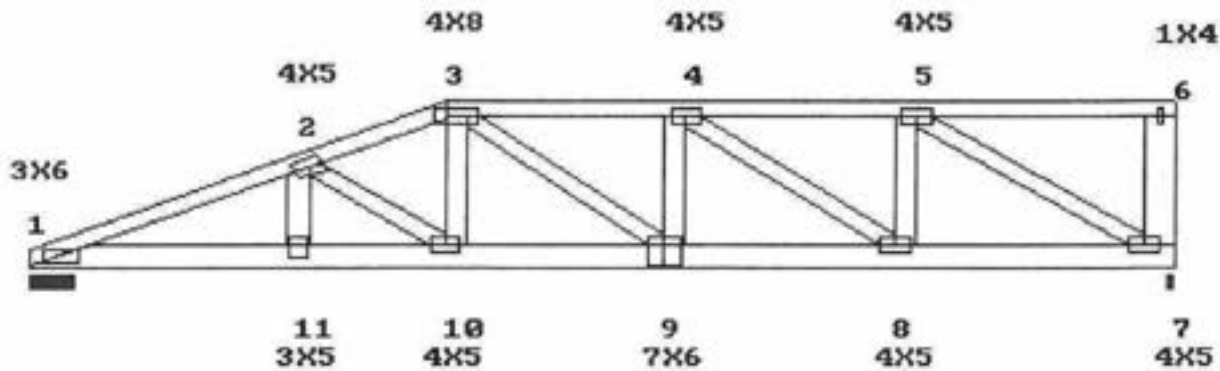
====<<<<ACES-32 Ver. 1.1>>>>=====[001885]====<<<<FL.QUALITY-MMH>>>>=====
 Customer : FOGLIA Thu Mar 1 10:08:10 2001
 Project #: 00469 Truss ID : GT16 Family #: 313
 Span : 16-7 B.Span : 16-8 Quant. : 2 Top Pitch : 6/12
 ==Bld(1/21/2000),v1.1=====

FORCES - LOAD CASE #1				REACTIONS - SIZE	
1-2= 3581	7-8= 1964	2-11= 1082	5-8= 1191	1= 1882	8.00
2-3= 2537	8-9= 2318	2-10= 1201	5-7= 2480	7= 1726	1.60
3-4= 2318	9-10= 2262	3-10= 853			
4-5= 1964	10-11= 3158	3-9= 76			
5-6= 0	11-1= 3158	4-9= 16			
6-7= 173		4-8= 475			

(78)

PROVIDE FOR 3253 LBS UPLIFT AT JOINT 1 (1.73)
 PROVIDE FOR 3177 LBS UPLIFT AT JOINT 7 (1.84)
 PROVIDE FOR 349 LBS HORIZ. REACTION AT JOINT 1
 PLATE OFFSETS (X=LEFT,Y=TOP):(j9=3,2),(j11=3,1.5),

3-10-8	6-0	9-4	12-8	16-7
3-10-8	2-1-8	3-4	3-4	3-11



3-10-8	6-0	9-4	12-8	16-7
3-10-8	2-1-8	3-4	3-4	3-11

L. HL TO PK:6-8-8
 LEFT HEIGHT:0-4-3 SPAN:16-7 RISE:3-4-3 RIGHT HEIGHT:3-4-3

LOADING (PSF)		MAX STRESSES		MINIMUM GRADE OF LUMBER	
TOP	30	15	TOP 1-2=0.560	TOP CHORD:2X4	No.2ND 19 SP
BOTT	0	10	BOTT 11-1=0.196	BOT CHORD:2X6	No.2D 19 SP
			LL.DEFL.@10=0.05 < L/360	WEBS	:2X4 No.3 19 SP

REPETITIVE STRESSES NOT USED

SPACING : 24.0 in. o. c.
 NO. OF MEMBERS = 2

LOADING	STRESS INCREASE	LOADING	PANEL(PLF) / JOINTS(LBS)
LUMBER	1.33	PLATE	1- 6= 90 7- 1= 20
TYPE		UNIFORM	8= 803 11= 1025
		CONCENTRATED	

[Signature]
 MAR 01 2001

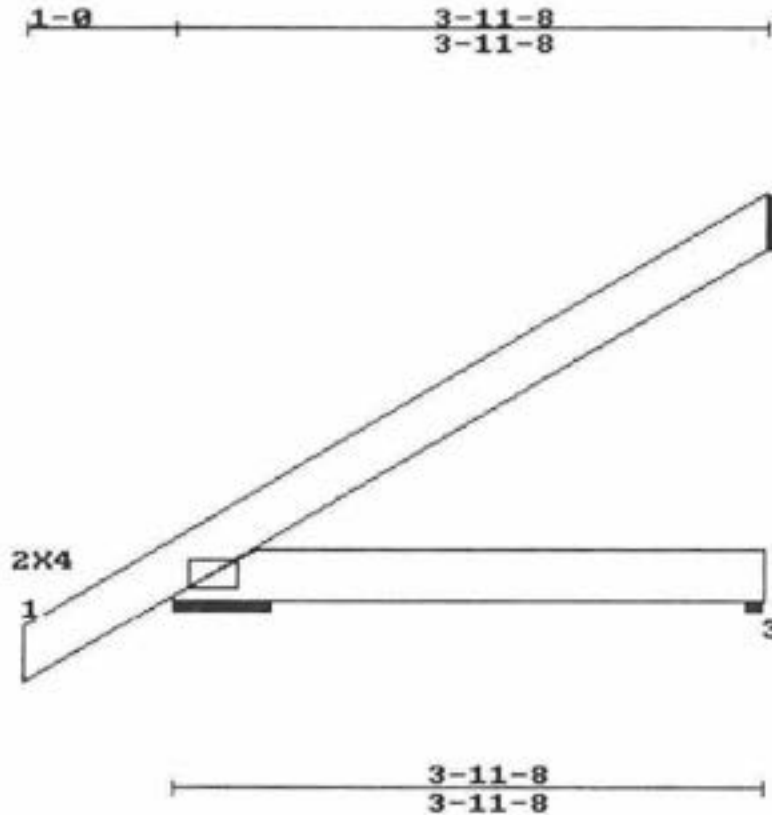
WEB: 6-7 TO BE 2X6 No.2D 19 SP
 RIGHT END VERTICAL IS NOT EXPOSED.
 2 MEMBERS NAILED TOG. W/1 ROW(S) OF .131x3 in. NAILS 12 in. o.c.(TOP CHS.),AND 1 ROW(S) OF .131x3 in. NAILS 10 in. o.c(BOTT. CHS.)
 For Webs use 1 ROW of NAILS 12 in. o.c.
 TRUSS HAS BEEN CHECKED FOR 145 M.P.H. WIND LOAD, WALL HEIGHT 16 FT, ENCLOSED BLDG. TYPE, ASSUMING 10 PSF TOTAL D.L.(7+3) (SBC-97).
 DEFLECTION(IN.) L.L= 0.05,D.L=0.05,T.L=0.10
 PLATES ARE MITEK M20-249,200 MANUFACTURED FROM ASTM A 446 GRD A GALVANIZED STEEL(EXCEPT AS SHOWN)
 PLATE MUST BE INSTALLED ON EA. FACE OF JOINT,SYMMETRICALLY(EXCEPT AS SHOWN)DESIGN CONFORMS W/NDS DESIGN SPECS, SSB,ANSI/TPI-95
 THIS DESIGN IS FOR TRUSS FABRICATION ONLY.FOR PERMANENT AND TEMPORARY BRACING(WHICH IS ALWAYS REQD)CONSULT BLDG ARCHITECT OR ENGINEER.

====<<<ACES-32 Ver. 1.1>>>====[002005]====<<<FL.QUALITY-MMH>>>====
 Customer : FOGLIA Wed Mar 7 07:14:00 2001
 Project #: 00469 Truss ID : C3B Family # : 205
 Span : 3-11-8 B.Span : 3-10-8 Quant. : 1 Top Pitch : 6/12
 ==Bld(1/21/2000),v1.1=====

FORCES - GRAVITY LOADS
 1-2=0 3-1=0
 2-3=0

REACTIONS - SIZE
 1=306 8.00
 3=36 1.50
 2=160 1.50

PROVIDE FOR 237 LBS UPLIFT AT JOINT 1 (0.78)
 PROVIDE FOR 247 LBS UPLIFT AT JOINT 2 (1.54)
 PROVIDE FOR 228 LBS HORIZ. REACTION AT JOINT 1



L. HL TO PK:4-5-2
 LEFT HEIGHT:0-4-3 SPAN:3-11-8 RISE:2-3-15 RIGHT HEIGHT:2-3-15

LOADING (PSF)		MAX STRESSES		MINIMUM GRADE OF LUMBER	
TOP	30	TOP	1-2=0.419	TOP CHORD:2X4	No.2ND 19 SP
BOTT	0	BOTT	3-1=0.062	BOT CHORD:2X4	No.2ND 19 SP
		LL.DEFL.	< L/360	WEBS	:2X4 No.3 19 SP

STR.INC.: LUMB = 1.33 PLATE = 1.33
 REPETITIVE STRESSES USED SPACING : 24.0 in. o. c.
 NO. OF MEMBERS = 1

TRUSS HAS BEEN CHECKED FOR 145 M.P.H. WIND LOAD, WALL HEIGHT 16 FT, ENCLOSED BLDG. TYPE, ASSUMING 10 PSF TOTAL D.L.(7+3) (SBC-97).
 DEFLECTION(IN.) L.L= 0.00,D.L=0.00,T.L=0.00
 PLATES ARE MITEK M20-249,200 MANUFACTURED FROM ASTM A 446 GRD A GALVANIZED STEEL(EXCEPT AS SHOWN)
 PLATE MUST BE INSTALLED ON EA. FACE OF JOINT,SYMMETRICALLY(EXCEPT AS SHOWN)DESIGN CONFORMS W/NDS DESIGN SPECS, SSBC,ANSI/TPI-95
 THIS DESIGN IS FOR TRUSS FABRICATION ONLY.FOR PERMANENT AND TEMPORARY BRACING(WHICH IS ALWAYS REQD)CONSULT BLDG ARCHITECT OR ENGINEER.

MAR 06 2001

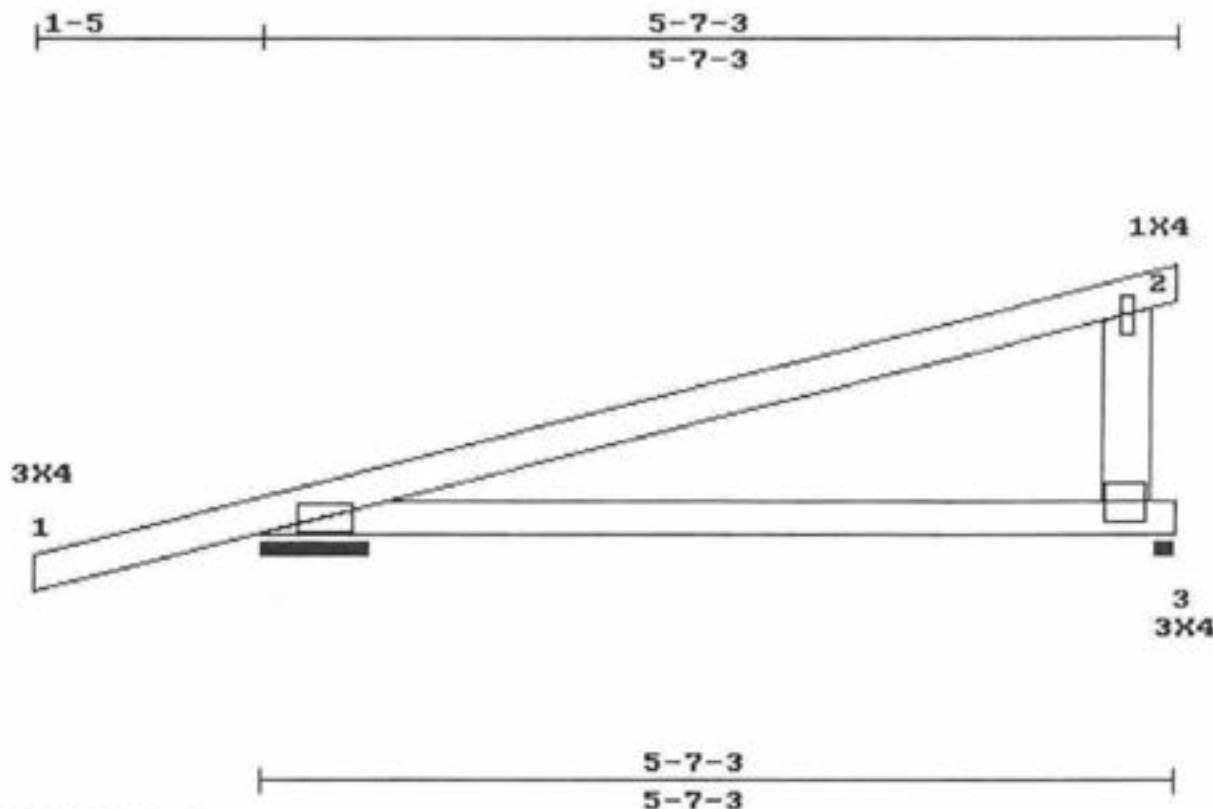
====<<<ACES-32 Ver. 1.1>>>=====[002004]====<<<FL.QUALITY-MMH>>>=====
 Customer : FOGLIA Wed Mar 7 07:13:43 2001
 Project #: 00469 Truss ID : CJ5C Family #: 204
 Span : 5-7-3 B.Span : 3-10-8 Quant. : 1 Top Pitch : 4.243
 ==Bld(1/21/2000),v1.1=====

FORCES - LOAD CASE #1
 1-2=0 3-1=0
 2-3=-172

REACTIONS - SIZE
 1=366 8.00
 3=210 1.50

PROVIDE FOR 568 LBS UPLIFT AT JOINT 1 (1.55)
 PROVIDE FOR 477 LBS UPLIFT AT JOINT 3 (2.27)
 PROVIDE FOR 230 LBS HORIZ. REACTION AT JOINT 1

80



L. HL TO PK:5-11-4
 LEFT HEIGHT:0-3-15

SPAN:5-7-3

RISE:2-3-12

RIGHT HEIGHT:2-3-12

LOADING (PSF)

	L	D
TOP	30	15
BOTT	0	10

MAX STRESSES

TOP	1-2=0.595
BOTT	3-1=0.111
LL.DEFL.	< L/360

MINIMUM GRADE OF LUMBER

TOP CHORD:2X4	No.2ND 19 SP
BOT CHORD:2X4	No.2ND 19 SP
WEBS	:2X4 No.3 19 SP

REPETITIVE STRESSES NOT USED

SPACING : 24.0 in. o. c.
 NO. OF MEMBERS = 1

LOADING	STRESS INCREASE	LOADING	PANEL(PLF) / JOINTS(LBS)
LUMBER	PLATE	TYPE	
1	1.33	1.33	UNIFORM 1- 2= 66 3- 1= 15

RIGHT END VERTICAL IS NOT EXPOSED.

TRUSS HAS BEEN CHECKED FOR 145 M.P.H. WIND LOAD, WALL HEIGHT 16 FT, ENCLOSED BLDG. TYPE, ASSUMING 10 PSF TOTAL D.L.(7+3) (SBC-97).
 DEFLECTION(IN.) L.L= 0.00,D.L=0.00,T.L=0.00

PLATES ARE MITEK M20-249,200 MANUFACTURED FROM ASTM A 446 GRD A GALVANIZED STEEL(EXCEPT AS SHOWN)

PLATE MUST BE INSTALLED ON EA. FACE OF JOINT,SYMMETRICALLY(EXCEPT AS SHOWN)DESIGN CONFORMS W/NDS DESIGN SPECS, S58C,ANSI/TPI-95

THIS DESIGN IS FOR TRUSS FABRICATION ONLY.FOR PERMANENT AND TEMPORARY BRACING(WHICH IS ALWAYS REQD)CONSULT BLDG ARCHITECT OR ENGINEER.

MAR 06 2001

====<<<<ACES-32 Ver. 1.1>>>>===== [001888] =====<<<<FL.QUALITY-MMH>>>>=====

Customer : FOGLIA

Project # : 00469

Span : 3-0

Truss ID : C3E

Quantity : 1

Thu Mar 1 10:08:56 2001

Family # : 205

Top Pitch : 6/12

==Bld(1/21/2000),v1.1=====

FORCES - GRAVITY LOADS

1-2= 0 3-1= 0
2-3= 0

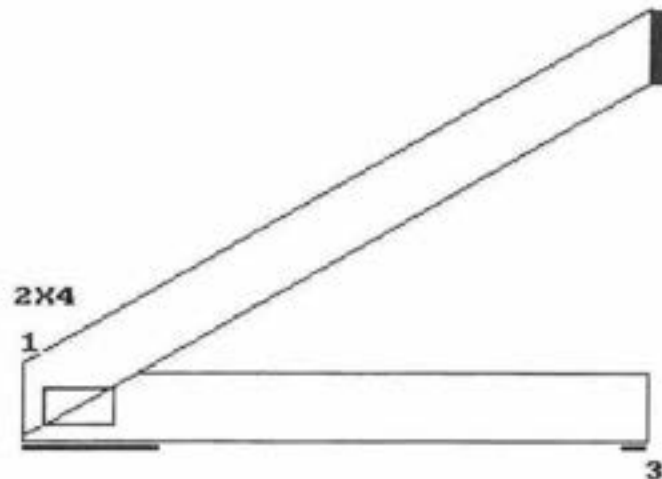
REACTIONS - SIZE

1=-143 8.00
3=-26 1.50
2=-117 1.50

(81)

PROVIDE FOR 79 LBS UPLIFT AT JOINT 1 (0.55)
PROVIDE FOR 183 LBS UPLIFT AT JOINT 2 (1.56)
PROVIDE FOR 172 LBS HORIZ. REACTION AT JOINT 1
(SYMMETRIC PLATING TURNED ON)

3-0
3-0



3-0
3-0

L. HL TO PK:3-4-4
LEFT HEIGHT:0-4-3

SPAN:3-0

RISE:1-10-3

RIGHT HEIGHT:1-10-3

LOADING (PSF)		MAX STRESSES		MINIMUM GRADE OF LUMBER	
TOP	30	TOP	1-2=0.225	TOP CHORD:2X4	No.2ND 19 SP
BOTT	0	BOTT	3-1=0.034	BOT CHORD:2X4	No.2ND 19 SP
		LL.DEFL.	< L/360	WEBS	:2X4 No.3 19 SP

STR.INC.: LUMB = 1.33 PLATE = 1.33
REPETITIVE STRESSES USED

SPACING : 24.0 in. o. c.
NO. OF MEMBERS = 1

TRUSS HAS BEEN CHECKED FOR 145 M.P.H. WIND LOAD, WALL HEIGHT 16 FT, ENCLOSED BLDG. TYPE, ASSUMING 10 PSF TOTAL D.L.(7*3) (SBC-97).
DEFLECTION(IN.) L.L= 0.00,D.L=0.00,T.L=0.00
PLATES ARE MITEK M20-249,200 MANUFACTURED FROM ASTM A 446 GRD A GALVANIZED STEEL(EXCEPT AS SHOWN)
PLATE MUST BE INSTALLED ON EA. FACE OF JOINT,SYMMETRICALLY(EXCEPT AS SHOWN)DESIGN CONFORMS W/NDS DESIGN SPECS, SS8C,ANSI/TPI-95
THIS DESIGN IS FOR TRUSS FABRICATION ONLY.FOR PERMANENT AND TEMPORARY BRACING(WHICH IS ALWAYS REQD)CONSULT BLDG ARCHITECT OR ENGINEER.

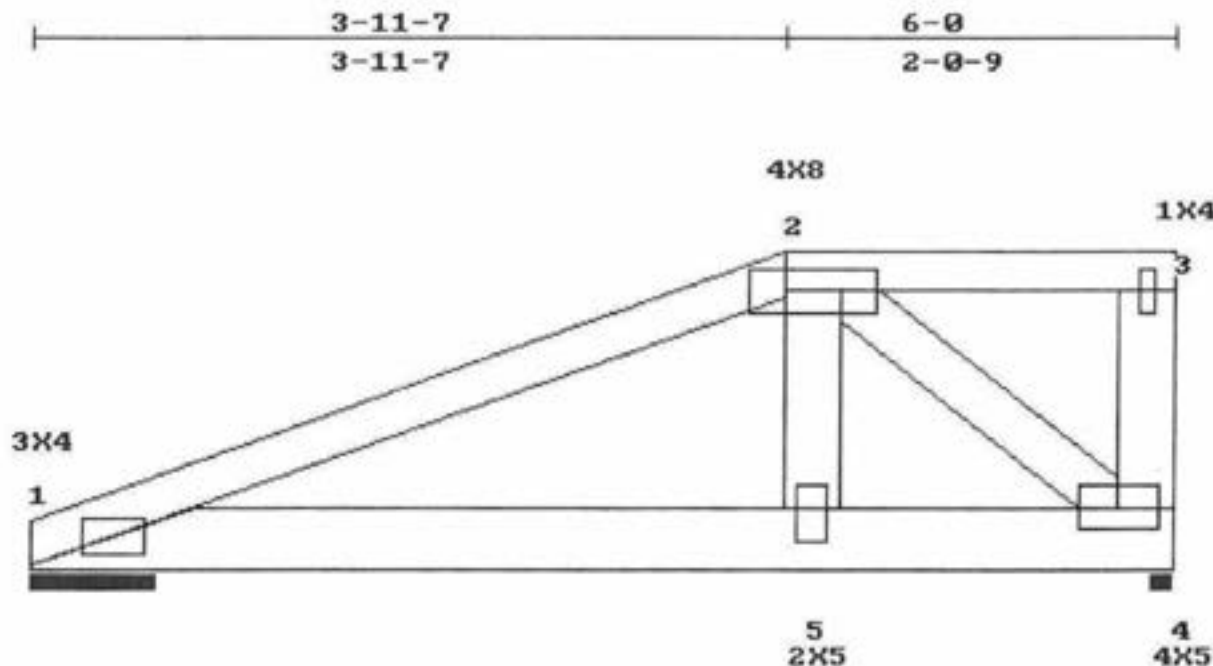
MAR 01 2001

====<<<ACES-32 Ver. 1.1>>>=====[002003]====<<<FL.QUALITY-MMH>>>=====
 Customer : FOGLIA Wed Mar 7 07:13:12 2001
 Project #: 00469 Truss ID : GT6 Family # : 313
 Span : 6-0 B.Span : 6-0 Quant. : 1 Top Pitch : 6/12

====8/6(1/21/2000),v1.1====
 FORCES - LOAD CASE #1 REACTIONS - SIZE
 1-2=810 4-5= 713 2-5= 790 1=901 8.00
 2-3= 0 5-1= 713 2-4=999 4=1026 1.50
 3-4=-131

PROVIDE FOR 1045 LBS UPLIFT AT JOINT 1 (1.16)
 PROVIDE FOR 1236 LBS UPLIFT AT JOINT 4 (1.21)
 PROVIDE FOR 229 LBS HORIZ. REACTION AT JOINT 1
 PLATE OFFSETS (X=LEFT,Y=TOP):[5=3,1],

82



L. HL TO PK:4-5-1
 LEFT HEIGHT:0-4-3 SPAN:6-0 RISE:2-3-14 RIGHT HEIGHT:2-3-14

LOADING (PSF) MAX STRESSES MINIMUM GRADE OF LUMBER
 TOP 30 15 TOP 1-2=0.364 TOP CHORD:2X4 No.2ND 19 SP
 BOTT 0 10 BOTT 4-5=0.298 BOT CHORD:2X6 No.2D 19 SP
 LL.DEFL.@5=0.01 < L/360 WEBS :2X4 No.3 19 SP

REPETITIVE STRESSES NOT USED

SPACING : 24.0 in. o. c.
 NO. OF MEMBERS = 1

LOADING	STRESS INCREASE	LOADING	PANEL(PLF) / JOINTS(LBS)
1	1.33	1.33	UNIFORM 1- 2= 90 2- 3= 132 4- 1= 196
			CONCENTRATED 5= 241

RIGHT END VERTICAL IS NOT EXPOSED.
 TRUSS HAS BEEN CHECKED FOR 145 M.P.H. WIND LOAD, WALL HEIGHT 16 FT, ENCLOSED BLDG. TYPE, ASSUMING 10 PSF TOTAL D.L.(7+3) (SBC-97).
 DEFLECTION(IN.) L.L= 0.01,D.L=0.01,T.L=0.01
 PLATES ARE MITEK M20-249,200 MANUFACTURED FROM ASTM A 446 GRD A GALVANIZED STEEL(EXCEPT AS SHOWN)
 PLATE MUST BE INSTALLED ON EA. FACE OF JOINT,SYMMETRICALLY(EXCEPT AS SHOWN)DESIGN CONFORMS W/NDG DESIGN SPECS, SSBG,ANSI/TPI-95
 THIS DESIGN IS FOR TRUSS FABRICATION ONLY.FOR PERMANENT AND TEMPORARY BRACING(WHICH IS ALWAYS REQD)CONSULT BLDG ARCHITECT OR ENGINEER.

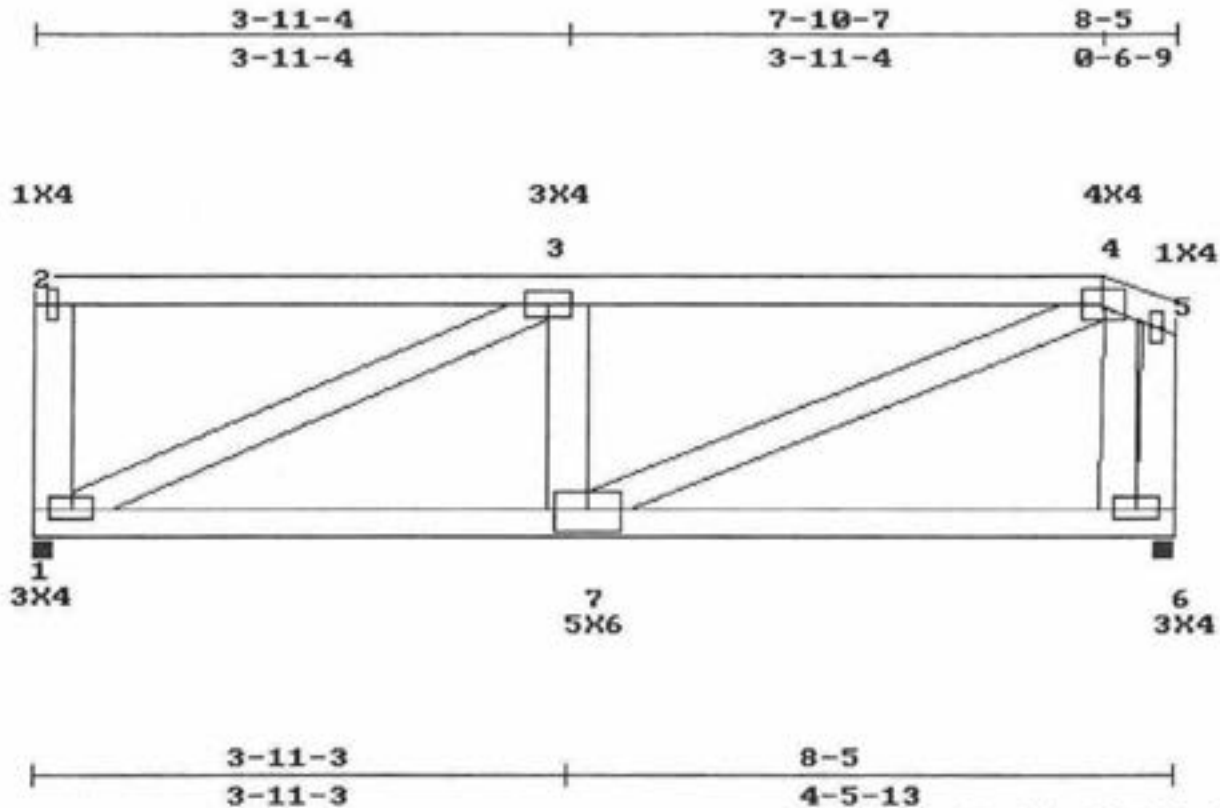
MAR 06 2001

====<<<ACES-32 Ver. 1.1>>>=====[001890]====<<<FL.QUALITY-MMH>>>=====
 Customer : FOGLIA Thu Mar 1 10:09:29 2001
 Project #: 00469 Truss ID : S13 Family # : special
 Span : 8-5 Quantity : 1 Top Pitch : 6/12
 ==Bld(1/21/2000),v1.1=====

FORCES - GRAVITY LOADS			REACTIONS - SIZE	
1-2=173	6-7= 75	3-1=-458	1=-454	2.00
2-3= 0	7-1= 389	3-7=-108	6=-454	2.00
3-4=-389		4-7= 368		
4-5= 0		4-6=-396		
5-6=-21				

(83)

PROVIDE FOR 410 LBS UPLIFT AT JOINT 1 (0.90)
 PROVIDE FOR 396 LBS UPLIFT AT JOINT 6 (0.87)
 PROVIDE FOR 25 LBS HORIZ. REACTION AT JOINT 1
 PLATE OFFSETS (X=LEFT,Y=TOP):[J7=3,2],(SYMMETRIC PLATING TURNED ON)



LEFT HEIGHT:2-8-4 SPAN:8-5 RISE:2-8-4 R. HL TO PK :0-7-5
 RIGHT HEIGHT:2-4-15

LOADING (PSF)		MAX STRESSES		MINIMUM GRADE OF LUMBER	
TOP	30	TOP	3-4=0.446	TOP CHORD:2X4	No.2ND 19 SP
BOTT	0	BOTT	7-1=0.143	BOT CHORD:2X4	No.2ND 19 SP
		LL.DEFL.@7=0.01	< L/360	WEBS	:2X4 No.3 19 SP

STR.INC.: LUMB = 1.33 PLATE = 1.33 SPACING : 24.0 in. o. c.
 REPETITIVE STRESSES USED NO. OF MEMBERS = 1

BOTH END VERTICALS ARE NOT EXPOSED.
 TRUSS HAS BEEN CHECKED FOR 145 M.P.H. WIND LOAD, WALL HEIGHT 16 FT, ENCLOSED BLDG. TYPE, ASSUMING 10 PSF TOTAL D.L.(7*3) (SBC-97).
 DEFLECTION(IN.) L.L= 0.01,D.L=0.00,T.L=0.01
 PLATES ARE MITEK M20-249,200 MANUFACTURED FROM ASTM A 446 GRD A GALVANIZED STEEL(EXCEPT AS SHOWN)
 PLATE MUST BE INSTALLED ON EA. FACE OF JOINT,SYMMETRICALLY(EXCEPT AS SHOWN)DESIGN CONFORMS W/ND S DESIGN SPECS, SSBC,ANSI/TPI-95
 THIS DESIGN IS FOR TRUSS FABRICATION ONLY.FOR PERMANENT AND TEMPORARY BRACING(WHICH IS ALWAYS REQD)CONSULT BLDG ARCHITECT OR ENGINEER.

MAR 01 2001

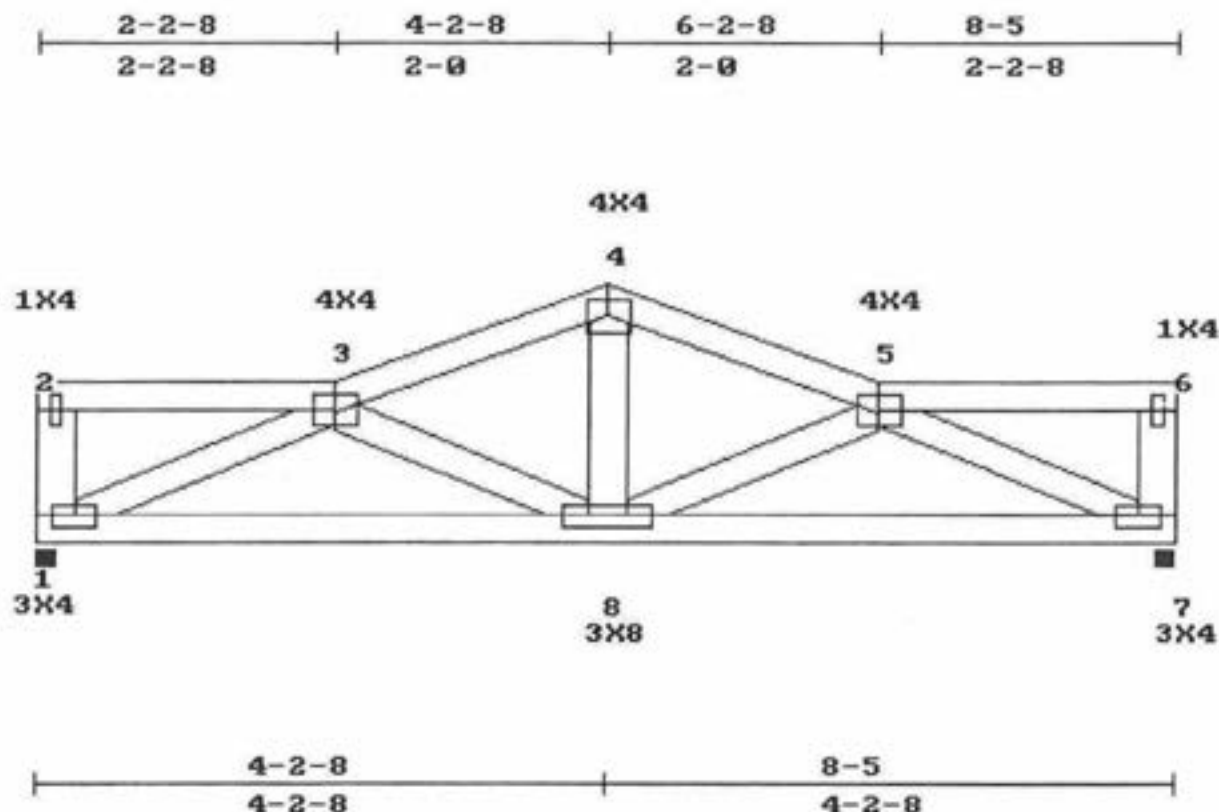
====<<<<ACES-32 Ver. 1.1>>>>=====[001891]====<<<<FL.QUALITY-MMH>>>>=====
 Customer : FOGLIA Thu Mar 1 10:09:37 2001
 Project #: 00469 Truss ID : S12 Family #: special
 Span : 8-5 Quantity : 1 Top Pitch : 6/12

==Bld(1/21/2000),v1.1=====

FORCES - GRAVITY LOADS			REACTIONS - SIZE	
1-2=96	7-8= 484	3-1=579	1=454	2.00
2-3= 0	8-1= 484	3-8=110	7=454	2.00
3-4=439		4-8= 208		
4-5=439		5-8=110		
5-6= 0		5-7=579		
6-7=96				

(84)

PROVIDE FOR 370 LBS UPLIFT AT JOINT 1 (0.81)
 PROVIDE FOR 370 LBS UPLIFT AT JOINT 7 (0.81)
 PROVIDE FOR 33 LBS HORIZ. REACTION AT JOINT 1
 (SYMMETRIC PLATING TURNED ON)



LEFT HEIGHT:1-8-3		SPAN:8-5		RISE:2-8-3		RIGHT HEIGHT:1-8-3	
LOADING (PSF)				MAX STRESSES		MINIMUM GRADE OF LUMBER	
TOP	L 30 D 15	TOP	3-4=0.193	TOP CHORD:	2X4 No.2ND 19 SP		
BOTT	0 10	BOTT	7-8=0.161	BOT CHORD:	2X4 No.2ND 19 SP		
		LL.DEFL.@8=0.01 < L/360	WEBS		:2X4 No.3 19 SP		
STR.INC.: LUMB = 1.33 PLATE = 1.33				SPACING : 24.0 in. o. c.			
REPETITIVE STRESSES USED				NO. OF MEMBERS = 1			

BOTH END VERTICALS ARE NOT EXPOSED.
 TRUSS HAS BEEN CHECKED FOR 145 M.P.H. WIND LOAD, WALL HEIGHT 16 FT, ENCLOSED BLDG. TYPE, ASSUMING 10 PSF TOTAL D.L.(7*3) (SBC-97).
 DEFLECTION(IN.) L.L= 0.01,D.L=0.01,T.L=0.01
 PLATES ARE MITEK M20-249,200 MANUFACTURED FROM ASTM A 446 GRD A GALVANIZED STEEL(EXCEPT AS SHOWN)
 PLATE MUST BE INSTALLED ON EA. FACE OF JOINT,SYMMETRICALLY(EXCEPT AS SHOWN)DESIGN CONFORMS W/NDS DESIGN SPECS, SBC,ANSI/TPI-95
 THIS DESIGN IS FOR TRUSS FABRICATION ONLY,FOR PERMANENT AND TEMPORARY BRACING(WHICH IS ALWAYS REQD)CONSULT BLDG ARCHITECT OR ENGINEER.

MAR 01 2001

====<<<ACES-32 Ver. 1.1>>>===== [001892] =====<<<FL.QUALITY-MMH>>>=====

Customer : FOGLIA Truss ID : S11 Thu Mar 1 10:09:46 2001

Project #: 00469 Quantity : 1 Family #: 101

Span : 8-5 Top Pitch : 6/12

==Bld(1/21/2000),v1.1=====

FORCES - GRAVITY LOADS

1-2=454 3-4= 393 2-4= 83

2-3=454 4-1= 393

REACTIONS - SIZE

1=454 2.00

3=454 2.00

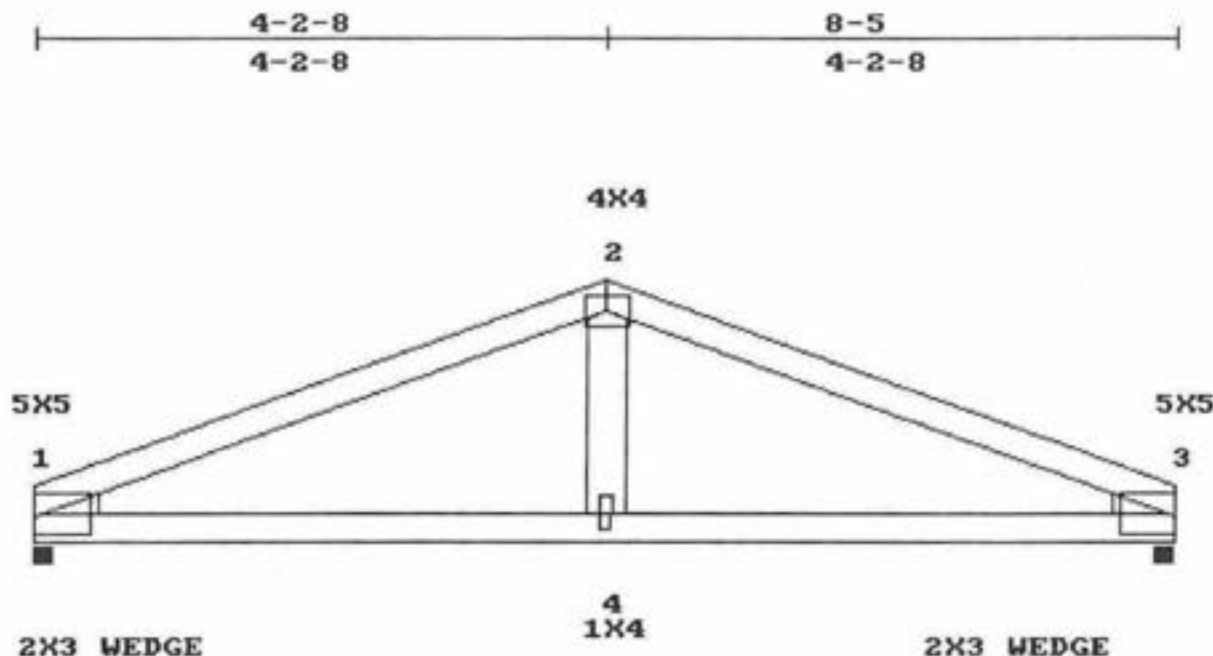
95

PROVIDE FOR 356 LBS UPLIFT AT JOINT 1 (0.79)

PROVIDE FOR 356 LBS UPLIFT AT JOINT 3 (0.79)

PROVIDE FOR 79 LBS HORIZ. REACTION AT JOINT 1

(SYMMETRIC PLATING TURNED ON)



L. HL TO PK:4-8-7

LEFT HEIGHT:0-7

SPAN:8-5

RISE:2-8-4

R. HL TO PK:4-8-7

RIGHT HEIGHT:0-7

LOADING (PSF)

TOP 30 15

BOTT 0 10

MAX STRESSES

TOP 2-3=0.598

BOTT 4-1=0.144

LL.DEFL.@4=0.01 < L/360

MINIMUM GRADE OF LUMBER

TOP CHORD:2X4 No.2ND 19 SP

BOT CHORD:2X4 No.2ND 19 SP

WEBS :2X4 No.3 19 SP

STR.INC.: LUMB = 1.33 PLATE = 1.33

REPETITIVE STRESSES USED

SPACING : 24.0 in. o. c.

NO. OF MEMBERS = 1

TRUSS HAS BEEN CHECKED FOR 145 M.P.H. WIND LOAD, WALL HEIGHT 16 FT, ENCLOSED BLDG. TYPE, ASSUMING 10 PSF TOTAL D.L.(7+3) (SBC-97).

DEFLECTION(IN.) L.L= 0.01,D.L=0.01,T.L=0.01

PLATES ARE MITEK M20-249,200 MANUFACTURED FROM ASTM A 446 GRD A GALVANIZED STEEL(EXCEPT AS SHOWN)

PLATE MUST BE INSTALLED ON EA. FACE OF JOINT,SYMMETRICALLY(EXCEPT AS SHOWN)DESIGN CONFORMS W/NDIS DESIGN SPECS, SSBC,ANSI/TPI-95

THIS DESIGN IS FOR TRUSS FABRICATION ONLY.FOR PERMANENT AND TEMPORARY BRACING(WHICH IS ALWAYS REQD)CONSULT BLDG ARCHITECT OR ENGINEER.

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MAR 01 2001

====<<<<ACES-32 Ver. 1.1>>>>=====[001893]=====<<<<FL.QUALITY-MMH>>>>=====

Customer : FOGLIA

Project # : 00469

Span : 9-4

Truss ID : H3

Quantity : 1

Thu Mar 1 10:09:59 2001

Family # : 314

Top Pitch : 6/12

==Bld(1/21/2000),v1.1=====

FORCES - LOAD CASE #1

1-2=1253 4-5= 1110 2-6= 316
2-3=1110 5-6= 1110 2-5= 0
3-4=1253 6-1= 1110 3-5= 316

REACTIONS - SIZE

1=908 2.00
4=908 2.00

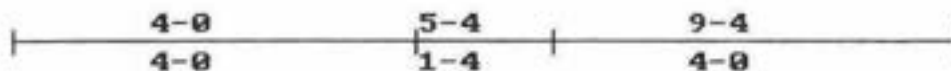
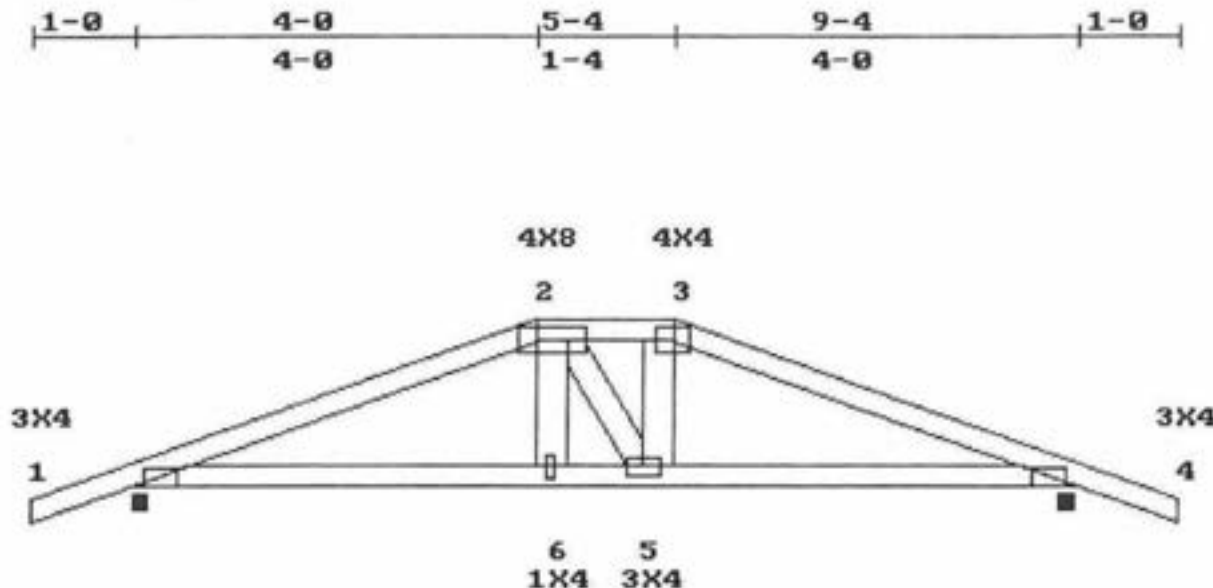
96

PROVIDE FOR 845 LBS UPLIFT AT JOINT 1 (0.93)

PROVIDE FOR 845 LBS UPLIFT AT JOINT 4 (0.93)

PROVIDE FOR 69 LBS HORIZ. REACTION AT JOINT 1

(SYMMETRIC PLATING TURNED ON)



L. HL TO PK:4-5-11

LEFT HEIGHT:0-4-3

SPAN:9-4

RISE:2-4-3

R. HL TO PK:4-5-11

RIGHT HEIGHT:0-4-3

LOADING (PSF)

TOP 30 15

BOTT 0 10

MAX STRESSES

TOP 3-4=0.644

BOTT 5-6=0.294

LL.DEFL.@6=0.02 < L/360

MINIMUM GRADE OF LUMBER

TOP CHORD:2X4 No.2ND 19 SP

BOT CHORD:2X4 No.2ND 19 SP

WEBS :2X4 No.3 19 SP

REPETITIVE STRESSES NOT USED

SPACING : 24.0 in. o. c.

NO. OF MEMBERS = 1

LOADING STRESS INCREASE LOADING PANEL(PLF) / JOINTS(LBS)

LUMBER PLATE TYPE

1 1.33 1.33

UNIFORM

CONCENTRATED

1- 2= 90

5= 257

2- 3= 135

6= 257

3- 4= 90

4- 5= 20

4- 5= 20

5- 6= 30

5- 6= 30

6- 1= 20

TRUSS HAS BEEN CHECKED FOR 145 M.P.H. WIND LOAD, WALL HEIGHT 16 FT, ENCLOSED BLDG. TYPE, ASSUMING 10 PSF TOTAL D.L.(7+3) (SBC-97).

DEFLECTION(IN.) L.L= 0.02,D.L=0.02,T.L=0.04

PLATES ARE MITEK M20-249,200 MANUFACTURED FROM ASTM A 446 GRD A GALVANIZED STEEL(EXCEPT AS SHOWN)

PLATE MUST BE INSTALLED ON EA. FACE OF JOINT,SYMMETRICALLY(EXCEPT AS SHOWN)DESIGN CONFORMS W/NDG DESIGN SPECS, SSBC,ANSI/TPI-95

THIS DESIGN IS FOR TRUSS FABRICATION ONLY.FOR PERMANENT AND TEMPORARY BRACING(WHICH IS ALWAYS REQD)CONSULT BLDG ARCHITECT OR ENGINEER.

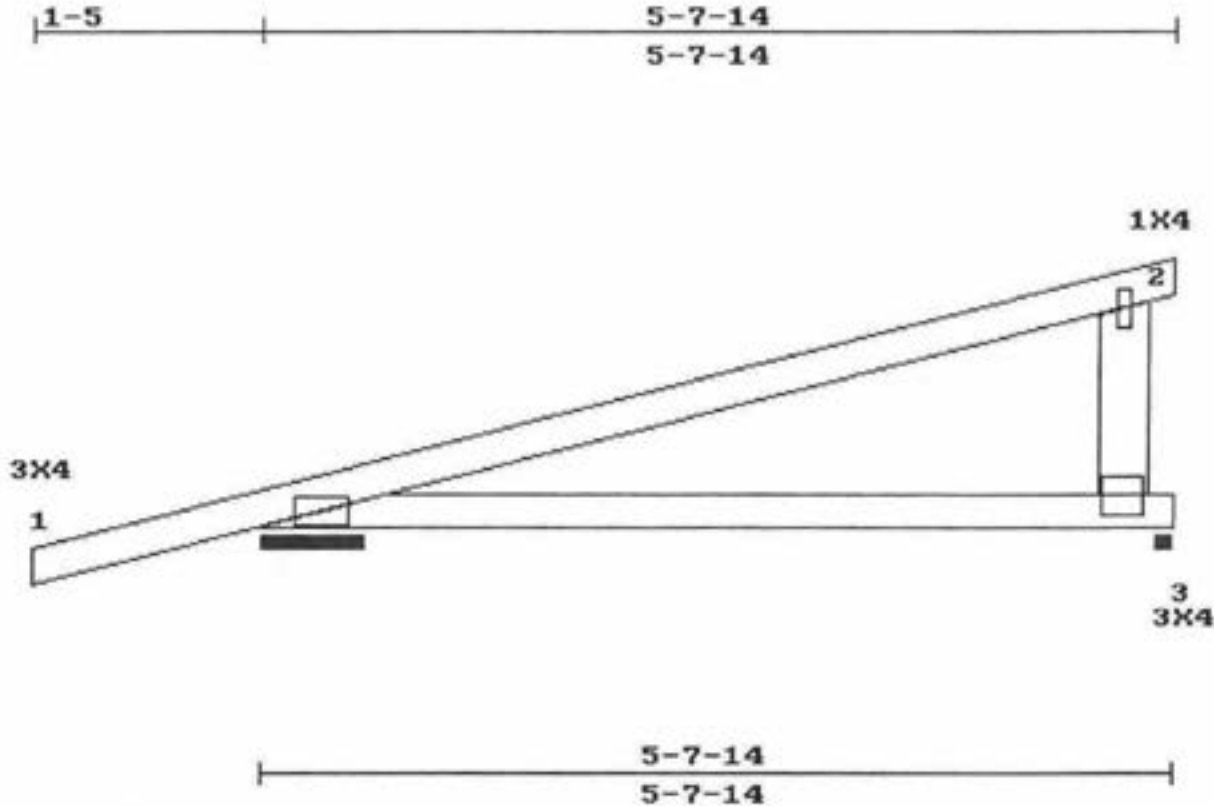
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MAR 0 1 2001

====<<<<ACES-32 Ver. 1.1>>>>====[001894]====<<<<FL.QUALITY-MMH>>>>====
 Customer : FOGLIA Thu Mar 1 10:10:09 2001
 Project #: 00469 Truss ID : CJ5 Family #: 204
 Span : 5-7-14 Quantity : 2 Top Pitch : 4.243/12

==Bld(1/21/2000),v1.1=====

FORCES - LOAD CASE #1		REACTIONS - SIZE	
1-2=0	3-1=0	1=370	8.00
2-3=-175		3=-214	1.50

PROVIDE FOR 575 LBS UPLIFT AT JOINT 1 (1.55)
 PROVIDE FOR 484 LBS UPLIFT AT JOINT 3 (2.26)
 PROVIDE FOR 232 LBS HORIZ. REACTION AT JOINT 1
 (SYMMETRIC PLATING TURNED ON)



L. HL TO PK:6-0		SPAN:5-7-14		RISE:2-3-15		RIGHT HEIGHT:2-3-15	
LEFT HEIGHT:0-3-15							
LOADING (PSF)		MAX STRESSES		MINIMUM GRADE OF LUMBER			
TOP	30 15	TOP	1-2=0.603	TOP CHORD:2X4 No.2ND 19 SP			
BOTT	0 10	BOTT	3-1=0.115	BOT CHORD:2X4 No.2ND 19 SP			
		LL.DEFL. < L/360		WEBS :2X4 No.3 19 SP			

REPETITIVE STRESSES NOT USED

SPACING : 24.0 in. o. c.
 NO. OF MEMBERS = 1

LOADING STRESS INCREASE	LOADING	PANEL(PLF) / JOINTS(LBS)
LUMBER PLATE	TYPE	
1 1.33 1.33	UNIFORM	1- 2= 67 3- 1= 15

RIGHT END VERTICAL IS NOT EXPOSED.
 TRUSS HAS BEEN CHECKED FOR 145 M.P.H. WIND LOAD, WALL HEIGHT 16 FT, ENCLOSED BLDG. TYPE, ASSUMING 10 PSF TOTAL D.L.(7+3) (SBC-97).
 DEFLECTION(IN.) L.L= 0.00,D.L=0.00,T.L=0.00
 PLATES ARE MITEK M20-249,200 MANUFACTURED FROM ASTM A 446 GRD A GALVANIZED STEEL(EXCEPT AS SHOWN)
 PLATE MUST BE INSTALLED ON EA. FACE OF JOINT,SYMMETRICALLY(EXCEPT AS SHOWN)DESIGN CONFORMS W/ND S DESIGN SPECS, S58C,ANSI/TP1-95
 THIS DESIGN IS FOR TRUSS FABRICATION ONLY.FOR PERMANENT AND TEMPORARY BRACING(WHICH IS ALWAYS REQD)CONSULT BLDG ARCHITECT OR ENGINEER.

MAR 01 2001


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====<<<ACES-32 Ver. 1.1>>>===== [ 001895 ] =====<<<FL.Quality-MMH>>>=====
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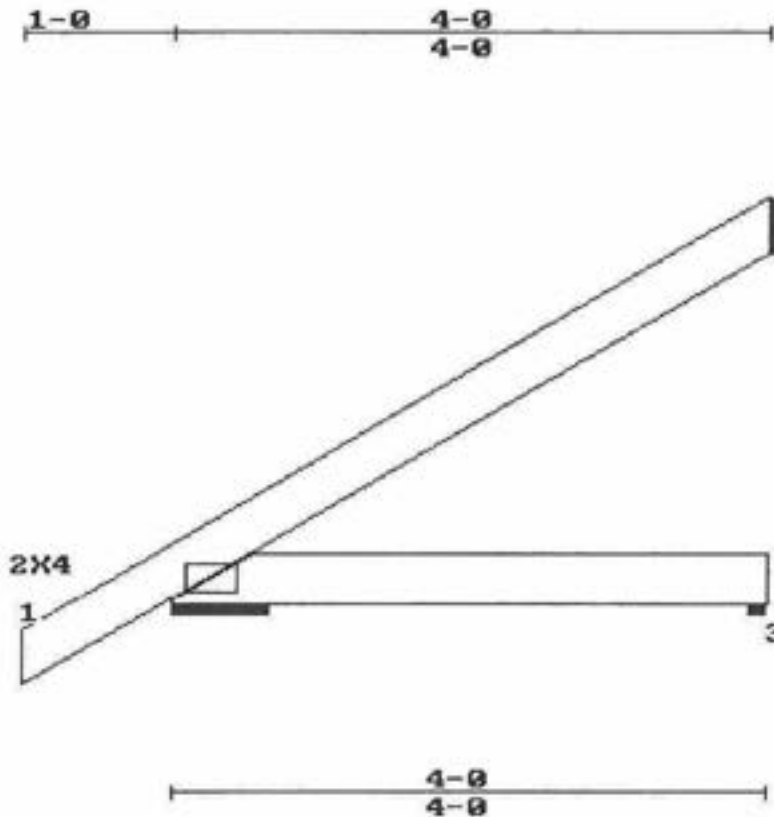
Customer : FOGLIA		Thu Mar 1 10:10:20 2001
Project #: 00469	Truss ID : J4	Family # : 205
Span : 4-0	Quantity : 2	Top Pitch : 6/12

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==Bld(1/21/2000).v1.1=====
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FORCES - GRAVITY LOADS
1-2=0
2-3=0
3-1=0

REACTIONS - SIZE	
1=-308	8.00
3=-36	1.50
2=-162	1.50

PROVIDE FOR 239 LBS UPLIFT AT JOINT 1 (0.78)
PROVIDE FOR 250 LBS UPLIFT AT JOINT 2 (1.54)
PROVIDE FOR 231 LBS HORIZ. REACTION AT JOINT 1
(SYMMETRIC PLATING TURNED ON)



L. HL TO PK:4-5-11
LEFT HEIGHT:0-4-3 SPAN:4-0 RISE:2-4-3 RIGHT HEIGHT:2-4-3

LOADING (PSF)		MAX STRESSES		MINIMUM GRADE OF LUMBER	
	L D	TOP	1-2=0.429	TOP CHORD:2X4	No.2ND 19 SP
TOP	30 15	BOTT	3-1=0.064	BOT CHORD:2X4	No.2ND 19 SP
BOTT	0 10	LL.DEFL.	< L/360	WEBS :2X4	No.3 19 SP
STR.INC.: LUMB = 1.33 PLATE = 1.33				SPACING : 24.0 in. o. c.	
REPETITIVE STRESSES USED				NO. OF MEMBERS = 1	

TRUSS HAS BEEN CHECKED FOR 145 M.P.H. WIND LOAD, WALL HEIGHT 16 FT, ENCLOSED BLDG. TYPE, ASSUMING 10 PSF TOTAL D.L. (7+3) (SBC-97).
DEFLECTION (IN.) L.L= 0.00, D.L=0.00, T.L=0.00
PLATES ARE MITEK M20-249,200 MANUFACTURED FROM ASTM A 446 GRD A GALVANIZED STEEL (EXCEPT AS SHOWN)
PLATE MUST BE INSTALLED ON EA. FACE OF JOINT, SYMMETRICALLY (EXCEPT AS SHOWN) DESIGN CONFORMS W/ WDS DESIGN SPECS, SBC, ANSI/TPI-95
THIS DESIGN IS FOR TRUSS FABRICATION ONLY. FOR PERMANENT AND TEMPORARY BRACING (WHICH IS ALWAYS REQD) CONSULT BLDG ARCHITECT OR ENGINEER.

MAR 01 2001

====<<<ACES-32 Ver. 1.1>>>===== [001896] =====<<<FL.QUALITY-MMH>>>=====

Customer : FOGLIA Truss ID : GT6A Thu Mar 1 10:10:36 2001

Project #: 00469 Quantity : 1 Family #: 201

Span : 6-0 Top Pitch : 6/12

====5ld(1/21/2000),v1.1=====

FORCES - LOAD CASE #1 REACTIONS - SIZE

1-2=877	4-5= 770	2-5= 549	1=-801	8.00
2-3= 0	5-1= 770	2-4=-859	4=-801	1.50
3-4=-132				

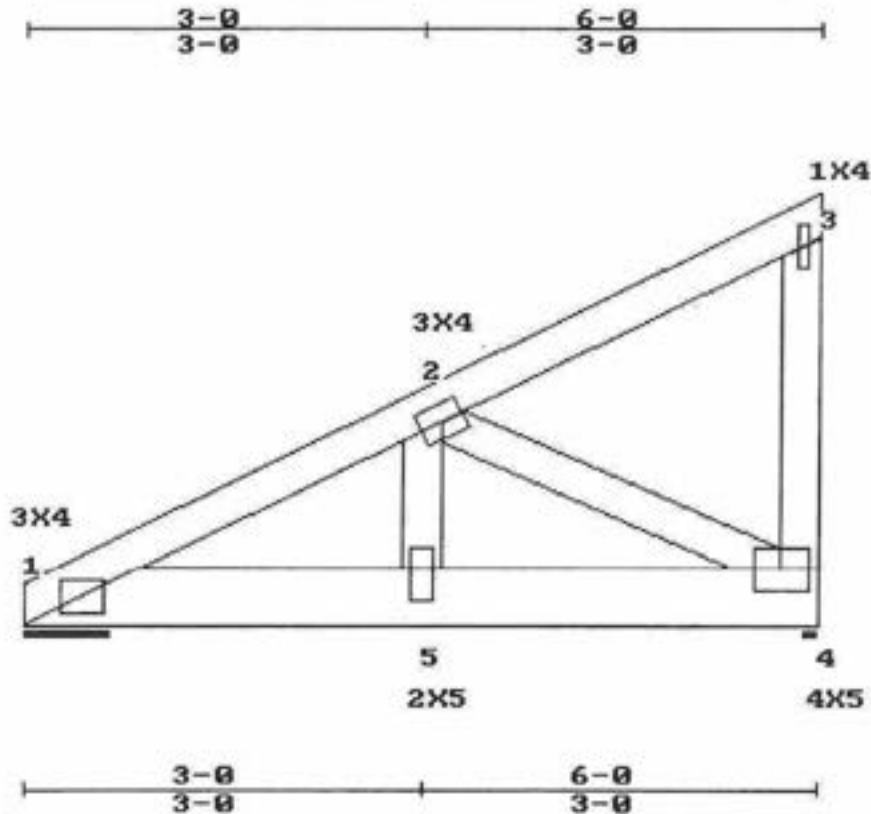
89

PROVIDE FOR 908 LBS UPLIFT AT JOINT 1 (1.13)

PROVIDE FOR 1082 LBS UPLIFT AT JOINT 4 (1.35)

PROVIDE FOR 338 LBS HORIZ. REACTION AT JOINT 1

PLATE OFFSETS (X=LEFT,Y=TOP):[J5=3,1],(SYMMETRIC PLATING TURNED ON)



L. HL TO PK:6-8-8 SPAN:6-0 RISE:3-4-3 RIGHT HEIGHT:3-4-3

LEFT HEIGHT:0-4-3

=====

LOADING (PSF)	MAX STRESSES	MINIMUM GRADE OF LUMBER
L D	TOP 1-2=0.327	TOP CHORD:2X4 No.2ND 19 SP
TOP 30 15	BOTT 4-5=0.234	BOT CHORD:2X6 No.2D 19 SP
BOTT 0 10	LL.DEFL.@5=0.01 < L/360	WEBS :2X4 No.3 19 SP

=====

REPETITIVE STRESSES NOT USED SPACING : 24.0 in. o. c.

NO. OF MEMBERS = 1

LOADING STRESS INCREASE	LOADING	PANEL(PLF) / JOINTS(LBS)
LUMBER PLATE	TYPE	
1 1.33 1.33	UNIFORM	1- 3= 90 4- 1= 196

RIGHT END VERTICAL IS NOT EXPOSED.

TRUSS HAS BEEN CHECKED FOR 145 M.P.H. WIND LOAD, WALL HEIGHT 16 FT, ENCLOSED BLDG. TYPE, ASSUMING 10 PSF TOTAL D.L.(7+3) (SBC-97).

DEFLECTION(IN.) L.L= 0.01,D.L=0.01,T.L=0.02

PLATES ARE MITEK M20-249,200 MANUFACTURED FROM ASTM A 446 GRD A GALVANIZED STEEL(EXCEPT AS SHOWN)

PLATE MUST BE INSTALLED ON EA. FACE OF JOINT,SYMMETRICALLY(EXCEPT AS SHOWN)DESIGN CONFORMS W/NDS DESIGN SPECS, SBC,ANSI/TPI-95

THIS DESIGN IS FOR TRUSS FABRICATION ONLY.FOR PERMANENT AND TEMPORARY BRACING(WHICH IS ALWAYS REQD)CONSULT BLDG ARCHITECT OR ENGINEER.

[Signature]

MAR 01 2001

====<<<<ACES-32 Ver. 1.1>>>>=====[001897]=====<<<<FL.QUALITY-MMH>>>>=====

Customer : FOGLIA

Project # : 00469

Span : 6-0

Truss ID : CA5

Quantity : 1

Thu Mar 1 10:10:48 2001

Family # : 205

Top Pitch : 6/12

==Bld(1/21/2000),v1.1=====

FORCES - GRAVITY LOADS

1-2=0

3-1=0

REACTIONS - SIZE

1=418 8.00

3=56 1.50

2=252 1.50

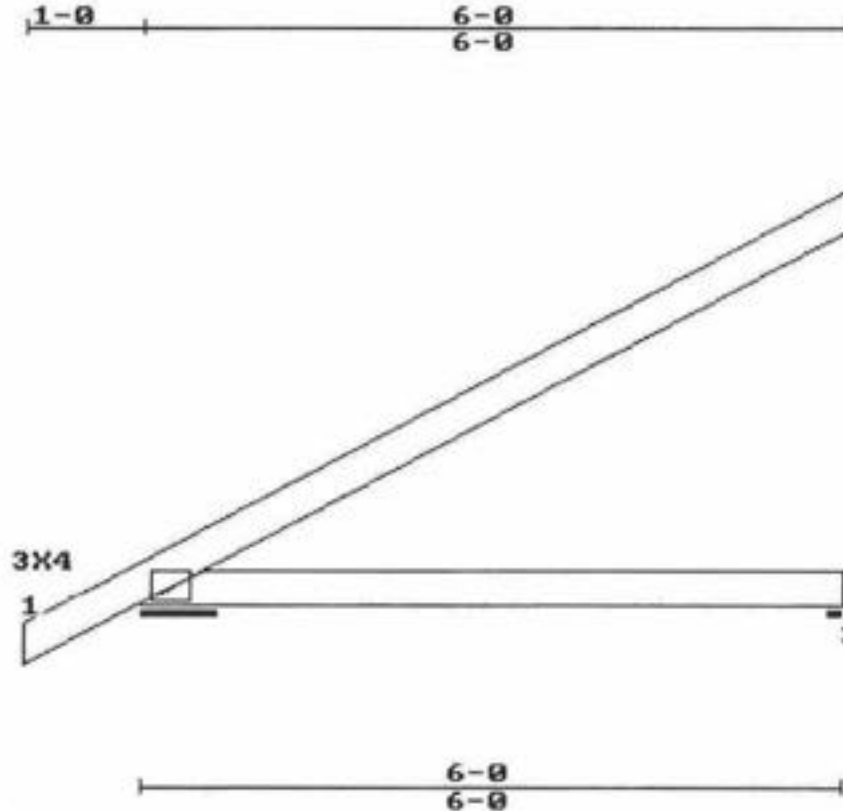
90

PROVIDE FOR 307 LBS UPLIFT AT JOINT 1 (0.73)

PROVIDE FOR 383 LBS UPLIFT AT JOINT 2 (1.52)

PROVIDE FOR 348 LBS HORIZ. REACTION AT JOINT 1

(SYMMETRIC PLATING TURNED ON)



L. HL TO PK:6-8-8

LEFT HEIGHT:0-4-3

SPAN:6-0

RISE:3-4-3

RIGHT HEIGHT:3-4-3

LOADING (PSF)

L D

MAX STRESSES

TOP 1-2=0.712

BOTT 3-1=0.152

LL.DEFL. < L/360

MINIMUM GRADE OF LUMBER

TOP CHORD:2X4 No.1D 19 SP

BOT CHORD:2X4 No.2ND 19 SP

WEBS :2X4 No.3 19 SP

STR.INC.: LUMB = 1.33 PLATE = 1.33

REPETITIVE STRESSES USED

SPACING : 24.0 in. o. c.

NO. OF MEMBERS = 1

TRUSS HAS BEEN CHECKED FOR 145 M.P.H. WIND LOAD, WALL HEIGHT 16 FT, ENCLOSED BLDG. TYPE, ASSUMING 10 PSF TOTAL D.L.(7+3) (SBC-97).

DEFLECTION(IN.) L.L= 0.00,D.L=0.00,T.L=0.00

PLATES ARE MITEK M20-249,200 MANUFACTURED FROM ASTM A 446 GRD A GALVANIZED STEEL(EXCEPT AS SHOWN)

PLATE MUST BE INSTALLED ON EA. FACE OF JOINT,SYMMETRICALLY(EXCEPT AS SHOWN)DESIGN CONFORMS W/NDS DESIGN SPECS, SSSC,ANSI/TP1-95

THIS DESIGN IS FOR TRUSS FABRICATION ONLY.FOR PERMANENT AND TEMPORARY BRACING(WHICH IS ALWAYS REQD)CONSULT BLDG ARCHITECT OR ENGINEER.

[Signature]
MAR 01 2001

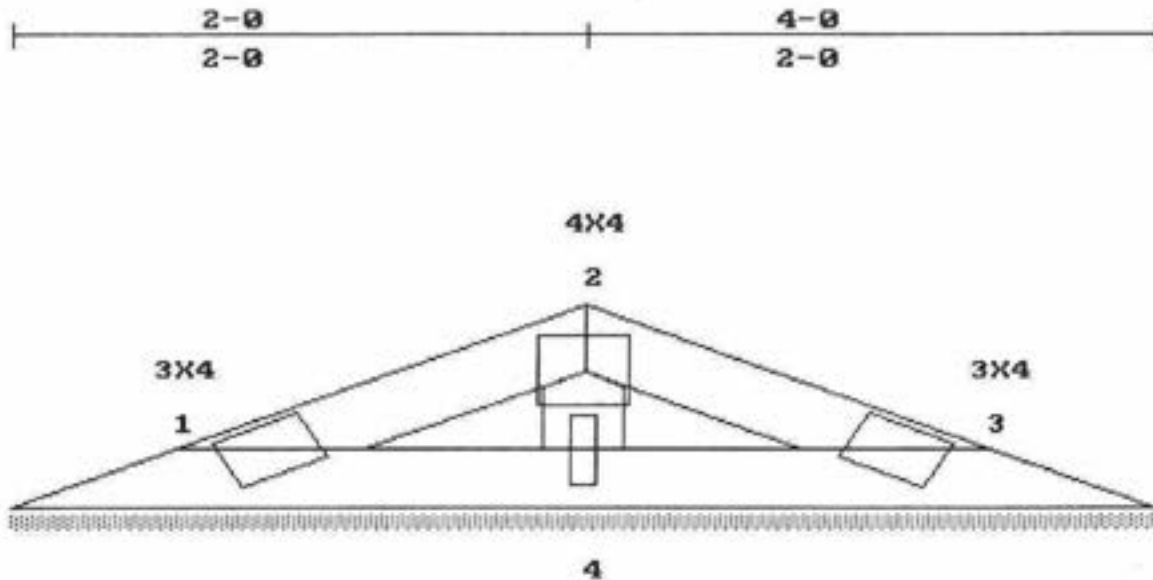
====<<<<ACES-32 Ver. 1.1>>>>===== [001898] =====<<<<FL.QUALITY-MMH>>>>=====

Customer : FOGLIA Thu Mar 1 10:10:57 2001
 Project #: 00469 Truss ID : V4 Family # : 101
 Span : 4-0 Quantity : 1 Top Pitch : 6/12

==81d(1/21/2000),v1.1=====

NOTES: (1) -Gable studs spaced at 24 inches o.c.
 (2) -Brace vertical studs in accordance with standard gable end detail (91)
 (3) -Continuous bearing provided along entire bottom chord
 (4) -Provide 1X4 plates at each end of gable stud unless otherwise noted

(SYMMETRIC PLATING TURNED ON)



L. HL TO PK:2-2-13 LEFT HEIGHT:0-0 SPAN:4-0 RISE:1-0 R. HL TO PK :2-2-13 RIGHT HEIGHT:0-0

LOADING (PSF)		MAX STRESSES	MINIMUM GRADE OF LUMBER	
TOP	30 15		TOP CHORD:2X4	No.2ND 19 SP
BOTT	0 10		BOT CHORD:2X4	No.2ND 19 SP
		LL.DEFL.@0=0.00 < L/360	WEBS	:2X4 No.3 19 SP

STR.INC.: LUMB = 1.33 PLATE = 1.33 SPACING : 24.0 in. o. c.
 REPETITIVE STRESSES USED NO. OF MEMBERS = 1

TRUSS HAS BEEN CHECKED FOR 145 M.P.H. WIND LOAD, WALL HEIGHT 16 FT, ENCLOSED BLDG. TYPE, ASSUMING 10 PSF TOTAL D.L.(7+3) (SBC-97).
 DEFLECTION(IN.) L.L= 0.00,D.L=0.00,T.L=0.00
 PLATES ARE MITEK M20-249,200 MANUFACTURED FROM ASTM A 446 GRD A GALVANIZED STEEL(EXCEPT AS SHOWN)
 PLATE MUST BE INSTALLED ON EA. FACE OF JOINT,SYMMETRICALLY(EXCEPT AS SHOWN)DESIGN CONFORMS W/ND5 DESIGN SPECS, SSBC,ANSI/TPI-95
 THIS DESIGN IS FOR TRUSS FABRICATION ONLY.FOR PERMANENT AND TEMPORARY BRACING(WHICH IS ALWAYS REQD)CONSULT BLDG ARCHITECT OR ENGINEER.

[Signature]
 9 MAR 6 1 2001

====<<<<ACES-32 Ver. 1.1>>>>===== [001899] =====<<<<FL.QUALITY-MMH>>>>=====

Customer : FOGLIA

Project #: 00469

Span : 8-0

Truss ID : V8

Quantity : 1

Thu Mar 1 10:11:08 2001

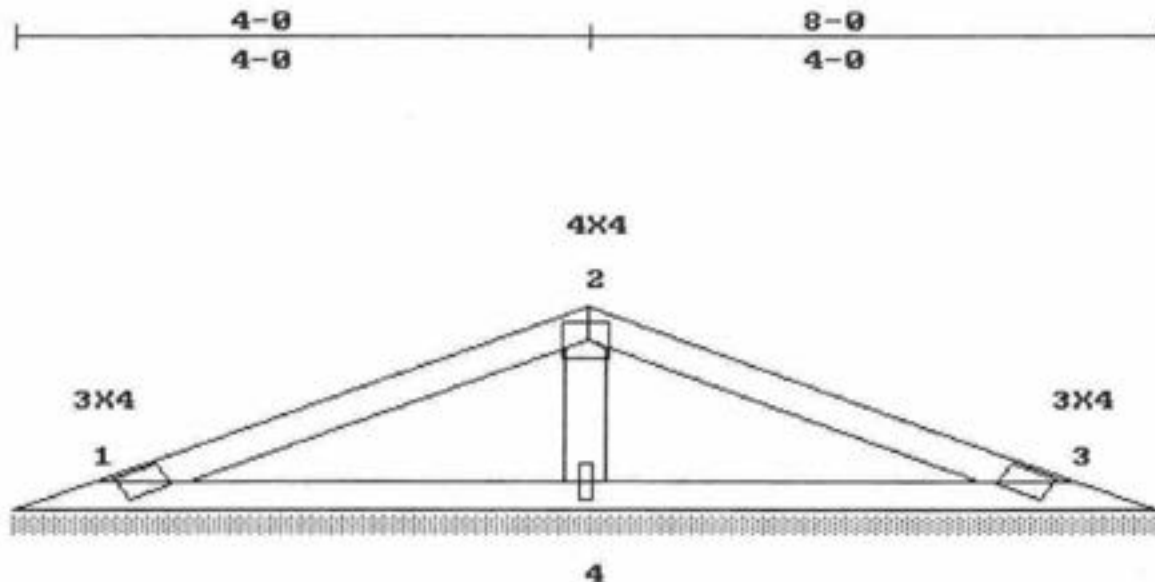
Family #: 101

Top Pitch : 6/12

==81d(1/21/2000),v1.1=====

NOTES: (1) -Gable studs spaced at 24 inches o.c.
(2) -Brace vertical studs in accordance with standard gable end detail (92)
(3) -Continuous bearing provided along entire bottom chord
(4) -Provide 1X4 plates at each end of gable stud unless otherwise noted

(SYMMETRIC PLATING TURNED ON)



L. HL TO PK:4-5-11

LEFT HEIGHT:0-0

SPAN:8-0

RISE:2-0

R. HL TO PK:4-5-11

RIGHT HEIGHT:0-0

LOADING (PSF)

MAX STRESSES

MINIMUM GRADE OF LUMBER

TOP 30 15

BOTT 0 10

LL.DEFL.@0=0.00 < L/360

TOP CHORD:2X4 No.2ND 19 SP

BOT CHORD:2X4 No.2ND 19 SP

WEBS :2X4 No.3 19 SP

STR.INC.: LUMB = 1.33 PLATE = 1.33

SPACING : 24.0 in. o. c.

REPETITIVE STRESSES USED

NO. OF MEMBERS = 1

TRUSS HAS BEEN CHECKED FOR 145 M.P.H. WIND LOAD, WALL HEIGHT 16 FT, ENCLOSED BLDG. TYPE, ASSUMING 10 PSF TOTAL D.L.(7+3) (SBC-97).

DEFLECTION(IN.) L.L= 0.00,D.L=0.00,T.L=0.00

PLATES ARE MITEK M20-249,200 MANUFACTURED FROM ASTM A 446 GRD A GALVANIZED STEEL(EXCEPT AS SHOWN)

PLATE MUST BE INSTALLED ON EA. FACE OF JOINT,SYMMETRICALLY(EXCEPT AS SHOWN)DESIGN CONFORMS W/ND S DESIGN SPECS, SSBC,ANSI/TP1-95

THIS DESIGN IS FOR TRUSS FABRICATION ONLY.FOR PERMANENT AND TEMPORARY BRACING(WHICH IS ALWAYS REQD)CONSULT BLDG ARCHITECT OR ENGINEER.

[Signature]

MAR 01 2001

====<<<ACES-32 Ver. 1.1>>>===== [001900] =====<<<FL.QUALITY-MMH>>>=====

Customer : FOGLIA

Project #: 00469

Span : 12-0

Truss ID : V12

Quantity : 1

Thu Mar 1 10:11:17 2001

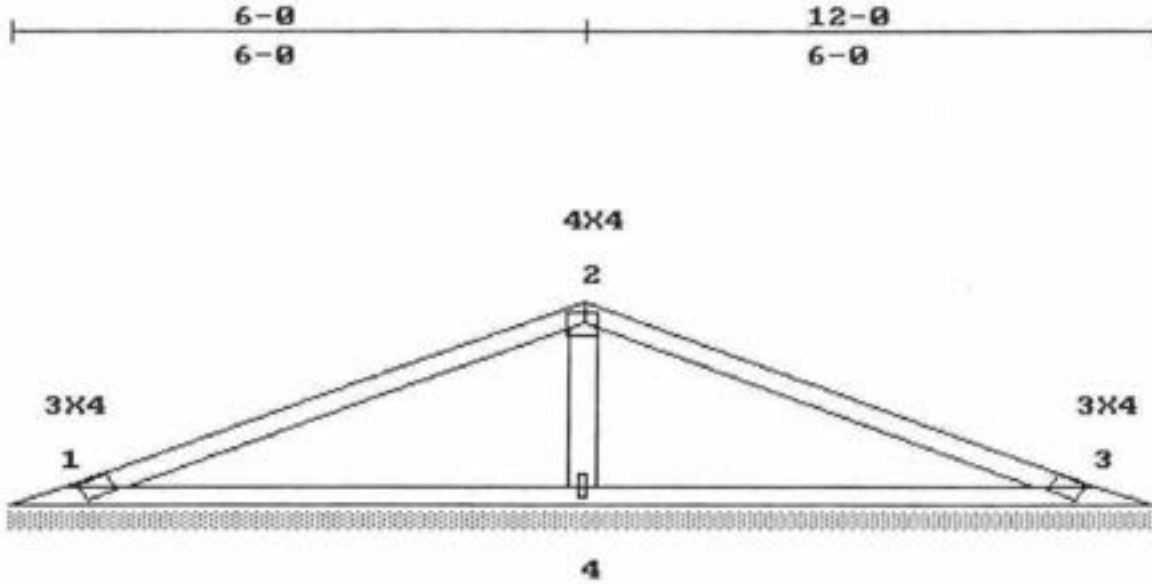
Family # : 101

Top Pitch : 6/12

==Bld(1/21/2000),v1.1=====

NOTES: (1) -Gable studs spaced at 48 inches o.c.
(2) -Brace vertical studs in accordance with standard gable end detail (93)
(3) -Continuous bearing provided along entire bottom chord
(4) -Provide 1X4 plates at each end of gable stud unless otherwise noted

(SYMMETRIC PLATING TURNED ON)



L. HL TO PK:6-8-8

LEFT HEIGHT:0-0

SPAN:12-0

RISE:3-0

R. HL TO PK :6-8-8

RIGHT HEIGHT:0-0

LOADING (PSF)

MAX STRESSES

MINIMUM GRADE OF LUMBER

TOP 30 15

TOP CHORD:2X4 No.2ND 19 SP

BOTT 0 10

BOT CHORD:2X4 No.2ND 19 SP

LL.DEFL.@0=0.00 < L/360

WEBS :2X4 No.3 19 SP

STR.INC.: LUMB = 1.33 PLATE = 1.33

SPACING : 24.0 in. o. c.

REPETITIVE STRESSES USED

NO. OF MEMBERS = 1

TRUSS HAS BEEN CHECKED FOR 145 M.P.H. WIND LOAD, WALL HEIGHT 16 FT, ENCLOSED BLDG. TYPE, ASSUMING 10 PSF TOTAL D.L.(7+3) (SBC-97).

DEFLECTION(IN.) L.L= 0.00,D.L=0.00,T.L=0.00

PLATES ARE MITEK M20-249,200 MANUFACTURED FROM ASTM A 446 GRD A GALVANIZED STEEL(EXCEPT AS SHOWN)

PLATE MUST BE INSTALLED ON EA. FACE OF JOINT,SYMMETRICALLY(EXCEPT AS SHOWN)DESIGN CONFORMS W/NDS DESIGN SPECS, SBC,ANSI/TPI-95

THIS DESIGN IS FOR TRUSS FABRICATION ONLY.FOR PERMANENT AND TEMPORARY BRACING(WHICH IS ALWAYS REQD)CONSULT BLDG ARCHITECT OR ENGINEER.

[Signature]

MAR 01 2001

====<<<<ACES-32 Ver. 1.1>>>>===== [001901]=====<<<<FL.QUALITY-MMH>>>>=====

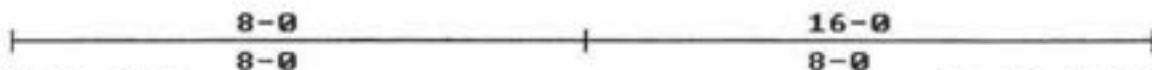
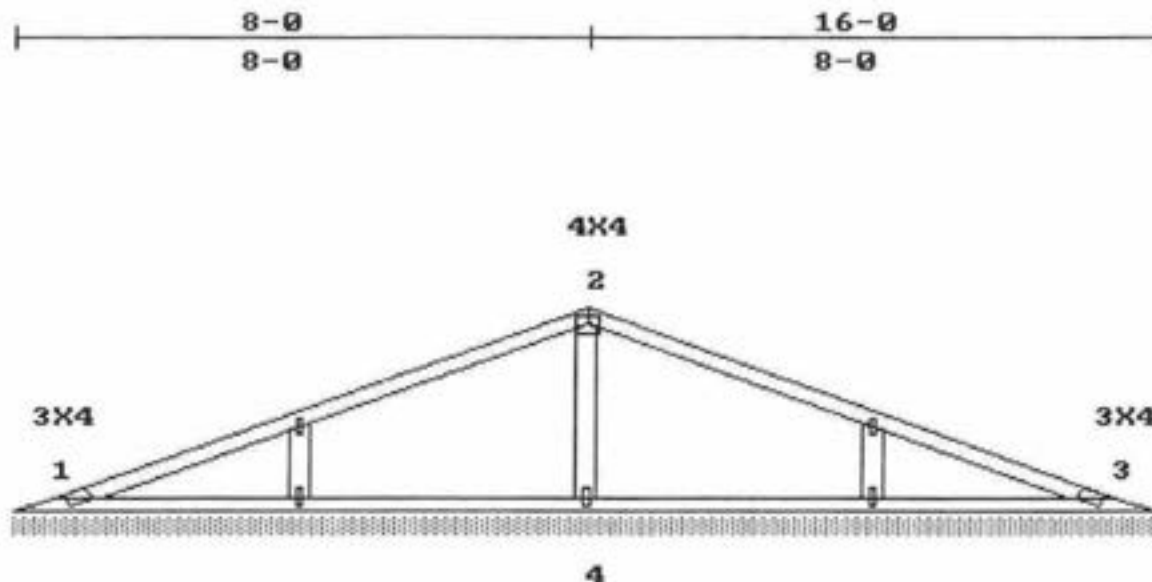
Customer : FOGLIA	Truss ID : V16	Thu Mar 1 10:11:25 2001
Project #: 00469	Quantity : 1	Family # : 101
Span : 16-0		Top Pitch : 6/12

==Bld(1/21/2000), v1.1=====

NOTES: (1) -Gable studs spaced at 48 inches o.c.
 (2) -Brace vertical studs in accordance with standard gable end detail
 (3) -Continuous bearing provided along entire bottom chord
 (4) -Provide 1X4 plates at each end of gable stud unless otherwise noted

94

(SYMMETRIC PLATING TURNED ON)



L. HL TO PK:8-11-5 LEFT HEIGHT:0-0 SPAN:16-0 RISE:4-0 R. HL TO PK :8-11-5 RIGHT HEIGHT:0-0

LOADING (PSF)		MAX STRESSES	MINIMUM GRADE OF LUMBER	
L	D		TOP CHORD:2X4	No.2ND 19 SP
TOP	30	15	BOT CHORD:2X4	No.2ND 19 SP
BOTT	0	10	WEBS	:2X4 No.3 19 SP
LL.DEFL.@0=0.00 < L/360				
STR.INC.: LUMB = 1.33 PLATE = 1.33			SPACING : 24.0 in. o. c.	
REPETITIVE STRESSES USED			NO. OF MEMBERS = 1	

TRUSS HAS BEEN CHECKED FOR 145 M.P.H. WIND LOAD, WALL HEIGHT 16 FT, ENCLOSED BLDG. TYPE, ASSUMING 10 PSF TOTAL D.L.(7+3) (SBC-97).
 DEFLECTION(IN.) L.L= 0.00,D.L=0.00,T.L=0.00
 PLATES ARE MITEK M20-249,200 MANUFACTURED FROM ASTM A 446 GRD A GALVANIZED STEEL(EXCEPT AS SHOWN)
 PLATE MUST BE INSTALLED ON EA. FACE OF JOINT,SYMMETRICALLY(EXCEPT AS SHOWN)DESIGN CONFORMS W/NDS DESIGN SPECS, SSB,ANSI/TP1-95
 THIS DESIGN IS FOR TRUSS FABRICATION ONLY.FOR PERMANENT AND TEMPORARY BRACING(WHICH IS ALWAYS REQD)CONSULT BLDG ARCHITECT OR ENGINEER.

[Signature]
 MAR 01 2001

====<<<ACES-32 Ver. 1.1>>>=====[002000]====<<<FL.QUALITY-MMH>>>=====
 Customer : FOGLIA Truss ID : T9A Wed Mar 7 07:11:38 2001
 Project #: 00469 Family #: 101
 Span : 14-8 B.Span : 14-8 Quant. : 1 Top Pitch : 6/12
 ==Bld(1/21/2000),v1.1=====

FORCES - GRAVITY LOADS

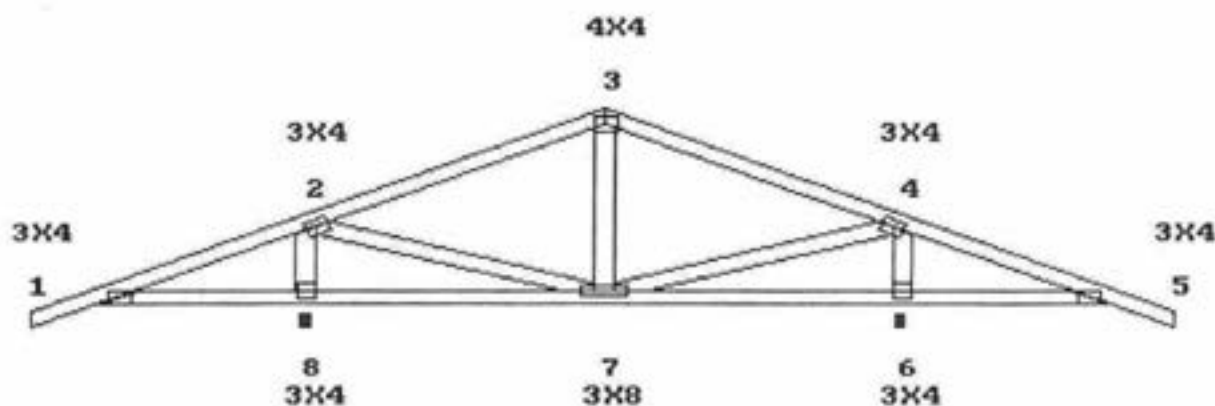
1-2= 603 5-6=-536 2-8=-843
 2-3=-63 6-7=-536 2-7= 629
 3-4=-63 7-8=-536 3-7=-334
 4-5= 603 8-1=-536 4-7= 629
 4-6=-843

REACTIONS - SIZE

8=917 2.00
 6=917 2.00

95

PROVIDE FOR 1174 LBS UPLIFT AT JOINT 8 (1.28)
 PROVIDE FOR 1174 LBS UPLIFT AT JOINT 6 (1.28)
 PROVIDE FOR 124 LBS HORIZ. REACTION AT JOINT 8



L. HL TO PK: 8-2-6 LEFT HEIGHT: 0-4-3 SPAN: 14-8 RISE: 4-0-3 R. HL TO PK: 8-2-6 RIGHT HEIGHT: 0-4-3

LOADING (PSF)		MAX STRESSES		MINIMUM GRADE OF LUMBER	
TOP	30	15	TOP 1-2=0.436	TOP CHORD: 2X4	No.2ND 19 SP
BOTT	0	10	BOTT 5-6=0.616	BOT CHORD: 2X4	No.2ND 19 SP
			LL.DEFL.@5=0.02 < L/360	WEBS: 2X4	No.3 19 SP

STR.INC.: LUMB = 1.33 PLATE = 1.33 SPACING : 24.0 in. o. c.
 REPETITIVE STRESSES USED NO. OF MEMBERS = 1

TRUSS HAS BEEN CHECKED FOR 145 M.P.H. WIND LOAD, WALL HEIGHT 16 FT, ENCLOSED BLDG. TYPE, ASSUMING 10 PSF TOTAL D.L.(7+3) (SBC-97).
 DEFLECTION(IN.) L.L= 0.02,D.L=0.02,T.L=0.05
 PLATES ARE MITEK M20-249,200 MANUFACTURED FROM ASTM A 446 GRD A GALVANIZED STEEL(EXCEPT AS SHOWN)
 PLATE MUST BE INSTALLED ON EA. FACE OF JOINT,SYMMETRICALLY(EXCEPT AS SHOWN)DESIGN CONFORMS W/ND'S DESIGN SPECS, SSB,ANSI/TP1-95
 THIS DESIGN IS FOR TRUSS FABRICATION ONLY.FOR PERMANENT AND TEMPORARY BRACING(WHICH IS ALWAYS REQD)CONSULT BLDG ARCHITECT OR ENGINEER.

[Signature]
 MAR 06 2001

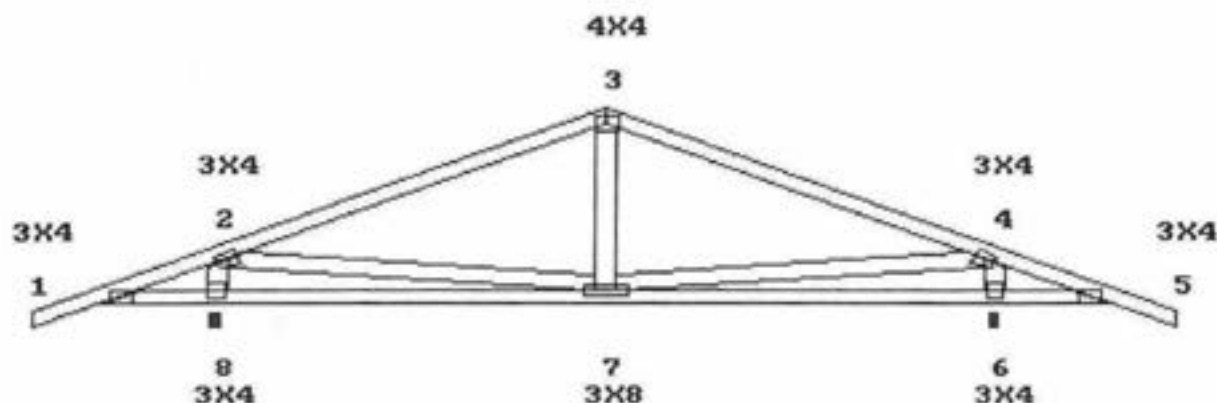
====<<<ACES-32 Ver. 1.1>>>=====[002001]====<<<FL.QUALITY-MMH>>>=====
 Customer : FOGLIA Wed Mar 7 07:11:48 2001
 Project #: 00469 Truss ID : T9 Family #: 101
 Span : 14-8 B.Span : 14-8 Quant. : 1 Top Pitch : 6/12
 ==Bld(1/21/2000),v1.1=====

FORCES - GRAVITY LOADS			REACTIONS - SIZE	
1-2= 435	5-6=386	2-8=843	8=917	2.00
2-3=432	6-7=386	2-7= 781	6=917	2.00
3-4=432	7-8=386	3-7=124		
4-5= 435	8-1=386	4-7= 781		
		4-6=843		

96

PROVIDE FOR 988 LBS UPLIFT AT JOINT 8 (1.08)
 PROVIDE FOR 988 LBS UPLIFT AT JOINT 6 (1.08)
 PROVIDE FOR 124 LBS HORIZ. REACTION AT JOINT 8

1-0 1-8 7-4 13-0 14-8 1-0
 1-8 5-8 5-8 1-8



1-8 7-4 13-0 14-8
 1-8 5-8 5-8 1-8

L. HL TO PK:8-2-6 LEFT HEIGHT:0-4-3 SPAN:14-8 RISE:4-0-3 R. HL TO PK:8-2-6 RIGHT HEIGHT:0-4-3

LOADING (PSF)		MAX STRESSES		MINIMUM GRADE OF LUMBER	
L	D	TOP	BOTT	TOP CHORD	BOT CHORD
30	15	2-3=0.869	5-6=0.860	2X4 No.2ND 19 SP	2X4 No.2ND 19 SP
0	10	LL.DEFL.@5=0.02 < L/360		WEBS	2X4 No.3 19 SP

STR.INC.: LUMB = 1.33 PLATE = 1.33 SPACING : 24.0 in. o. c.
 REPETITIVE STRESSES USED NO. OF MEMBERS = 1

TRUSS HAS BEEN CHECKED FOR 145 M.P.H. WIND LOAD, WALL HEIGHT 16 FT, ENCLOSED BLDG. TYPE, ASSUMING 10 PSF TOTAL D.L.(7+3) (SBC-97).
 DEFLECTION(IN.) L.L= 0.02,D.L=0.01,T.L=0.03
 PLATES ARE MITEK M20-249,200 MANUFACTURED FROM ASTM A 446 GRD A GALVANIZED STEEL(EXCEPT AS SHOWN)
 PLATE MUST BE INSTALLED ON EA. FACE OF JOINT,SYMMETRICALLY(EXCEPT AS SHOWN)DESIGN CONFORMS W/ND5 DESIGN SPECS, S5BC,ANSI/TPI-95
 THIS DESIGN IS FOR TRUSS FABRICATION ONLY.FOR PERMANENT AND TEMPORARY BRACING(WHICH IS ALWAYS REQD)CONSULT BLDG ARCHITECT OR ENGINEER.

MAR 06 2001

====<<<ACES-32 Ver. 1.1>>>===== [002002] =====<<<PL.QUALITY-MMH>>>=====

Customer : FOGLIA Wed Mar 7 07:11:58 2001

Project #: 00469 Truss ID : T8 Family # : 101

Span : 14-8 B.Span : 14-8 Quant. : 1 Top Pitch : 6/12

==Bld(1/21/2000),v1.1=====

FORCES - GRAVITY LOADS REACTIONS - SIZE

1-2=876 3-4= 780 2-4= 145 1=908 2.00

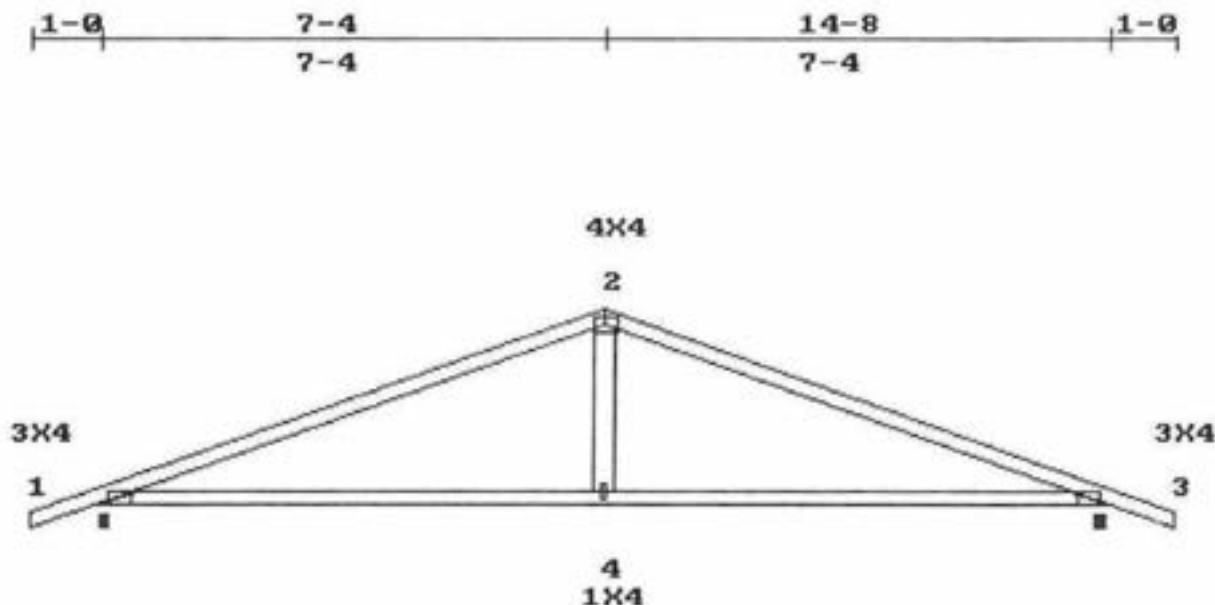
2-3=876 4-1= 780 3=908 2.00

PROVIDE FOR 757 LBS UPLIFT AT JOINT 1 (0.83)

PROVIDE FOR 757 LBS UPLIFT AT JOINT 3 (0.83)

PROVIDE FOR 124 LBS HORIZ. REACTION AT JOINT 1

97



L. HL TO PK:8-2-6 R. HL TO PK:8-2-6

LEFT HEIGHT:0-4-3 SPAN:14-8 RISE:4-0-3 RIGHT HEIGHT:0-4-3

LOADING (PSF)		MAX STRESSES		MINIMUM GRADE OF LUMBER	
TOP	30	TOP	2-3=0.760	TOP CHORD:2X4	SS 19 SP
BOTT	0	BOTT	4-1=0.377	BOT CHORD:2X4	No.2ND 19 SP
		LL.DEFL.@4=0.02 < L/360		WEBS	:2X4 No.3 19 SP

STR.INC.: LUMB = 1.33 PLATE = 1.33 SPACING : 24.0 in. o. c.

REPETITIVE STRESSES USED NO. OF MEMBERS = 1

TRUSS HAS BEEN CHECKED FOR 145 M.P.H. WIND LOAD, WALL HEIGHT 16 FT, ENCLOSED BLDG. TYPE, ASSUMING 10 PSF TOTAL D.L.(7*3) (SBC-97).

DEFLECTION(IN.) L.L= 0.02,D.L=0.02,T.L=0.04

PLATES ARE MITEK M20-249,200 MANUFACTURED FROM ASTM A 446 GRD A GALVANIZED STEEL(EXCEPT AS SHOWN)

PLATE MUST BE INSTALLED ON EA. FACE OF JOINT,SYMMETRICALLY(EXCEPT AS SHOWN)DESIGN CONFORMS W/NDS DESIGN SPECS, S58C,ANSI/TPI-95

THIS DESIGN IS FOR TRUSS FABRICATION ONLY.FOR PERMANENT AND TEMPORARY BRACING(WHICH IS ALWAYS REQD)CONSULT BLDG ARCHITECT OR ENGINEER.

MAR 06 2001

====<<<<ACES-32 Ver. 1.1>>>>===== [002009] =====<<<<FL.QUALITY-MMH>>>>=====

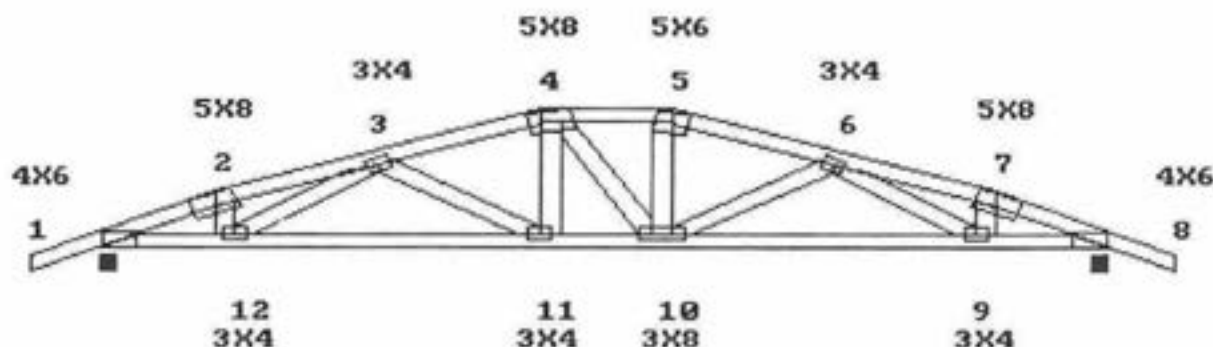
Customer : FOGLIA
 Project # : 00469
 Span : 14-8 B.Span : 14-8 Truss ID : T1
 2-4 4.26/12, 5-7 4.26/12, Quant. : 1
 Wed Mar 7 07:17:19 2001
 Family # : spec.
 Top Pitch : 6/12
 ==Bld(1/21/2000),v1.1=====

FORCES - LOAD CASE #1				REACTIONS - SIZE	
1-2=-2852	7-8=-2853	8-9= 2477	2-12= 266	6-10=-371	1=-1609 3.04
2-3=-2628		9-10= 2706	3-12=-283	6-9=-283	8=-1609 3.03
3-4=-2556		10-11= 2406	3-11=-370	7-9= 266	
4-5=-2406		11-12= 2705	4-11= 501		
5-6=-2556		12-1= 2477	4-10= 0		
6-7=-2629			5-10= 501		

98

PROVIDE FOR 2184 LBS UPLIFT AT JOINT 1 (1.36)
 PROVIDE FOR 2184 LBS UPLIFT AT JOINT 8 (1.36)
 PROVIDE FOR 86 LBS HORIZ. REACTION AT JOINT 1
 PLATE OFFSETS (X=LEFT,Y=TOP): [j2=4,3], [j7=4,3],

1-0 1-8 4-0-5 6-4-9 8-3-7 10-7-12 13-0 14-8 1-0
 1-8 2-4-5 2-4-5 1-10-14 2-4-5 2-4-5 1-8



1-8 6-4-9 8-3-7 13-0 14-8
 1-8 4-8-9 1-10-14 4-8-9 1-8

L. HL TO PK:1-10-6 LEFT HEIGHT:0-4-3 SPAN:14-8 RISE:2-10-4 R. HL TO PK:1-10-6 RIGHT HEIGHT:0-4-3

LOADING (PSF)		MAX STRESSES		MINIMUM GRADE OF LUMBER	
L	D	TOP	BOTT	TOP CHORD:	WEBS
30	15	7-8=0.736	9-10=0.623	2X4 No.2ND 19 SP	2X4 No.2ND 19 SP
0	10	LL.DEFL.@11=0.10 < L/360		2X4 No.3 19 SP	

REPETITIVE STRESSES NOT USED

SPACING : 24.0 in. o. c.
 NO. OF MEMBERS = 1

LOADING	STRESS INCREASE	LOADING	PANEL(PLF) / JOINTS(LBS)
LUMBER	PLATE	TYPE	
1	1.33	1.33	UNIFORM 1- 2= 90 2- 7= 170 7- 8= 90 8- 9= 20 9-12= 25 12- 1= 20
			CONCENTRATED 9= 25 10= 200 11= 200 12= 25

TRUSS HAS BEEN CHECKED FOR 145 M.P.H. WIND LOAD, WALL HEIGHT 16 FT, ENCLOSED BLDG. TYPE, ASSUMING 10 PSF TOTAL D.L.(7+3) (SBC-97).
 DEFLECTION(IN.) L.L.= 0.10,D.L.=0.09,T.L.=0.19
 PLATES ARE MITEK M20-249,200 MANUFACTURED FROM ASTM A 446 GRD A GALVANIZED STEEL(EXCEPT AS SHOWN)
 PLATE MUST BE INSTALLED ON EA. FACE OF JOINT,SYMMETRICALLY(EXCEPT AS SHOWN)DESIGN CONFORMS W/NDG DESIGN SPECS, SSB,ANSI/TPI-95
 THIS DESIGN IS FOR TRUSS FABRICATION ONLY.FOR PERMANENT AND TEMPORARY BRACING(WHICH IS ALWAYS REQD)CONSULT BLDG ARCHITECT OR ENGINEER.

[Signature]
 MMH 06 2001

====<<<ACES-32 Ver. 1.1>>>=====[001907]====<<<FL.QUALITY-MMH>>>=====

Customer : FOGLIA

Project #: 00469

Span : 2-4

Truss ID : JD2

Quantity : 1

Thu Mar 1 10:13:44 2001

Family #: 205

Top Pitch : 6/12

==Bld(1/21/2000),v1.1=====

FORCES - GRAVITY LOADS

1-2= 0

3-1= 0

2-3= 0

REACTIONS - SIZE

1=-229 2.00

3=-22 2.00

2=-97 1.50

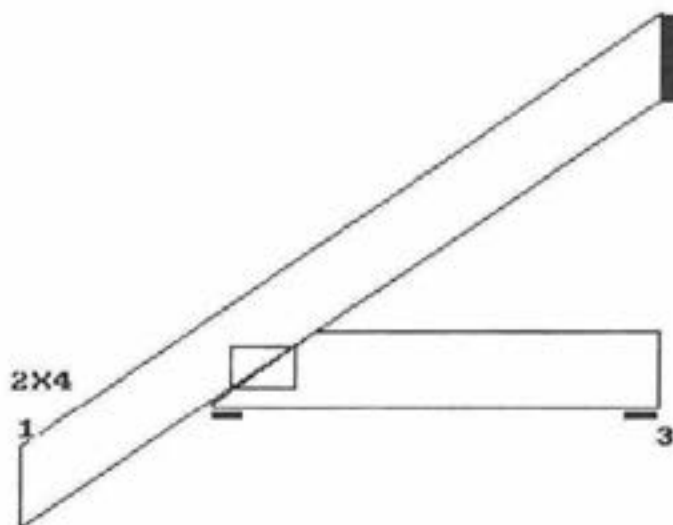
99

PROVIDE FOR 197 LBS UPLIFT AT JOINT 1 (0.86)

PROVIDE FOR 147 LBS UPLIFT AT JOINT 2 (1.51)

PROVIDE FOR 133 LBS HORIZ. REACTION AT JOINT 1
(SYMMETRIC PLATING TURNED ON)

1-0 2-4
2-4



2-4
2-4

L. HL TO PK:2-7-5

LEFT HEIGHT:0-4-3

SPAN:2-4

RISE:1-6-3

RIGHT HEIGHT:1-6-3

LOADING (PSF)

L D

TOP 30 15

BOTT 0 10

MAX STRESSES

TOP 1-2=0.126

BOTT 3-1=0.019

LL.DEFL. < L/360

MINIMUM GRADE OF LUMBER

TOP CHORD:2X4 No.2ND 19 SP

BOT CHORD:2X4 No.2ND 19 SP

WEBS :2X4 No.3 19 SP

STR.INC.: LUMB = 1.33 PLATE = 1.33

REPETITIVE STRESSES USED

SPACING : 24.0 in. o. c.

NO. OF MEMBERS = 1

TRUSS HAS BEEN CHECKED FOR 145 M.P.H. WIND LOAD, WALL HEIGHT 16 FT, ENCLOSED BLDG. TYPE, ASSUMING 10 PSF TOTAL D.L.(7+3) (SBC-97).
DEFLECTION(IN.) L.L= 0.00,D.L=0.00,T.L=0.00

PLATES ARE MITEK M20-249,200 MANUFACTURED FROM ASTM A 446 GRD A GALVANIZED STEEL(EXCEPT AS SHOWN)

PLATE MUST BE INSTALLED ON EA. FACE OF JOINT,SYMMETRICALLY(EXCEPT AS SHOWN)DESIGN CONFORMS W/NDS DESIGN SPECS, S88C,ANSI/TPI-95

THIS DESIGN IS FOR TRUSS FABRICATION ONLY.FOR PERMANENT AND TEMPORARY BRACING(WHICH IS ALWAYS REQD)CONSULT BLDG ARCHITECT OR ENGINEER.

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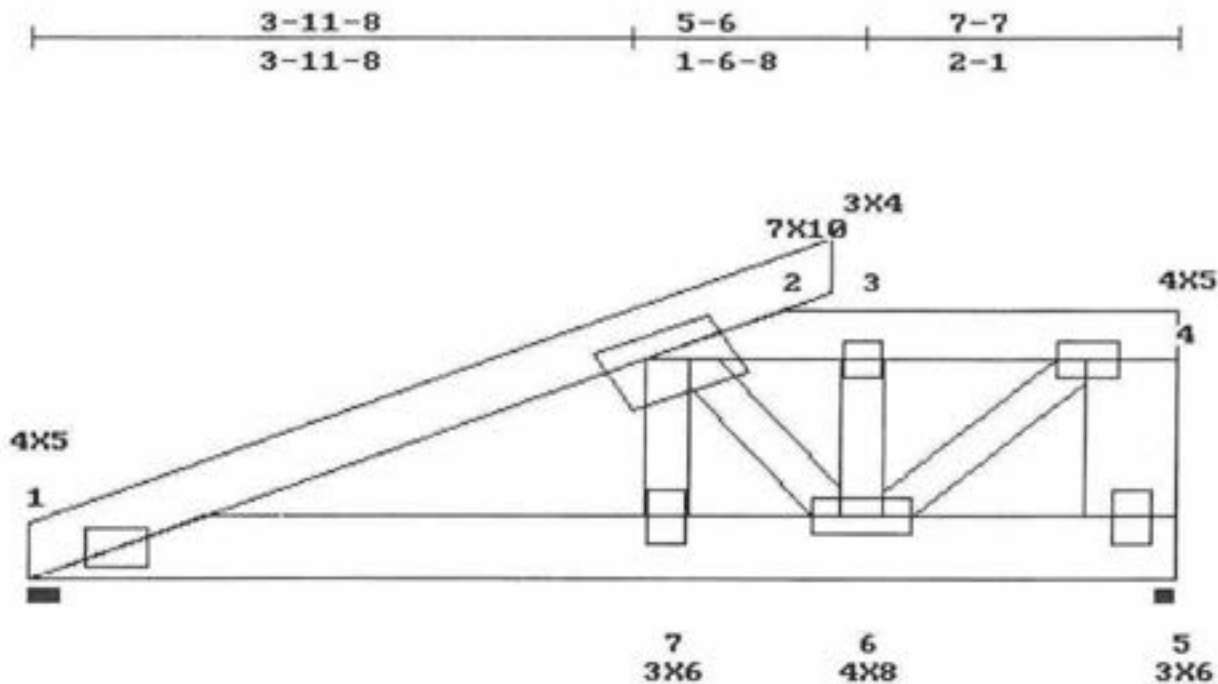
MAR 01 2001

====<<<<ACES-32 Ver. 1.1>>>>=====[002006]====<<<<FL.QUALITY-MMH>>>>=====
 Customer : FOGLIA Wed Mar 7 07:14:51 2001
 Project # : 00469 Truss ID : T10 Family # : 313
 Span : 7-7 B.Span : 8-3-8 Quant. : 4 Top Pitch : 6/12
 ==Bld(1/21/2000),v1.1=====

FORCES - LOAD CASE #1			REACTIONS - SIZE	
1-2=-2420	5-6= 0	2-7= 54	1=-4368	2.58
2-3=-2467	6-7= 2152	2-6= 513	5=-2692	1.66
3-4=-2467	7-1= 2152	3-6=-2040		
4-5=-2672		4-6= 3463		

100

PROVIDE FOR 4262 LBS UPLIFT AT JOINT 1 (0.98)
 PROVIDE FOR 2814 LBS UPLIFT AT JOINT 5 (1.05)
 PROVIDE FOR 233 LBS HORIZ. REACTION AT JOINT 1
 PLATE OFFSETS (X=LEFT,Y=TOP):[j2=5,3],



L. HL TO PK:4-5-2
 LEFT HEIGHT:0-6-7 SPAN:7-7 RISE:3-2-3 RIGHT HEIGHT:2-6-3

LOADING (PSF)		MAX STRESSES		MINIMUM GRADE OF LUMBER	
TOP	30	TOP	1-2=0.182	TOP CHORD:2X6	No.2D 19 SP
BOTT	0	BOTT	7-1=0.061	BOT CHORD:2X8	SS 19 SP
		LL.DEFL.@6=0.01 < L/360		WEBS	:2X4 No.3 19 SP

REPETITIVE STRESSES NOT USED

SPACING : 24.0 in. o. c.
 NO. OF MEMBERS = 2

LOADING	STRESS INCREASE	LOADING	PANEL(PLF) / JOINTS(LBS)
LUMBER	PLATE	TYPE	
1	1.33	UNIFORM	1- 4= 240 5- 1= 20
		CONCENTRATED	1= 2760 3= 1613 6= 760

WEB: 4-5 TO BE 2X8 SS 19 SP
 RIGHT END VERTICAL IS NOT EXPOSED.
 2 MEMBERS NAILED TOG, W/1 ROW(S) OF .131x3 in. NAILS 3 in. o.c.(TOP CHS.),AND 1 ROW(S) OF .131x3 in. NAILS 12 in. o.c(BOTT. CHS.)
 For Webs use 1 ROW of NAILS 12 in. o.c.
 TRUSS HAS BEEN CHECKED FOR 145 M.P.H. WIND LOAD, WALL HEIGHT 16 FT, ENCLOSED BLDG. TYPE, ASSUMING 10 PSF TOTAL D.L.(7+3) (SBC-97).
 DEFLECTION(IN.) L.L= 0.01,D.L=0.01,T.L=0.02
 PLATES ARE MITEK M20-249,200 MANUFACTURED FROM ASTM A 446 GRD A GALVANIZED STEEL(EXCEPT AS SHOWN)
 PLATE MUST BE INSTALLED ON EA. FACE OF JOINT,SYMMETRICALLY(EXCEPT AS SHOWN)DESIGN CONFORMS W/NDS DESIGN SPECS, S58C,ANSI/TP1-95
 THIS DESIGN IS FOR TRUSS FABRICATION ONLY.FOR PERMANENT AND TEMPORARY BRACING(WHICH IS ALWAYS REGD)CONSULT BLDG ARCHITECT OR ENGINEER.

[Signature]
 MAR 00 2001

====<<<<ACES-32 Ver. 1.1>>>>===== [002008] =====<<<<FL.QUALITY-MMH>>>>=====

Customer : FOGLIA

Project # : 00469

Span : 7-11-8 B.Span : 8-8

Truss ID : S1

Quant. : 1

Wed Mar 7 07:17:03 2001

Family # : 201

Top Pitch : 6/12

==Bld(1/21/2000),v1.1=====

FORCES - GRAVITY LOADS

1-2=-433 4-5= 0 2-5=-339

2-3=-426 5-1= 384 3-5= 568

3-4= 36

REACTIONS - SIZE

1=-543 2.00

3=-484 2.00

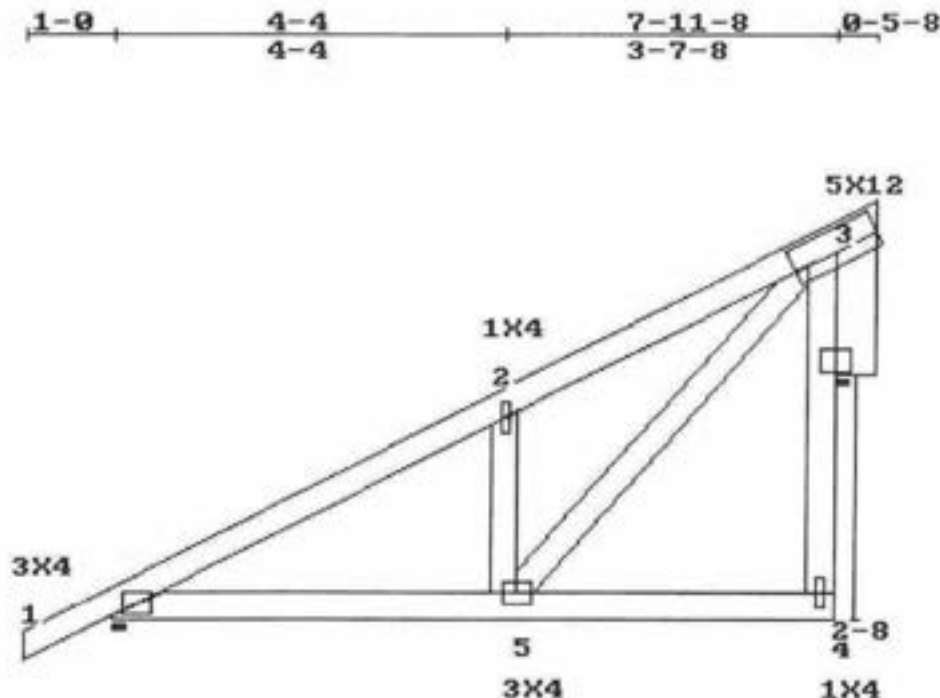
101

PROVIDE FOR 394 LBS UPLIFT AT JOINT 1 (0.73)

PROVIDE FOR 558 LBS UPLIFT AT JOINT 3 (1.15)

PROVIDE FOR 463 LBS HORIZ. REACTION AT JOINT 1

PLATE OFFSETS (X=LEFT,Y=TOP):[J3=2,3],



4-4 7-11-8
4-4 3-7-8

L. HL TO PK:8-10-12

LEFT HEIGHT:0-4-3

SPAN:7-11-8

RISE:4-6-11

RIGHT HEIGHT:4-3-15

LOADING (PSF)

L D

TOP 30 15

BOTT 0 10

MAX STRESSES

TOP 2-3=0.505

BOTT 5-1=0.134

LL.DEFL.@5=0.01 < L/360

MINIMUM GRADE OF LUMBER

TOP CHORD:2X4 No.2ND 19 SP

BOT CHORD:2X4 No.2ND 19 SP

WEBS :2X4 No.3 19 SP

STR.INC.: LUMB = 1.33 PLATE = 1.33

REPETITIVE STRESSES USED

SPACING : 24.0 in. o. c.

NO. OF MEMBERS = 1

RIGHT END VERTICAL IS NOT EXPOSED.

TRUSS HAS BEEN CHECKED FOR 145 M.P.H. WIND LOAD, WALL HEIGHT 16 FT, ENCLOSED BLDG. TYPE, ASSUMING 10 PSF TOTAL D.L.(7+3) (SBC-97).

DEFLECTION(IN.) L.L= 0.01,D.L=0.01,T.L=0.01

PLATES ARE MITEK M20-249,200 MANUFACTURED FROM ASTM A 446 GRD A GALVANIZED STEEL(EXCEPT AS SHOWN)

PLATE MUST BE INSTALLED ON EA. FACE OF JOINT,SYMMETRICALLY(EXCEPT AS SHOWN)DESIGN CONFORMS W/NDS DESIGN SPECS, SSB,ANSI/TPI-95

THIS DESIGN IS FOR TRUSS FABRICATION ONLY.FOR PERMANENT AND TEMPORARY BRACING(WHICH IS ALWAYS REQD)CONSULT BLDG ARCHITECT OR ENGINEER.

MAR 06 2001

====<<<<ACES-32 Ver. 1.1>>>>===== [001910] =====<<<<FL.QUALITY-MMH>>>>=====

Customer : FOGLIA

Project #: 00469

Span : 1-9-5

Truss ID : CJ1A

Quantity : 2

Thu Mar 1 10:14:29 2001

Family # : 201

Top Pitch : 5.54/12

==Bld(1/21/2000),v1.1=====

FORCES - LOAD CASE #1

REACTIONS - SIZE

1-2= 0

3-1= 0

1=139

2.00

2-3=-16

3=-20

1.50

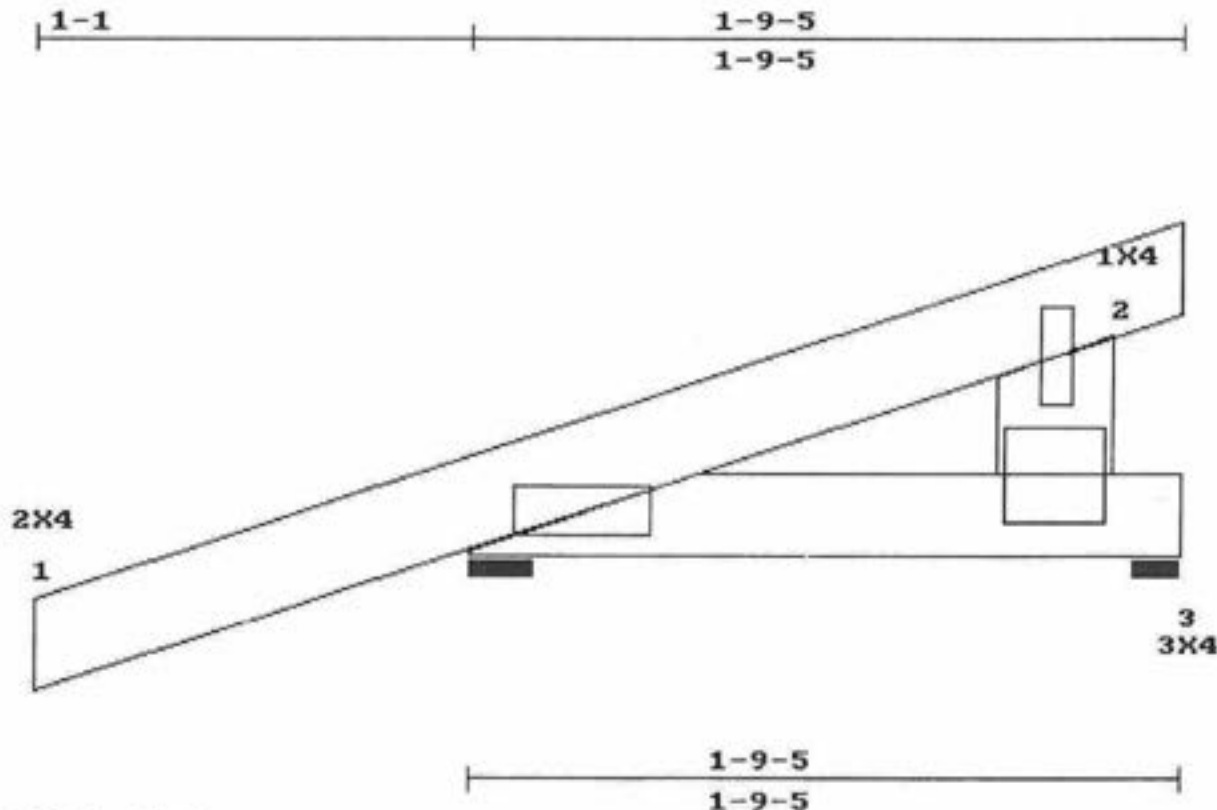
102

PROVIDE FOR 213 LBS UPLIFT AT JOINT 1 (1.53)

PROVIDE FOR 122 LBS UPLIFT AT JOINT 3 (6.07)

PROVIDE FOR 93 LBS HORIZ. REACTION AT JOINT 1

(SYMMETRIC PLATING TURNED ON)



L. HL TO PK:1-11-8

LEFT HEIGHT:0-4-3

SPAN:1-9-5

RISE:1-2

RIGHT HEIGHT:1-2

LOADING (PSF)

TOP 30 15

BOTT 0 10

MAX STRESSES

TOP 1-2=0.075

BOTT 3-1=0.003

LL.DEFL. < L/360

MINIMUM GRADE OF LUMBER

TOP CHORD:2X4 No.2ND 19 SP

BOT CHORD:2X4 No.2ND 19 SP

WEBS :2X4 No.3 19 SP

REPETITIVE STRESSES NOT USED

SPACING : 24.0 in. o. c.

NO. OF MEMBERS = 1

LOADING STRESS INCREASE LOADING

LUMBER PLATE TYPE

1 1.33 1.33 UNIFORM

PANEL(PLF) / JOINTS(LBS)

1- 2= 20 3- 1= 4

RIGHT END VERTICAL IS NOT EXPOSED.

TRUSS HAS BEEN CHECKED FOR 145 M.P.H. WIND LOAD, WALL HEIGHT 16 FT, ENCLOSED BLDG. TYPE, ASSUMING 10 PSF TOTAL D.L.(7+3) (SBC-97).

DEFLECTION(IN.) L.L= 0.00,D.L=0.00,T.L=0.00

PLATES ARE MITEK M20-249,200 MANUFACTURED FROM ASTM A 446 GRD A GALVANIZED STEEL(EXCEPT AS SHOWN)

PLATE MUST BE INSTALLED ON EA. FACE OF JOINT,SYMMETRICALLY(EXCEPT AS SHOWN)DESIGN CONFORMS W/NDS DESIGN SPECS, S58C,ANSI/TPI-95

THIS DESIGN IS FOR TRUSS FABRICATION ONLY.FOR PERMANENT AND TEMPORARY BRACING(WHICH IS ALWAYS REQD)CONSULT BLDG ARCHITECT OR ENGINEER.

[Signature]

MAR 01 2001

====<<<<ACES-32 Ver. 1.1>>>>===== [001911] =====<<<<FL.QUALITY-MMH>>>>=====

Customer : FOGLIA

Project # : 00469

Span : 2-2-8

Truss ID : CC1

Quantity : 2

Thu Mar 1 10:14:41 2001

Family # : 205

Top Pitch : 6/12

==Bld(1/21/2000),v1.1=====

FORCES - GRAVITY LOADS

1-2= 0 3-1= 0
2-3= 0

REACTIONS - SIZE

1=223 2.00
3=21 1.50
2=93 1.50

103

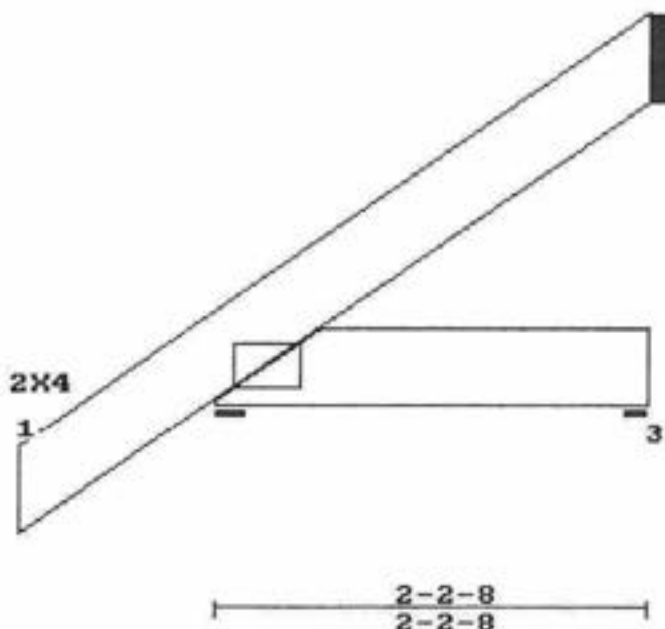
PROVIDE FOR 194 LBS UPLIFT AT JOINT 1 (0.87)

PROVIDE FOR 139 LBS UPLIFT AT JOINT 2 (1.50)

PROVIDE FOR 125 LBS HORIZ. REACTION AT JOINT 1

(SYMMETRIC PLATING TURNED ON)

1-0 2-2-8
2-2-8



L. HL TO PK:2-5-10

LEFT HEIGHT:0-4-3

SPAN:2-2-8

RISE:1-5-7

RIGHT HEIGHT:1-5-7

LOADING (PSF)

TOP 30 15

BOTT 0 10

MAX STRESSES

TOP 1-2=0.111

BOTT 3-1=0.017

LL.DEFL. < L/360

MINIMUM GRADE OF LUMBER

TOP CHORD:2X4 No.2ND 19 SP

BOT CHORD:2X4 No.2ND 19 SP

WEBS :2X4 No.3 19 SP

STR.INC.: LUMB = 1.33 PLATE = 1.33

REPETITIVE STRESSES USED

SPACING : 24.0 in. o. c.

NO. OF MEMBERS = 1

TRUSS HAS BEEN CHECKED FOR 145 M.P.H. WIND LOAD, WALL HEIGHT 16 FT, ENCLOSED BLDG. TYPE, ASSUMING 10 PSF TOTAL D.L.(7+3) (SBC-97).

DEFLECTION(IN.) L.L= 0.00,D.L=0.00,T.L=0.00

PLATES ARE MITEK M20-249,200 MANUFACTURED FROM ASTM A 446 GRD A GALVANIZED STEEL(EXCEPT AS SHOWN)

PLATE MUST BE INSTALLED ON EA. FACE OF JOINT,SYMMETRICALLY(EXCEPT AS SHOWN)DESIGN CONFORMS W/NDS DESIGN SPECS, SSBC,ANSI/TP1-95

THIS DESIGN IS FOR TRUSS FABRICATION ONLY.FOR PERMANENT AND TEMPORARY BRACING(WHICH IS ALWAYS REQD)CONSULT BLDG ARCHITECT OR ENGINEER.

[Signature]
MAR 01 2001

====<<<ACES-32 Ver. 1.1>>>=====[001912]====<<<FL.QUALITY-MMH>>>=====

Customer : FOGLIA

Project #: 00469

Span : 4-1

Truss ID : CC3

Quantity : 2

Thu Mar 1 10:14:51 2001

Family # : 205

Top Pitch : 6/12

==Q(d(1/21/2000),v1.1=====

FORCES - GRAVITY LOADS

1-2= 0

3-1= 0

2-3= 0

REACTIONS - SIZE

1=-327 2.00

3=-39 1.50

2=-177 1.50

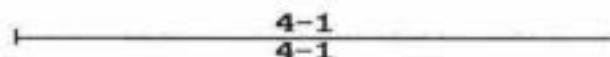
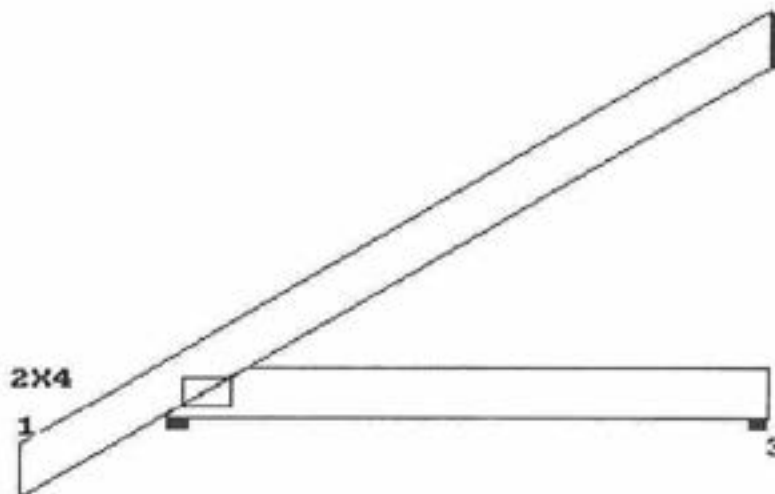
(109)

PROVIDE FOR 258 LBS UPLIFT AT JOINT 1 (0.79)

PROVIDE FOR 264 LBS UPLIFT AT JOINT 2 (1.49)

PROVIDE FOR 235 LBS HORIZ. REACTION AT JOINT 1

(SYMMETRIC PLATING TURNED ON)



L. HL TO PK:4-6-13

LEFT HEIGHT:0-4-3

SPAN:4-1

RISE:2-4-11

RIGHT HEIGHT:2-4-11

LOADING (PSF)

L D

TOP 30 15

BOTT 0 10

MAX STRESSES

TOP 1-2=0.449

BOTT 3-1=0.067

LL.DEFL. < L/360

MINIMUM GRADE OF LUMBER

TOP CHORD:2X4 No.2ND 19 SP

BOT CHORD:2X4 No.2ND 19 SP

WEBS :2X4 No.3 19 SP

STR.INC.: LUMB = 1.33 PLATE = 1.33

REPETITIVE STRESSES USED

SPACING : 24.0 in. o. c.

NO. OF MEMBERS = 1

TRUSS HAS BEEN CHECKED FOR 145 M.P.H. WIND LOAD, WALL HEIGHT 16 FT, ENCLOSED BLDG. TYPE, ASSUMING 10 PSF TOTAL D.L.(7+3) (SBC-97).

DEFLECTION(IN.) L.L= 0.00,D.L=0.00,T.L=0.00

PLATES ARE MITEK M20-249,200 MANUFACTURED FROM ASTM A 446 GRD A GALVANIZED STEEL(EXCEPT AS SHOWN)

PLATE MUST BE INSTALLED ON EA. FACE OF JOINT,SYMMETRICALLY(EXCEPT AS SHOWN)DESIGN CONFORMS W/WDS DESIGN SPECS, SSSC,ANSI/TPI-95

THIS DESIGN IS FOR TRUSS FABRICATION ONLY.FOR PERMANENT AND TEMPORARY BRACING(WHICH IS ALWAYS REQD)CONSULT BLDG ARCHITECT OR ENGINEER.

[Signature]

MAR 01 2001

====<<<<ACES-32 Ver. 1.1>>>>=====[001913]====<<<<FL.QUALITY-MMH>>>>=====

Customer : FOGLIA

Project # : 00469

Span : 2-5-9

Truss ID : C2

Quantity : 8

Thu Mar 1 10:15:00 2001

Family # : 205

Top Pitch : 6/12

==Bld(1/21/2000),v1.1=====

FORCES - GRAVITY LOADS

1-2= 0

3-1= 0

2-3= 0

REACTIONS - SIZE

1=-237 2.00

3=-23 1.50

2=-104 1.50

105

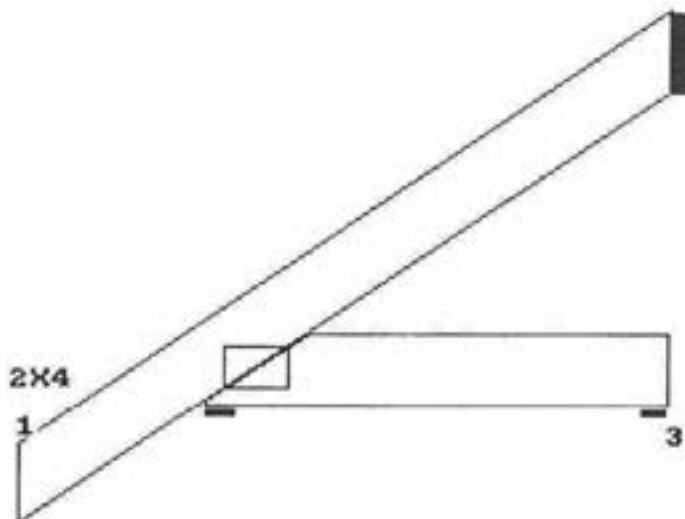
PROVIDE FOR 203 LBS UPLIFT AT JOINT 1 (0.85)

PROVIDE FOR 156 LBS UPLIFT AT JOINT 2 (1.50)

PROVIDE FOR 140 LBS HORIZ. REACTION AT JOINT 1

(SYMMETRIC PLATING TURNED ON)

1-0 2-5-9
2-5-9



2-5-9
2-5-9

L. HL TO PK:2-9-1

LEFT HEIGHT:0-4-3

SPAN:2-5-9

RISE:1-6-15

RIGHT HEIGHT:1-6-15

LOADING (PSF)

L D

TOP 30 15

BOTT 0 10

MAX STRESSES

TOP 1-2=0.144

BOTT 3-1=0.022

LL.DEFL. < L/360

MINIMUM GRADE OF LUMBER

TOP CHORD:2X4 No.2ND 19 SP

BOT CHORD:2X4 No.2ND 19 SP

WEBS :2X4 No.3 19 SP

STR.INC.: LUMB = 1.33 PLATE = 1.33

REPETITIVE STRESSES USED

SPACING : 24.0 in. o. c.

NO. OF MEMBERS = 1

TRUSS HAS BEEN CHECKED FOR 145 M.P.H. WIND LOAD, WALL HEIGHT 16 FT, ENCLOSED BLDG. TYPE, ASSUMING 10 PSF TOTAL D.L.(7+3) (SBC-97).

DEFLECTION(IN.) L.L= 0.00,D.L=0.00,T.L=0.00

PLATES ARE MITEK M20-249,200 MANUFACTURED FROM ASTM A 446 GRD A GALVANIZED STEEL(EXCEPT AS SHOWN)

PLATE MUST BE INSTALLED ON EA. FACE OF JOINT,SYMMETRICALLY(EXCEPT AS SHOWN)DESIGN CONFORMS W/NDS DESIGN SPECS, SIBC,ANSI/TPI-95

THIS DESIGN IS FOR TRUSS FABRICATION ONLY.FOR PERMANENT AND TEMPORARY BRACING(WHICH IS ALWAYS REQD)CONSULT BLDG ARCHITECT OR ENGINEER.

[Signature]

MAR 01 2001

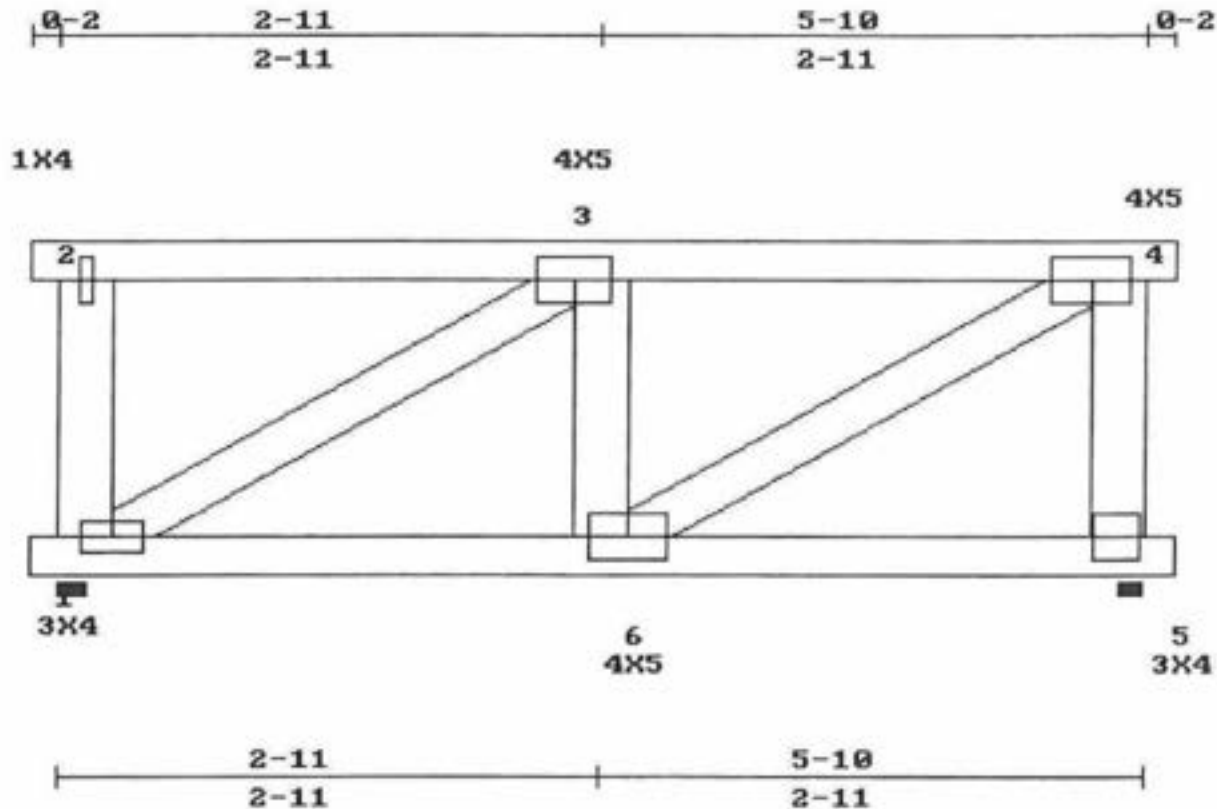
====<<<ACBS-32 Ver. 1.1>>>=====[001914]====<<<FL.QUALITY-MMH>>>=====
 Customer : FOGLIA Thu Mar 1 10:15:21 2001
 Project #: 00469 Truss ID : FG8 Family # : 301
 Span : 5-10 Quantity : 4 Top Pitch : /12

==Bld(1/21/2000),v1.1=====

FORCES - LOAD CASE #1			REACTIONS - SIZE	
1-2=-340	5-6= 0	3-1=-601	1=-739	2.00
2-3= 0	6-1= 473	3-6=-311	5=-739	1.50
3-4=-473		4-6= 599		
4-5=-711				

(106)

PROVIDE FOR 964 LBS UPLIFT AT JOINT 1 (1.30)
 PROVIDE FOR 964 LBS UPLIFT AT JOINT 5 (1.30)
 (SYMMETRIC PLATING TURNED ON)



LEFT HEIGHT:2-6-3		SPAN:5-10		RISE:2-6-3		RIGHT HEIGHT:2-6-3	
LOADING (PSF)		MAX STRESSES		MINIMUM GRADE OF LUMBER			
TOP	30 15	TOP	3-4=0.228	TOP CHORD:2X4	No.2ND	19	SP
BOTT	0 10	BOTT	6-1=0.062	BOT CHORD:2X4	No.2ND	19	SP
		LL.DEFL.@6=0.00	< L/360	WEBS	:2X4	No.3	19 SP

REPETITIVE STRESSES NOT USED

SPACING : 24.0 in. o. c.
 NO. OF MEMBERS = 2

LOADING STRESS INCREASE	LOADING	PANEL(PLF) / JOINTS(LBS)
LUMBER PLATE	TYPE	
1 1.33 1.33	UNIFORM	2- 4= 240 5- 1= 20

BOTH END VERTICALS ARE NOT EXPOSED.
 2 MEMBERS NAILED TOG. W/1 ROW(S) OF .131x3 in. NAILS 11 in. o.c.(TOP CHS.),AND 1 ROW(S) OF .131x3 in. NAILS 12 in. o.c(BOTT. CHS.)
 For Webs use 1 ROW of NAILS 12 in. o.c.
 TRUSS HAS BEEN CHECKED FOR 145 M.P.H. WIND LOAD, WALL HEIGHT 16 FT, ENCLOSED BLDG. TYPE, ASSUMING 10 PSF TOTAL D.L.(7+3) (SBC-97).
 DEFLECTION(IN.) L.L= 0.00,D.L=0.00,T.L=0.01
 PLATES ARE MITEK M20-249,200 MANUFACTURED FROM ASTM A 446 GRD A GALVANIZED STEEL(EXCEPT AS SHOWN)
 PLATE MUST BE INSTALLED ON EA. FACE OF JOINT,SYMMETRICALLY(EXCEPT AS SHOWN)DESIGN CONFORMS W/ND5 DESIGN SPECS, S5BC,ANSI/TP1-95
 THIS DESIGN IS FOR TRUSS FABRICATION ONLY.FOR PERMANENT AND TEMPORARY BRACING(WHICH IS ALWAYS REQD)CONSULT BLDG ARCHITECT OR ENGINEER.

[Signature]
 MAR 01 2001

====<<<<ACES-32 Ver. 1.1>>>>===== [001915] =====<<<<FL.QUALITY-MMH>>>>=====

Customer : FOGLIA Thu Mar 1 10:15:32 2001

Project #: 00469 Truss ID : CJ5A Family #: 201

Span : 5-4-9 Quantity : 2 Top Pitch : 5.54/12

==Bld(1/21/2000),v1.1=====

FORCES - LOAD CASE #1 REACTIONS - SIZE

1-2= 0 3-1= 0 1=-314 2.00

2-3=-159 3=-194 1.50

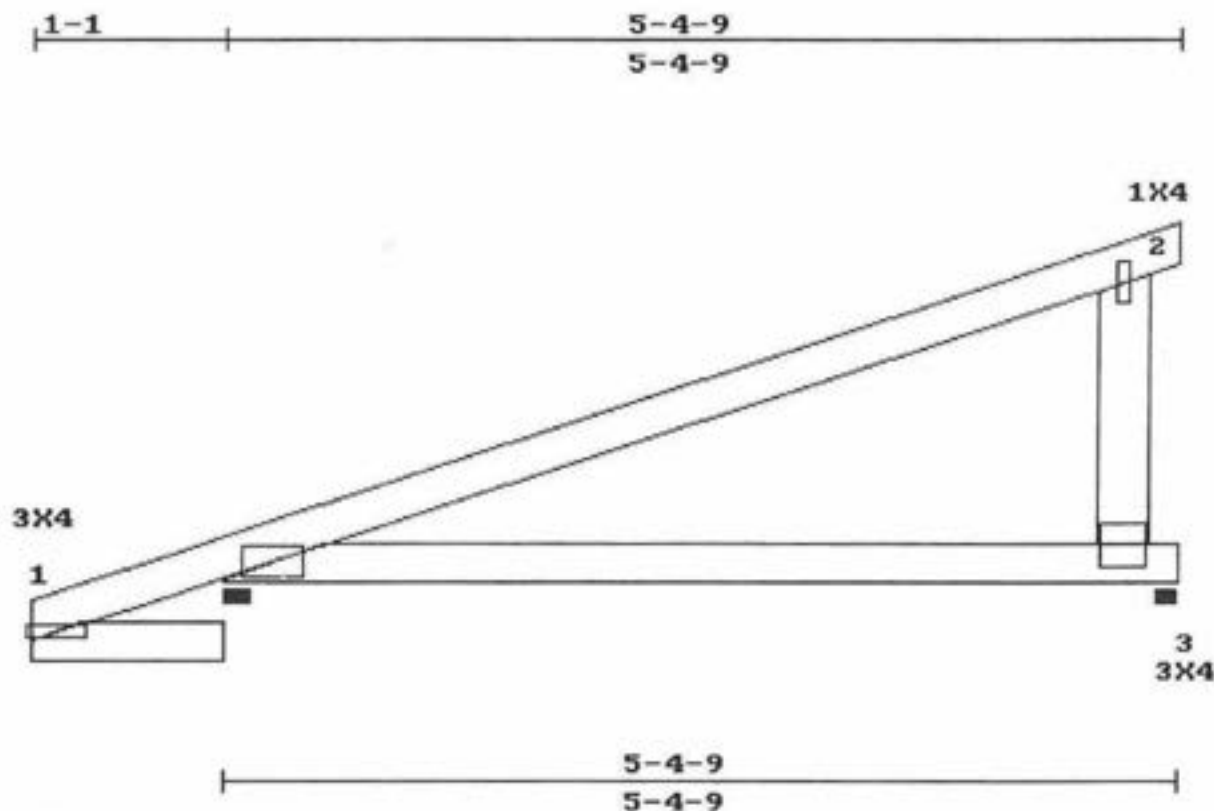
107

PROVIDE FOR 502 LBS UPLIFT AT JOINT 1 (1.60)

PROVIDE FOR 501 LBS UPLIFT AT JOINT 3 (2.58)

PROVIDE FOR 289 LBS HORIZ. REACTION AT JOINT 1

(SYMMETRIC PLATING TURNED ON)



L. HL TO PK:5-11-2 SPAN:5-4-9 RISE:2-10 RIGHT HEIGHT:2-10

LEFT HEIGHT:0-4-3

LOADING (PSF) MAX STRESSES MINIMUM GRADE OF LUMBER

TOP 30 15 TOP CHORD:2X4 No.2ND 19 SP

BOTT 0 10 BOT CHORD:2X4 No.2ND 19 SP

LL.DEFL. < L/360 WEBS :2X4 No.3 19 SP

REPETITIVE STRESSES NOT USED

SPACING : 24.0 in. o. c.

NO. OF MEMBERS = 1

LOADING STRESS INCREASE LOADING PANEL(PLF) / JOINTS(LBS)

LUMBER PLATE TYPE

1 1.33 1.33 UNIFORM 1- 2= 61 3- 1= 14

RIGHT END VERTICAL IS NOT EXPOSED.

TRUSS HAS BEEN CHECKED FOR 145 M.P.H. WIND LOAD, WALL HEIGHT 16 FT, ENCLOSED BLDG. TYPE, ASSUMING 10 PSF TOTAL D.L.(7+3) (SBC-97).

DEFLECTION(IN.) L.L= 0.00,D.L=0.00,T.L=0.00

PLATES ARE MITEK M20-249,200 MANUFACTURED FROM ASTM A 446 GRD A GALVANIZED STEEL(EXCEPT AS SHOWN)

PLATE MUST BE INSTALLED ON EA. FACE OF JOINT,SYMMETRICALLY(EXCEPT AS SHOWN)DESIGN CONFORMS W/MDS DESIGN SPECS, SSSC,ANSI/TPI-95

THIS DESIGN IS FOR TRUSS FABRICATION ONLY.FOR PERMANENT AND TEMPORARY BRACING(WHICH IS ALWAYS REQD)CONSULT BLDG ARCHITECT OR ENGINEER.

[Signature]

MAR 01 2001

====<<<ACES-32 Ver. 1.1>>>===== [001916] =====<<<FL.QUALITY-MMH>>>=====

Customer : FOGLIA

Project #: 00469

Span : 5-0

Truss ID : CC5

Quantity : 1

Thu Mar 1 10:15:42 2001

Family # : 205

Top Pitch : 6/12

==sld(1/21/2000),v1.1=====

FORCES - GRAVITY LOADS

1-2= 0 3-1= 0

2-3= 0

REACTIONS - SIZE

1=-363 8.00

3=-46 1.50

2=-207 1.50

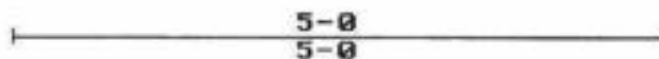
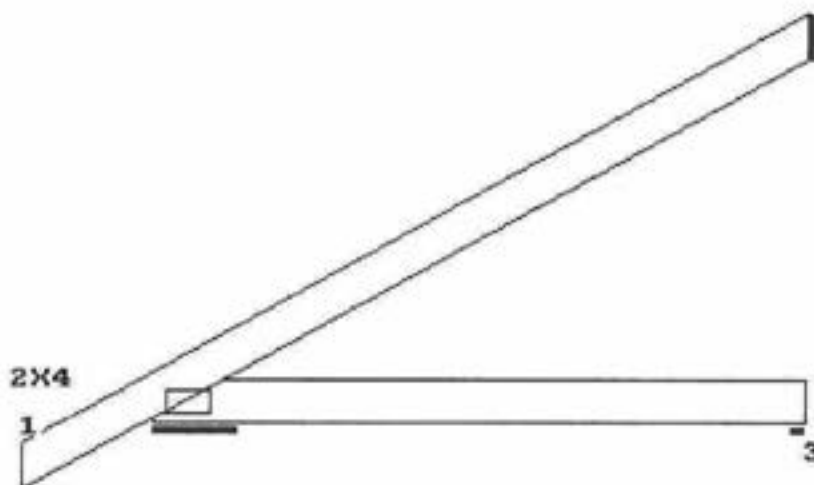
(108)

PROVIDE FOR 273 LBS UPLIFT AT JOINT 1 (0.75)

PROVIDE FOR 316 LBS UPLIFT AT JOINT 2 (1.53)

PROVIDE FOR 289 LBS HORIZ. REACTION AT JOINT 1

(SYMMETRIC PLATING TURNED ON)



L. HL TO PK:5-7-1

LEFT HEIGHT:0-4-3

SPAN:5-0

RISE:2-10-3

RIGHT HEIGHT:2-10-3

LOADING (PSF)

L D

TOP 30 15

BOTT 0 10

MAX STRESSES

TOP 1-2=0.702

BOTT 3-1=0.103

LL.DEFL. < L/360

MINIMUM GRADE OF LUMBER

TOP CHORD:2X4 No.2ND 19 SP

BOT CHORD:2X4 No.2ND 19 SP

WEBS :2X4 No.3 19 SP

STR.INC.: LUMB = 1.33 PLATE = 1.33

REPETITIVE STRESSES USED

SPACING : 24.0 in. o. c.

NO. OF MEMBERS = 1

TRUSS HAS BEEN CHECKED FOR 145 M.P.H. WIND LOAD, WALL HEIGHT 16 FT, ENCLOSED BLDG. TYPE, ASSUMING 10 PSF TOTAL D.L.(7+3) (SBC-97).

DEFLECTION(IN.) L.L= 0.00,D.L=0.00,T.L=0.00

PLATES ARE MITEK M20-249,200 MANUFACTURED FROM ASTM A 446 GRD A GALVANIZED STEEL(EXCEPT AS SHOWN)

PLATE MUST BE INSTALLED ON EA. FACE OF JOINT,SYMMETRICALLY(EXCEPT AS SHOWN)DESIGN CONFORMS W/ND5 DESIGN SPECS, SSBC,ANSI/TPI-95

THIS DESIGN IS FOR TRUSS FABRICATION ONLY.FOR PERMANENT AND TEMPORARY BRACING(WHICH IS ALWAYS REQD)CONSULT BLDG ARCHITECT OR ENGINEER.

[Signature]

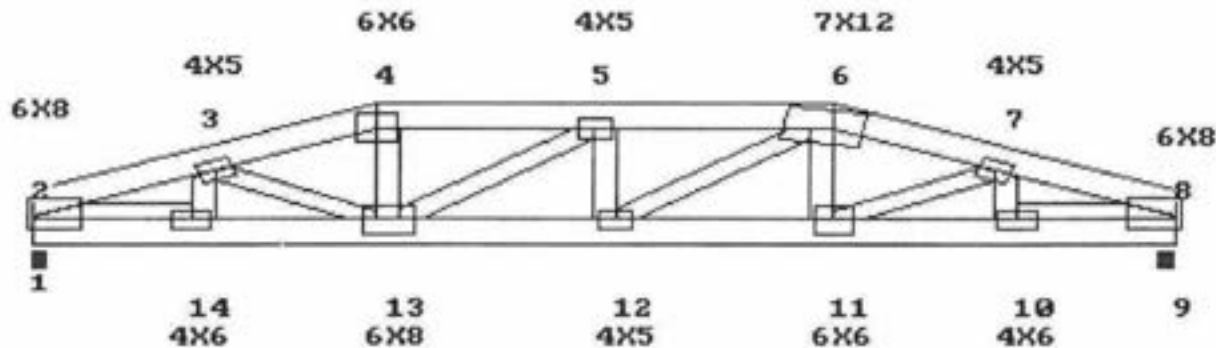
MAR 01 2001

====<<<<ACES-32 Ver. 1.1>>>>=====[002007]====<<<<FL.QUALITY-MMH>>>>=====
 Customer : FOGLIA Wed Mar 7 07:16:10 2001
 Project #: 00469 Truss ID : GT14 Family #: 314
 Span : 14-5 B.Span : 14-5 Quant. : 2 Top Pitch : 4.24/
 ==8(d(1/21/2000),v1.1=====

FORCES - LOAD CASE #1				REACTIONS - SIZE	
1-2=-3727	7-8=-5826	9-10= 0	2-14= 5675	6-12= 909	1=-3759 2.68
2-3=-5787	8-9=-3723	10-11= 5517	3-14=-1431	6-11= 621	9=-3756 2.75
3-4=-6505		11-12= 6137	3-13= 752	7-11= 720	
4-5=-6121		12-13= 6876	4-13= 1145	7-10=-1427	
5-6=-6876		13-14= 5474	5-13=-928	8-10= 5716	
6-7=-6522		14-1= 0	5-12=-440		

PROVIDE FOR 5310 LBS UPLIFT AT JOINT 1 (1.41)
 PROVIDE FOR 5453 LBS UPLIFT AT JOINT 9 (1.45)
 PROVIDE FOR 50 LBS HORIZ. REACTION AT JOINT 1
 PLATE OFFSETS (X=LEFT,Y=TOP):[J2=4,2.5],[J6=6,4],[J8=4,2.5],

2-1-15	4-3-15	7-2-8	10-1-1	12-3	14-5
2-1-15	2-1-15	2-10-9	2-10-9	2-1-15	2-1-15



2-1-15	4-3-15	7-2-8	10-1-1	12-3	14-5
2-1-15	2-1-15	2-10-9	2-10-9	2-1-15	2-1-15

L. HL TO PK:4-7-1 LEFT HEIGHT:0-11-14 SPAN:14-5 RISE:2-6-4 R. HL TO PK:4-7-1 RIGHT HEIGHT:0-11-14

LOADING (PSF)		MAX STRESSES		MINIMUM GRADE OF LUMBER	
TOP	30	15	TOP 5-6=0.666	TOP CHORD:2X6	No.2D 19 SP
BOTT	0	10	BOTT 12-13=0.418	BOT CHORD:2X6	No.2D 19 SP
			LL.DEFL.@12=0.08 < L/360	WEBS :2X4	No.3 19 SP

REPETITIVE STRESSES NOT USED

SPACING : 24.0 in. o. c.
 NO. OF MEMBERS = 2

LOADING STRESS INCREASE	LOADING LUMBER PLATE TYPE	PANEL(PLF) / JOINTS(LBS)
1	1.33 1.33	UNIFORM 2- 8= 340 9- 1= 31
		CONCENTRATED 4= 200 6= 200 11= 910 13= 910

BOTH END VERTICALS ARE NOT EXPOSED.
 2 MEMBERS NAILED TOG. W/1 ROW(S) OF .131x3 in. NAILS 7 in. o.c.(TOP CHS.),AND 1 ROW(S) OF .131x3 in. NAILS 6 in. o.c(BOTT. CHS.)
 For Webs use 1 ROW of NAILS 12 in. o.c.
 TRUSS HAS BEEN CHECKED FOR 145 M.P.H. WIND LOAD, WALL HEIGHT 16 FT, ENCLOSED BLDG. TYPE, ASSUMING 10 PSF TOTAL D.L.(7+3) (SBC-97).
 DEFLECTION(IN.) L.L= 0.08,D.L=0.07,T.L=0.15
 PLATES ARE MITEK M20-249,200 MANUFACTURED FROM ASTM A 446 GRD A GALVANIZED STEEL(EXCEPT AS SHOWN)
 PLATE MUST BE INSTALLED ON EA. FACE OF JOINT,SYMMETRICALLY(EXCEPT AS SHOWN)DESIGN CONFORMS W/NDOS DESIGN SPECS, SSSC,ANSI/TPI-95
 THIS DESIGN IS FOR TRUSS FABRICATION ONLY.FOR PERMANENT AND TEMPORARY BRACING(WHICH IS ALWAYS REQD)CONSULT BLDG ARCHITECT OR ENGINEER.

MAR 06 2001

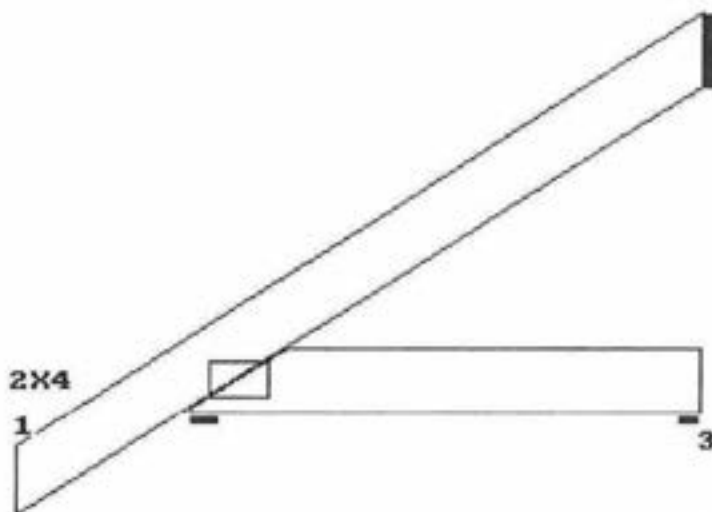
====<<<ACES-32 Ver. 1.1>>>=====[001918]====<<<FL.QUALITY-MMH>>>=====
 Customer : FOGLIA Thu Mar 1 10:16:36 2001
 Project #: 00469 Truss ID : CC2 Family #: 205
 Span : 2-11 Quantity : 2 Top Pitch : 6/12
 ==Rld(1/21/2000),v1.1=====

FORCES - GRAVITY LOADS		REACTIONS - SIZE	
1-2= 0	3-1= 0	1=-262	2.00
2-3= 0		3=-28	1.50
		2=-125	1.50

(110)

PROVIDE FOR 218 LBS UPLIFT AT JOINT 1 (0.83)
 PROVIDE FOR 187 LBS UPLIFT AT JOINT 2 (1.50)
 PROVIDE FOR 167 LBS HORIZ. REACTION AT JOINT 1
 (SYMMETRIC PLATING TURNED ON)

1-0 2-11
2-11



2-11
2-11

L. HL TO PK:3-3-2
 LEFT HEIGHT:0-4-3 SPAN:2-11 RISE:1-9-11 RIGHT HEIGHT:1-9-11

LOADING (PSF)		MAX STRESSES		MINIMUM GRADE OF LUMBER	
L	D	TOP	1-2=0.211	TOP CHORD:2X4	No.2ND 19 SP
TOP	30	BOTT	3-1=0.032	BOT CHORD:2X4	No.2ND 19 SP
BOTT	0	LL.DEFL.	< L/360	WEBS	:2X4 No.3 19 SP

STR.INC.: LUMB = 1.33 PLATE = 1.33 SPACING : 24.0 in. o. c.
 REPETITIVE STRESSES USED NO. OF MEMBERS = 1

TRUSS HAS BEEN CHECKED FOR 145 M.P.H. WIND LOAD, WALL HEIGHT 16 FT, ENCLOSED BLDG. TYPE, ASSUMING 10 PSF TOTAL D.L.(7+3) (SBC-97).
 DEFLECTION(IN.) L.L= 0.00,D.L=0.00,T.L=0.00
 PLATES ARE MITEK M20-249,200 MANUFACTURED FROM ASTM A 446 GRD A GALVANIZED STEEL(EXCEPT AS SHOWN)
 PLATE MUST BE INSTALLED ON EA. FACE OF JOINT,SYMMETRICALLY(EXCEPT AS SHOWN)DESIGN CONFORMS W/ND S DESIGN SPECS, SSBC,ANSI/TP1-95
 THIS DESIGN IS FOR TRUSS FABRICATION ONLY.FOR PERMANENT AND TEMPORARY BRACING(WHICH IS ALWAYS REQD)CONSULT BLDG ARCHITECT OR ENGINEER.

[Signature]

MAR 01 2001

====<<<<ACES-32 Ver. 1.1>>>>=====[001919]=====<<<<FL.QUALITY-MMH>>>>=====

Customer : FOGLIA Truss ID : FG5 Thu Mar 1 10:16:50 2001

Project #: 00469 Quantity : 4 Family # : 301

Span : 4-0 Top Pitch : /12

==Bld(1/21/2000),v1.1=====

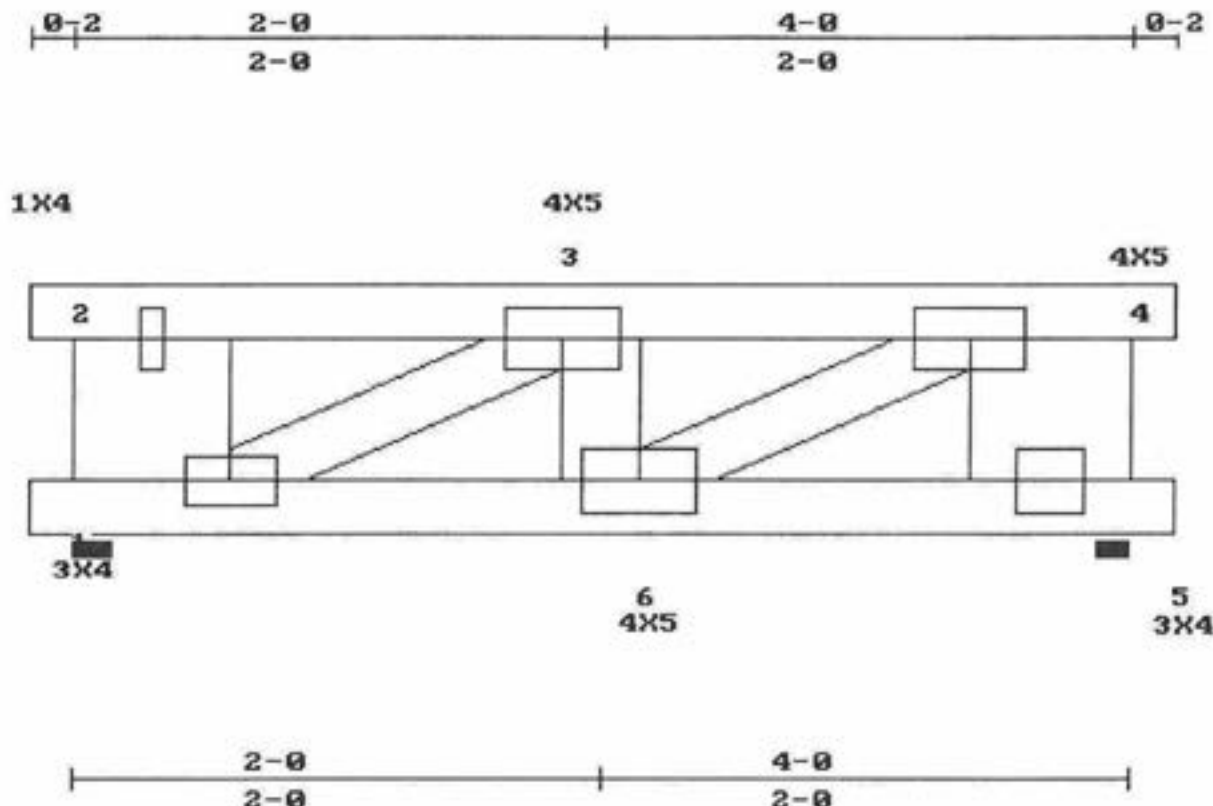
FORCES - LOAD CASE #1			REACTIONS - SIZE	
1-2=-326	5-6= 0	3-1=-730	1=-694	2.00
2-3= 0	6-1= 642	3-6=-306	5=-694	1.50
3-4=-642		4-6= 729		
4-5=-674				

PROVIDE FOR 815 LBS UPLIFT AT JOINT 1 (1.17)

PROVIDE FOR 852 LBS UPLIFT AT JOINT 5 (1.23)

PROVIDE FOR 139 LBS HORIZ. REACTION AT JOINT 1

(SYMMETRIC PLATING TURNED ON)



LEFT HEIGHT:1-4 SPAN:4-0 RISE:1-4 RIGHT HEIGHT:1-4

LOADING (PSF)			MAX STRESSES		MINIMUM GRADE OF LUMBER	
TOP	L 30	D 15	TOP	3-4=0.164	TOP CHORD:2X4	No.2ND 19 SP
BOTT	0	10	BOTT	6-1=0.067	BOT CHORD:2X4	No.2ND 19 SP
			LL.DEFL.@6=0.00 < L/360	WEBS	:2X4	No.3 19 SP

REPETITIVE STRESSES NOT USED

SPACING : 24.0 in. o. c.

NO. OF MEMBERS = 2

LOADING	STRESS INCREASE	LOADING	PANEL(PLF) / JOINTS(LBS)
LUMBER	PLATE	TYPE	
1	1.33	1.33	UNIFORM
			2- 4= 340 5- 1= 20

WEB: 1-2; 4-5 TO BE 2X8 SS 19 SP

BOTH END VERTICALS ARE EXPOSED.

2 MEMBERS NAILED TOG. W/1 ROW(S) OF .131x3 in. NAILS 12 in. o.c.(BOTT. CHS.),AND 16d NAILS 12 in. o.c.(TOP CHORD)

For Webs use 1 ROW of NAILS 12 in. o.c.

****NOTE:Special hanger/connection is reqd. to dist. top chord uniform load(s) to all plies.****

TRUSS HAS BEEN CHECKED FOR 145 M.P.H. WIND LOAD, WALL HEIGHT 16 FT, ENCLOSED BLDG. TYPE, ASSUMING 10 PSF TOTAL D.L.(7+3) (SBC-97).

DEFLECTION(IN.) L.L= 0.00,D.L=0.00,T.L=0.01

PLATES ARE MITEK M20-249,200 MANUFACTURED FROM ASTM A 446 GRD A GALVANIZED STEEL(EXCEPT AS SHOWN)

PLATE MUST BE INSTALLED ON EA. FACE OF JOINT,SYMMETRICALLY(EXCEPT AS SHOWN)DESIGN CONFORMS W/ND S DESIGN SPECS, SIBC,ANSI/TP1-95

THIS DESIGN IS FOR TRUSS FABRICATION ONLY.FOR PERMANENT AND TEMPORARY BRACING(WHICH IS ALWAYS REQD)CONSULT BLDG ARCHITECT OR ENGINEER.

[Signature]

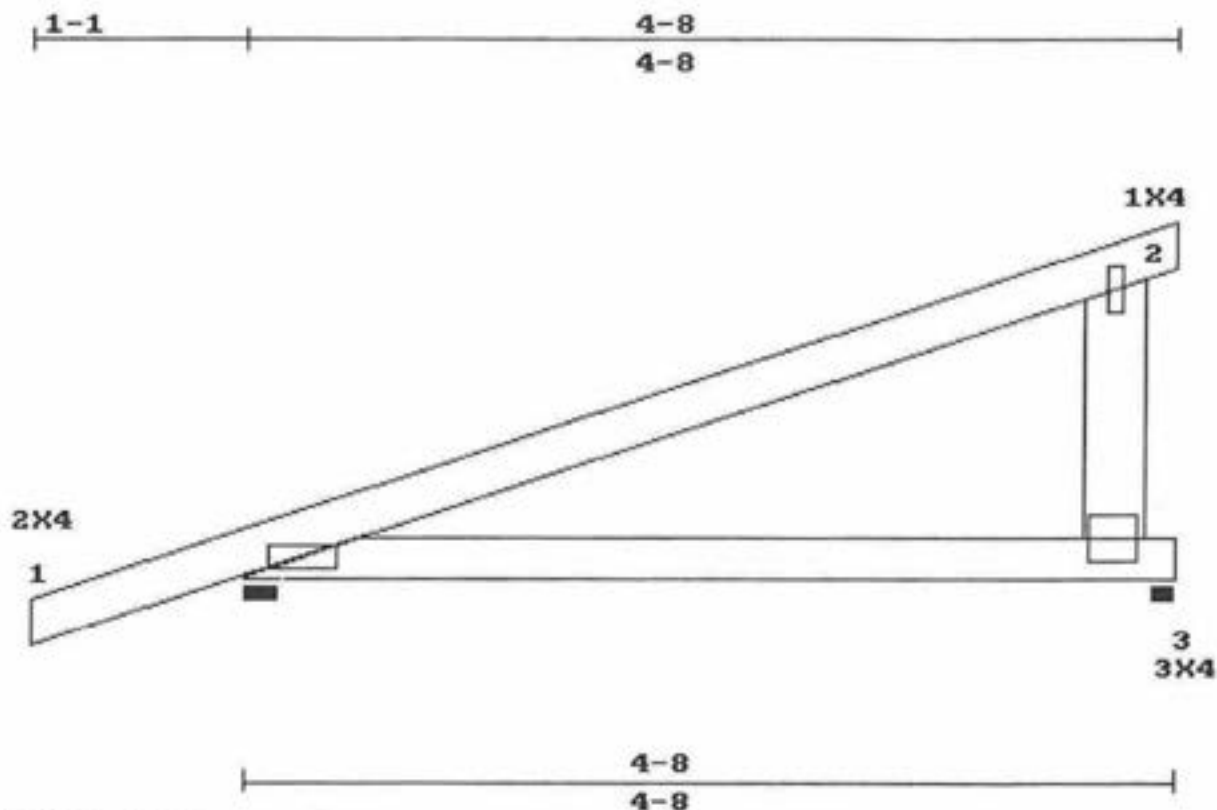
MAR 01 2001

====<<<ACES-32 Ver. 1.1>>>=====[001920]====<<<FL.QUALITY-MMH>>>=====
 Customer : FOGLIA Thu Mar 1 10:17:01 2001
 Project #: 00469 Truss ID : CJ4 Family # : 201
 Span : 4-8 Quantity : 2 Top Pitch : 5.54/12
 ==Bld(1/21/2000),v1.1=====

FORCES - LOAD CASE #1		REACTIONS - SIZE
1-2= 0	3-1= 0	1=-265 2.00
2-3=-119		3=-146 1.50

(12)

PROVIDE FOR 432 LBS UPLIFT AT JOINT 1 (1.63)
 PROVIDE FOR 414 LBS UPLIFT AT JOINT 3 (2.84)
 PROVIDE FOR 250 LBS HORIZ. REACTION AT JOINT 1
 (SYMMETRIC PLATING TURNED ON)



L. HL TO PK:5-1-11		SPAN:4-8		RISE:2-6-1		RIGHT HEIGHT:2-6-1	
LEFT HEIGHT:0-4-3							
LOADING (PSF)		MAX STRESSES		MINIMUM GRADE OF LUMBER			
	L D	TOP	1-2=0.507	TOP CHORD:2X4		No.2ND 19 SP	
TOP	30 15	BOTT	3-1=0.060	BOT CHORD:2X4		No.2ND 19 SP	
BOTT	0 10	LL.DEFL.	< L/360	WEBS :2X4		No.3 19 SP	

REPETITIVE STRESSES NOT USED

SPACING : 24.0 in. o. c.
 NO. OF MEMBERS = 1

LOADING	STRESS INCREASE	LOADING	PANEL(PLF) / JOINTS(LBS)
LUMBER	PLATE	TYPE	
1	1.33 1.33	UNIFORM	1- 2= 53 3- 1= 12

RIGHT END VERTICAL IS NOT EXPOSED.
 TRUSS HAS BEEN CHECKED FOR 145 M.P.H. WIND LOAD, WALL HEIGHT 16 FT, ENCLOSED BLDG. TYPE, ASSUMING 10 PSF TOTAL D.L.(7+3) (SBC-97).
 DEFLECTION(IN.) L.L= 0.00,D.L=0.00,T.L=0.00
 PLATES ARE MITEK M20-249,200 MANUFACTURED FROM ASTM A 446 GRD A GALVANIZED STEEL(EXCEPT AS SHOWN)
 PLATE MUST BE INSTALLED ON EA. FACE OF JOINT,SYMMETRICALLY(EXCEPT AS SHOWN)DESIGN CONFORMS W/NDS DESIGN SPECS, SBC,ANSI/TP1-95
 THIS DESIGN IS FOR TRUSS FABRICATION ONLY.FOR PERMANENT AND TEMPORARY BRACING(WHICH IS ALWAYS REQD)CONSULT BLDG ARCHITECT OR ENGINEER.

[Signature]
 MAR 01 2001

====<<<ACES-32 Ver. 1.1>>>=====[001921]====<<<FL.QUALITY-MMH>>>=====
 Customer : FOGLIA Thu Mar 1 10:17:13 2001
 Project #: 00469 Truss ID : CD4 Family # : 205
 Span : 4-4 Quantity : 3 Top Pitch : 6/12

==Bld(1/21/2000),v1.1=====

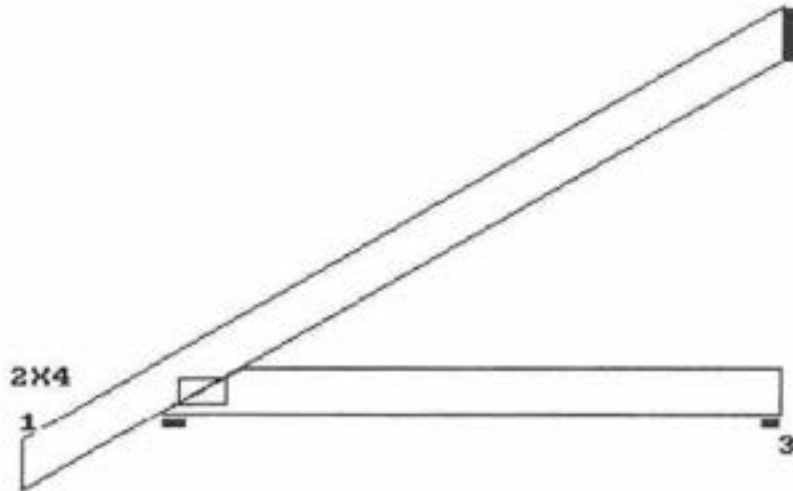
FORCES - GRAVITY LOADS
 1-2= 0 3-1= 0
 2-3= 0

REACTIONS - SIZE
 1=340 2.00
 3=42 1.50
 2=188 1.50

113

PROVIDE FOR 267 LBS UPLIFT AT JOINT 1 (0.78)
 PROVIDE FOR 281 LBS UPLIFT AT JOINT 2 (1.49)
 PROVIDE FOR 250 LBS HORIZ. REACTION AT JOINT 1
 (SYMMETRIC PLATING TURNED ON)

1-0 4-4
 4-4



4-4
 4-4

L. HL TO PK:4-10-2
 LEFT HEIGHT:0-4-3

SPAN:4-4

RISE:2-6-3

RIGHT HEIGHT:2-6-3

LOADING (PSF)

MAX STRESSES

MINIMUM GRADE OF LUMBER

TOP 30 15

TOP 1-2=0.512

TOP CHORD:2X4 No.2ND 19 SP

BOTT 0 10

BOTT 3-1=0.076

BOT CHORD:2X4 No.2ND 19 SP

LL.DEFL. < L/360

WEBS :2X4 No.3 19 SP

STR.INC.: LUMB = 1.33 PLATE = 1.33

SPACING : 24.0 in. o. c.

REPETITIVE STRESSES USED

NO. OF MEMBERS = 1

TRUSS HAS BEEN CHECKED FOR 145 M.P.H. WIND LOAD, WALL HEIGHT 16 FT, ENCLOSED BLDG. TYPE, ASSUMING 10 PSF TOTAL D.L.(7+3) (SBC-97).

DEFLECTION(IN.) L.L= 0.00,D.L=0.00,T.L=0.00

PLATES ARE MITEK M20-249,200 MANUFACTURED FROM ASTM A 446 GRD A GALVANIZED STEEL(EXCEPT AS SHOWN)

PLATE MUST BE INSTALLED ON EA. FACE OF JOINT,SYMMETRICALLY(EXCEPT AS SHOWN)DESIGN CONFORMS W/NDS DESIGN SPECS, SSB,ANSI/TP1-95

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[Signature]

MAR 01 2001

====<<<ACES-32 Ver. 1.1>>>=====[001922]=====<<<FL.QUALITY-MMH>>>=====

Customer : FOGLIA Truss ID : FG4 Thu Mar 1 10:17:26 2001

Project # : 00469 Quantity : 2 Family # : 301

Span : 8-4 Top Pitch : /12

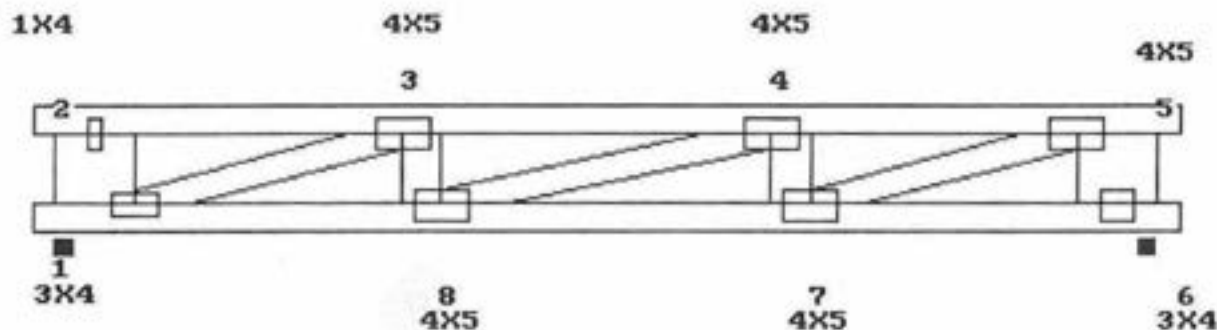
== ald(1/21,2000),v1.1 ==

FORCES - LOAD CASE #1			REACTIONS - SIZE	
1-2=-525	6-7= 0	3-1=-3123	1=-1678	2.00
2-3= 0	7-8= 2924	3-8= 59	6=-1678	1.50
3-4=-2913	8-1= 2913	4-8=-12		
4-5=-2924		4-7=-1067		
5-6=-1651		5-7= 3132		

114

PROVIDE FOR 1922 LBS UPLIFT AT JOINT 1 (1.15)
 PROVIDE FOR 1940 LBS UPLIFT AT JOINT 6 (1.16)
 PROVIDE FOR 139 LBS HORIZ. REACTION AT JOINT 1
 (SYMMETRIC PLATING TURNED ON)

0-2 2-9-5 5-6-11 8-4 0-2
 2-9-5 2-9-5 2-9-5



2-9-5 5-6-11 8-4
 2-9-5 2-9-5 2-9-5

LEFT HEIGHT:1-4 SPAN:8-4 RISE:1-4 RIGHT HEIGHT:1-4

LOADING (PSF)			MAX STRESSES		MINIMUM GRADE OF LUMBER	
TOP	30	15	TOP	3-4=0.538	TOP CHORD:2X4	No.2ND 19 SP
BOTT	0	10	BOTT	7-8=0.289	BOT CHORD:2X4	No.2ND 19 SP
			LL.DEFL.@8=0.03 < L/360	WEBS	:2X4	No.3 19 SP

REPETITIVE STRESSES NOT USED

SPACING : 24.0 in. o. c.
 NO. OF MEMBERS = 2

LOADING	STRESS INCREASE	LUMBER	PLATE	LOADING TYPE	PANEL(PLF) / JOINTS(LBS)
1	1.33	1.33	UNIFORM	2- 5= 390 6- 1= 20	

WEB: 1-2; 5-6 TO BE 2X8 SS 19 SP

BOTH END VERTICALS ARE EXPOSED.

2 MEMBERS NAILED TOG. W/1 ROW(S) OF .131x3 in. NAILS 12 in. o.c.(BOTT. CHS.),AND 16d NAILS 12 in. o.c.(TOP CHORD)

For Webs use 1 ROW of NAILS 12 in. o.c.

****NOTE:Special hanger/connection is reqd. to dist. top chord uniform load(s) to all plies.****

TRUSS HAS BEEN CHECKED FOR 145 M.P.H. WIND LOAD, WALL HEIGHT 16 FT, ENCLOSED BLDG. TYPE, ASSUMING 10 PSF TOTAL D.L.(7+3) (SBC-97).

DEFLECTION(IN.) L.L= 0.03,D.L=0.03,T.L=0.06

PLATES ARE MITEK M20-249,200 MANUFACTURED FROM ASTM A 446 GRD A GALVANIZED STEEL(EXCEPT AS SHOWN)

PLATE MUST BE INSTALLED ON EA. FACE OF JOINT,SYMMETRICALLY(EXCEPT AS SHOWN)DESIGN CONFORMS W/NDOS DESIGN SPECS, SSBC,ANSI/TPI-95

THIS DESIGN IS FOR TRUSS FABRICATION ONLY.FOR PERMANENT AND TEMPORARY BRACING(WHICH IS ALWAYS REQD)CONSULT BLDG ARCHITECT OR ENGINEER.

[Signature]
 MAR 01 2001

BUILDING CODE COMPLIANCE OFFICE
METRO-DADE FLAGLER BUILDING
140 WEST FLAGLER STREET, SUITE 1603
MIAMI, FLORIDA 33130-1583
(305) 375-2901
FAX (305) 375-2908

PRODUCT CONTROL DIVISION
(305) 375-2902
FAX (305) 372-6339

PRODUCT CONTROL NOTICE OF ACCEPTANCE

Southeastern Metals Manufacturing Co., Inc.
11801 Industry Drive
Jacksonville, FL 32226

Prod. examine

Product application for Product Approval of:

→ "5-V Crimp" Metal Roofing Panels

under Chapter 8 of the Miami-Dade County Code governing the use of Alternate Materials and Types of Construction, and completely described in the plans, specifications and calculations as submitted by: Construction Research Laboratory, Inc. and Hurricane Test Laboratory, Inc. has been recommended for acceptance by the Building Code Compliance Office to be used in Dade County, Florida under the specific conditions set forth on pages 2-4 and the standard conditions on page 5.

This approval shall not be valid after the expiration date stated below. The Building Code Compliance Office reserves the right to secure this product or material at any time from a jobsite or manufacturer's plant for quality control testing. If this product or material fails to perform in the approved manner, the Building Code Compliance Office may revoke, modify, or suspend the use of such product or material immediately. The Building Code Compliance Office reserves the right to revoke this approval, if it is determined by the Building Code Compliance Office that this product or material fails to meet the requirements of the South Florida Building Code.

The expense of such testing will be incurred by the manufacturer.

→ ACCEPTANCE NO.: 98-0429.09 Renews & Revises: 97-0404.05
EXPIRES: 06/23/01

Raul Rodriguez
Raul Rodriguez
Product Control Supervisor

THIS IS THE COVERSHEET, SEE ADDITIONAL PAGES FOR SPECIFIC AND GENERAL CONDITIONS BUILDING CODE COMMITTEE

This application for Product Approval has been reviewed by the Miami-Dade County Building Code Compliance Office and approved by the Building Code Committee to be used in Dade County, Florida under the conditions set forth above.

*6/11/01 TOWN OF SEWALL'S POINT
REVIEW*

BLDG OFFICIAL

*TOWN COPY
102 ABKIE CT.*

APPROVED: 06/23/98

FILE

Charles Danger
Charles Danger, P.E.
Director
Building Code Compliance Dept.
Miami-Dade County

PN 5228

(RPG. SUB 5250)



BUILDING CODE COMPLIANCE OFFICE
METRO-DADE FLAGLER BUILDING
140 WEST FLAGLER STREET, SUITE 1803
MIAMI, FLORIDA 33130-1563
(305) 375-2901
FAX (305) 375-2908**PRODUCT CONTROL NOTICE OF ACCEPTANCE**

Binnings Pan American

2805 N.E. 185 St.

Miami

FL 33163

PRODUCT CONTROL DIVISION

(305) 375-2902

FAX (305) 372-6339

Your application for Product Approval of:

*Series PA-14 Aluminum Single Hung Window*under Chapter 8 of the Metropolitan Dade County Code governing the use of Alternate Materials and Types of Construction, and completely described in the plans, specifications and calculations as submitted by: *Applicant, along with Drawing No. PA-14DC-01, Sheets 1 thru 8 of 8.*

has been recommended for acceptance by the Building Code Compliance office to be used in Dade County, Florida under the specific conditions set forth on pages 2 et. seq. and the Standard Conditions on page 3.

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Acceptance No.: 97-1027.09 (Revises No.: 96-0220.01)Expires: 07/09/01Raul Rodriguez
Product Control Supervisor**THIS IS THE COVERSHEET, SEE ADDITIONAL PAGES FOR SPECIFIC AND GENERAL CONDITIONS****BUILDING CODE COMMITTEE**

This application for Product Approval has been reviewed by the Metropolitan Dade County Building Code Compliance Department and approved by the Building Code Committee to be used in Dade County, Florida under the conditions set forth above.

1/15/01 TOWN OF SEVILLE'S POINT
Review:

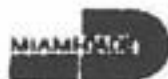
BLOG OFFICIAL

Charles Danger, P.E.
Director
Building Code Compliance Dept.
Metropolitan Dade CountyApproved: 07/09/98

FILE

TOWN - COPY
102 ABILE CT.

PN 5228

IMPACT FRENCH DOOR @ REAR PATIO

MIAMI-DADE COUNTY, FLORIDA
METRO-DADE FLAGLER BUILDING

BUILDING CODE COMPLIANCE OFFICE
METRO-DADE FLAGLER BUILDING
140 WEST FLAGLER STREET, SUITE 1603
MIAMI, FLORIDA 33130-1363
(305) 375-2901 FAX (305) 375-2908

PRODUCT CONTROL NOTICE OF ACCEPTANCE

Vinyl Tech/Progressive Glass Technology
1070 Technology Drive
Nokomis FL 34275

CONTRACTOR LICENSING SECTION
(305) 375-2337 FAX (305) 375-2338

CONTRACTOR ENFORCEMENT SECTION
(305) 375-2946 FAX (305) 375-2909

PRODUCT CONTROL DIVISION
(305) 375-2902 FAX (305) 375-4339

Your application for Product Approval of:

SWD-101 Outswing Aluminum French Door

under Chapter 8 of the Code of Miami-Dade County governing the use of Alternate Materials and Types of Construction, and completely described herein, has been recommended for acceptance by the Miami-Dade County Building Code Compliance Office (BCCO) under the conditions specified herein.

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The expense of such testing will be incurred by the manufacturer.

Acceptance No.: 99-0128.01 (Revises No.: 98-0506.02)

Expires: 11/22/2001

Raul Rodriguez
Chief Product Control Division

THIS IS THE COVERSHEET, SEE ADDITIONAL PAGES FOR SPECIFIC AND GENERAL CONDITIONS

BUILDING CODE & PRODUCT REVIEW COMMITTEE

This application for Product Approval has been reviewed by the BCCO and approved by the Building Code and Product Review Committee to be used in Dade County, Florida under the conditions set forth above.

Francisco J. Quintana, R.A.
Director
Miami-Dade County
Building Code Compliance Office

Approved: 02/18/1999

1 of 3



BUILDING CODE COMPLIANCE OFFICE
METRO-DADE FLAGLER BUILDING
140 WEST FLAGLER STREET, SUITE 1603
MIAMI, FLORIDA 33130-1563
(305) 375-2901 FAX (305) 375-2908**PRODUCT CONTROL NOTICE OF ACCEPTANCE**T.M. Window & Door Co.
601 N.W. 12th Ave.
Pompano Beach FL 33069CONTRACTOR LICENSING SECTION
(305) 375-2527 FAX (305) 375-2558CONTRACTOR ENFORCEMENT SECTION
(305) 375-2966 FAX (305) 375-2908PRODUCT CONTROL DIVISION
(305) 375-2902 FAX (305) 372-6339

Your application for Product Approval of:

Series 200 Outswing Aluminum French Door w/Sidelites(Heavy Duty Reinforced)

under Chapter 8 of the Code of Miami-Dade County governing the use of Alternate Materials and Types of Construction, and completely described herein, has been recommended for acceptance by the Miami-Dade County Building Code Compliance Office (BCCO) under the conditions specified herein.

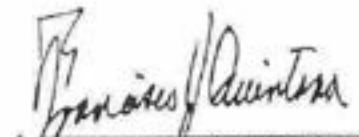
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The expense of such testing will be incurred by the manufacturer.

Acceptance No.: 99-0524.05Expires: 06/30/2000Raul Rodriguez
Chief Product Control Division**THIS IS THE COVERSHEET, SEE ADDITIONAL PAGES FOR SPECIFIC AND GENERAL
CONDITIONS****BUILDING CODE & PRODUCT REVIEW COMMITTEE**

This application for Product Approval has been reviewed by the BCCO and approved by the Building Code and Product Review Committee to be used in Dade County, Florida under the conditions set forth above.

Francisco J. Quintana, R.A.
Director
Miami-Dade County
Building Code Compliance OffiApproved: 07/15/1999

1 of 3

BUILDING CODE COMPLIANCE OFFICE
METRO-DADE FLAGLER BUILDING
140 WEST FLAGLER STREET, SUITE 1603
MIAMI, FLORIDA 33130-1563
(305) 375-2901 FAX (305) 375-2908CONTRACTOR LICENSING SECTION
(305) 375-2527 FAX (305) 375-2558CONTRACTOR ENFORCEMENT SECTION
(305) 375-2966 FAX (305) 375-2908PRODUCT CONTROL DIVISION
(305) 375-2902 FAX (305) 372-6339**PRODUCT CONTROL NOTICE OF ACCEPTANCE**Binnings Pan American
2805 N.E. 185 St.
Miami

EL 33163

Your application for Product Approval of:

Series PA-9, Models A, B, and C Aluminum Sliding Glass Door

under Chapter 8 of the Metropolitan Dade County Code governing the use of Alternate Materials and Types of Construction, and completely described in the plans, specifications and calculations as submitted by Applicant, along with Drawing No. PA-09-DC, Sheets 1 thru 6 of 6. (This NOA renews NOA No. 95-0831.01.)

has been recommended for acceptance by the Building Code Compliance office to be used in Dade County, Florida under the specific conditions set forth on pages 2 et. seq. and the Standard Conditions on page 3.

This approval shall not be valid after the expiration date stated below. The Office of Code Compliance reserves the right to secure this product or material at anytime from a jobsite or manufacturer's plant for quality control testing. If this product or material fails to perform in the approved manner, the Code Compliance Office may revoke, modify, or suspend the use of such product or material immediately. The applicant shall re-evaluate this product or material should any amendments to the South Florida Building Code be enacted affecting this product or material. The Building Code Compliance Office reserves the right to revoke this approval, if it is determined by the Building Code Compliance Office that this product or material fails to meet the requirements of the South Florida Building Code. The expense of such testing will be incurred by the manufacturer.

Acceptance No.: 98-1110.06Expires: 11/16/01

Raul Rodriguez
Product Control Supervisor

**THIS IS THE COVERSHEET, SEE ADDITIONAL PAGES FOR SPECIFIC AND GENERAL
CONDITIONS**

BUILDING CODE COMMITTEE

This application for Product Approval has been reviewed by the Metropolitan Dade County Building Code Compliance Department and approved by the Building Code Committee to be used in Dade County, Florida under the conditions set forth above.


Charles Danger, P.E.
Director
Building Code Compliance Dep
Metropolitan Dade CountyApproved: 12/03/98



MIAMI-DADE COUNTY, FLORIDA
METRO-DADE FLAGLER BUILDING

BUILDING CODE COMPLIANCE OFFICE
METRO-DADE FLAGLER BUILDING
140 WEST FLAGLER STREET, SUITE 1603
MIAMI, FLORIDA 33130-1563
(305) 375-2901 FAX (305) 375-2908

PRODUCT CONTROL NOTICE OF ACCEPTANCE

Alufab Hurricane Shutters, Inc.
2353 Ali Baba Avenue
Opa Locka FL 33054

CONTRACTOR LICENSING SECTION
(305) 375-2527 FAX (305) 375-2558

CONTRACTOR ENFORCEMENT SECTION
(305) 375-2966 FAX (305) 375-2908

PRODUCT CONTROL DIVISION
(305) 375-2902 FAX (305) 372-6339

Your application for Product Approval of:

20 gauge Galvanized Steel Storm Panel Shutter

under Chapter 8 of the Code of Miami-Dade County governing the use of Alternate Materials and Types of Construction, and completely described herein, has been recommended for acceptance by the Miami-Dade County Building Code Compliance Office (BCCO) under the conditions specified herein.

This approval shall not be valid after the expiration date stated below. BCCO reserves the right to secure this product or material at anytime from a jobsite or manufacturer's plant for quality control testing.

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The expense of such testing will be incurred by the manufacturer.

Acceptance No.: 98-0915.03 (Revises No.: 96-1107.02)

Expires: 07/17/2000

Raul Rodriguez
Chief Product Control Division

THIS IS THE COVERSHEET, SEE ADDITIONAL PAGES FOR SPECIFIC AND GENERAL CONDITIONS

BUILDING CODE & PRODUCT REVIEW COMMITTEE

This application for Product Approval has been reviewed by the BCCO and approved by the Building Code and Product Review Committee to be used in Dade County, Florida under the conditions set forth above.

Francisco J. Quintana, R.A.
Director
Miami-Dade County
Building Code Compliance Of

Approved: 01/20/1999

1 of 3





16-0129
MIAMI-DADE COUNTY, FLORIDA
METRO-DADE FLAGLER BUILDING

BUILDING CODE COMPLIANCE OFFICE
METRO-DADE FLAGLER BUILDING
140 WEST FLAGLER STREET, SUITE 1603
MIAMI, FLORIDA 33130-1563
(305) 375-2901 FAX (305) 375-2908

PRODUCT CONTROL NOTICE OF ACCEPTANCE

DAB Door Company, Inc.
12195 NW 98th Avenue
Hialeah Gardens FL 33016

CONTRACTOR LICENSING SECTION
(305) 375-2527 FAX (305) 375-2558

CONTRACTOR ENFORCEMENT SECTION
(305) 375-2966 FAX (305) 375-2908

PRODUCT CONTROL DIVISION
(305) 375-2902 FAX (305) 372-6339

Your application for Product Approval of:

Sectional Residential Garage Door

under Chapter 8 of the Metropolitan Dade County Code governing the use of Alternate Materials and Types of Construction, and completely described in the plans, specifications and calculations as submitted by: *applicant, along with Dab Door Co. Inc. drawing No 98-05, sheet 1 to 3 of 3, dated 02/23/98, revised on 08/24/98, signed and sealed by H. Farooq-PE.*

has been recommended for acceptance by the Building Code Compliance office to be used in Dade County, Florida under the specific conditions set forth on pages 2 et. seq. and the Standard Conditions on page 3.

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Acceptance No.: 98-0901.09 (Revises No.: 98-0409.04)

Expires: 08/14/01


Raul Rodriguez
Product Control Supervisor

THIS IS THE COVERSHEET, SEE ADDITIONAL PAGES FOR SPECIFIC AND GENERAL CONDITIONS

BUILDING CODE COMMITTEE

This application for Product Approval ~~has been~~ reviewed by the Metropolitan Dade County Building Code Compliance Department and approved by the Building Code Committee to be used in Dade County, Florida under the conditions set forth above.


Charles Danger, P.E.
Director
Building Code Compliance Dept.
Metropolitan Dade County

Approved: 10/22/98

-1-





MIAMI-DADE COUNTY, FLORIDA
METRO-DADE FLAGLER BUILDING

BUILDING CODE COMPLIANCE OFFICE
METRO-DADE FLAGLER BUILDING
140 WEST FLAGLER STREET, SUITE 1603
MIAMI, FLORIDA 33130-1563
(305) 375-2901 FAX (305) 375-2908

CONTRACTOR LICENSING SECTION
(305) 375-2527 FAX (305) 375-2558

CONTRACTOR ENFORCEMENT SECTION
(305) 375-2966 FAX (305) 375-2908

PRODUCT CONTROL DIVISION
(305) 375-2902 FAX (305) 372-6339

PRODUCT CONTROL NOTICE OF ACCEPTANCE

All Weather Control, Inc.
1505 South 30 Avenue
Hollywood FL 33020

Your application for Product Approval of:

"Hurri Down" Aluminum Bahama Shutter

under Chapter 8 of the Code of Miami-Dade County governing the use of Alternate Materials and Types of Construction, and completely described herein, has been recommended for acceptance by the Miami-Dade County Building Code Compliance Office (BCCO) under the conditions specified herein.

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The expense of such testing will be incurred by the manufacturer.

Acceptance No.: 99-0326.12

Expires: 04/18/2002

Raul Rodriguez
Chief Product Control Division

THIS IS THE COVERSHEET, SEE ADDITIONAL PAGES FOR SPECIFIC AND GENERAL CONDITIONS

BUILDING CODE & PRODUCT REVIEW COMMITTEE

This application for Product Approval has been reviewed by the BCCO and approved by the Building Code and Product Review Committee to be used in Dade County, Florida under the conditions set forth above.

Francisco J. Quintana, R.A.
Director

Miami-Dade County
Building Code Compliance Office

Approved: 06/03/1999

1 of 3





METROPOLITAN DADE COUNTY, FLORIDA
METRO-DADE FLAGLER BUILDING

BUILDING CODE COMPLIANCE OFFICE
METRO-DADE FLAGLER BUILDING
140 WEST FLAGLER STREET, SUITE 1803
MIAMI, FLORIDA 33130-1583

(305) 375-2901
FAX (305) 375-2908

PRODUCT CONTROL NOTICE OF ACCEPTANCE

Binnings Pan American

2805 N.E. 185 St.

Miami

FL 33163

PRODUCT CONTROL DIVISION

(305) 375-2902

FAX (305) 372-6339

Your application for Product Approval of:

Series PA-14 Aluminum Single Hung Window

under Chapter 8 of the Metropolitan Dade County Code governing the use of Alternate Materials and Types of Construction, and completely described in the plans, specifications and calculations as submitted by:

Applicant, along with Drawing No. PA-14DC-01, Sheets 1 thru 8 of 8.

has been recommended for acceptance by the Building Code Compliance office to be used in Dade County, Florida under the specific conditions set forth on pages 2 et. seq. and the Standard Conditions on page 3.

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Acceptance No.: 97-1027.09 (Revises No.: 96-0220.01)

Expires: 07/09/01

Raul Rodriguez
Product Control Supervisor

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CONDITIONS

BUILDING CODE COMMITTEE

This application for Product Approval has been reviewed by the Metropolitan Dade County Building Code Compliance Department and approved by the Building Code Committee to be used in Dade County, Florida under the conditions set forth above.

1/15/01 TOWN OF SEVERAL'S POINT
REVIEWED:
BLOG OFFICIAL

Charles Danger, P.E.
Director
Building Code Compliance Dept.
Metropolitan Dade County

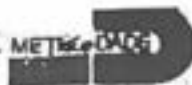
Approved: 07/09/98

FILE

TOWN - COPY
102 ABBIE CT.

PN 5228



**METROPOLITAN DADE COUNTY, FLORIDA
METRO-DADE FLAGLER BUILDING****BUILDING CODE COMPLIANCE OFFICE
METRO-DADE FLAGLER BUILDING
140 WEST FLAGLER STREET, SUITE 1603
MIAMI, FLORIDA 33130-1563
(305) 375-2901
FAX (305) 375-2908****PRODUCT CONTROL NOTICE OF ACCEPTANCE****Designer Windows of Palm Beaches
6451 E. Rogers Cir.
Boca Raton FL 33487****PRODUCT CONTROL DIVISION
(305) 375-2902
FAX (305) 372-6339**

Your application for Product Approval of:

*72" x 72" Rectangular Aluminum Fixed Window*under Chapter 8 of the Metropolitan Dade County Code governing the use of Alternate Materials and Types of Construction, and completely described in the plans, specifications and calculations as submitted by: *Applicant, along with Drawing No. W97-001, Sheet 1 of 1.*

has been recommended for acceptance by the Building Code Compliance office to be used in Dade County, Florida under the specific conditions set forth on pages 2 et. seq. and the Standard Conditions on page 3.

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Acceptance No.: 97-0723.04**Expires: 04/09/01**
**Raul Rodriguez
Product Control Supervisor****THIS IS THE COVERSHEET, SEE ADDITIONAL PAGES FOR SPECIFIC AND GENERAL
CONDITIONS****BUILDING CODE COMMITTEE**

This application for Product Approval has been reviewed by the Metropolitan Dade County Building Code Compliance Department and approved by the Building Code Committee to be used in Dade County, Florida under the conditions set forth above.

5.18.99
**Charles Danger, P.E.
Director
Building Code Compliance Dept.
Metropolitan Dade County****Approved: 04/09/98**

-1-



Designer Windows of Palm BeachesACCEPTANCE No.: 97-0723.04APPROVED : APR 09 1998EXPIRES : APR 09 2001NOTICE OF ACCEPTANCE: SPECIFIC CONDITIONS**1. SCOPE**

- 1.1 This approves an aluminum fixed window as described in section 2 of this Notice of Acceptance, designed to comply with the South Florida Building Code, (SFBC), 1994 Edition for Miami-Dade County, for the locations where the pressure requirements, as determined by SFBC Chapter 23, do not exceed the Design Pressure Rating values indicated in the approved drawings.

2. PRODUCT DESCRIPTION

The 72" wide x 72" Rectangular Aluminum Fixed Window" and its components shall be constructed in strict compliance with the following document: Drawing No. W97-001, titled "72" wide x 72" Rectangular Window", Sheet 1 of 1, dated 01/31/97, revised on 02/14/98, signed and sealed by K. Venkatesan, P.E., bearing the Miami-Dade County Product Control approval stamp with the Notice of Acceptance number and approval date by the Miami-Dade County Product Control Division. These documents shall hereinafter be referred to as the approved drawings.

3. LIMITATIONS

- 3.1 This approval applies to single unit applications of aluminum fixed window, as shown in approved drawings.
- 3.2 Units with dimensions equal to or smaller than those shown in approved drawings shall qualify under this approval.
- 3.3 Installation is limited by the Design Pressure Rating shown in approved drawings.

4. INSTALLATION:

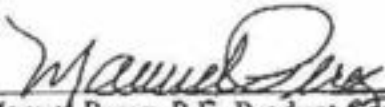
- 4.1 The aluminum fixed window and its components shall be constructed in strict compliance with the approved drawings.
- 4.2 Hurricane protection system (shutters): the installation of this unit will require a hurricane protection system.

5. LABELING

- 5.1 Each unit shall bear a permanent label with the manufacturer's name or logo, city, state and following statement: "Miami-Dade County Product Control Approved".

6. BUILDING PERMIT REQUIREMENTS

- 6.1 Application for building permit shall be accompanied by two copies of the following:
- 6.1.1 This Notice of Acceptance.
- 6.1.2 Duplicate copies of the approved drawings, as identified in section 2 of this Notice of Acceptance, clearly marked to show the components selected for the proposed installation.
- 6.1.3 Any other documents required by the Building Official or the South Florida Building Code (SFBC) in order to properly evaluate the installation of this system.


Manuel Perez, P.E. Product Control Examiner
Product Control Division



MIAMI-DADE COUNTY, FLORIDA
METRO-DADE FLAGLER BUILDING

BUILDING CODE COMPLIANCE OFFICE
METRO-DADE FLAGLER BUILDING
140 WEST FLAGLER STREET, SUITE 1603
MIAMI, FLORIDA 33130-1563
(305) 375-2901 FAX (305) 375-2908

PRODUCT CONTROL NOTICE OF ACCEPTANCE

T.M. Window & Door Co.
601 N.W. 12th Ave.
Pompano Beach FL 33069

CONTRACTOR LICENSING SECTION
(305) 375-2527 FAX (305) 375-2558

CONTRACTOR ENFORCEMENT SECTION
(305) 375-2966 FAX (305) 375-2908

PRODUCT CONTROL DIVISION
(305) 375-2902 FAX (305) 372-6339

Your application for Product Approval of:

Series 200 Outswing Aluminum French Door w/Sidelites(Heavy Duty Reinforced)

under Chapter 8 of the Code of Miami-Dade County governing the use of Alternate Materials and Types of Construction, and completely described herein, has been recommended for acceptance by the Miami-Dade County Building Code Compliance Office (BCCO) under the conditions specified herein.

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The expense of such testing will be incurred by the manufacturer.

Acceptance No.: 99-0524.05

Expires: 06/30/2000

Raul Rodriguez
Chief Product Control Division

THIS IS THE COVERSHEET, SEE ADDITIONAL PAGES FOR SPECIFIC AND GENERAL
CONDITIONS

BUILDING CODE & PRODUCT REVIEW COMMITTEE

This application for Product Approval has been reviewed by the BCCO and approved by the Building Code and Product Review Committee to be used in Dade County, Florida under the conditions set forth above.

Francisco J. Quintana, R.A.
Director
Miami-Dade County
Building Code Compliance Office

Approved: 07/15/1999

1 of 3



LOT
IMPACT FRENCH DOOR @ REAR PATIO



MIAMI-DADE COUNTY, FLORIDA
METRO-DADE FLAGLER BUILDING

BUILDING CODE COMPLIANCE OFFICE
METRO-DADE FLAGLER BUILDING
140 WEST FLAGLER STREET, SUITE 1403
MIAMI, FLORIDA 33130-1343
(305) 375-2901 FAX (305) 375-2908

PRODUCT CONTROL NOTICE OF ACCEPTANCE

Vinyl Tech/Progressive Glass Technology
1070 Technology Drive
Nokomis FL 34275

CONTRACTOR LICENSING SECTION
(305) 375-2527 FAX (305) 375-2558

CONTRACTOR ENFORCEMENT SECTION
(305) 375-2946 FAX (305) 375-2908

PRODUCT CONTROL DIVISION
(305) 375-2902 FAX (305) 375-4339

Your application for Product Approval of:

SWD-101 Outswing Aluminum French Door

under Chapter 8 of the Code of Miami-Dade County governing the use of Alternate Materials and Types of Construction, and completely described herein, has been recommended for acceptance by the Miami-Dade County Building Code Compliance Office (BCCO) under the conditions specified herein.

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The expense of such testing will be incurred by the manufacturer.

Acceptance No. 99-0128.01 (Revises No.: 98-0506.02)

Expires: 11/22/2001

Raul Rodriguez

Chief Product Control Division

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BUILDING CODE & PRODUCT REVIEW COMMITTEE

This application for Product Approval has been reviewed by the BCCO and approved by the Building Code and Product Review Committee to be used in Dade County, Florida under the conditions set forth above.

Francisco J. Quintana, R.A.

Director
Miami-Dade County
Building Code Compliance Office

Approved: 02/18/1999

1 of 3





MIAMI-DADE COUNTY, FLORIDA
METRO-DADE FLAGLER BUILDING

BUILDING CODE COMPLIANCE OFFICE
METRO-DADE FLAGLER BUILDING
140 WEST FLAGLER STREET, SUITE 1603
MIAMI, FLORIDA 33130-1563
(305) 375-2901 FAX (305) 375-2908

PRODUCT CONTROL NOTICE OF ACCEPTANCE

T.M. Window & Door Co.
601 N.W. 12th Ave.
Pompano Beach FL 33069

CONTRACTOR LICENSING SECTION
(305) 375-2527 FAX (305) 375-2558

CONTRACTOR ENFORCEMENT SECTION
(305) 375-2966 FAX (305) 375-2908

PRODUCT CONTROL DIVISION
(305) 375-2902 FAX (305) 372-6339

Your application for Product Approval of:

Series 200 Outswing Aluminum French Door w/Sidelites(Heavy Duty Reinforced)

under Chapter 8 of the Code of Miami-Dade County governing the use of Alternate Materials and Types of Construction, and completely described herein, has been recommended for acceptance by the Miami-Dade County Building Code Compliance Office (BCCO) under the conditions specified herein.

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The expense of such testing will be incurred by the manufacturer.

Acceptance No.: 99-0524.05

Expires: 06/30/2000

Raul Rodriguez
Chief Product Control Division

THIS IS THE COVERSHEET, SEE ADDITIONAL PAGES FOR SPECIFIC AND GENERAL CONDITIONS

BUILDING CODE & PRODUCT REVIEW COMMITTEE

This application for Product Approval has been reviewed by the BCCO and approved by the Building Code and Product Review Committee to be used in Dade County, Florida under the conditions set forth above.

Francisco J. Quintana, R.A.
Director
Miami-Dade County
Building Code Compliance Office

Approved: 07/15/1999

1 of 3





MIAMI-DADE COUNTY, FLORIDA
METRO-DADE FLAGLER BUILDING

BUILDING CODE COMPLIANCE OFFICE
METRO-DADE FLAGLER BUILDING
140 WEST FLAGLER STREET, SUITE 1603
MIAMI, FLORIDA 33130-1563
(305) 375-2901 FAX (305) 375-2908

PRODUCT CONTROL NOTICE OF ACCEPTANCE

Binnings Pan American

2805 N.E. 185 St.

Miami

FL 33163

CONTRACTOR LICENSING SECTION
(305) 375-2527 FAX (305) 375-2558

CONTRACTOR ENFORCEMENT SECTION
(305) 375-2966 FAX (305) 375-2908

PRODUCT CONTROL DIVISION
(305) 375-2902 FAX (305) 372-6339

Your application for Product Approval of:

Series PA-9, Models A, B, and C Aluminum Sliding Glass Door

under Chapter 8 of the Metropolitan Dade County Code governing the use of Alternate Materials and Types of Construction, and completely described in the plans, specifications and calculations as submitted by: *Applicant, along with Drawing No. PA-09-DC, Sheets 1 thru 6 of 6. (This NOA renews NOA No. 95-0831.01.)*

has been recommended for acceptance by the Building Code Compliance office to be used in Dade County, Florida under the specific conditions set forth on pages 2 et. seq. and the Standard Conditions on page 3.

This approval shall not be valid after the expiration date stated below. The Office of Code Compliance reserves the right to secure this product or material at anytime from a jobsite or manufacturer's plant for quality control testing. If this product or material fails to perform in the approved manner, the Code Compliance Office may revoke, modify, or suspend the use of such product or material immediately. The applicant shall re-evaluate this product or material should any ammendments to the South Florida Building Code be enacted affecting this product or material. The Building Code Compliance Office reserves the the right to revoke this approval, if it is determined by the Building Code Compliance Office that this product or material fails to meet the requirements of the South Florida Building Code. The expense of such testing will be incurred by the manufacturer.

Acceptance No.: 98-1110.06

Expires: 11/16/01

Raul Rodriguez
Product Control Supervisor

THIS IS THE COVERSHEET, SEE ADDITIONAL PAGES FOR SPECIFIC AND GENERAL
CONDITIONS

BUILDING CODE COMMITTEE

This application for Product Approval has been reviewed by the Metropolitan Dade County Building Code Compliance Department and approved by the Building Code Committee to be used in Dade County, Florida under the conditions set forth above.

Charles Danger, P.E.
Director
Building Code Compliance Dept.
Metropolitan Dade County

Approved: 12/03/98



MIAMI-DADE COUNTY, FLORIDA
METRO-DADE FLAGLER BUILDING

BUILDING CODE COMPLIANCE OFFICE
METRO-DADE FLAGLER BUILDING
140 WEST FLAGLER STREET, SUITE 1603
MIAMI, FLORIDA 33130-1563
(305) 375-2901 FAX (305) 375-2908

PRODUCT CONTROL NOTICE OF ACCEPTANCE

Alufab Hurricane Shutters, Inc.
2353 Ali Baba Avenue
Opa Locka FL 33054

CONTRACTOR LICENSING SECTION
(305) 375-2527 FAX (305) 375-2558

CONTRACTOR ENFORCEMENT SECTION
(305) 375-2966 FAX (305) 375-2908

PRODUCT CONTROL DIVISION
(305) 375-2902 FAX (305) 372-6339

Your application for Product Approval of:

20 gauge Galvanized Steel Storm Panel Shutter

under Chapter 8 of the Code of Miami-Dade County governing the use of Alternate Materials and Types of Construction, and completely described herein, has been recommended for acceptance by the Miami-Dade County Building Code Compliance Office (BCCO) under the conditions specified herein.

This approval shall not be valid after the expiration date stated below. BCCO reserves the right to secure this product or material at anytime from a jobsite or manufacturer's plant for quality control testing.

If this product or material fails to perform in the approved manner, BCCO may revoke, modify, or suspend the use of such product or material immediately. BCCO reserves the right to revoke this approval, if it is determined BCCO that this product or material fails to meet the requirements of the South Florida Building Code.

The expense of such testing will be incurred by the manufacturer.

Acceptance No.: 98-0915.03 (Revises No.: 96-1107.02)

Expires: 07/17/2000

Raul Rodriguez
Chief Product Control Division

THIS IS THE COVERSHEET, SEE ADDITIONAL PAGES FOR SPECIFIC AND GENERAL
CONDITIONS

BUILDING CODE & PRODUCT REVIEW COMMITTEE

This application for Product Approval has been reviewed by the BCCO and approved by the Building Code and Product Review Committee to be used in Dade County, Florida under the conditions set forth above.

Francisco J. Quintana, R.A.
Director

Miami-Dade County
Building Code Compliance Office

Approved: 01/20/1999

1 of 3





BUILDING CODE COMPLIANCE OFFICE
METRO-DADE FLAGLER BUILDING
140 WEST FLAGLER STREET, SUITE 1603
MIAMI, FLORIDA 33130-1363
(305) 375-2901 FAX (305) 375-2908

PRODUCT CONTROL NOTICE OF ACCEPTANCE

DAB Door Company, Inc.
12195 NW 98th Avenue
Hialeah Gardens FL 33016

CONTRACTOR LICENSING SECTION
(305) 375-2527 FAX (305) 375-2558

CONTRACTOR ENFORCEMENT SECTION
(305) 375-2966 FAX (305) 375-2908

PRODUCT CONTROL DIVISION
(305) 375-2902 FAX (305) 372-6339

Your application for Product Approval of:

Sectional Residential Garage Door

under Chapter 8 of the Metropolitan Dade County Code governing the use of Alternate Materials and Types of Construction, and completely described in the plans, specifications and calculations as submitted by: *applicant, along with Dab Door Co. Inc. drawing No 98-05, sheet 1 to 3 of 3, dated 02/23/98, revised on 08/24/98, signed and sealed by H. Farooq-PE.*

has been recommended for acceptance by the Building Code Compliance office to be used in Dade County, Florida under the specific conditions set forth on pages 2 et. seq. and the Standard Conditions on page 3.

This approval shall not be valid after the expiration date stated below. The Office of Code Compliance reserves the right to secure this product or material at anytime from a jobsite or manufacturer's plant for quality control testing. If this product or material fails to perform in the approved manner, the Code Compliance Office may revoke, modify, or suspend the use of such product or material immediately. The applicant shall re-evaluate this product or material should any amendments to the South Florida Building Code be enacted affecting this product or material. The Building Code Compliance Office reserves the right to revoke this approval, if it is determined by the Building Code Compliance Office that this product or material fails to meet the requirements of the South Florida Building Code. The expense of such testing will be incurred by the manufacturer.

Acceptance No.: 98-0901.09 (Revises No.: 98-0409.04)

Expires: 08/14/01


Raul Rodriguez
Product Control Supervisor

THIS IS THE COVERSHEET, SEE ADDITIONAL PAGES FOR SPECIFIC AND GENERAL CONDITIONS

BUILDING CODE COMMITTEE

This application for Product Approval ~~has~~ been reviewed by the Metropolitan Dade County Building Code Compliance Department and approved by the Building Code Committee to be used in Dade County, Florida under the conditions set forth above.


Charles Danger, P.E.
Director
Building Code Compliance Dept.
Metropolitan Dade County

Approved: 10/22/98

-1-





MIAMI-DADE COUNTY, FLORIDA
METRO-DADE FLAGLER BUILDING

BUILDING CODE COMPLIANCE OFFICE
METRO-DADE FLAGLER BUILDING
140 WEST FLAGLER STREET, SUITE 1603
MIAMI, FLORIDA 33130-1563
(305) 375-2901 FAX (305) 375-2908

CONTRACTOR LICENSING SECTION
(305) 375-2527 FAX (305) 375-2558

CONTRACTOR ENFORCEMENT SECTION
(305) 375-2966 FAX (305) 375-2908

PRODUCT CONTROL DIVISION
(305) 375-2002 FAX (305) 375-6339

PRODUCT CONTROL NOTICE OF ACCEPTANCE

All Weather Control, Inc.
1505 South 30 Avenue
Hollywood FL 33020

Your application for Product Approval of:
"Hurri Down" Aluminum Bahama Shutter

under Chapter 8 of the Code of Miami-Dade County governing the use of Alternate Materials and Types of Construction, and completely described herein, has been recommended for acceptance by the Miami-Dade County Building Code Compliance Office (BCCO) under the conditions specified herein.

This approval shall not be valid after the expiration date stated below. BCCO reserves the right to secure this product or material at anytime from a jobsite or manufacturer's plant for quality control testing.

If this product or material fails to perform in the approved manner, BCCO may revoke, modify, or suspend the use of such product or material immediately. BCCO reserves the right to revoke this approval, if it is determined BCCO that this product or material fails to meet the requirements of the South Florida Building Code.

The expense of such testing will be incurred by the manufacturer.

Acceptance No.: 99-0326.12

Expires: 04/18/2002

Raul Rodriguez
Chief Product Control Division

THIS IS THE COVERSHEET, SEE ADDITIONAL PAGES FOR SPECIFIC AND GENERAL CONDITIONS

BUILDING CODE & PRODUCT REVIEW COMMITTEE

This application for Product Approval has been reviewed by the BCCO and approved by the Building Code and Product Review Committee to be used in Dade County, Florida under the conditions set forth above.

Francisco J. Quintana, R.A.
Director

Miami-Dade County
Building Code Compliance Office

Approved: 06/03/1999

1 of 3



MASTER PERMIT NO. 5228

TOWN OF SEWALL'S POINT

Date 8/29/01

BUILDING PERMIT NO. 5521

Building to be erected for FOGLIA CONTR. CORP.

Type of Permit IRRIGATION

Applied for by FOGLIA CONTR. CORP.

(Contractor)

Building Fee \$130.00

Subdivision SEWALL'S MEADOW Lot 22

Block

Radon Fee 60.00

Address 102 AIRBIE COURT

Impact Fee

Type of structure S.F.R. (UNDER CONST.)

A/C Fee

8/29/01 "AFTER PART" PERMIT
- DOUBLE FEE

Electrical Fee

Parcel Control Number:

Plumbing Fee

13-38-41-013-000-00220-00000

Roofing Fee 6.00

Amount Paid \$66.00 Check # 4581 Cash

Other Fees (RV) 3.00

Total Construction Cost \$ 2,450.00

TOTAL Fees \$330.00
\$66.00

Signed [Signature]

Applicant

Signed

[Signature]
Town Building Inspector OFFICIAL

BUILDING PERMIT

FORM BOARD SURVEY DATE _____
COMPACTION TESTS DATE _____
GROUND ROUGH DATE _____
SOIL POISONING DATE _____
FOOTINGS / PIERS DATE _____
SLAB ON GRADE DATE _____
TIE-BEAMS & COLUMNS DATE _____
STRAPS AND ANCHORS DATE _____
DRIVEWAY DATE _____
AS-BUILT SURVEY DATE _____

SHEATHING DATE _____
FRAMING DATE _____
INSULATION DATE _____
ROOF DRY-IN DATE _____
ROOF FINAL DATE _____
METER FINAL DATE _____
AS BUILT SURVEY DATE _____
STORM PANELS DATE _____
LANDCAPE & GRADE DATE _____
FINAL INSPECTION DATE 9/5/01

FLOOD ZONE _____

LOWEST HABITABLE FLOOR ELEV. _____

24 HOURS NOTICE REQUIRED FOR INSPECTIONS.

CALL 287-2455

WORK HOURS – 8:00 AM UNTIL 5:00 PM

MONDAY THROUGH SATURDAY

☐ New Construction ☐ Remodel ☐ Addition ☐ Demolition

This permit must be visible from the street, accessible to the inspector.

FURTHER CONDITIONS ARE SET FORTH IN THE APPLICATION FOR PERMIT,
NOTATIONS ON THE APPROVED SUBMITTALS, AND ATTACHMENTS IN THE PERMIT FILE.
DO NOT FASTEN THIS OR ANY OTHER SIGN TO A TREE!

TOWN OF SEWALL'S POINT

Building Department - Inspection Log

Date of Inspection: ☐ Mon ☐ Wed ☒ Fri AUGUST 31, 2001; Page 3 of 3.

PERMIT	OWNER/ADDRESS/CONTR.	INSPECTION TYPE	RESULTS	NOTES/COMMENTS:
✓ 5380	GIFFORD	POOL DECK	Passed	~~Co. per. Test.~~
(16)	85 N. SEWALL'S POINT RD A&G CONCRETE POOLS			Waterfall area only INSPECTOR: <u>J 8/31</u>
✓ 5489	STRACUZZI (285-2815) (260-1443 con)	U/G PLUMBING	Passed	
S (7)	12 RIO VISTA DR. RUCCOLO			INSPECTOR: <u>J 8/31</u>
✓ 5324	HECKENBERG	MAS. WALL - FINAL	Passed	PN 5405 - ENTRY COL. (STATUS?)
(13)	SLAGOON ISLAND CT. U/B (JAMIE 718-2777)	ENTRY COL. - FINAL	Passed	INSPECTOR: <u>J 8/31</u>
✓ T/R	TENNY	FIELD VERIF.	Passed	(AMENDED PLAN -)
S (9)	105 SEWALL'S POINT RD GORDON'S TREE SERVICE		PN 0494 8/31/01	(W/O GUMBO LIMBO) INSPECTOR: <u>J 8/31</u>
✓ T/R	AYRES	FIELD VERIF.	Passed	
S (8)	155 RIVER RD. MONTE'S TREE SERVICE		PN 0493 8/31/01	INSPECTOR: <u>J 8/31</u>
✓ 5482	RAO	IRRIGATION - FINAL	Passed	MPN 5192
(15)	30 CASTLE HILL WAY FRITZ IRRIGATION			INSPECTOR: <u>J 8/31</u>
✓ 5521	FOGLIA	IRRIGATION - FINAL	Passed	MPN 5228
S (12)	102 ABBIE COURT FOGLIA CONTRACTING			INSPECTOR: <u>J 8/31</u>

OTHER: _____

Southern Irrigation, Inc.
5207 SW Moore Street
Palm City, FL 34990
561-288-1883
561-288-1894 fax



FILE
IRRG.
PERMIT
5521
(MPN 5228)

August 21, 2001

Town Of Sewalls Point
15 Sewalls Point Road
Sewalls Point, FL 34996

Mr. Edward Arnold:

Lot 22 Sewalls Meadow, 102 Abbie Court, as per section 22-146 is installed as a low volume irrigation system with a rain sensor devise.

Martin County Competency #SP00734

Sincerely,

Robin G. Henn
Sec. / Tres.



**Town of Sewall's Point
BUILDING PERMIT APPLICATION**

RECEIVED
AUG 21 2001

Bldg. Permit Number: 5521

Owner or Titleholder's Name FCGUA CONTRACTING CORP Phone No. (FH) 75-8808
 Street: 7428 WYDS Rd. City CORAL SPRINGS State: FL Zip 33007
 Legal Description of Property: 102 ABBIE COURT
 Parcel Number: 13-38-4-013-000-00220-00000

Location of Job Site: LOT 22 SEWALLS MEADOW
 TYPE OF WORK TO BE DONE: IRRIGATION SYSTEM

CONTRACTOR/Company Name: SEWELL IRRIGATION Phone No. (FH) 308-7083
 Street: SAME AS ABOVE City _____ State: _____ Zip _____
 State Registration: _____ State License: _____

ARCHITECT: _____ Phone No. () _____
 Street: _____ City _____ State: _____ Zip _____

ENGINEER: _____ Phone No. () _____
 Street: _____ City _____ State: _____ Zip _____

AREA SQUARE FOOTAGE - SEWER - ELECTRIC:
 Living Area: _____ Garage Area: _____ Carport: _____ Accessory Bldg: _____
 Covered Patio: _____ Scr. Porch: _____ Wood Deck: _____
 Type Sewage: _____ Septic Tank Permit # from Health Dept. _____
 New Electrical Service Size: _____ AMPS

FLOOD HAZARD INFORMATION
 Flood zone: _____ Minimum Base Flood Elevation (BFE): _____ NGVD
 Proposed first habitable floor finished elevation: _____ NGVD (minimum 1 foot above BFE)

COSTS AND VALUES
 Estimated cost of construction or Improvement: \$ 2450⁰⁰
 Estimated Fair Market Value (FMV) prior to improvement: \$ _____
 If Improvement is cost greater than 50% of Fair Market Value? YES _____ NO _____

Approved by Town Engineer _____ (if required)
 Date: _____
 Approved by Building Official: [Signature]
 Date: 8/24/01

NOTICE: In addition to the requirements of this permit, there may be additional restrictions applicable to this property that may be found in the public records of COUNTY OF MARTIN, and there may be additional permits required from other governmental entities such as water management districts, state and federal agencies.
 Replat required upon completion of slab or footing inspection And Prior to any further inspections.

AC 5910992

STATE OF FLORIDA
DEPARTMENT OF BUSINESS AND PROFESSIONAL REGULATION
CONSTRUCTION INDUSTRY LICENSING BOARD

DATE BATCH NUMBER LICENSE NO.

07/03/2002 00900145
The Florida Building & Construction Trades Council
Under the provisions of Chapter 489
Expiration date: AUG 31, 2002

FOGLIA, JOSEPH N.
FOGLIA CONTRACTING CORP
7428 WILES RD
CORAL SPRINGS

JEB BUSH
GOVERNOR

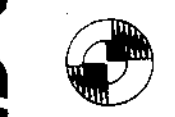
DISPLAY AS REQUIRED BY LAW

CYNTHIA A. HENDERSON
SECRETARY

CG-C038434

REVISIONS	BY
ADD SITE DATA 01/13/2001	W.G.H.

OFFICE PHONE # (561) 286-7694
OFFICE FAX # (561) 220-7993



1501 DECKER AVENUE SUITE 419-D
STUART, FL 34994

RECEIVED
JAN 15 2001
BY: [Signature]

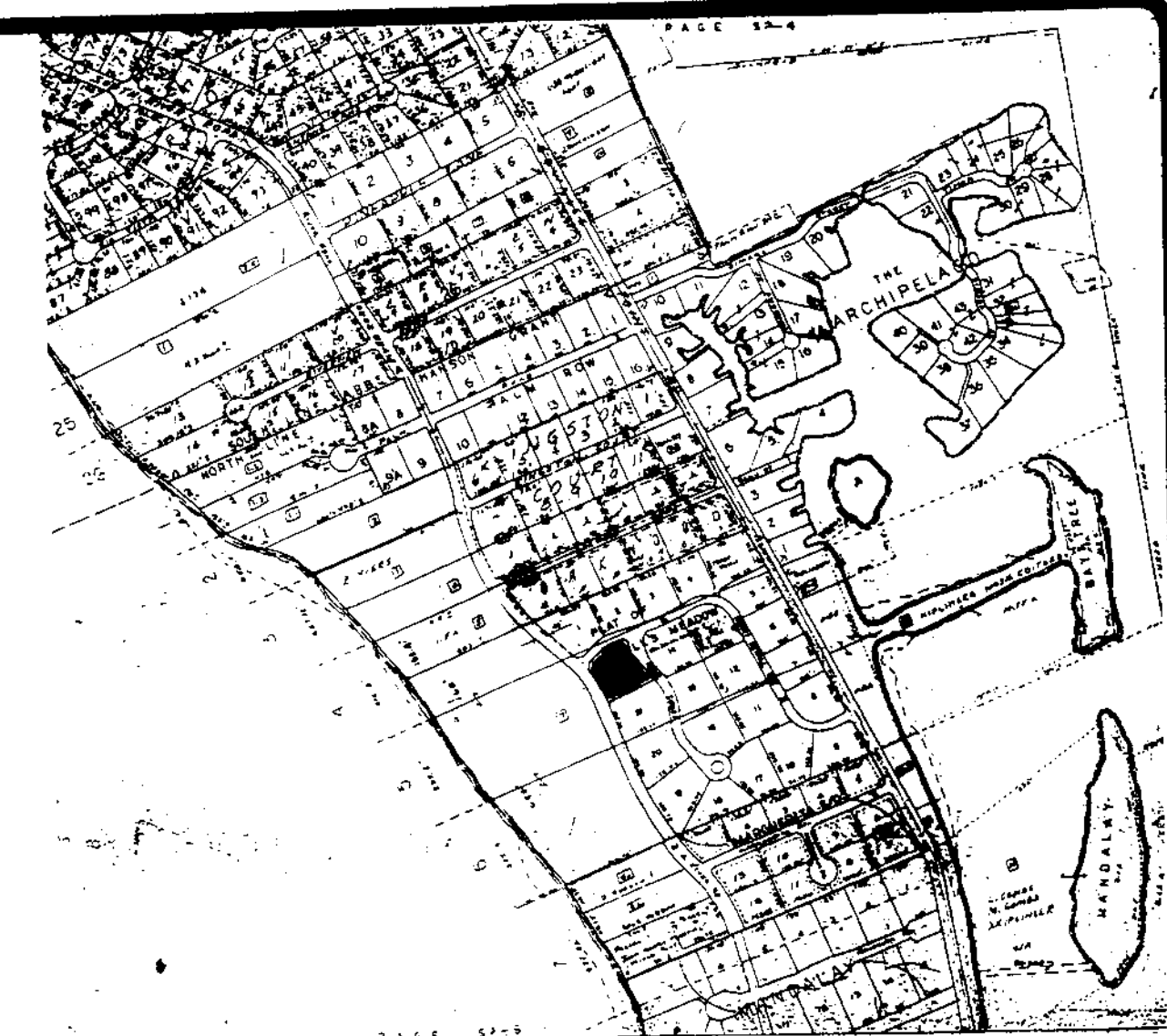
DRAWN W.G.H.
CHECKED E.R.S.
DATE (FIELD) 10/1/2000
SCALE 1" = 10.00'
JOB NO. 690-06-010
SHEET 1
OF ONE SHEETS

ACCURIGHT LAND SURVEYING, INC.

BOUNDARY SURVEY

PREPARED FOR:
FOGLIA CONTRACTING CORPORATION

LOCATION MAP



LEGAL DESCRIPTION

KNOWN AS LOT 22, "SEWALL'S MEADOW", ACCORDING TO THE PLAT THEREOF, AS RECORDED IN PLAT BOOK 14, PAGE 32, PUBLIC RECORDS OF MARTIN COUNTY, FLORIDA.

THE GENERAL SLOPE OF THE PROPERTY, RECORDED EASEMENTS FROM THE RECORD PLAT, FILLED AREAS AND DRAINAGE FEATURES ARE AS SHOWN.

SITE PLAN/SURVEY

1/5/00 TOWN OF SEWALL'S POINT REVIEW
[Signature]
BLDG. OFFICIAL

FILE TOWN COPY
102 ABBIE CT

PD 5228

SITE DATA
TOTAL LOT SIZE - 25,130.3 S.F. - 100%
IMPERVIOUS AREA BREAKDOWN
A/C, GARAGE, ENTRY & PATIO - 4,470 S.F. - 17.8%
POOL AREA, WALKS & PATS - 1,515 S.F. - 6%
DRIVEWAY - 2615 S.F. - 10.4%
TOTAL IMPERVIOUS AREA - 8,600 S.F. - 34.2%

$$\begin{aligned} \text{MAX. FIB. FL. ELEV.} &= (A+B+C) \div 3 + 1.5 \\ &= (8.5 + 10.8 + 13.0) \div 3 + 1.5 = 12.0 \end{aligned}$$

SURVEYOR'S NOTES:

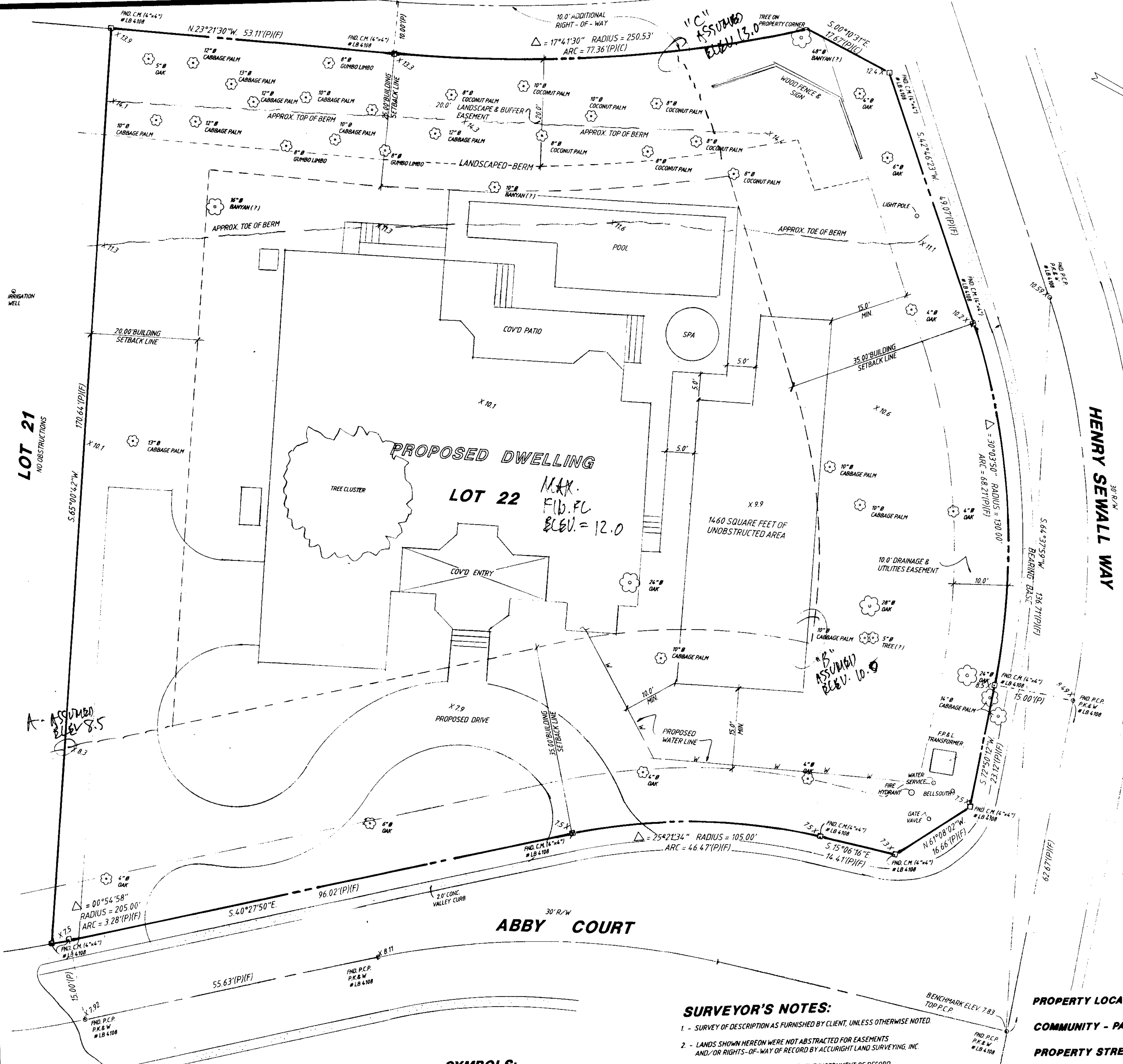
- SURVEY OF DESCRIPTION AS FURNISHED BY CLIENT, UNLESS OTHERWISE NOTED
- LANDS SHOWN HEREON WERE NOT ABSTRACTED FOR EASEMENTS AND/OR RIGHTS-OF-WAY OF RECORD BY ACCURIGHT LAND SURVEYING, INC.
- ALL BEARINGS ARE REFERENCED TO THE INSTRUMENT OF RECORD AS SHOWN HEREON, UNLESS OTHERWISE NOTED
- ELEVATIONS SHOWN HEREON ARE RELATIVE TO NATIONAL GEODETIC VERTICAL DATUM OF 1929, UNLESS OTHERWISE NOTED. SEE ABOVE FOR REFERENCE BENCH MARK IF APPLICABLE.
- THERE ARE NO ABOVE GROUND ENCROACHMENTS, UNLESS OTHERWISE NOTED

SYMBOLS:

- Δ - DELTA / CENTRAL ANGLE
- ⊕ - CENTERLINE
- - DRAINAGE FLOW
- X 11.32 - EXISTING ELEVATION
- ⊕ - PROPERTY LINE

LEGEND:

- (PI) - DENOTES DISTANCE, ANGLE OR BEARING BY FURNISHED DESCRIPTION
- (PI) - DENOTES MEASURED DISTANCE, ANGLE OR BEARING
- (C) - CALCULATED DISTANCE, ANGLE OR DISTANCE
- CONC. - CONCRETE
- FND - FOUND
- C.M. - CONCRETE MONUMENT
- COVD. - COVERED
- ELEV. - ELEVATION
- C.B.S. - CONCRETE BLOCK STRUCTURE
- R/W - RIGHT OF WAY
- I.B. - IRON BAR



SURVEYOR'S CERTIFICATE

WE HEREBY CERTIFY THAT THE BOUNDARY SURVEY AS SHOWN HEREON IS TRUE AND CORRECT TO THE BEST OF OUR KNOWLEDGE AND BELIEF AS SURVEYED UNDER OUR DIRECTION. WE FURTHER CERTIFY THAT IT MEETS THE MINIMUM TECHNICAL STANDARDS UNDER RULE 61G17 - 6 FLORIDA ADMINISTRATIVE CODE, PURSUANT TO CHAPTER 472.027 FLORIDA STATUTES.

NOT VALID WITHOUT THE SIGNATURE AND THE ORIGINAL RAISED SEAL OF A FLORIDA LICENSED SURVEYOR AND MAPPER

ACCURIGHT LAND SURVEYING, INC.

STARKEY - PROFESSIONAL SURVEYOR & MAPPER
GISTRATION NO. 4459 - STATE OF FLORIDA

EARLE R. RL

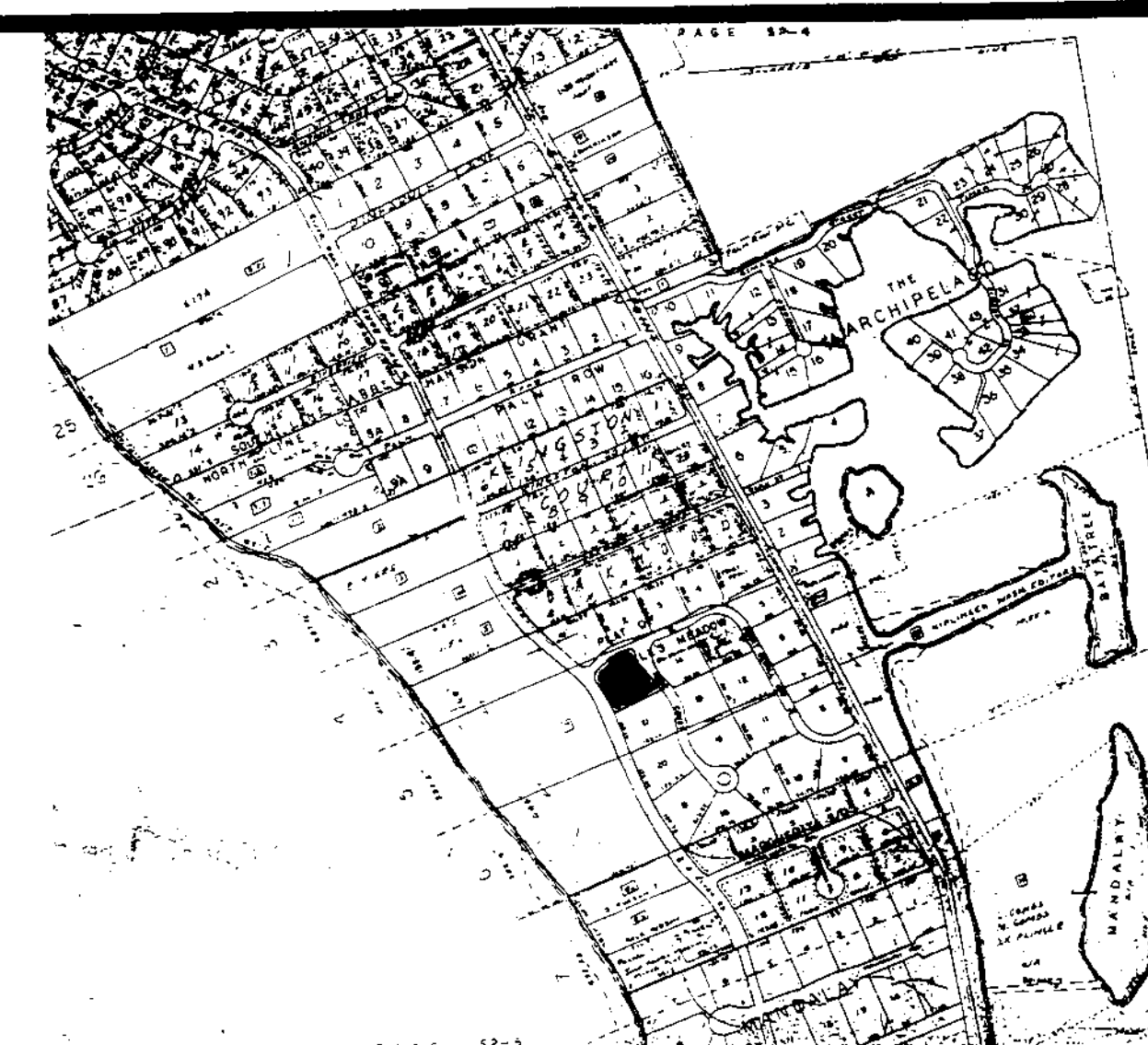
REVISIONS	BY
ADD SITE DATA 01/13/2001	W.G.H.
TIE-IN 02/10/2001	W.G.H.
FORM BOARD TIE-IN 02/24/2001	W.G.H.

OFFICE PHONE # (561) 286-7694
OFFICE FAX # (561) 220-7993

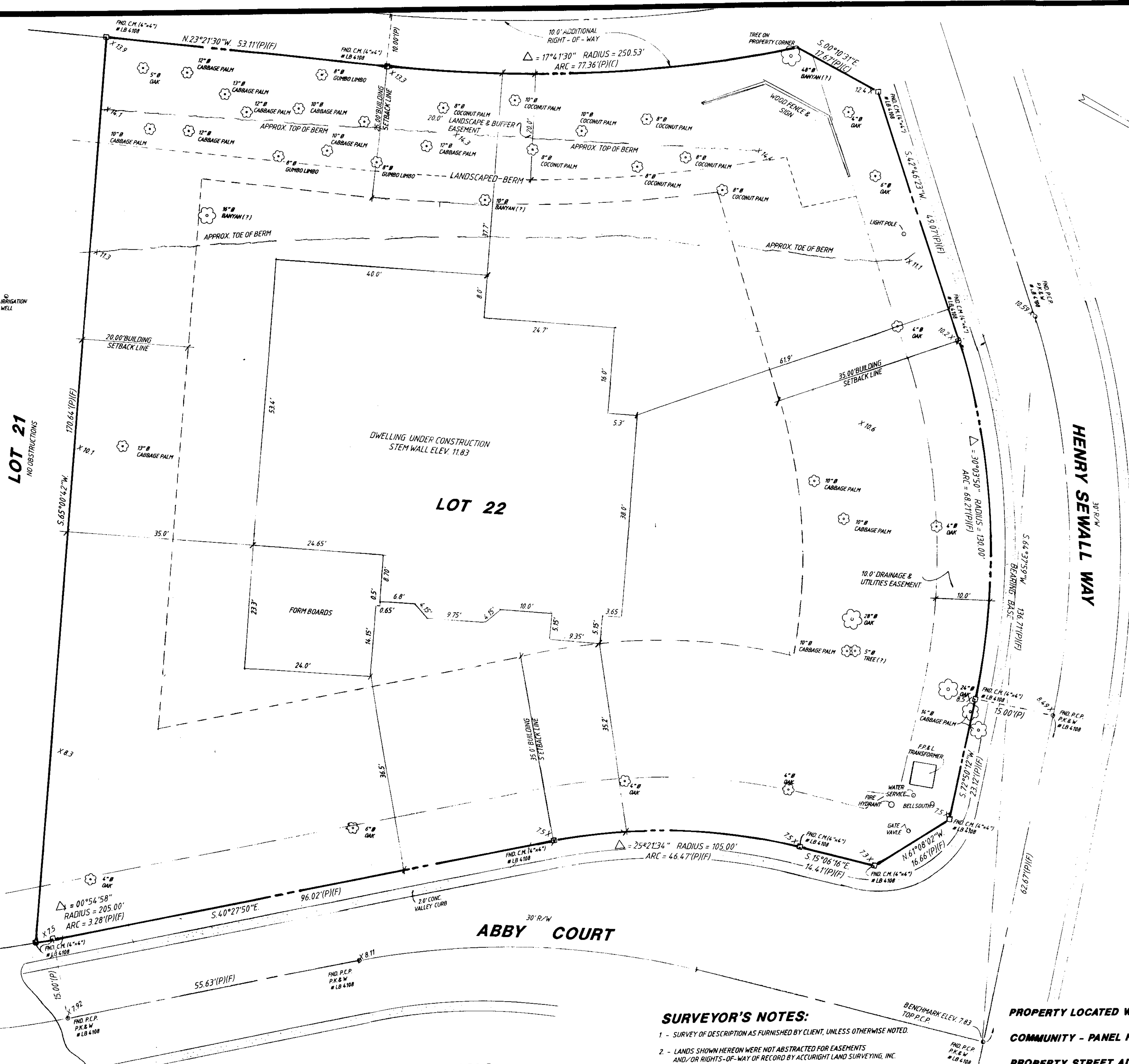
ACCURIGHT LAND SURVEYING, INC.

1501 DECKER AVENUE, SUITE 419-D
STUART, FL 34994

DRAWN W.G.H.
CHECKED E.R.S.
DATE (FIELD) 10/1/2000
SCALE 1" = 10.00'
JOB NO. 690-06-010
SHEET 1
OF ONE SHEETS



LOCATION MAP



HENRY SEWALL WAY

ABBY COURT

LEGAL DESCRIPTION

KNOWN AS LOT 22, "SEWALL'S MEADOW", ACCORDING TO THE PLAT THEREOF, AS RECORDED IN PLAT BOOK 14, PAGE 32, PUBLIC RECORDS OF MARTIN COUNTY, FLORIDA.

RECEIVED
FEB 26 2001

BY: J. STEWART (E.S.) W. GARAGE

FILE TOWN COPY
102 ABBIE CT.

PN 5228

THE GENERAL SLOPE OF THE PROPERTY, RECORDED EASEMENT(S) FROM THE RECORD PLAT, FILLED AREAS AND DRAINAGE FEATURES ARE AS SHOWN

SITE DATA
TOTAL LOT SIZE - 25,130.3 S.F. - 100%
IMPERVIOUS AREA BREAKDOWN
A/C, GARAGE, ENTRY & PATIO - 4,470 S.F. - 17.8%
POOL AREA, WALKS & PATIOS - 1,515 S.F. - 6%
DRIVEWAY - 2615 S.F. - 10.4%
TOTAL IMPERVIOUS AREA - 8,600 S.F. - 34.2%

SURVEYOR'S NOTES:

- 1 - SURVEY OF DESCRIPTION AS FURNISHED BY CLIENT, UNLESS OTHERWISE NOTED.
- 2 - LANDS SHOWN HEREON WERE NOT ABSTRACTED FOR EASEMENTS AND/OR RIGHTS-OF-WAY OF RECORD BY ACCURIGHT LAND SURVEYING, INC.
- 3 - ALL BEARINGS ARE REFERENCED TO THE INSTRUMENT OF RECORD AS SHOWN HEREON, UNLESS OTHERWISE NOTED.
- 4 - ELEVATIONS SHOWN HEREON ARE RELATIVE TO NATIONAL GEODETIC VERTICAL DATUM OF 1929, UNLESS OTHERWISE NOTED. SEE ABOVE FOR REFERENCE BENCH MARK IF APPLICABLE.
- 5 - THERE ARE NO ABOVE GROUND ENCROACHMENTS, UNLESS OTHERWISE NOTED.

PROPERTY LOCATED WITHIN FLOOD ZONE: "C"

COMMUNITY - PANEL NUMBER: 120164 0002 D

PROPERTY STREET ADDRESS: 102 ABBIE COURT
STUART, FL 34996

CERTIFIED TO: FOGLIA CONTRACTING CORP., A FLORIDA CORPORATION;
MCARTHY, SUMMERS, BOBKO, MCKEY, WOOD & SAWYER, P.A.;
ATTORNEY'S TITLE INSURANCE FUND, INC.;
FIDELITY FEDERAL SAVINGS BANK OF FLORIDA;
TOWN OF SEWALL'S POINT

SURVEYOR'S CERTIFICATE

WE HEREBY CERTIFY THAT THE BOUNDARY SURVEY AS SHOWN HEREON IS TRUE AND CORRECT TO THE BEST OF OUR KNOWLEDGE AND BELIEF AS SURVEYED UNDER OUR DIRECTION. WE FURTHER CERTIFY THAT IT MEETS THE MINIMUM TECHNICAL STANDARDS UNDER RULE 61G17 - 6 FLORIDA ADMINISTRATIVE CODE, PURSUANT TO CHAPTER 472.027 FLORIDA STATUTES.

NO VALID WITHOUT THE SIGNATURE AND THE ORIGINAL RAISED SEAL OF A FLORIDA LICENSED SURVEYOR AND MAPPER

ACCURIGHT LAND SURVEYING, INC.

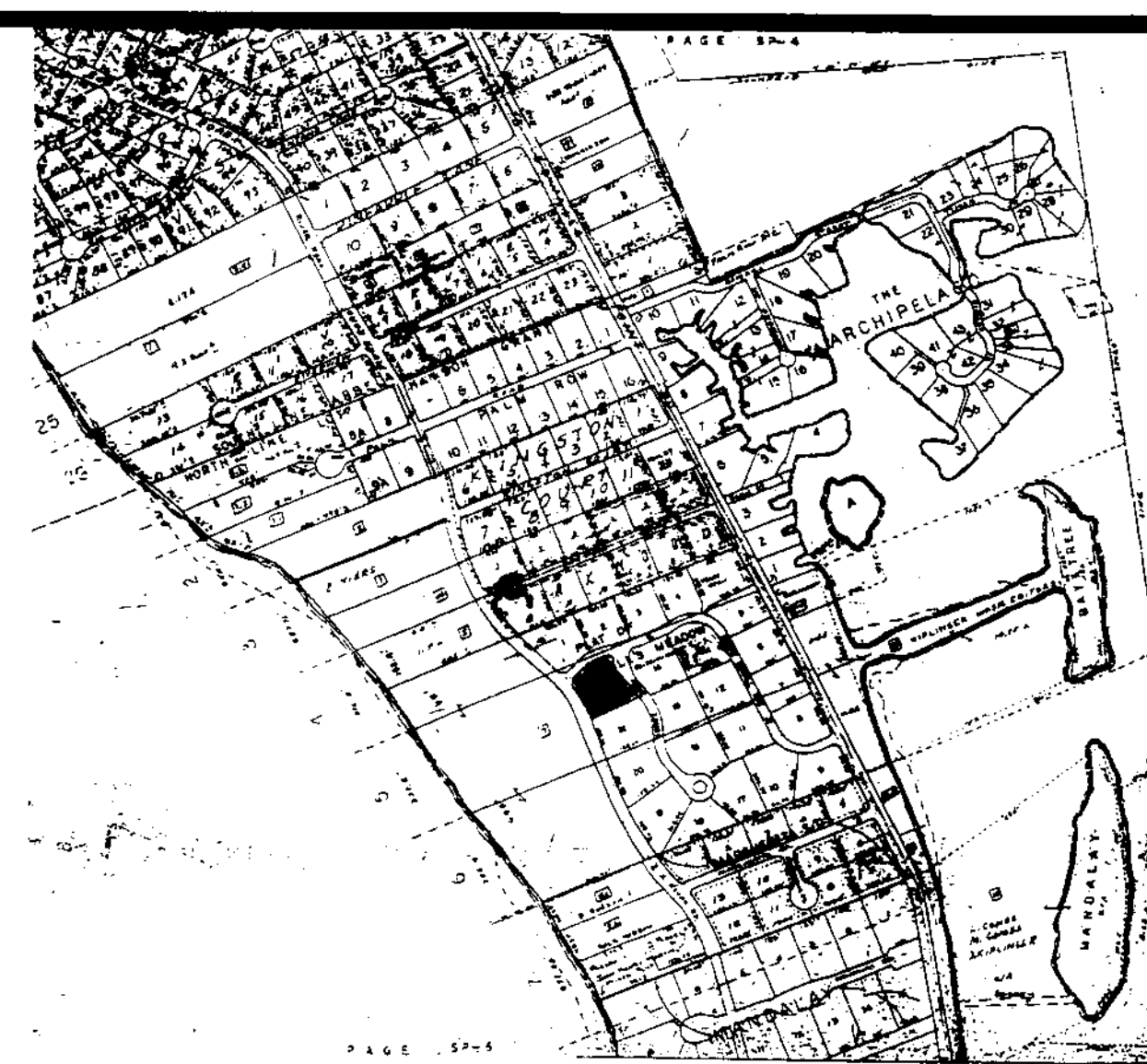
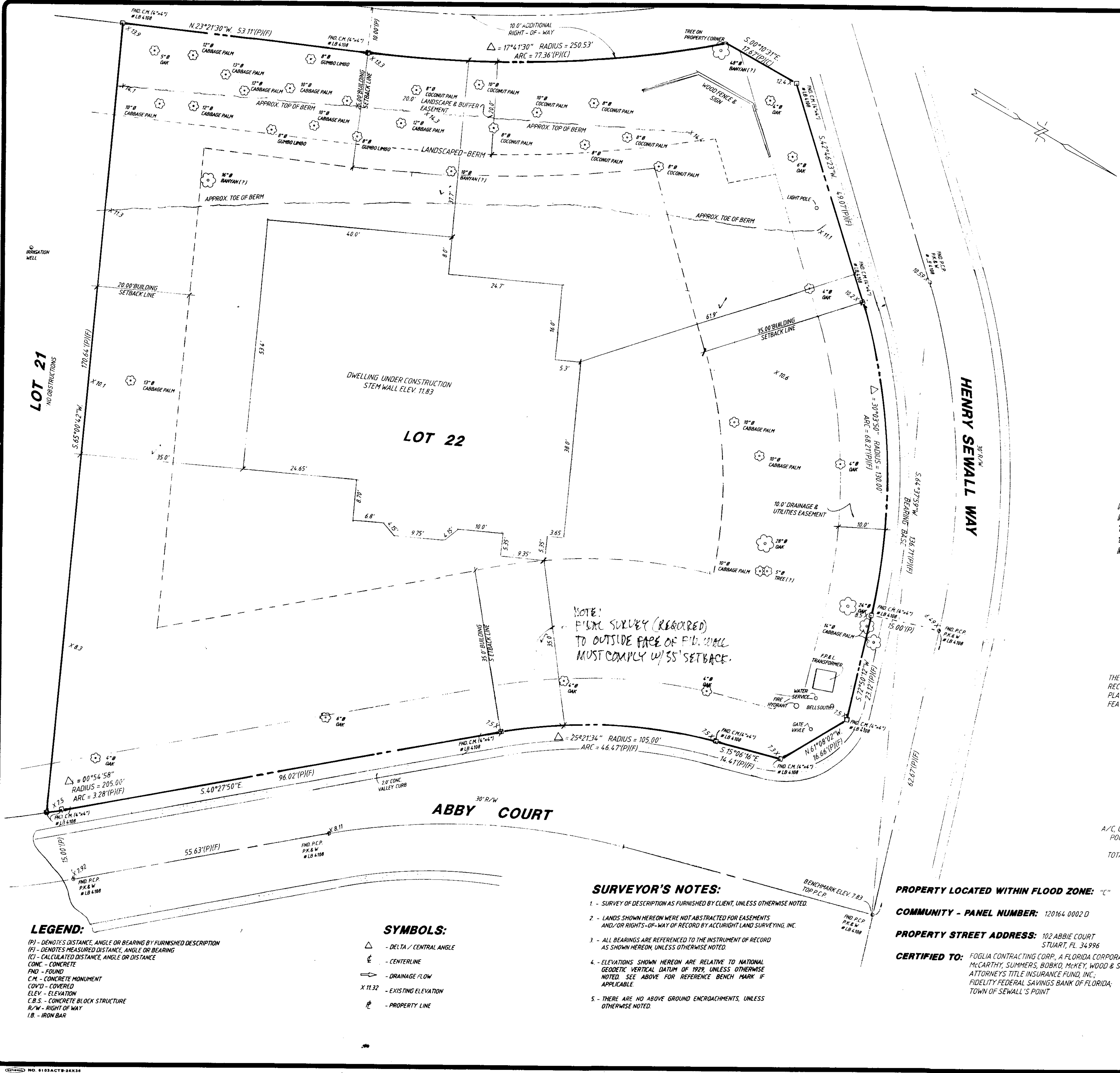
EARLE R. STARKEY - PROFESSIONAL SURVEYOR & MAPPER
REGISTRATION NO. 4459 - STATE OF FLORIDA

LEGEND:

- (P) - DENOTES DISTANCE, ANGLE OR BEARING BY FURNISHED DESCRIPTION
- (F) - DENOTES MEASURED DISTANCE, ANGLE OR BEARING
- (C) - CALCULATED DISTANCE, ANGLE OR DISTANCE
- CONC. - CONCRETE
- FND - FOUND
- CM - CONCRETE MONUMENT
- COVD - COVERED
- ELEV. - ELEVATION
- C.B.S. - CONCRETE BLOCK STRUCTURE
- R/W - RIGHT OF WAY
- I.B. - IRON BAR

SYMBOLS:

- Δ - DELTA / CENTRAL ANGLE
- ⊙ - CENTERLINE
- - DRAINAGE FLOW
- X 11.32 - EXISTING ELEVATION
- ℙ - PROPERTY LINE



LOCATION MAP

LEGAL DESCRIPTION

KNOWN AS LOT 22, "SEWALL'S MEADOW", ACCORDING TO THE PLAT THEREOF, AS RECORDED IN PLAT BOOK 14, PAGE 32, PUBLIC RECORDS OF MARTIN COUNTY, FLORIDA.

PREPARED FOR:
FOGLIA CONTRACTING CORPORATION

BOUNDARY SURVEY

ACCURIGHT LAND SURVEYING, INC.

OFFICE PHONE # (561) 286-7694
OFFICE FAX # (561) 220-7993
1501 DECKER AVENUE, SUITE 419-D
STUART, FL 34994

THE GENERAL SLOPE OF THE PROPERTY, RECORDED EASEMENTS FROM THE RECORD PLAT, FILLED AREAS AND DRAINAGE FEATURES ARE AS SHOWN

RECEIVED
FEB 12 2001
BY: [Signature]

STEM WALL (FORMER) SURVEY

FILE TOWN COPY
LOZ ABIE CT

PN 5228

SITE DATA
TOTAL LOT SIZE - 25,130.3 S.F. - 100%
IMPERVIOUS AREA BREAKDOWN
A/C, GARAGE, ENTRY & PATIO - 4,470 S.F. - 17.8%
POOL AREA, WALKS & PATS - 1,515 S.F. - 6%
DRIVEWAY - 2615 S.F. - 10.4%
TOTAL IMPERVIOUS AREA - 8,600 S.F. - 34.2%

SURVEYOR'S NOTES:

- SURVEY OF DESCRIPTION AS FURNISHED BY CLIENT, UNLESS OTHERWISE NOTED.
- LANDS SHOWN HEREON WERE NOT ABSTRACTED FOR EASEMENTS AND/OR RIGHTS-OF-WAY OF RECORD BY ACCURIGHT LAND SURVEYING, INC.
- ALL BEARINGS ARE REFERENCED TO THE INSTRUMENT OF RECORD AS SHOWN HEREON, UNLESS OTHERWISE NOTED.
- ELEVATIONS SHOWN HEREON ARE RELATIVE TO NATIONAL GEODETIC VERTICAL DATUM OF 1929, UNLESS OTHERWISE NOTED. SEE ABOVE FOR REFERENCE BENCH MARK IF APPLICABLE.
- THERE ARE NO ABOVE GROUND ENCROACHMENTS, UNLESS OTHERWISE NOTED.

PROPERTY LOCATED WITHIN FLOOD ZONE: "C"

COMMUNITY - PANEL NUMBER: 120164 0002 D

PROPERTY STREET ADDRESS: 102 ABIE COURT
STUART, FL 34996

CERTIFIED TO: FOGLIA CONTRACTING CORP., A FLORIDA CORPORATION;
MC CARTHY, SUMMERS, BOBKO, MCKEY, WOOD & SAWYER, P.A.;
ATTORNEYS TITLE INSURANCE FUND, INC.;
FIDELITY FEDERAL SAVINGS BANK OF FLORIDA;
TOWN OF SEWALL'S POINT

SURVEYOR'S CERTIFICATE

WE HEREBY CERTIFY THAT THE BOUNDARY SURVEY AS SHOWN HEREON IS TRUE AND CORRECT TO THE BEST OF OUR KNOWLEDGE AND BELIEF AS SURVEYED UNDER OUR DIRECTION. WE FURTHER CERTIFY THAT IT MEETS THE MINIMUM TECHNICAL STANDARDS UNDER RULE 61G17 - 6 FLORIDA ADMINISTRATIVE CODE, PURSUANT TO CHAPTER 472.027 FLORIDA STATUTES.

NOT VALID WITHOUT THE SIGNATURE AND THE ORIGINAL RAISED SEAL OF A FLORIDA LICENSED SURVEYOR AND MAPPER

ACCURIGHT LAND SURVEYING, INC.

EARLE R. STAFFKEY - PROFESSIONAL SURVEYOR & MAPPER
REGISTRATION NO. 4459 - STATE OF FLORIDA

LEGEND:

- (P) - DENOTES DISTANCE, ANGLE OR BEARING BY FURNISHED DESCRIPTION
(F) - DENOTES MEASURED DISTANCE, ANGLE OR BEARING
(C) - CALCULATED DISTANCE, ANGLE OR DISTANCE
CONC. - CONCRETE
FND - FOUND
C.M. - CONCRETE MONUMENT
COVD - COVERED
ELEV - ELEVATION
C.B.S. - CONCRETE BLOCK STRUCTURE
R/W - RIGHT OF WAY
I.B. - IRON BAR

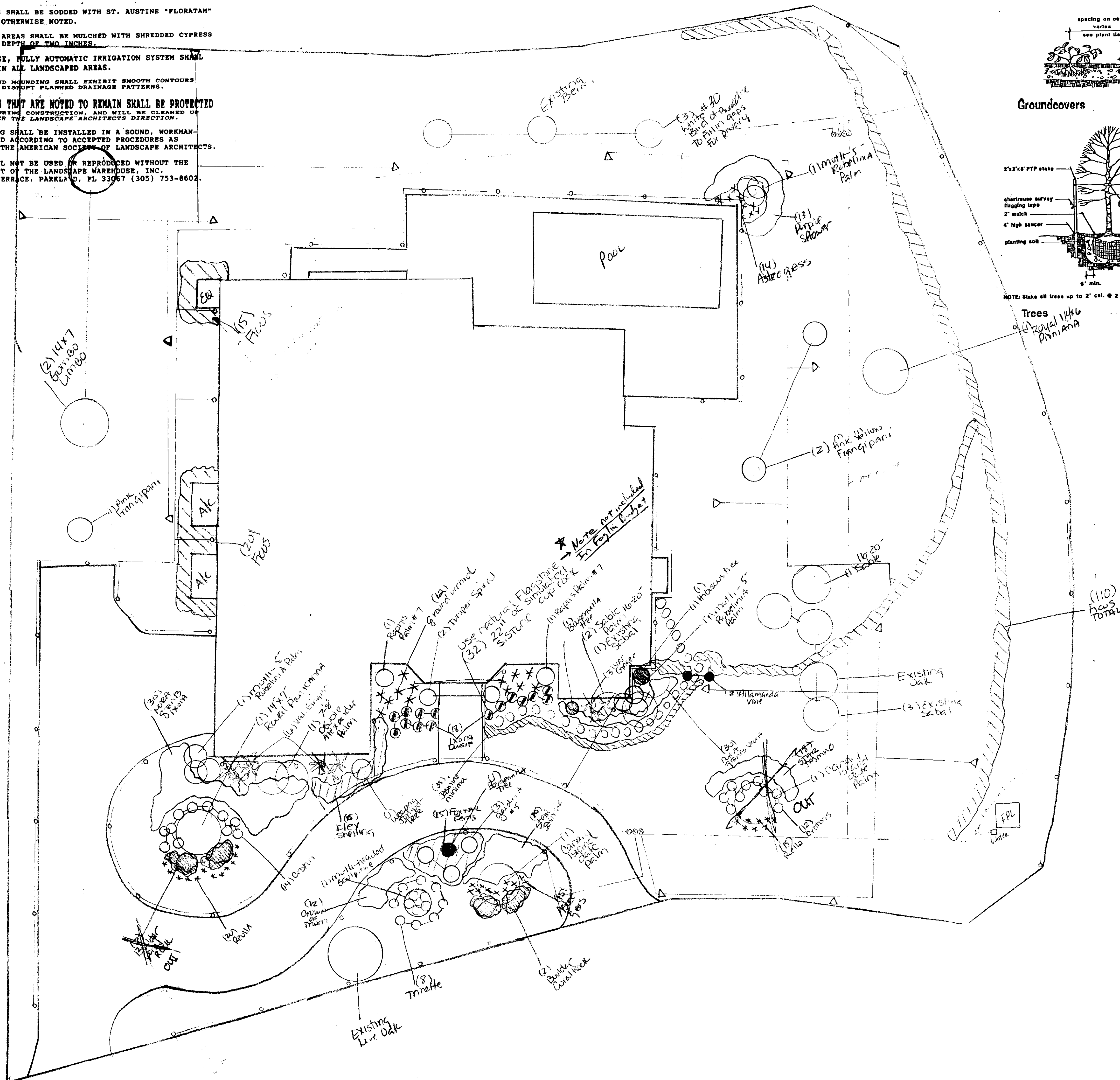
SYMBOLS:

- Δ - DELTA / CENTRAL ANGLE
C - CENTERLINE
DRAINAGE FLOW
X 11.32 - EXISTING ELEVATION
P - PROPERTY LINE

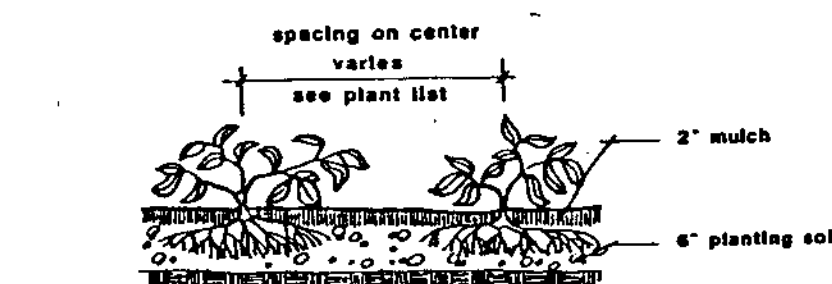
REVISIONS	BY
ADD SITE DATA	W.G.H.
TIE-IN	W.G.H.
02/10/2001	

DRAWN	W.G.H.
CHECKED	E.R.S.
DATE (FIELD)	10/11/2000
SCALE	1" = 10.00'
JOB NO.	690-06-010
SHEET	1
OF ONE SHEETS	

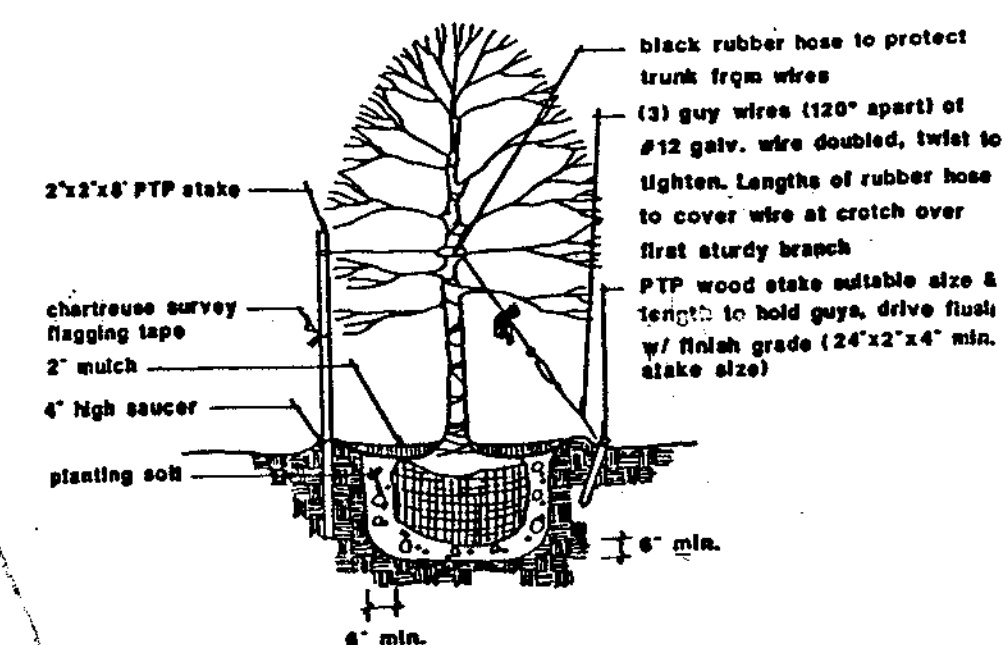
8. THIS PLAN SHALL NOT BE USED OR REPRODUCED WITHOUT THE WRITTEN CONSENT OF THE LANDSCAPE WAREHOUSE, INC.
7250 N.W. 82 TERRACE, PARKLAND, FL 33067 (305) 753-8602



Planting Details

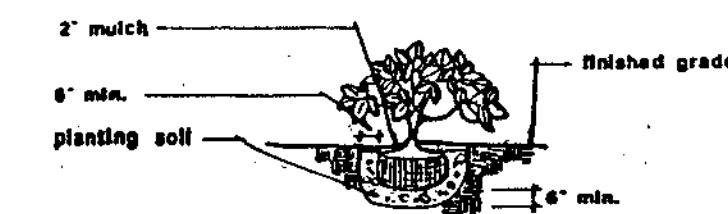


Groundcovers

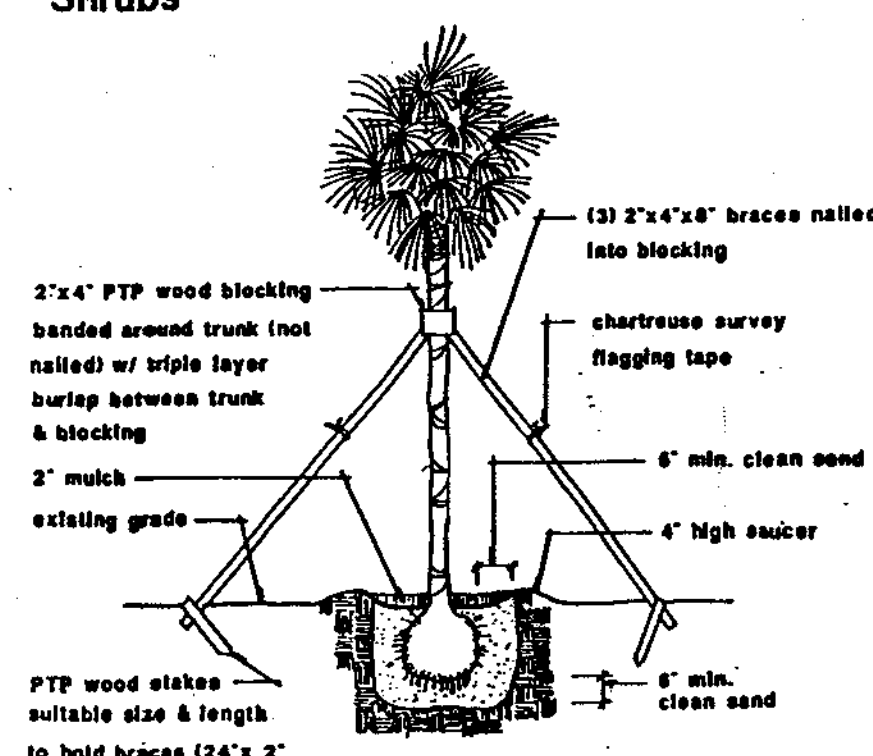


NOTE: Stake all trees up to 2" cal. @ 2 per tree. Guy all trees larger than 2" cal.

Trees



Shrubs



Straight Trunk Palms

PLANT LIST		
QTY	SIZE	DESCRIPTION
2	14x7	Gumbo Limbo
2	14x7	Royal Poinamia
3	10' 20'	Sable Palm
1	4' 5'	Hibiscus tree
1	11'	Weeping Juniper tree
2	5' 7'	Raphis Palm
1	12'	Double Alexander Palm
2	#10	Burg. villa tree
2	10'	Canary Island Date Palm
3	#30	White Bird of Paradise
1	#10	Mutli-headed Sculpture
3	5'	Mutli-headed Robelina Palm
2	#3	Antimanda Vine Yellow
15	#3	Ilex Shelling
9	#3	var Ginger
8	#3	Titnette
3	#5	Gordonia
3	#10	Frangipani Pink (2) Yellow (1)
13	#3	Purple Shaker
145	#3	Ficus
26	#3	Crotons
38	#3	Star Jasmine
60	#3	Nor A Crap B Ixora
15	#3	Fox tail Ferns
12	#3	Crown of Thorns
35	#1	Ruellia
29	#1	Aster grass
18	#3	Dwarf Ixora
2	#10	Juniper Sculpture Spiral
14	#3	Ground Orchid
35	#1	Asimina minima
37	22"	Stepping Stone
*	*	Coastal Rock Boulders
*	*	Florist #1 Sod
160	12x6's	Cypress Mulch approx

6/24/61 TALL OF SEWELL'S POINT
REVISION: 3232
BEG OFFICIAL

FILE TOWD COPY
102 ABILE COURT

PN 5521

[illegible]

102 ABBE CT.

Sewall's Meadow
Lot 22 Abbie Court
FOGLIA CUSTOM HOMES

The landscape warehouse
7250 NW 82 Terrace
Parkland, FL 33067
(954) 753-8602

DRAWN 7-17-01
CHECKED 7-18-01
DATE <i>7-18-01</i>
SCALE
JOB NO.
SHEET
OF
SHEETS

OFFICE PHONE # (561) 286 - 7694
OFFICE FAX # (561) 220 - 7993



BOUNDARY SURVEY

501 DECKER AVENUE, SUITE 419-D

DRAWN
WGH
CHECKED
ERS
DATE (FIELD)
10/11/2000
SCALE
1" = 10.00'
JOB NO
690-06-010'
SHEET
1
OF ONE SHEETS



-  - DELTA / CENTRAL ANG
-  - CENTERLINE
-  - DRAINAGE FLOW
-  - EXISTING ELEVATION
-  - PROPERTY LINE

- 1 - SURVEY OF DESCRIPTION AS FURNISHED BY CLIENT, UNLESS OTHERWISE NOTED.
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- 5 - THERE ARE NO ABOVE GROUND ENCROACHMENTS, UNLESS OTHERWISE NOTED

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FTG.	SIZE D x W	REINFORCING	ELEVATION	REMARKS
F1	12" x 16"	2 #5 BOT.	MONOLITHIC	
F2	12" MIN x 12"	1 #5 BOT.	MONOLITHIC	GARAGE STEP
F3	12" x 20"	2 #5 BOT.	-0'-8" BELOW GRADE MIN. UNO. & SYM.	STEM WALL
F4	10" x 12"	1 #5 BOT.	MONOLITHIC	SHOWER STEP
F5	16" x 20"	2 #5 BOT.	MONOLITHIC	PROVIDE 3/4" RECESS @ PERIMETER EDGE
F6	12" x 24" x 24"	3 #5 E.W. BOT.	-1'-0" BLW GRADE	CONCRETE PAD
F7	12" x 30" x 30"	3 #5 E.W. BOT.	-1'-0" BLW GRADE	CONCRETE PAD
F8	12" x 48" x 48"	4 #5 E.W. BOT.	-1'-0" BLW GRADE	CONCRETE PAD

COL.	SIZE	VERT. REINF. OR CAP PL. & BOLTS	COL. TIES OR BASE PL. & BOLTS	REMARKS
C1	8" x 8"	1 #5	---	MASONRY GROUT FILLED CELL
C2	8" x 16"	2 #5	---	MASONRY GROUT FILLED CELL
C3	8" x 8" x 8"	2 #5	---	MASONRY GROUT FILLED CELL - MBCT990
C4	8" x 16" x 5"	6 #5	#3 @ 8" C/C	CONCRETE
C5	TS 3x3x 1/4	1/4" STL TRUSS BUCKET SEE DETAIL 12/7	PL 5/8 x 8 x 8 W/ (4) 1/2" A.B.	STEEL COLUMN SEE DETAIL 12/7
C6	TS 4x4x 1/4	1/4" STL TRUSS BUCKET SEE DETAIL 13/7	PL 5/8 x 8 x 8 W/ (4) 1/2" A.B.	STEEL COLUMN SEE DETAIL 13/7
C7	(3) 2 x 4	(2) SEMCO RTPG836 W/ (16) 16d COMM NAILS	SEMCO MTS27B W/16 16d COMM NAILS	WOOD POST SEE DETAIL 10/7
C8	8" x 12"	2 #5	---	MASONRY GROUT FILLED CELL - MBCT994
C9	8" x 12"	2 #5	---	MASONRY GROUT FILLED CELL - AO

FOUNDATION NOTES:

FILL UNDER ALL SLABS SHALL BE CLEAN SAND AND SHALL BE COMPACTED TO A MINIMUM OF 95% MAXIMUM DENSITY AS PER ASTM D-1557. CONTRACTOR SHALL VERIFY AFTER COMPACTION.

FOOTINGS HAVE BEEN DESIGNED FOR A SOIL BEARING CAPACITY OF 2500 PSF. CONTRACTOR SHALL SUBMIT SOIL REPORT TO BUILDING DEPT. FOR SITE ALLOWABLE BEARING CAPACITY PRIOR TO INSTALLATION OF FOOTING.

FOUNDATION SLAB SHALL BE 4" THICK 2500 PSI CONCRETE WITH 8X8 10/10 W.W.F. (DBL. REIN. 30" FROM SUPPORT AT ALL PERIMETER EDGES) ON VAPOR BARRIER.

ALL FOOTING THICKNESSES ARE MINIMUM. DEPTHS MAY BE INCREASED 12" AS REQUIRED PER SITE CONDITIONS WITHOUT ADDITIONAL REINFORCEMENT.

PROVIDE MINIMUM 3/4" BLOCK RECESS @ ALL PERIMETER EDGES RECEIVING BLOCK. DOORS SHALL BE RECESSED 1-1/2" UNO. FOOTING DEPTHS SHALL BE MEASURED FROM RECESSED EDGE.

CONCRETE PADS LOCATED AT PERIMETER OF BUILDING SHALL BE CONSIDERED EXTERIOR AND BE SET MIN. 4'-8" BELOW THE EXTERIOR GRADE UNLESS NOTED OTHERWISE.

PROVIDE ELECTRICAL CONDUIT IN SLABS AS REQUIRED PER ELECTRICAL LAYOUT.

PROVIDE CORNER BARS FOR CONTINUITY OF REINFORCEMENT AROUND CORNERS AND CHANGES IN DIRECTION. LONGITUDINAL STEEL SHALL BE BENT AROUND CORNERS 48 BAR DIAMETERS OR BY ADDING MATCHING REINFORCEMENT STEEL EXTENDING 48 BAR DIAMETERS FROM EACH CORNER OR CHANGE IN DIRECTION.

ACCESSORY USE SLABS SUCH AS A/C COMPRESSOR SLABS SHALL BE NOT LESS THAN FOUR (4) INCHES THICK. THE USE OF A MONOLITHIC FOOTING AROUND PERIMETER EDGE OF THE SLAB IS NOT REQUIRED. SUCH SLABS SHALL BE PLACED ON CLEAN, THOROUGHLY COMPACTED, SAND OR CRUSHED ROCK FREE FROM ORGANICS, DEBRIS OR OTHER DELETERIOUS MATERIALS.

THIS PLAN HAS BEEN RATIONALLY ANALYZED TO OMIT TRANSFER BARS IN THE MONOLITHIC FOOTING EXCEPT WHERE INDICATED ON PLAN.

PLUMBING NOTES:

ALL CLEANOUTS SHALL COME UP TO GRADE.

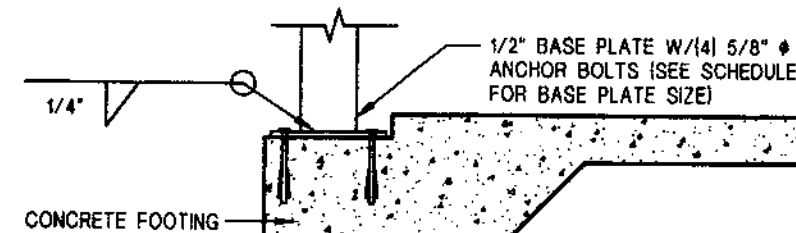
NO VENT STACKS SHALL PASS THROUGH ROOF CRICKETS OR VALLEYS.

FIXTURE SUPPLY SIZES SHALL CONFORM TO SECTION 484.13 OF S.F.B.C.

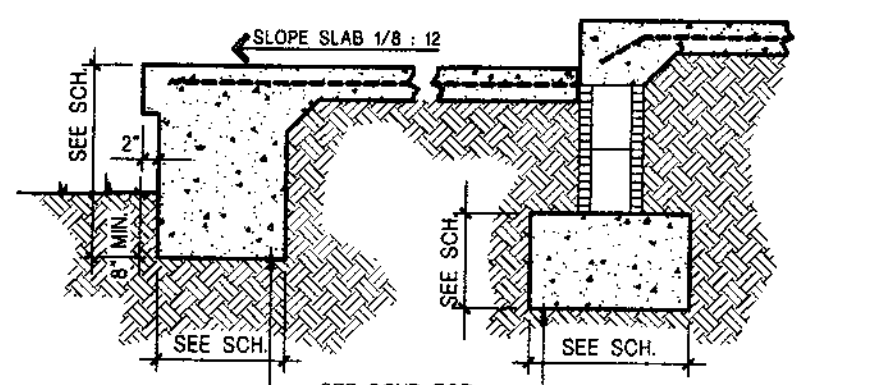
ALL WATER CLOSETS, LAVATORY, SHOWERHEADS, URINALS AND SINKS SHALL COMPLY WITH TABLE 46-22 OF S.F.B.C. 1999 EDITION.

ALL SHOWERS AND BATH AND SHOWER COMBINATIONS SHALL BE PROTECTED WITH A CONTROL VALVE OF THE PRESSURE BALANCE, THE THERMOSTATIC MIXING OR THE COMBINATION PRESSURE BALANCE/ THERMOSTATIC TYPE. HANDLE POSITION STOPS SHALL BE ADJUSTED IN ACCORDANCE WITH MANUFACTURER'S INSTRUCTIONS AT TIME OF INSTALLATION TO A MAXIMUM MIXED WATER OUTLET TEMPERATURE OF 120 DEGREES FAHRENHEIT. ALL SHOWER OR BATH-SHOWER COMBINATION VALVES SHALL COMPLY W/ S.F.B.C. 484.17 (b) (2) 1999 EDITION.

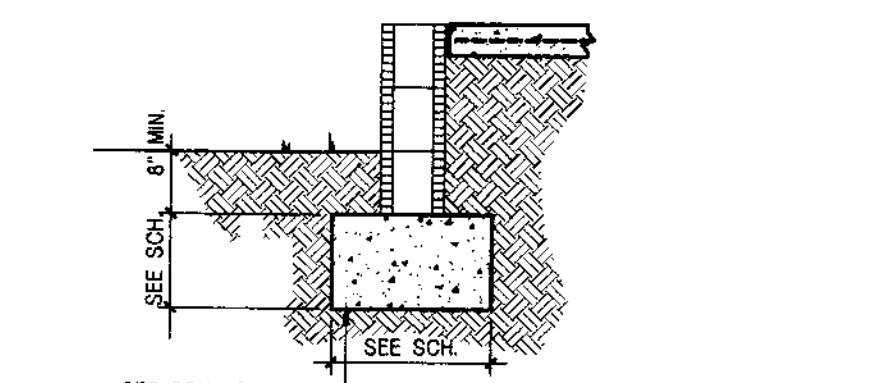
PLUMBING STACK PERMITTED TO PASS THRU FOOTING.



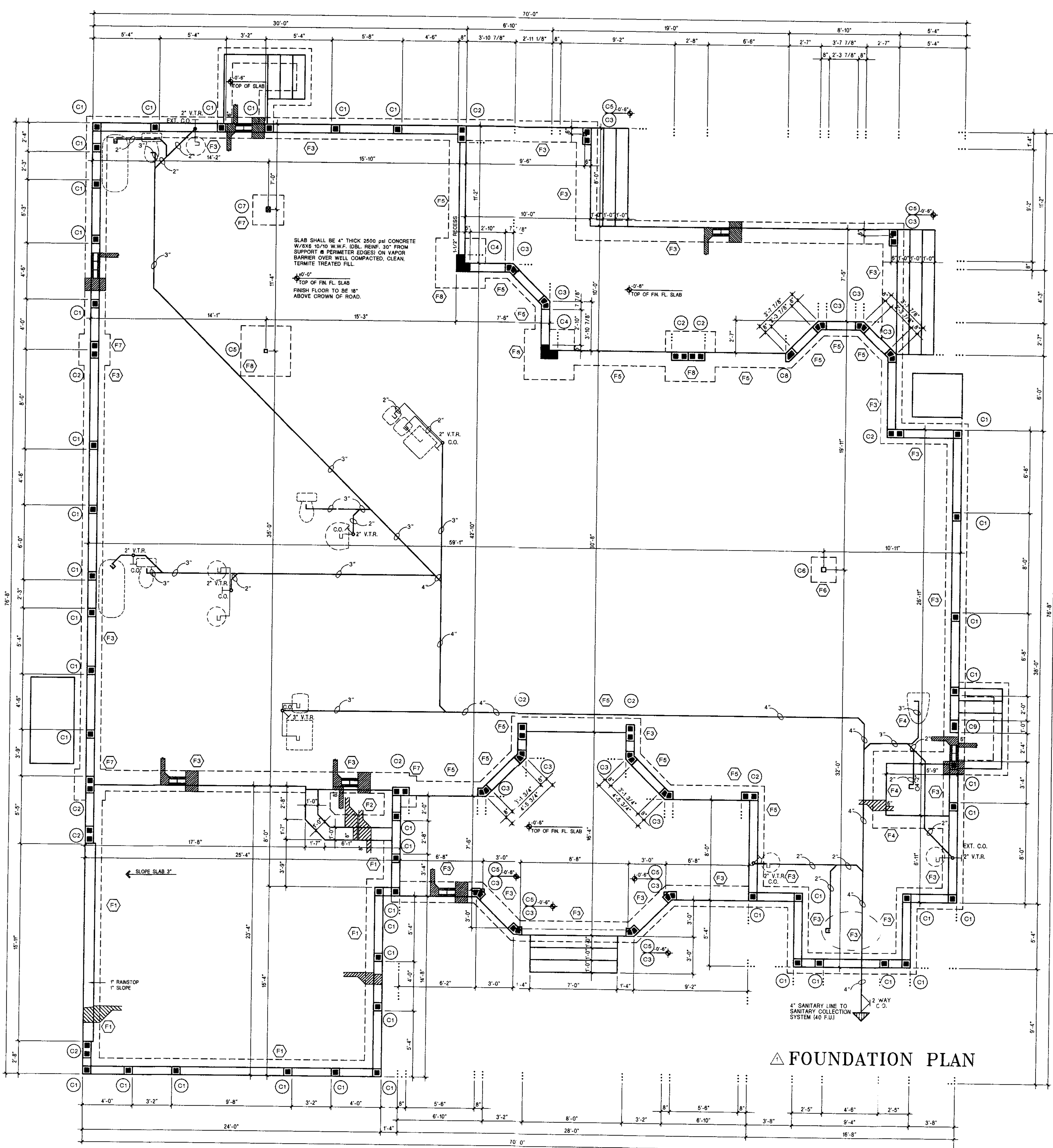
DETAIL 1 COLUMN BASE
SCALE: NONE



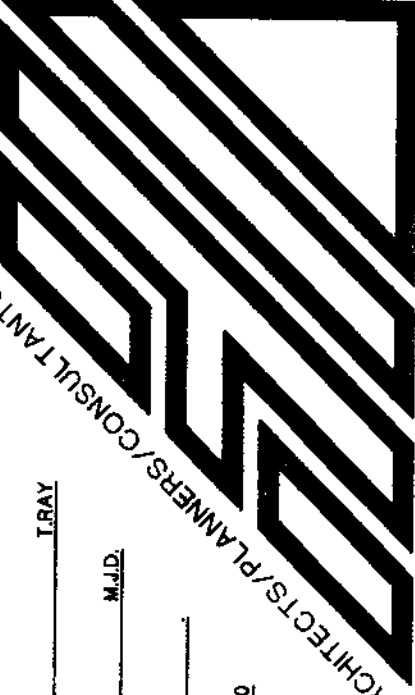
DETAIL 2 STEM WALL @ ENTRY
SCALE: NONE



DETAIL 3 STEM WALL
SCALE: NONE



FOUNDATION PLAN



A NEW CUSTOM RESIDENCE
DR. & MRS. ROBERT CUSHING

THE CAPTIVA

LOT 22

SEAWALL'S MEADOWS
MARTIN COUNTY, FLORIDA

HOMES BY ARCHITECTNIC INC.

7430 WILES ROAD
CORAL SPRINGS, FLORIDA 33067

FOR: BY:

FOGLIA CUSTOM HOMES

SEAWALL'S MEADOWS
MARTIN COUNTY, FLORIDA

HOMES BY ARCHITECTNIC INC.

7430 WILES ROAD
CORAL SPRINGS, FLORIDA 33067

PHONE: (954) 755-0690
FAX: (954) 755-8801

Revisions:

2-04-00 CLIENT REVISION M.J.D.

1-04-00

1-04-00

1-04-00

1-04-00

All ideas designs arrangements and plans are the property of the architect and shall be used only for the project and site for which they were prepared. No part of this drawing or any part thereof shall be reproduced, stored in a retrieval system, or transmitted in any form or by any means, electronic, mechanical, photocopying, recording, or by any information storage and retrieval system, without the prior written permission of the architect. The architect shall not be responsible for any errors or omissions in this drawing or any part thereof, nor shall the architect be liable for any damages or losses of any kind, including but not limited to, consequential damages, arising out of the use of this drawing or any part thereof. The architect's liability shall be limited to the professional services rendered by the architect and shall not extend to any other services or to any other damages or losses of any kind, including but not limited to, consequential damages, arising out of the use of this drawing or any part thereof. The architect's liability shall be limited to the professional services rendered by the architect and shall not extend to any other services or to any other damages or losses of any kind, including but not limited to, consequential damages, arising out of the use of this drawing or any part thereof.

GENERAL NOTES:

ALL WORK SHALL CONFORM TO THE REQUIREMENTS OF THE SOUTH FLORIDA BUILDING CODE.
ALL DIMENSIONS AND CONDITIONS SHALL BE VERIFIED IN THE FIELD BY THE CONTRACTOR BEFORE PROCEEDING WITH WORK. FIGURE DIMENSIONS TAKE PRECEDENCE OVER SCALE DIMENSIONS AT ALL TIMES.

CONCRETE
ALL CONCRETE TO HAVE A COMPRESSIVE STRENGTH OF 3000 P.S.I. AT 28 DAYS UNLESS NOTED OTHERWISE. CONCRETE PER ASTM C96. CONCRETE WORK PER ACI 301.
ALL WELDED WIRE FABRIC, W.W.F., SHALL CONFORM TO ASTM A-185 AND BE GRADE 60, F_y = 60 KSI.
ALL REINFORCED STEEL TO BE ASTM A-615 GRADE 60, BENT, LAPPED AND PLACED IN ACCORDANCE WITH THESE DRAWINGS AND THE LATEST SPECIFICATIONS IN ACI 318.

COLUMNS
ALL STEEL COLUMNS SHALL BE 1 HOUR FIRE RATED WITH 2 LAYERS OF 1/2" GYPSUM WALLBOARD APPLIED PER S.F.B.C. TABLE 37A #24.
GLASS BLOCK
ALL GLASS BLOCK SHALL BE 3-7/8" MIN. & BE PROVIDED WITH # 6 GA. X 1" WIDE PANEL ANCHORS @ PERIMETER. ANCHORS SHALL BE LAPPED WITH ALL REINFORCING 6" MIN. CONNECT TO CONCRETE COLUMNS OR BEAMS PER ENGINEER'S DETAIL FOR REINFORCING SCHEDULE.
MORTAR TO HAVE COMPRESSIVE STRENGTH TYPE S - 1800 psi AT 20 DAYS EXPANSION STRIPS TO BE PLACED PER MFG. SPECS.
ALL GLASS BLOCK SHALL BE INSTALLED PER S.F.B.C. SECTION 2704.10 REFER TO ATTACHED ENGINEER'S DETAIL FOR REINFORCING SCHEDULE.

ENGINEERED UNIT MASONRY NOTES:

CONCRETE LIMITS
1. TO BE ASTM C 90-90. ASTM GRADES N OR NI

2. PROVIDE PRECAST LINTELS AS NECESSARY

MORTAR TYPE
M OR S PER ASTM C 270-95. (CEMENT/LIME)

GROUT
1. GROUT SHALL CONFORM TO ASTM DESIGNATION C475-83 (2500 psi)

2. THE MIX DESIGN SHALL BE APPROVED BY THE ENGINEER.

3. ALL GROUT SHALL BE PLACED CONSISTENTLY, WHICH MEANS THAT CONSISTENCY BE AS FLUID AS POSSIBLE FOR POURING WITHOUT SEGREGATION OF THE CONSTITUENT PARTS. (9" ± 1" SLUMP)

4. THE USE OF ADMIXTURES SHALL NOT BE PERMITTED WITHOUT WRITTEN CONSENT OF THE ENGINEER.

REINFORCING VERTICAL
a. ASTM A615-90 PER REINFORCING SECTION (GRADE 60).

b. WHEN A FOUNDATION DOWEL DOES NOT LINE UP WITH A VERTICAL CORE, IT SHALL NOT BE SLOPED MORE THAN ONE HORIZONTAL IN SIX VERTICAL. ALIGNMENT, EVEN THOUGH IT IS IN AN ADJACENT CELL TO THE VERTICAL WALL REINFORCING.

c. VERTICAL REINFORCEMENT STEEL SHALL HAVE A MINIMUM CLEARANCE OF ONE-HALF INCH FROM THE MASONRY.

d. VERTICAL REINFORCEMENT SHALL BE AS SHOWN ON PLANS.

e. VERTICAL REINFORCEMENT EACH SIDE OF ANY OPENING SHALL BE CONTINUOUS TO THE BEAM. PRECAST LINTELS SHALL HAVE OPENINGS TO ALLOW REINFORCING BARS TO CONTINUE UNINTERRUPTED.

f. ALL BAR SPLICES SHALL BE 48" BAR DIA.

HORIZONTAL
a. HORIZONTAL REINFORCEMENT SHALL BE PROVIDED AS SHOWN ON WALL SECTION IF ANY IS REQUIRED.

GENERAL
1. ALL REINFORCING MASONRY WORK DESIGN SHALL CONFORM TO ACI-830-82/ASCE 5-92/TMS 402-92 AND ALL CONSTRUCTION SHALL BE IN ACCORDANCE WITH ACI 530.1-92/ASCE 5-92/TMS 602-92 (F_m = 1500 psi).

2. REINFORCED MASONRY UNITS HAVE BEEN DESIGNED BY AN ENGINEER. REFER TO ATTACHED STRUCTURAL CALCULATIONS W/SIGNED & SEALED ENGINEERING LETTER.

3. STRUCTURE HAS BEEN DESIGNED IN ACCORDANCE WITH ASCE 7-93.

INSPECTION
ALL ENGINEERED UNIT MASONRY WORK SHALL REQUIRE INSPECTION BY SPECIAL INSPECTOR.

BUILDING IDENTIFICATION
S.F.B.C. 512.02(1) REQUIRES APPROVED NUMBERS OR ADDRESSES BE PLACED ON ALL NEW AND EXISTING BUILDINGS IN SUCH POSITION AS TO BE PLAINLY VISIBLE AND LEGIBLE FROM THE STREET OR ROAD FRONTING THE PROPERTY.

SECTION 314.3 OF THE BROWARD COUNTY CODE FURTHER REQUIRES, "FOR EACH BUILDING, OR PORTION OF BUILDING, HAVING AN ADDRESS NUMBER, THE OWNER OR PERSON IN CHARGE OF THE BUILDING, OR OF EACH PORTION, SHALL CAUSE SUCH ADDRESS NUMBER TO BE DISPLAYED AT ALL TIMES, IN NUMERALS ON OR ADJACENT TO THE BUILDING, OR PORTION, SO THAT THE NUMERALS ARE READABLE FROM THE STREET OR THOROUGHFARE TO WHICH THE NUMBER APPLIES. NUMERALS SHALL BE AT LEAST THREE INCHES IN HEIGHT FOR RESIDENTIAL BUILDINGS."

ANCHORAGE
METAL AND WIRE LATH:
FASTENING SHALL BE PER S.F.B.C. 3502 AND ANSI A423 WITH MIN. 1-3/8" PENETRATION FOR HORIZONTAL SUPPORTS;
3/4" PENETRATION ON VERTICAL SUPPORTS.

GARAGE
GARAGE WALLS AND CEILINGS SHALL BE 1 HR. FIRE RATED. CEILING (S.F.B.C. TABLE 37-C #42) SHALL BE (1) LAYER 5/8" TYPE "X" GYPSUM WALLBOARD, APPLIED AT RIGHT ANGLES TO STEEL FURRING CHANNELS W/ 1-1/4" LONG NO. 6 DRYWALL SCREWS 6" C/C AND CHANNELS 16" C/C. COMPOUND SHALL COVER JOINTS. APPROVED INSULATION OF FIBERGLASS OR MINERAL WOOL BATTIS, OR BLOWN IN FIBERGLASS MINERAL WOOL OR CELLULOSE INSULATION HAVING A MINIMUM INSULATING VALUE OF R-11. WALLS (S.F.B.C. TABLE 37-B #72) SHALL BE NO. 25 GAUGE CHANNEL-SHAPED STUDS 16" C/C WITH (1) FULL LENGTH LAYER OF 5/8" TYPE "X" GYPSUM WALLBOARD APPLIED VERTICALLY ATTACHED WITH 1-1/4" LONG NO. 6 DRYWALL SCREWS TO EACH SIDE. SCREWS ARE 8" O.C. AROUND THE PERIMETER AND 12" C/C ON THE INTERMEDIATE STUD.

DOORS & WINDOWS
PER S.F.B.C. SECTION 312.2 - ALL BEDROOMS REQUIRE AN OUTSIDE WINDOW OR DOOR OPERABLE FROM THE INSIDE WITHOUT THE USE OF TOOLS AND PROVIDING A CLEAR OPENING OF NOT LESS THAN 20 INCHES WIDTH, 24 INCHES IN HEIGHT AND 5.7 SQ.FT. IN AREA. THE BOTTOM OF THE OPENING SHALL NOT BE MORE THAN 44" OFF THE FLOOR AND NO PART OF THE OPERATING MECHANISM SHALL BE PLACED HIGHER THAN 54" ABOVE FINISHED FLOOR.

OPERATING MECHANISMS ON SINGLE HUNG WINDOWS SHALL BE AT OR NEAR THE SILL.
WINDOWS INDICATED WITH (E) MUST BE MANUFACTURED TO CONFORM TO THE BUILDING CODE WITH RESPECT TO MINIMUM EMERGENCY EGRESS REQUIREMENTS PER A.F.P.A. 101 CHAPTER 22.

ALL SLIDING GLASS DOORS AND FRENCH DOORS SHALL BE TEMPERED PER S.F.B.C. SECTION 3508.

ALL FIXED GLASS SHALL MEET THE REQUIREMENTS OF THE S.F.B.C. SECTION 3508.2. ALL GLASS SHALL BE SINGLE TINT.

TOLERANCE
FRAME TO BUCK TOLERANCE SHALL BE PER WINDOW MFG.

BUCK TO OPENING TOLERANCE SHALL BE MAX. 1/4" TOTAL OPENING.

ALL GARAGE DOOR MIN. 2 X 6 P.T. WOOD BUCKS (HEADER AND JAMB) SHALL BE ATTACHED W/ 1/2" X ANCHOR BOLT W/ MIN. 4" EMBEDMENT AND DRIVEN NOT LESS THAN 3" FROM EDGE OF CONCRETE. SPACING SHALL BE 12" C/C STAGGERED (8" WIDTH; 9" C/C FOR 16" WIDTH).

ALTERNATE 1: 1/4" X TAP-ON # 9" C/C STAGGERED (MIN. 1-3/4" EMBEDMENT) OR RAWL-STUD EXPANSION ANCHOR (1400 SERIES) 3/8" X 1-3/4" MIN. EMBEDMENT, MIN. 5" PER VERTICAL JAMB, FASTENERS LOCATED 18" C/C MAX.

ALTERNATE 2: "HIT" R.F. XOM FASTENERS (145 #, 3" LONG, 1-1/2" EMBEDMENT) @ 8" C/C NOTE: ALL FASTENERS SHALL BE SUPPLIED W/ NUTS & WASHERS.

SHUTTERS
REFER TO MFG'S SPECIFICATIONS & SHOP DWGS FOR INSTALLATION AND CONNECTION DETAILS. ALL SHUTTERS SHALL BE REVIEWED & APPROVED BY THE ARCHITECT OF RECORD. SHUTTERS SHALL MEET AND COMPLY WITH S.F.B.C. AND SATISFY THE REQUIREMENTS OF DADE COUNTY PRODUCT APPROVAL.

SHUTTERS SHALL BE PROVIDED AT ALL WINDOWS UNLESS THE WINDOW HAS AN APPROVED PRODUCT APPROVAL NUMBER & COMPLIES W/ ALL REQUIRED TESTING.

FINISHES
MIRRORS IN EXCESS OF 6 S.F. SHALL BE DIRECTLY SECURED TO SUPPORTS AND SHALL NOT BE HUNG PER S.F.B.C. 3508.4 (g).

BATHROOM FLOORS AND BASE SHALL BE IMPERVIOUS MATERIALS PER S.F.B.C. SECTION 2908. BEARING & EXTERIOR FRAME WALLS HAVE BEEN DESIGNED BY RATIONAL ANALYSIS TO REDUCE THE MINIMUM THICKNESS/HEIGHT ALLOWABLE PER S.F.B.C. 2905.1.

WOOD COLUMNS SHALL BE 1 HOUR FIRE RATED REFER TO DETAIL FOR FIRE RATING SPECIFICATIONS COMPLYING WITH THE S.F.B.C.

WOOD
ALL STRUCTURAL LUMBER TO BE DOUGLAS FIR-LARCH NO2 OR BETTER F_b = 1200 P.S.I.

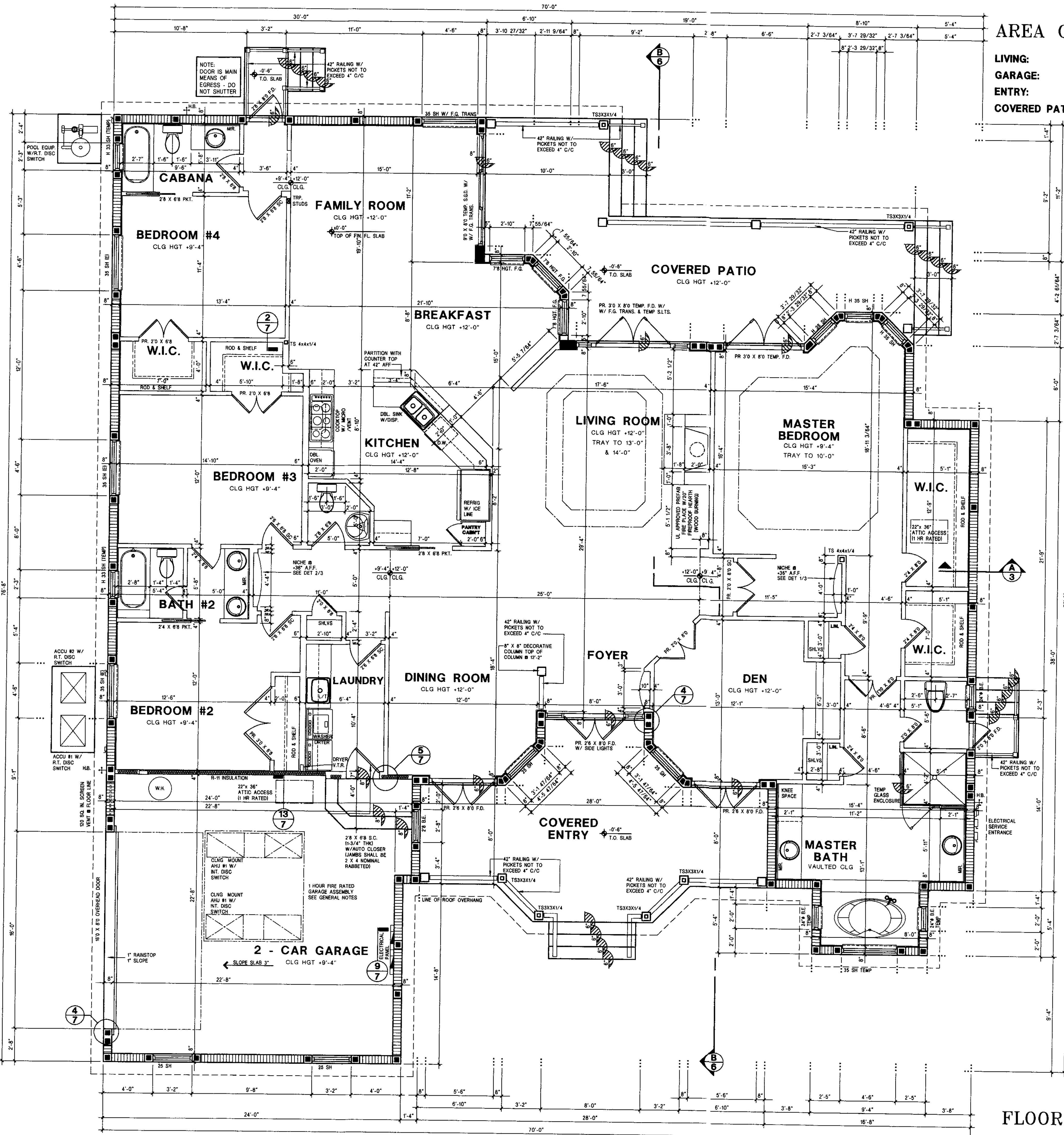
ALL LUMBER IN CONTACT WITH MASONRY OR CONCRETE SHALL BE PRESSURE TREATED.

ANCHORAGE OF ALL WOOD FRAMING SHALL BE CONTINUOUS FROM THE FOUNDATION TO THE ROOF AND SHALL MEET THE REQUIREMENTS OF S.F.B.C. SECTION 2908.

BEARING & EXTERIOR FRAME WALLS HAVE BEEN DESIGNED BY RATIONAL ANALYSIS TO REDUCE THE MINIMUM THICKNESS/HEIGHT ALLOWABLE PER S.F.B.C. 2905.1.

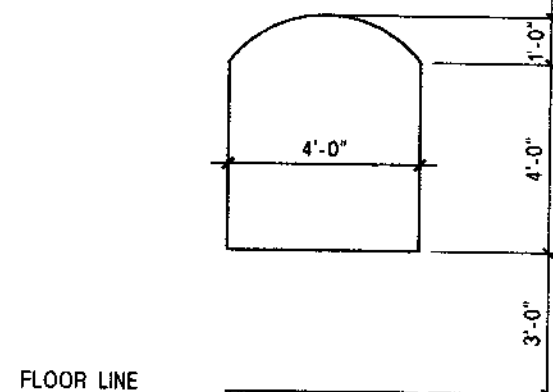
WOOD COLUMNS SHALL BE 1 HOUR FIRE RATED REFER TO DETAIL FOR FIRE RATING SPECIFICATIONS COMPLYING WITH THE S.F.B.C.

December 04, 2000 - 14:45
0264-311.dwg - M.J.D.

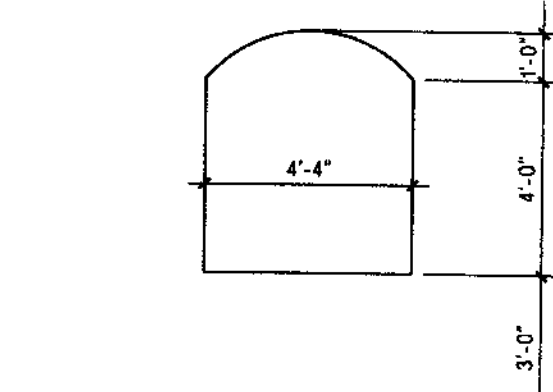


AREA CALCULATIONS

LIVING: 3,202 SQ. FT.
GARAGE: 570 SQ. FT.
ENTRY: 306 SQ. FT.
COVERED PATIO: 392 SQ. FT.



DETAIL NICHES DETAIL 1
SCALE: 1/4" = 1'-0"



DETAIL NICHES DETAIL 2
SCALE: 1/4" = 1'-0"

FLOOR PLAN

LOT 22

THE CAPTIVA

FOR: BY:

DR. & MRS. ROBERT CUSHING

FOGLIA CUSTOM HOMES

SEAWALL'S MEADOWS

MARTIN COUNTY, FLORIDA

HOMES BY ARCHITECTNIC INC.

7430 WILES ROAD

CORAL SPRINGS, FLORIDA 33067

PHONE: (954) 755-0690

FAX: (954) 755-8801

DESIGNED BY: TRAY

DRAWN BY: M.J.D.

CHECKED BY:

DATE: OCTOBER 17, 2000

SCALE: 1/4" = 1'-0"

PROJECT #: 0264

SHEETS 8

3

GENERAL NOTES:

ALL WORK SHALL CONFORM TO THE REQUIREMENTS OF THE SOUTH FLORIDA BUILDING CODE.

ALL DIMENSIONS AND CONDITIONS SHALL BE VERIFIED IN THE FIELD BY THE CONTRACTOR BEFORE PROCEEDING WITH WORK. FIGURE DIMENSIONS TAKE PRECEDENCE OVER SCALE DIMENSIONS AT ALL TIMES.

CONCRETE
ALL CONCRETE TO HAVE A COMPRESSIVE STRENGTH OF 3000 P.S.I. AT 28 DAYS UNLESS NOTED OTHERWISE. CONCRETE PER ASTM C99. CONCRETE WORK PER ACI 301.

ALL WELDED WIRE FABRIC, WWF, SHALL CONFORM TO ASTM A-185 AND BE GRADE 60, (fy = 60 KSI).

ALL REINFORCED STEEL TO BE ASTM A-615 GRADE 60, BENT, LAPPED AND PLACED IN ACCORDANCE WITH THESE DRAWINGS AND THE LATEST SPECIFICATIONS IN ACI 318.

COLUMNS
ALL STEEL COLUMNS SHALL BE 1 HOUR FIRE RATED WITH 2 LAYERS OF 1/2" GYPSUM WALLBOARD APPLIED PER S.F.B.C. TABLE 37A #24.

GLASS BLOCK
ALL GLASS BLOCK SHALL BE 3-7/8" MIN. & BE PROVIDED WITH 1/8" GA. X 1" WIDE PANEL ANCHORS PER PERIMETER. ANCHORS SHALL BE LAPPED WITH ALL REINFORCING 6" MIN. CONNECT TO CONCRETE COLUMNS OR BEAMS PER ENGINEER'S DETAIL FOR REINFORCING SCHEDULE.

MORTAR TO HAVE COMPRESSIVE STRENGTH TYPE S - 1800 psi AT 20 DAYS EXPANSION STRIPS TO BE PLACED PER MFR. SPECS.

ALL GLASS BLOCK SHALL BE INSTALLED PER S.F.B.C. SECTION 2704.10 REFER TO ATTACHED ENGINEER'S DETAIL FOR REINFORCING SCHEDULE.

ENGINEERED UNIT MASONRY NOTES:
CONCRETE UNITS
1. TO BE ASTM C 90-90. ASTM GRADES IN OR IN
2. PROVIDE PRECAST LINTELS AS NECESSARY

MORTAR TYPE
1. M OR S PER ASTM C 270-95. (CEMENT/LIME)
GROUT
1. GROUT SHALL CONFORM TO ASTM DESIGNATION C478-83 (2500 psi)
2. THE MIX DESIGN SHALL BE APPROVED BY THE ENGINEER.
3. ALL GROUT SHALL BE FLUID CONSISTENCY, WHICH MEANS THAT CONSISTENCY BE AS FLUID AS POSSIBLE FOR POURING WITHOUT SEGREGATION OF THE CONSTITUENT PARTS. (3" ± 1" SLUMP)
4. THE USE OF ADMIXTURES SHALL NOT BE PERMITTED WITHOUT WRITTEN CONSENT OF THE ENGINEER.

REINFORCING VERTICAL
a. ASTM A615-90 PER REINFORCING SECTION (GRADE 60).
b. WHEN A FOUNDATION DOWEL DOES NOT LINE UP WITH A VERTICAL CORE, IT SHALL NOT BE SLOPED MORE THAN ONE HORIZONTAL IN SIX VERTICAL, EVEN THOUGH IT IS HORIZONTAL IN AN ADJACENT CELL TO THE VERTICAL WALL REINFORCING.
c. VERTICAL REINFORCEMENT STEEL SHALL HAVE A MINIMUM CLEARANCE OF ONE-HALF INCH FROM THE MASONRY.
d. VERTICAL REINFORCEMENT SHALL BE AS SHOWN ON PLANS.
e. VERTICAL REINFORCEMENT EACH SIDE OF ANY OPENING SHALL BE CONTINUOUS TO THE BEAM. PRECAST LINTELS SHALL HAVE OPENINGS TO ALLOW REINFORCING BARS TO CONTINUE UNINTERRUPTED.
1. ALL BAR SPLICES SHALL BE 48" BAR DIA.

HORIZONTAL
a. HORIZONTAL REINFORCEMENT SHALL BE PROVIDED AS SHOWN ON WALL SECTION IF ANY IS REQUIRED).

GENERAL
1. ALL REINFORCING MASONRY WORK DESIGN SHALL CONFORM TO ACI-530-92/ ASCE 5-92/TMS 402-92 AND ALL CONSTRUCTION SHALL BE IN ACCORDANCE WITH ACI 530-92/ASCE 5-92/TMS 402-92 (1999).
2. REINFORCED MASONRY UNITS HAVE BEEN DESIGNED BY AN ENGINEER. REFER TO ATTACHED STRUCTURAL CALCULATIONS W/SIGNED & SEALED ENGINEERING LETTER.
3. STRUCTURE HAS BEEN DESIGNED IN ACCORDANCE WITH ASCE 7-93.

INSPECTION:
ALL ENGINEERED UNIT MASONRY WORK SHALL REQUIRE INSPECTION BY SPECIAL INSPECTOR.

BUILDING IDENTIFICATION
S.F.B.C. 5212.06(1) REQUIRES APPROVED NUMBERS OR ADDRESSES BE PLACED ON ALL NEW AND EXISTING BUILDINGS IN SUCH POSITION AS TO BE PLAINLY VISIBLE AND LEGIBLE FROM THE STREET OR ROAD FRONTING THE PROPERTY. SAID NUMBERS SHALL CONTRAST WITH THEIR BACKGROUND.

SECTION 31-3 OF THE BROWARD COUNTY CODE FURTHER REQUIRES, "FOR EACH BUILDING, OR PORTION OF BUILDING, HAVING AN ADDRESS NUMBER, THE OWNER OR PERSON IN CHARGE OF THE BUILDING, OR OF EACH PORTION, SHALL CAUSE SUCH ADDRESS NUMBER TO BE DISPLAYED AT ALL TIMES, IN NUMERALS ON OR ADJACENT TO THE BUILDING, OR PORTION, SO THAT THE NUMERALS ARE READABLE FROM THE STREET OR THOROUGHFARE TO WHICH THE NUMBER APPLIES. NUMERALS SHALL BE AT LEAST THREE INCHES IN HEIGHT FOR RESIDENTIAL BUILDINGS."

ANCHORAGE
METAL AND WIRE LATH.
FASTENING SHALL BE PER S.F.B.C. 3502 AND ANSI A42.3 WITH MIN. 1-3/8" PENETRATION FOR HORIZONTAL SUPPORTS;
3/4" PENETRATION ON VERTICAL SUPPORTS.

GARAGE
GARAGE WALLS AND CEILINGS SHALL BE 1 HR. FIRE RATED. CEILING (S.F.B.C. TABLE 37-C #42) SHALL BE (1) LAYER 5/8" TYPE "X" GYPSUM WALLBOARD APPLIED AT RIGHT ANGLES TO STEEL FLOORING CHANNELS W/ 1-1/4" LONG, NO. 6 DRYWALL SCREWS 8" C/C AND CHANNELS "6" C/C COMPOUND SHALL COVER JOINTS. APPROVED INSULATION OF FIBERGLASS OR MINERAL WOOL BATT, OR BLOWN-IN FIBERGLASS MINERAL WOOL, OR CELLULOSE INSULATION HAVING A MINIMUM INSULATING VALUE OF R-11. WALLS (S.F.B.C. TABLE 37-B #72) SHALL BE NO. 25 GAUGE CHANNEL-SHAPED STUDS 16" C/C WITH 1/2" FULL LENGTH LAYER OF 5/8" TYPE "X" GYPSUM WALLBOARD APPLIED VERTICALLY ATTACHED WITH 1-1/4" LONG NO. 6 DRYWALL SCREWS TO EACH SIDE. SCREWS ARE 8" O.C. AROUND THE PERIMETER AND 12" C/C ON THE INTERMEDIATE STUD.

DOORS & WINDOWS
PER S.F.B.C. SECTION 3122.2 - ALL BEDROOMS REQUIRE AN OUTSIDE WINDOW OR DOOR OPERABLE FROM THE INSIDE WITHOUT THE USE OF TOOLS AND PROVIDING A CLEAR OPENING OF NOT LESS THAN 20 INCHES WIDTH, 24 INCHES IN HEIGHT AND 5.7 SQ.FT. IN AREA. THE BOTTOM OF THE OPENING SHALL NOT BE MORE THAN 44" OFF THE FLOOR AND NO PART OF THE OPERATING MECHANISM SHALL BE PLACED HIGHER THAN 54" ABOVE FINISHED FLOOR.

OPERATING MECHANISMS ON SINGLE HUNG WINDOWS SHALL BE AT OR NEAR THE SILL.
WINDOWS INDICATED WITH (E) MUST BE MANUFACTURED TO CONFORM TO THE BUILDING CODE WITH RESPECT TO MINIMUM EMERGENCY EGRESS REQUIREMENTS PER NFPA 101 CHAPTER 22.2.
ALL SLIDING GLASS DOORS AND FRENCH DOORS SHALL BE TEMPERED PER S.F.B.C. SECTION 3508.
ALL FIXED GLASS SHALL MEET THE REQUIREMENTS OF THE S.F.B.C. SECTION 3508.2. ALL GLASS SHALL BE SINGLE TINT.

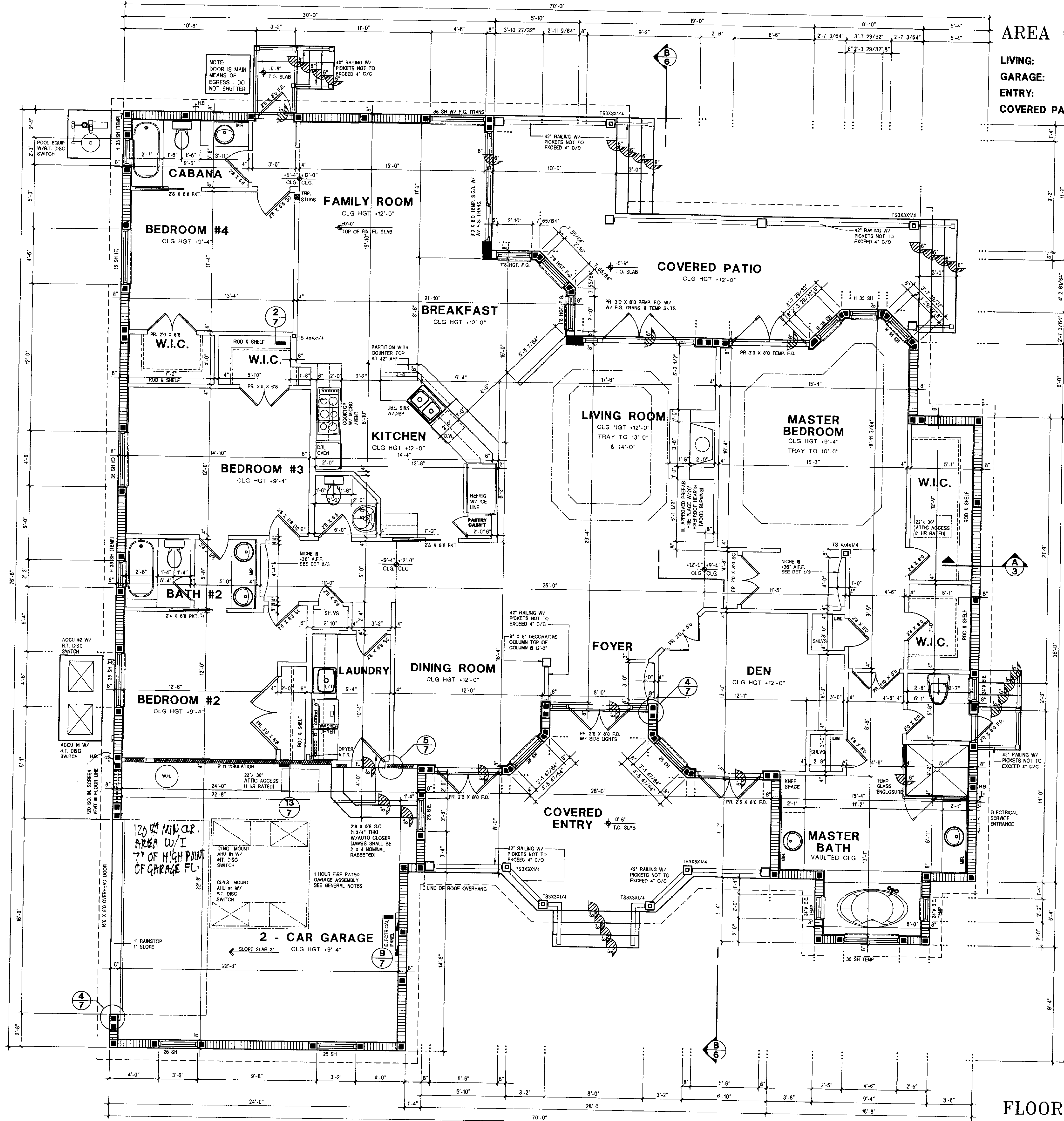
TOLERANCE
FRAME TO BUCK TOLERANCE SHALL BE PER WINDOW MFR.
BUCK TO DRIVING TOLERANCE SHALL BE MAX. 1/4" TOTAL OPENING.
ALL GARAGE DOOR MIN. 2" X 8" P.T. WOOD BUCKS (HEADER AND JAMB) SHALL BE ATTACHED W/ 1/2" X ANCHOR BOLT W/MIN 4" EMBEDMENT AND DRIVEN NOT LESS THAN 3" FROM EDGE OF CONCRETE. SPACING SHALL BE 12" C/C STAGGERED 18" WIDTH; 8" C/C FOR 16" WIDTH. ALTERNATE: 1/4" X TAP-CON 8" 5" C/C STAGGERED 18" 1-3/4" EMBEDMENT OR RAWL-STUD EXPANSION ANCHOR (1400 SERIES) 3/8" X 1-3/4" MIN. EMBEDMENT, MIN. 5 PER VERTICAL JAMB, FASTENERS LOCATED 16" C/C MAX.
ALTERNATE 2: "HIT" P.A.F. XMI FASTENERS (144 5/8", 3" LONG; 1-1/2" EMBEDMENT) 8" C/C NOTE: ALL FASTENERS SHALL BE SUPPLIED W/NUTS & WASHERS

SHUTTERS
REFER TO MFR'S SPECIFICATIONS & SHOP DWGS FOR INSTALLATION AND CONNECTION DETAILS. ALL SHUTTERS SHALL BE REVIEWED & APPROVED BY THE ARCHITECT OF RECORD. SHUTTERS SHALL MEET AND COMPLY WITH S.F.B.C. AND SATISFY THE REQUIREMENTS OF DATE COUNTY PRODUCT APPROVAL. SHUTTERS SHALL BE PROVIDED AT ALL WINDOWS UNLESS THE WINDOW HAS AN APPROVED PRODUCT APPROVAL NUMBER & COMPLES W/ALL REQUIRED TESTING.

FINISHES
MIRRORS IN EXCESS OF 9 S.F. SHALL BE DIRECTLY SECURED TO SUPPORTS AND SHALL NOT BE HUNG PER S.F.B.C. 3508.4 (d).
BATHROOM FLOORS AND BASE SHALL BE IMPERVIOUS MATERIALS PER SOUTH FLORIDA BUILDING CODE SECTION 1408.2(b). THE SELECTION BY OWNER.

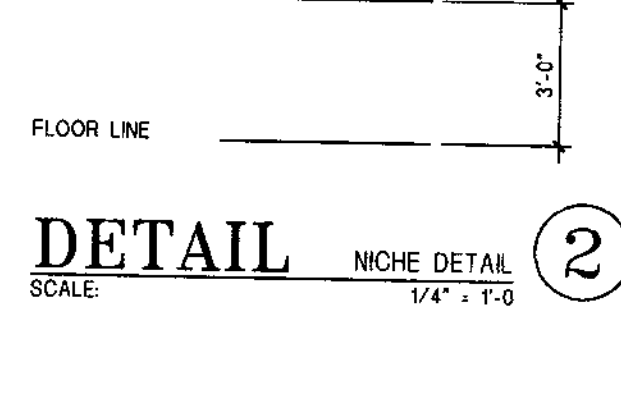
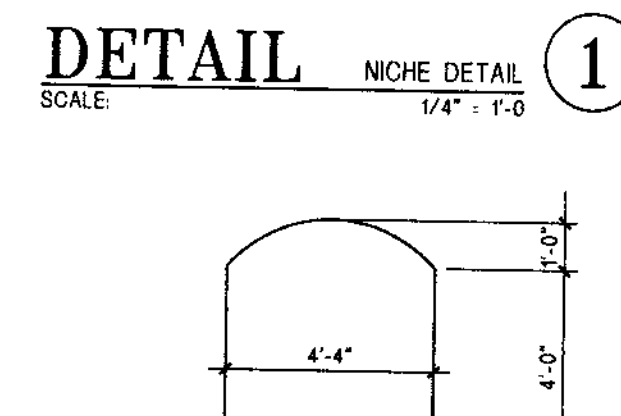
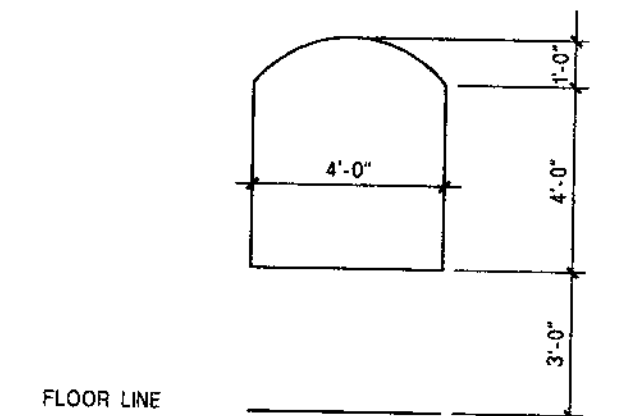
NON-BEARING INTERIOR PARTITIONS
ALL INTERIOR NON-BEARING PARTITIONS SHALL BE 3/8" METAL STUDS STUDS & RUNNERS SHALL BE OF CHANNEL OR "C" SHAPE TYPE MIN 25 GA. HOT DIPPED GALV. @ 24" C/C MAX. SPACING WITH 1/2" MIN. GYPSUM WALLBOARD EACH SIDE. SPANS BETWEEN DRYWALL SUPPORTS SHALL NOT EXCEED 24" C/C. FIRESTOPS SHALL BE PROVIDED AT CEILINGS AND FLOOR LEVELS AND AT MID HGT. OR NOT TO EXCEED 8' HEIGHT.

WOOD
ALL STRUCTURAL LUMBER TO BE DOUGLAS FIR-LARCH NO2 OR BETTER F_b = 1200 P.S.I.
ALL LUMBER IN CONTACT WITH MASONRY OR CONCRETE SHALL BE PRESSURE TREATED.
ANCHORAGE OF ALL WOOD FRAMING SHALL BE CONTINUOUS FROM THE FOUNDATION TO THE ROOF AND SHALL MEET THE REQUIREMENTS OF S.F.B.C. SECTION 2908.
BEARING & EXTERIOR FRAME WALLS HAVE BEEN DESIGNED BY RATIONAL ANALYSIS TO REDUCE THE MINIMUM THICKNESS/HEIGHT ALLOWABLE PER S.F.B.C. 2903.1.
WOOD COLUMNS SHALL BE 1 HOUR FIRE RATED REFER TO DETAIL FOR FIRE RATING SPECIFICATIONS COMPLYING WITH THE S.F.B.C.



AREA CALCULATIONS

LIVING: 3,202 SQ. FT.
GARAGE: 570 SQ. FT.
ENTRY: 306 SQ. FT.
COVERED PATIO: 392 SQ. FT.



FLOOR PLAN

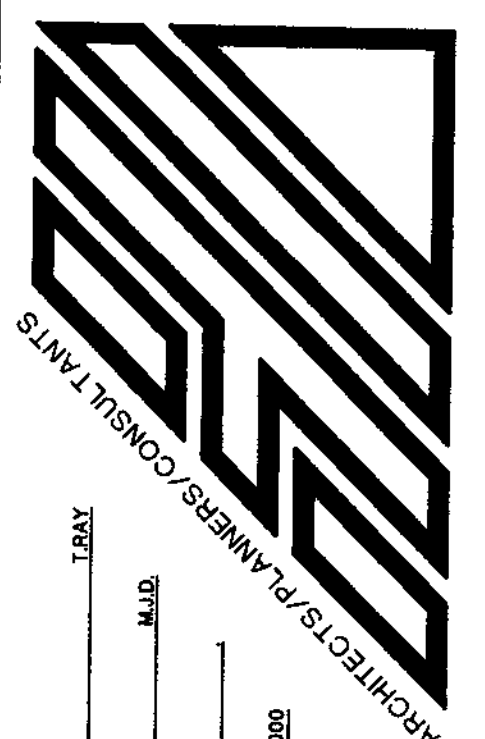
SAVE FOR H.D. APPROVAL
(SEE A-3 - REV 1 10/30/00)
13 BEARIT BAY

Martin County Health Department
THIS PLAN IS APPROVED FOR:
Septic System: Approval # 43-55-02900
Well Location: Approval # 43-
Other: Approval # 43-
By: [Signature] Date: 11/1/02
As Seen To The Plans Must Be Approved By The Health Department

LOT 22

THE CAPTIVA

FOR: BY:
A NEW CUSTOM RESIDENCE
DR. & MRS. ROBERT CUSHING
FOGLIA CUSTOM HOMES
SEWALL'S MEADOWS
MARTIN COUNTY, FLORIDA
HOMES BY ARCHITECT INC.
SEWALL'S POINT,
7430 WILES ROAD
CORAL SPRINGS, FLORIDA 33067
PHONE: (954) 755-0690
FAX: (954) 755-8801

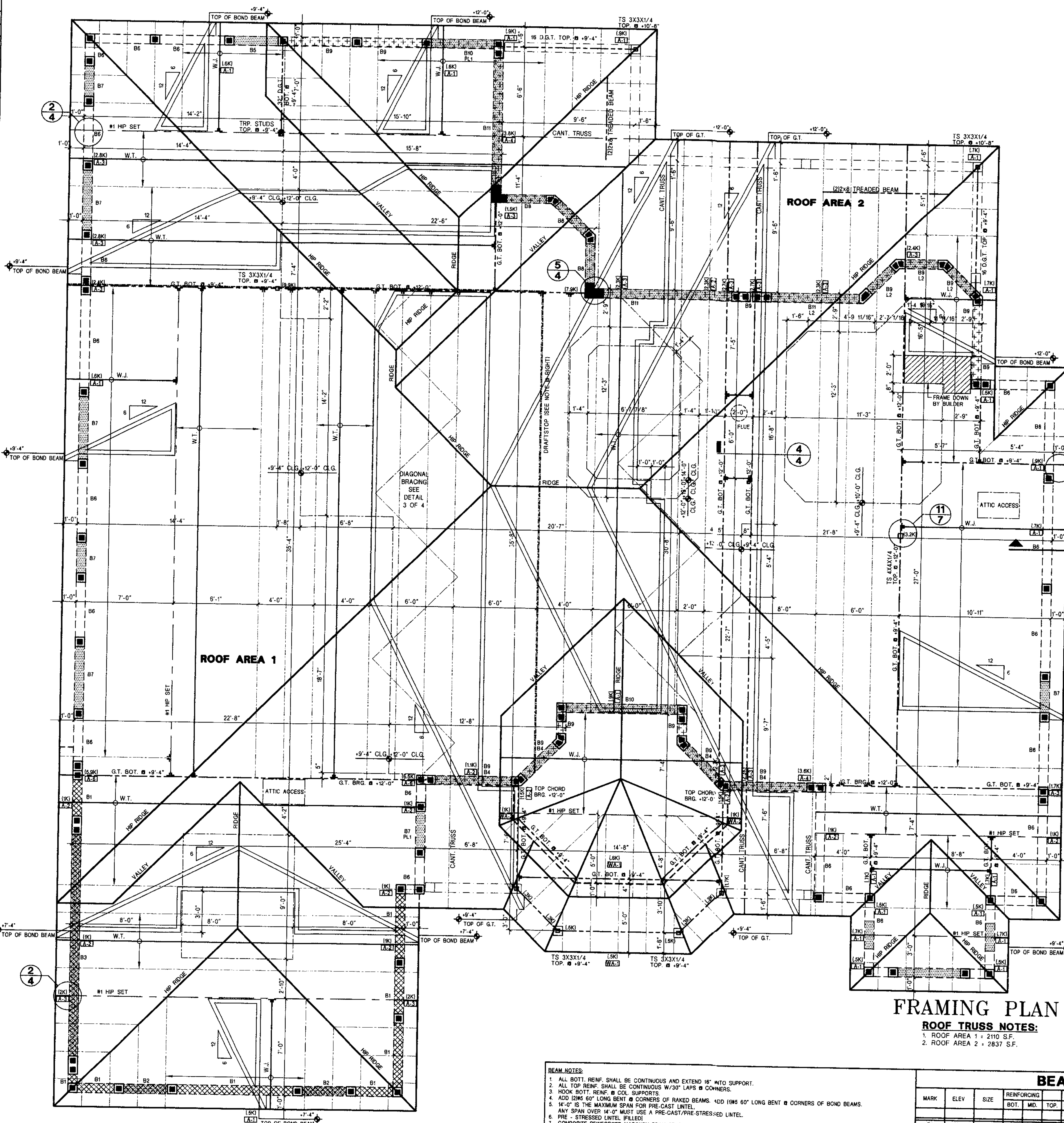
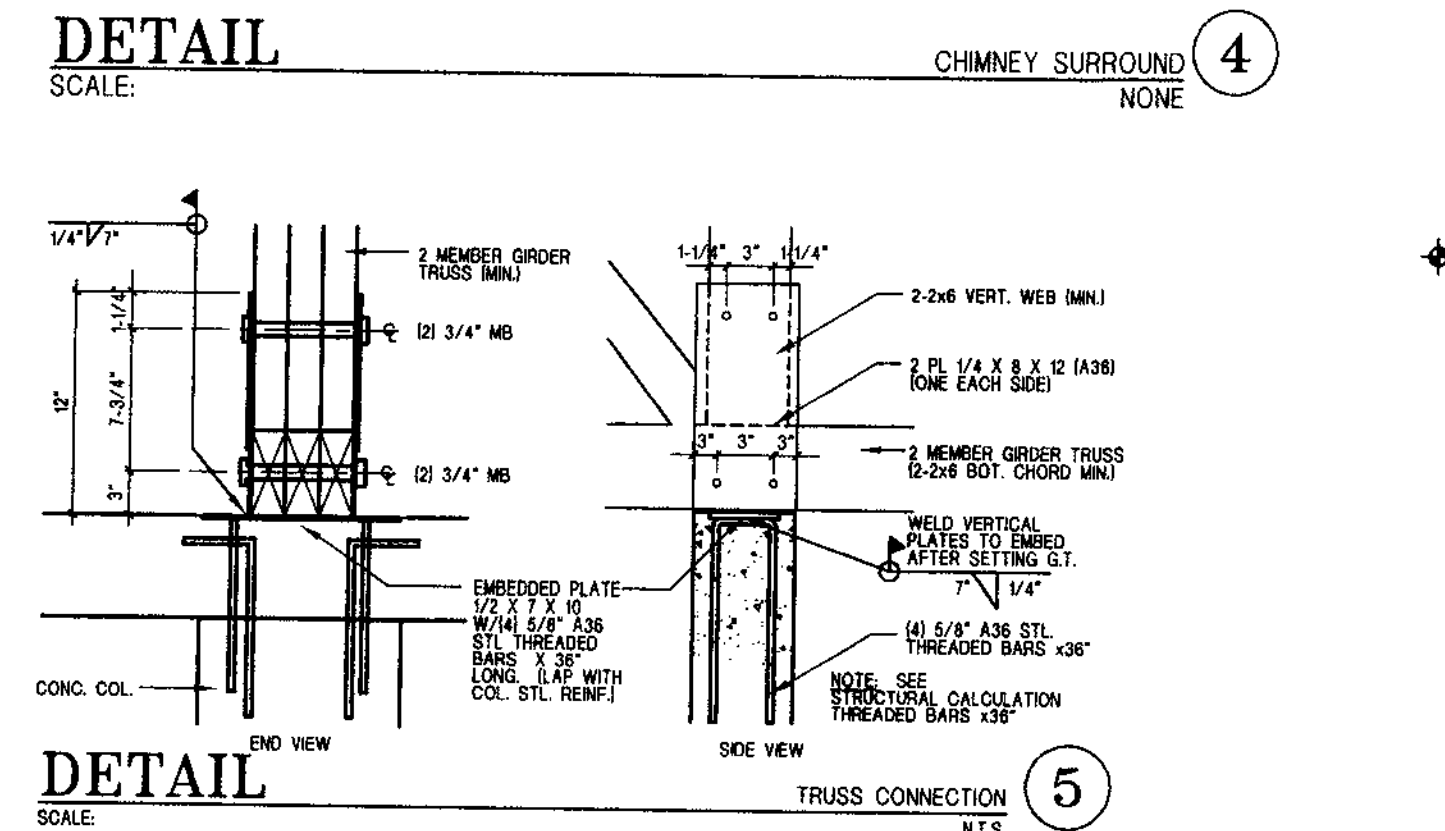
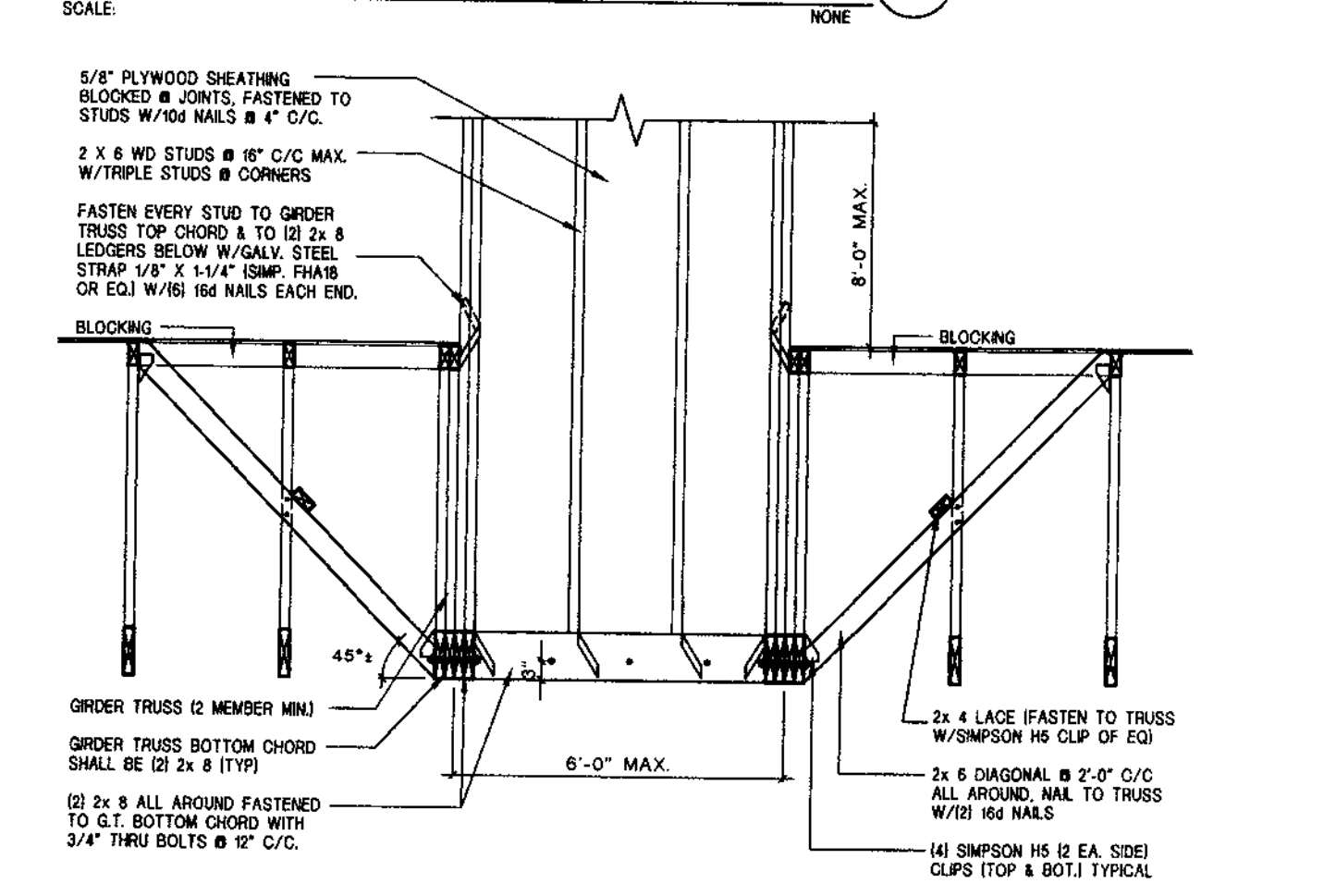
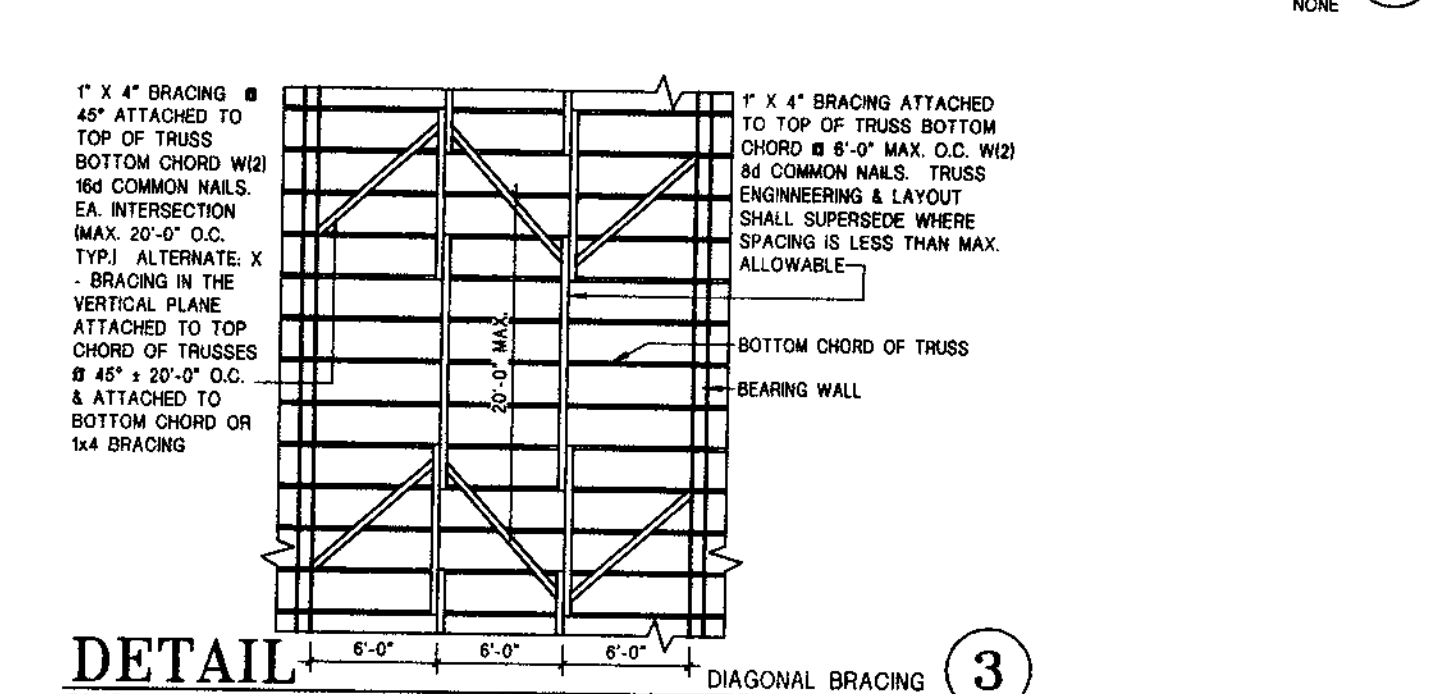
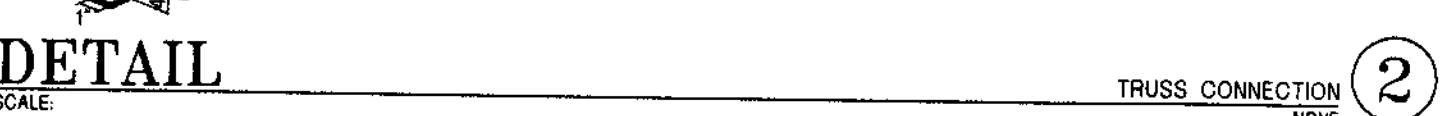
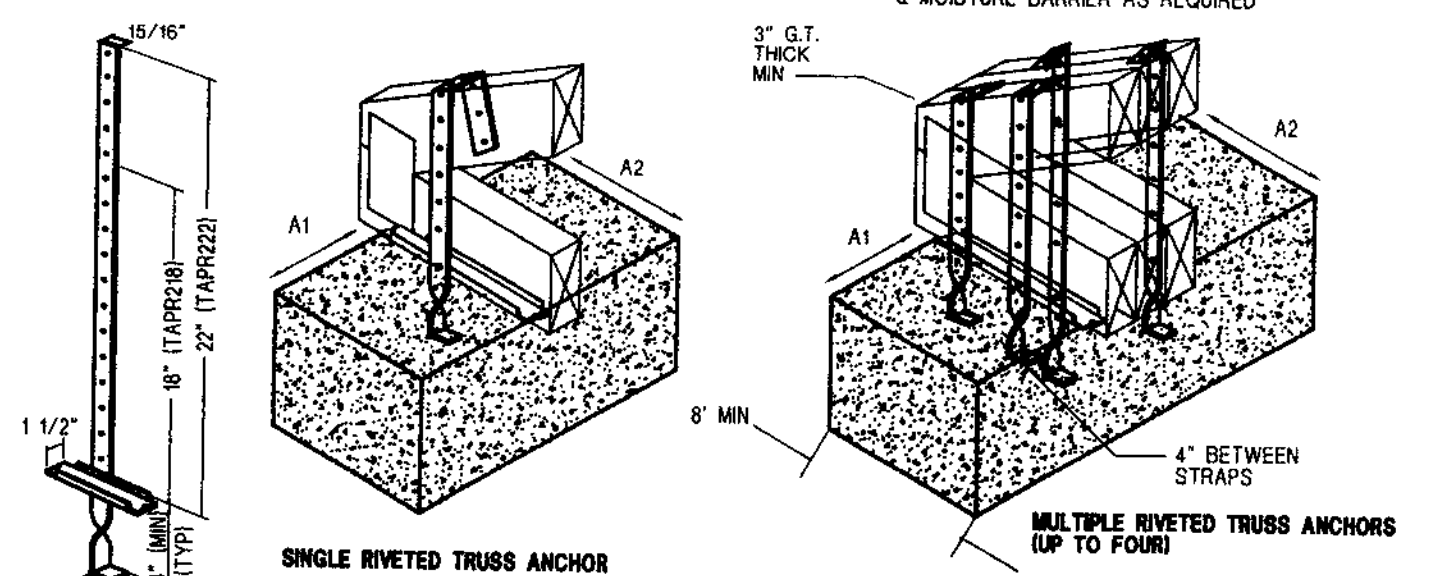


Designed by: T.B.A.
Drawn by: M.A.D.
Checked by: [Signature]
Date: OCTOBER 17, 2000
Scale: 1/4" = 1'-0"
Project #: 00284
Sheets: 3 of 8
Sheet 3

RIVETED TRUSS ANCHOR SCHEDULE									
MARK	NUMBER OF STRAPS	SIZE	PRODUCT CODE	GAUGE	DIMENSIONS		NUMBER OF 16d COMM. NAILS	ALLOWABLE LOADS	
					OUT OF CONCRETE	OVERALL LENGTH		LATERAL	UPLIFT
A-1	1	20	TAPR218	14	18	20	5	595	210
A-2	1	20	TAPR218	14	18	20	8	595	210
A-3	2	20	TAPR218	14	18	20	8 EACH	1190	420
A-4	2	26	TAPR222	14	22	26	11 EACH	1190	420
A-5	3	26	TAPR222	14	22	26	11 EACH	1785	630
A-6	4	26	TAPR222	14	22	26	11 EACH	2380	8840

- USE ALL FASTENERS IN SCHEDULE TO ACHIEVE VALUES INDICATED. MINIMUM EDGE DISTANCE IS 2"
- ALLOW 4" EMBEDMENT IN CONCRETE
- "TAPR" IS "TAP" WITH A RIVETED BEAM SEAT, EITHER STRAP YIELDS THE SAME UPLIFT
- DESIGN LOADS BASED ON USE OF SOUTHERN YELLOW PINE. SG=0.55

NOTE: (1) STRAPS ARE TAPR AND (2) ARE TAP PROVIDED ONE BEAM SEAT PER SIDE & MOISTURE BARRIER AS REQUIRED



FRAMING PLAN
ROOF TRUSS NOTES:
 1. ROOF AREA 1 = 2110 S.F.
 2. ROOF AREA 2 = 2837 S.F.

BEAM NOTES:

- ALL BOT. REIN. SHALL BE CONTINUOUS AND EXTEND 18" INTO SUPPORT.
- HOOK BOT. REIN. @ CORNERS OF RAISED BEAMS. 4D (96 60" LONG BENT @ CORNERS OF BOND BEAMS.
- 14'-0" IS THE MAXIMUM SPAN FOR PRE-CAST UNITS.
- ANY SPAN OVER 14'-0" MUST USE A PRE-CAST/PRE-STRESSED UNITS.
- PRE-CAST UNITS (P.U.)
- COMPOSITE REINFORCED MASONRY BEAM SECTIONS SHALL HAVE THE FOLLOWING MIN. SAFE LOADS CAPACITY AS CERTIFIED BY MANUFACTURER.

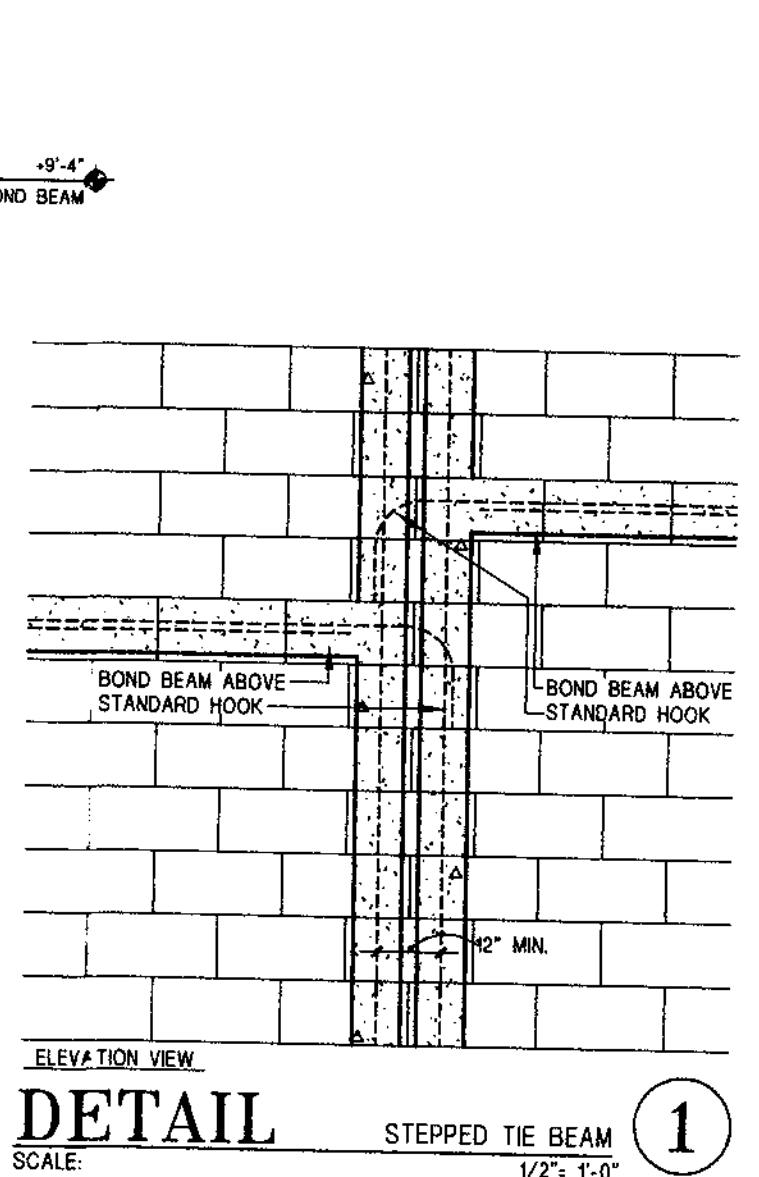
GRAVITY	LATERAL	UPLIFT
1. 1400 P.L.F.	280 P.L.F.	464 P.L.F.
2. 2244 P.L.F.	360 P.L.F.	1288 P.L.F.
3. 1003 P.L.F.	280 P.L.F.	403 P.L.F.
4. 2033 P.L.F.	360 P.L.F.	1042 P.L.F.
5. 2610 P.L.F.	464 P.L.F.	1466 P.L.F.

BEAM SCHEDULE

MARK	ELEV	SIZE	REINFORCING	STRIP	SEE	REMARKS	SEE DESIGNATION
B1	7'-4"	8"x16"	(1) #5	(1) #5	4	# MASONRY OPENINGS	BF16-1B/1T
B2	7'-4"	8"x16"	(1) #5	(1) #5	7c	# GARAGE DOOR OPENINGS USE L1	BF20-1B/1T
B3	7'-4"	8"x16"	(1) #5	(1) #5	4,7a,8	# MASONRY OPENINGS	BF16-1B/1T
B4	9'-4"	8"x16"	(1) #5	(1) #5	7a	# 4"x4" PL RECESSED	BR14-1B/1T
B5	9'-4"	8"x16"	(1) #5	(1) #5	7c	# MASONRY OPENINGS	BF16-1B/1T
B6	12'-0"	8"x16"	(1) #5	(1) #5	7c	# MASONRY OPENINGS	BF16-1B/1T
B7	12'-0"	8"x16"	(1) #5	(1) #5	4	# CONCRETE BEAM W/ #3 CLOSED STIRRUPS	CONC.
B8	12'-0"	8"x16"	(1) #5	(1) #5	7c	# MASONRY OPENINGS	BF16-1B/1T
B9	12'-0"	8"x16"	(1) #5	(1) #5	7c	# MASONRY OPENINGS	BF16-1B/1T
B10	12'-0"	8"x16"	(1) #5	(1) #5	7c	# MASONRY OPENINGS	BF16-1B/1T
B11	12'-0"	8"x16"	(1) #5	(1) #5	7c	# MASONRY OPENINGS	BF16-1B/1T

BEARING HEIGHTS	
SYMBOL	DESCRIPTION
	7'-4" TOP OF BOND BEAM
	9'-4" TOP OF BOND BEAM
	12'-0" TOP OF BOND BEAM
	INDICATES OPENING BELOW

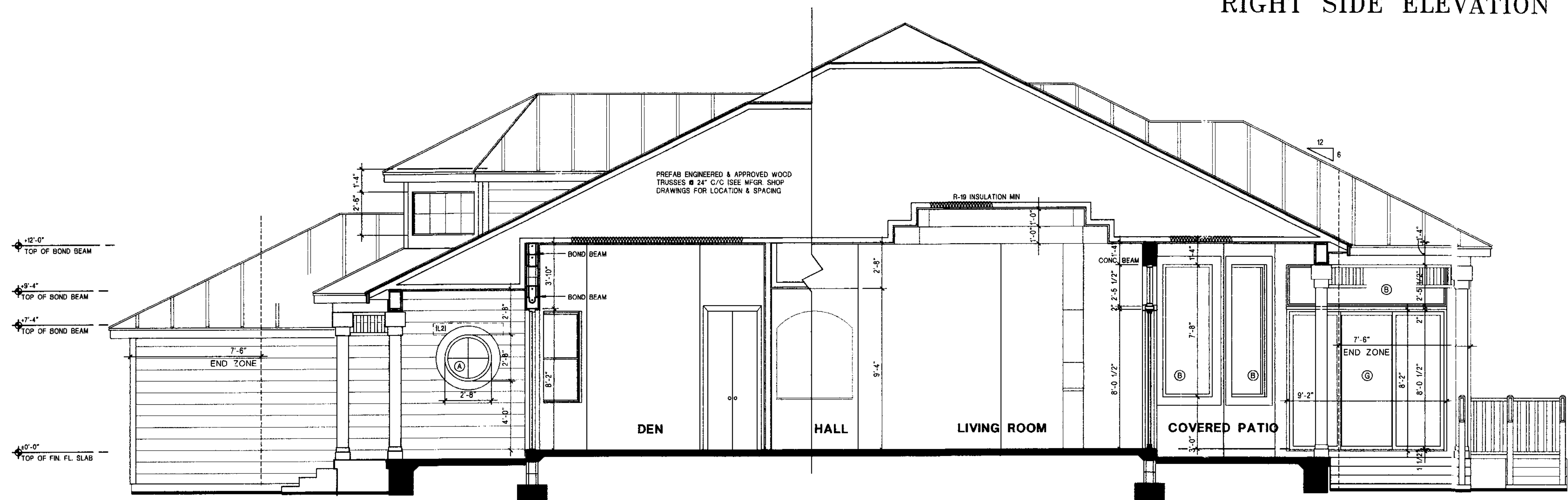
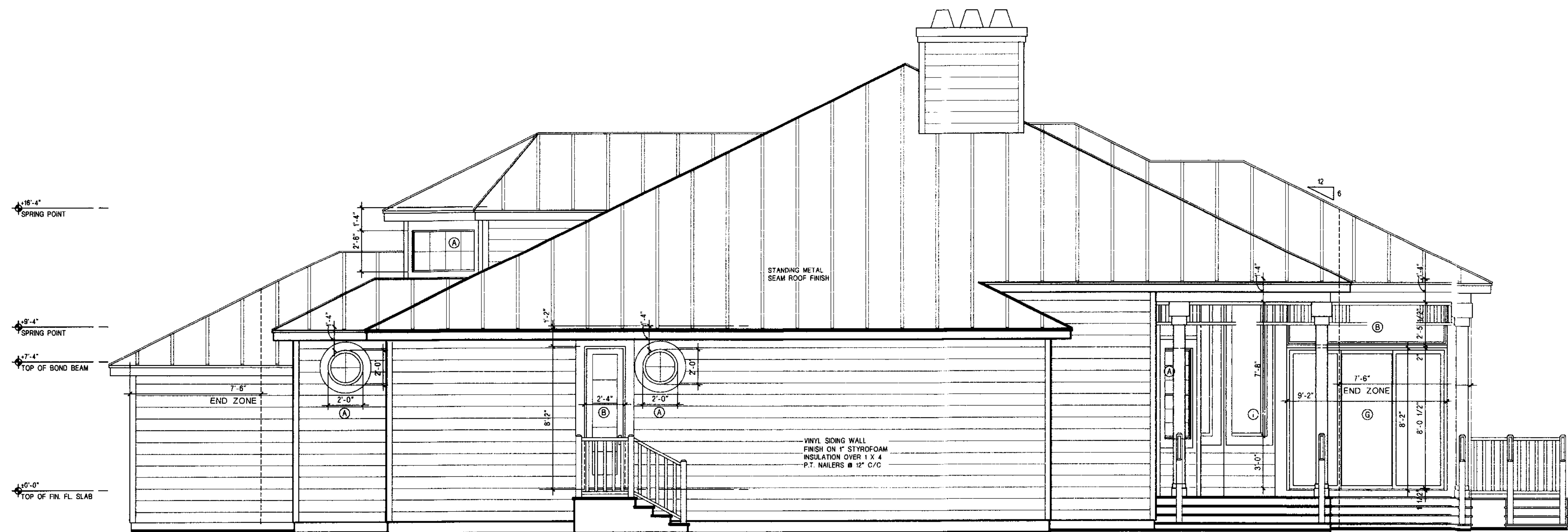
- NOTES:**
- ROOF DESIGN LIVE LOAD = 30 PSF.
 - ROOF DESIGN DEAD LOAD = 26 PSF.
 - NET UPLIFT = 26 PSF.
 - SHOP DRAWINGS SHALL BE SUBMITTED TO THE ARCHITECT BY AN APPROVED TRUSS MANUFACTURER.
 - FINAL DESIGN OF TRUSS BUCKETS AND CONNECTIONS SHALL BE DONE AFTER COMPLETION OF TRUSS ENGINEERING BY AN APPROVED TRUSS MANUFACTURER.
 - PROVIDE DRAFT-SITE BY CLADDING ONE SIDE OF TRUSS W/1 LAYER OF 1/2" GYPSUM WALLBOARD FOR CEILING AREAS INCLUSIVE OF OVERHANG EXCEEDING 3000 S.F. & FLOOR AREAS NOT TO EXCEED 1000 S.F.
 - PROVIDE A MIN OF (2) 2x UNDER ALL GIRDERS & HEADERS AND AT ENDS OF ALL FREE STANDING WALLS UNLESS NOTED OTHERWISE STUDS SHALL BE SAME DIMENSION AS WALL.
 - ELEVATIONS ARE TAKEN FROM LIVING ROOM FINISH FLOOR SLAB WHICH SHALL BE 10'-0".
 - B.P. INDICATES BEARING PARTITION WITH WOOD STUDS @ 16" C/C UNDO AND (3) LAYER 5/8" TYPE 'X' GYPSUM WALLBOARD EACH SIDE. REFER TO PLAN FOR WALL THICKNESS.
 - THE BRACING FOR THE ROOF SYSTEM HAD BEEN DESIGNED FOR BOTH POSITIVE AND NEGATIVE PRESSURE AS IN ACCORDANCE WITH THE REQUIREMENTS OF CHAPTER 23 OF S.F.B.C. PER 2906.17. NO ADDITIONAL BRACING IS REQUIRED.
 - LATERAL BRACING SHALL BE PER T.P.I. SPECIFICATIONS.
 - ROOF SHEATHING SHALL BE ATTACHED TO WOOD MEMBERS WITH "ATLAS COLLATED NAIL" #10D774 & #10D775 2-1/2" X .131 COATED PLAIN SHANK, PNEUMATIC NAIL.
 - ROOFING ANCHOR SHEETS SHALL BE FASTENED TO NOMINAL 5/8" 30# PLYWOOD SHEATHING WITH 1" LONG 120# COLD ROLLING NAIL "SEMO" PRODUCT NO. HJ5ATV, OR EQUAL. SPACING SHALL BE 4" C/C AT EDGE AND 6" C/C IN THE FIELD UP TO A HEIGHT OF 20 FEET ABOVE GRADE PER REQUIREMENTS OF S.F.B.C. SECTION 3402.2(c).
 - WT INDICATES WOOD TRUSSES @ 24" o/c UNDO.
 - WJ INDICATES WOOD JACKS @ 24" o/c UNDO.
 - PRECAST MANUFACTURERS SPECIFICATIONS & CERTIFIED SAFE LOAD CAPACITIES SHALL BE SUBMITTED TO THE ENGINEER FOR REVIEW & APPROVAL.
 - PRECAST UNITS SHALL BE INSTALLED IN ACCORDANCE W/ MANUFACTURERS SPECIFICATIONS.
 - (K) - INDICATES UPLIFT IN KIPS @ DESIGNATED BEARINGS.
 - ALL TRUSSES TO CONCRETE CONNECTIONS SHALL BE W/SEMO (TAPR218 W/8) 86d COMMON NAILS UNDO. MAX UPLIFT = 1339# ALL ALL WOOD JACKS (MAX LENGTH = 13'-0" USE ONLY (4) 10d COMMON NAILS MAX UPLIFT = 668#)



THE CAPTIVA LOT 22

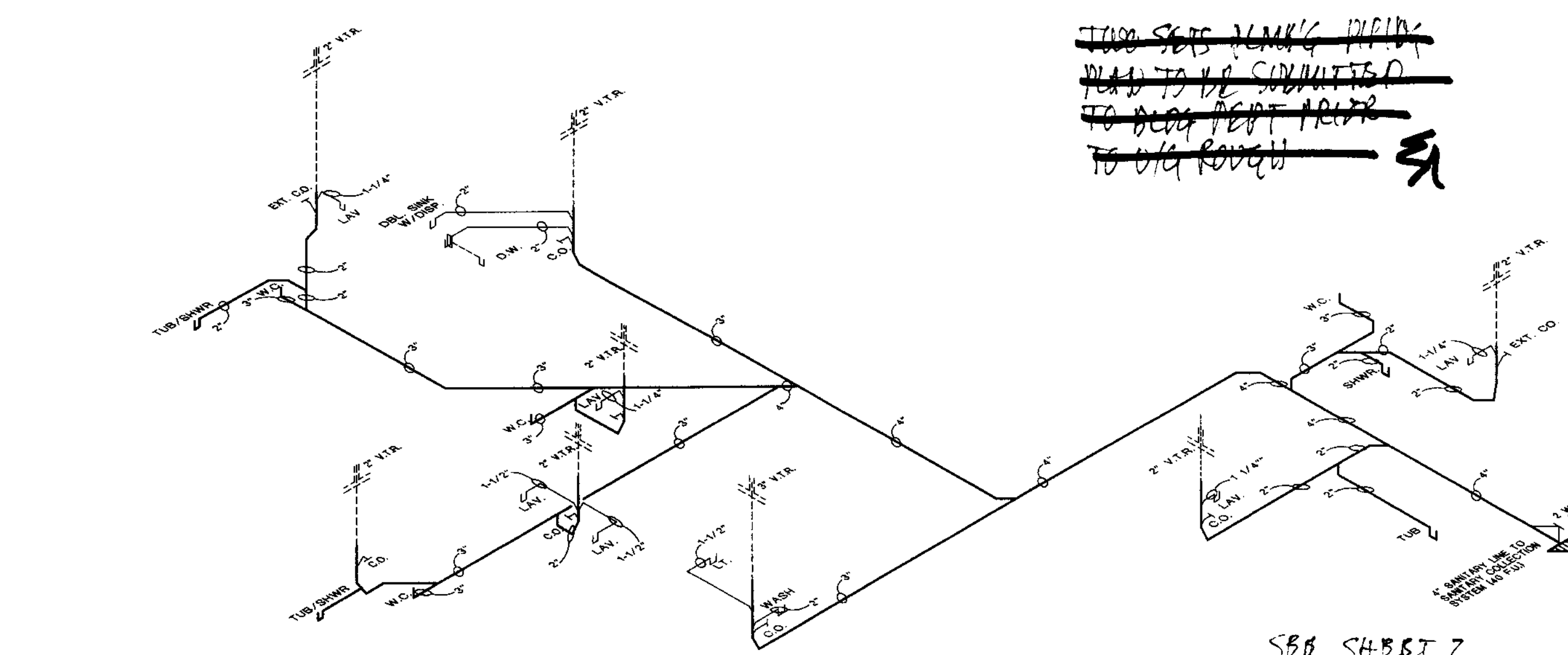
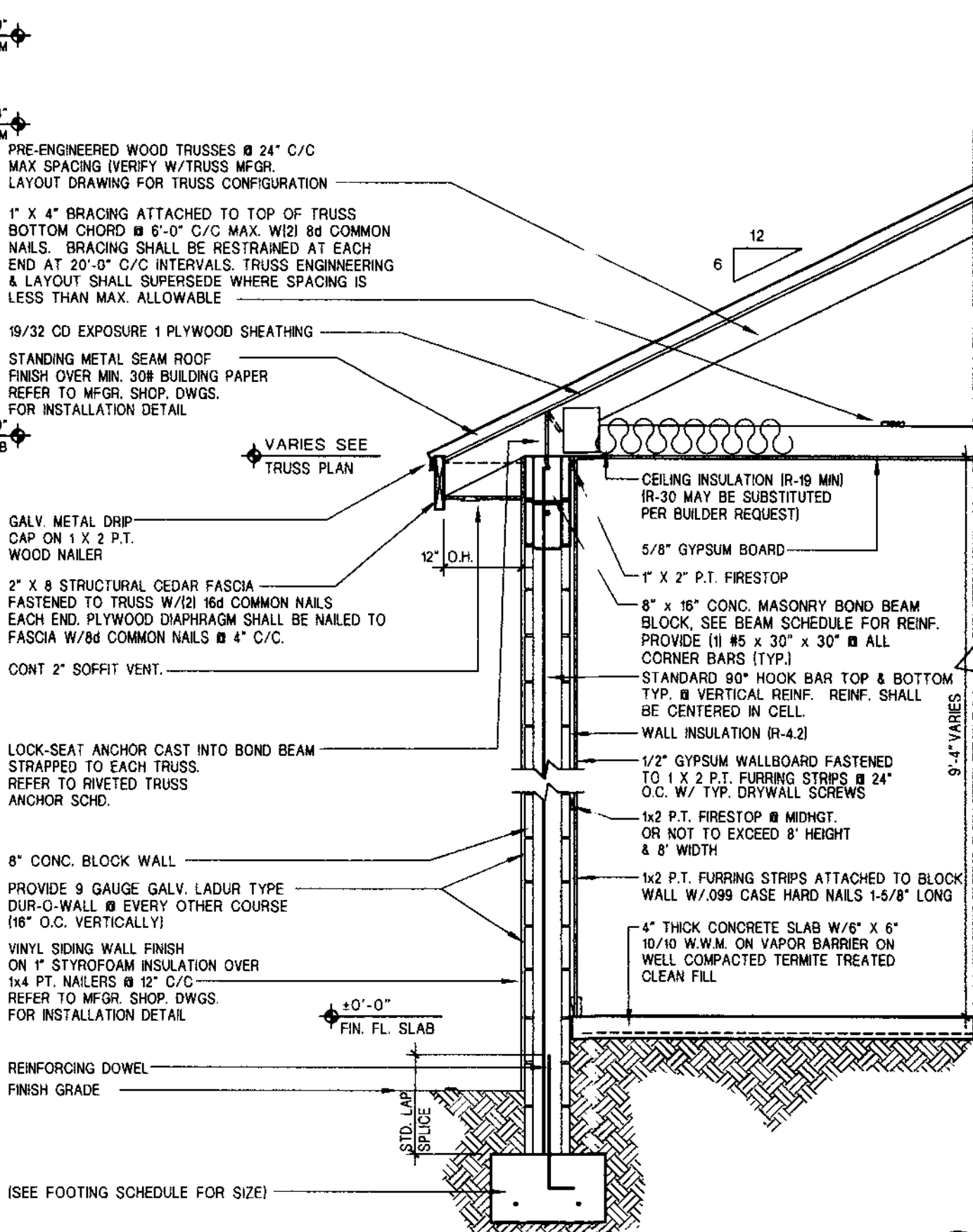
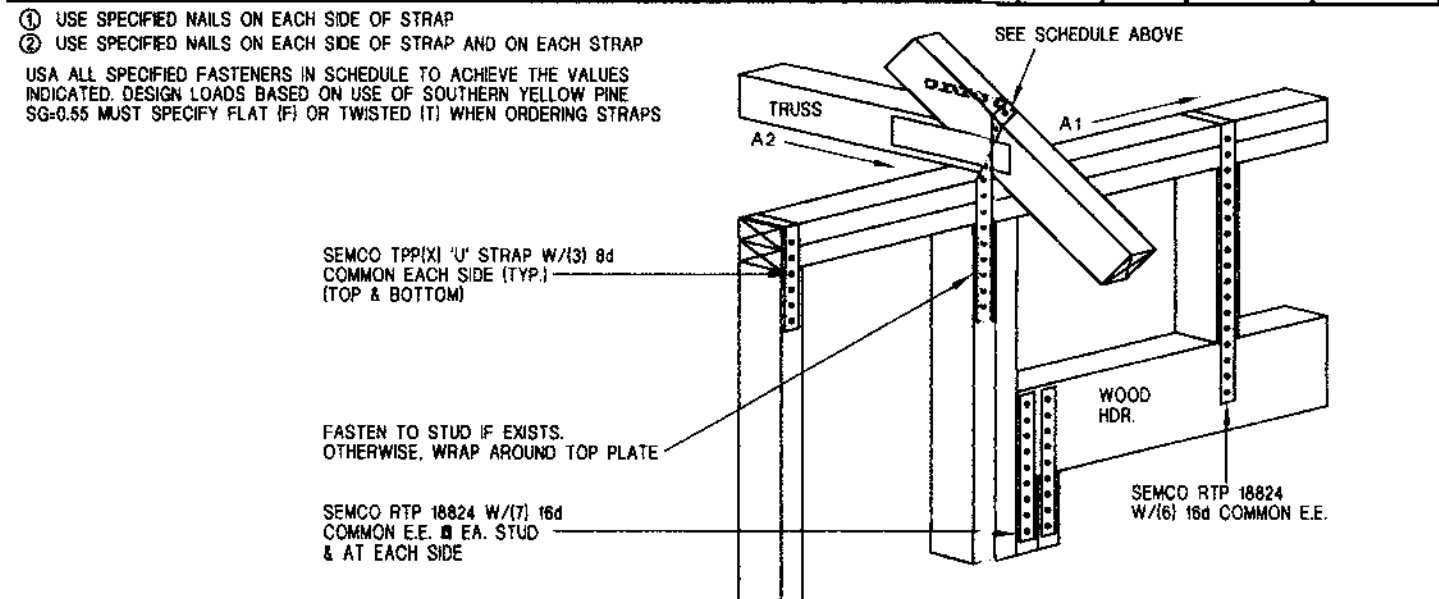
FOR: DR. & MRS. ROBERT CUSHING
 BY: FOGLIA CUSTOM HOMES
 SEWALL'S MEADOWS
 MARTIN COUNTY, FLORIDA
 HOMES BY ARCHITECTNIC INC.
 7430 WILES ROAD
 CORAL SPRINGS, FLORIDA 33067
 PHONE: (954) 755-0690
 FAX: (954) 755-8801

Designed by: TAYLOR
 Drawn by: MILD
 Checked by: OCTOBER 17, 2000
 Date: 1/4" = 1'-0"
 Scale: 00004
 Project #: 8
 Sheets



~~TWO SETS PLUMBING PLAN TO BE SUBMITTED TO BLDG DEPT. PRIOR TO OIG REVIEW~~

WOOD TO WOOD STRAP SCHEDULE									
MARK	NUMBER OF STRAPS	SIZE	PRODUCT CODE (SEWCO)	GAUGE	DIMENSIONS CONNECTION	NUMBER OF 16d COMM. NAILS	ALLOWABLE LOADS		
							LATERAL	UPLIFT	16d COM.
WA-1	1	16	RTPGA816	14	WD/WD	24	5	210	595
WA-2	1	16	RTPGA816	14	WD/WD	24	7	210	595
WA-3	1	16	RTPGA816	14	WD/WD	24	9	210	595
WA-4	2	24	RTPGA824	14	WD/WD	24	9	420	1120
WA-5	2	24	RTPGA824	14	WD/WD	24	12	420	1120
WA-6	3	24	RTPGA824	14	WD/WD	24	12	420	1120



THE CAPTIVA LOT 22

FOR: A NEW CUSTOM RESIDENCE DR. & MRS. ROBERT CUSHING

BY: FOGLIA CUSTOM HOMES

FLORIDA

SEWALL'S MEADOWS MARTIN COUNTY

HOMES BY ARCHITECT INC.

7430 WILES ROAD CORAL SPRINGS, FLORIDA 33067

PHONE: (954) 755-0690

FAX: (954) 755-8801

Designed by: TRAY

Drawn by: M.D.

Checked by: M.D.

Date: OCTOBER 17, 2000

Scale: 1/4" = 1'-0"

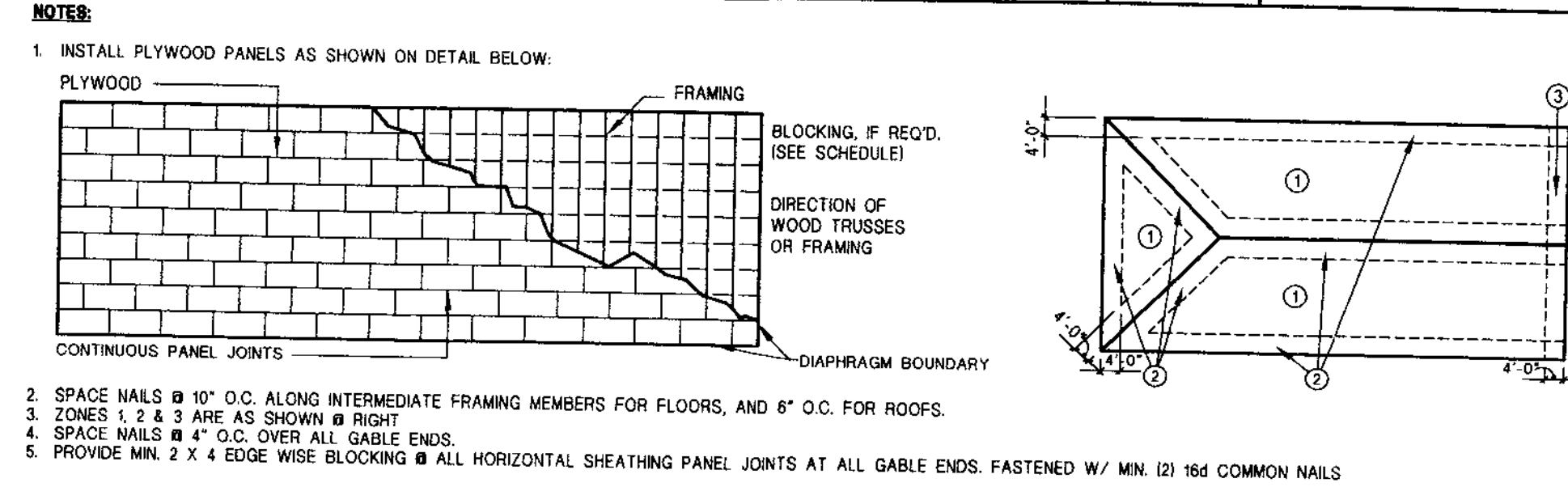
Project #: 00284

6 of 9

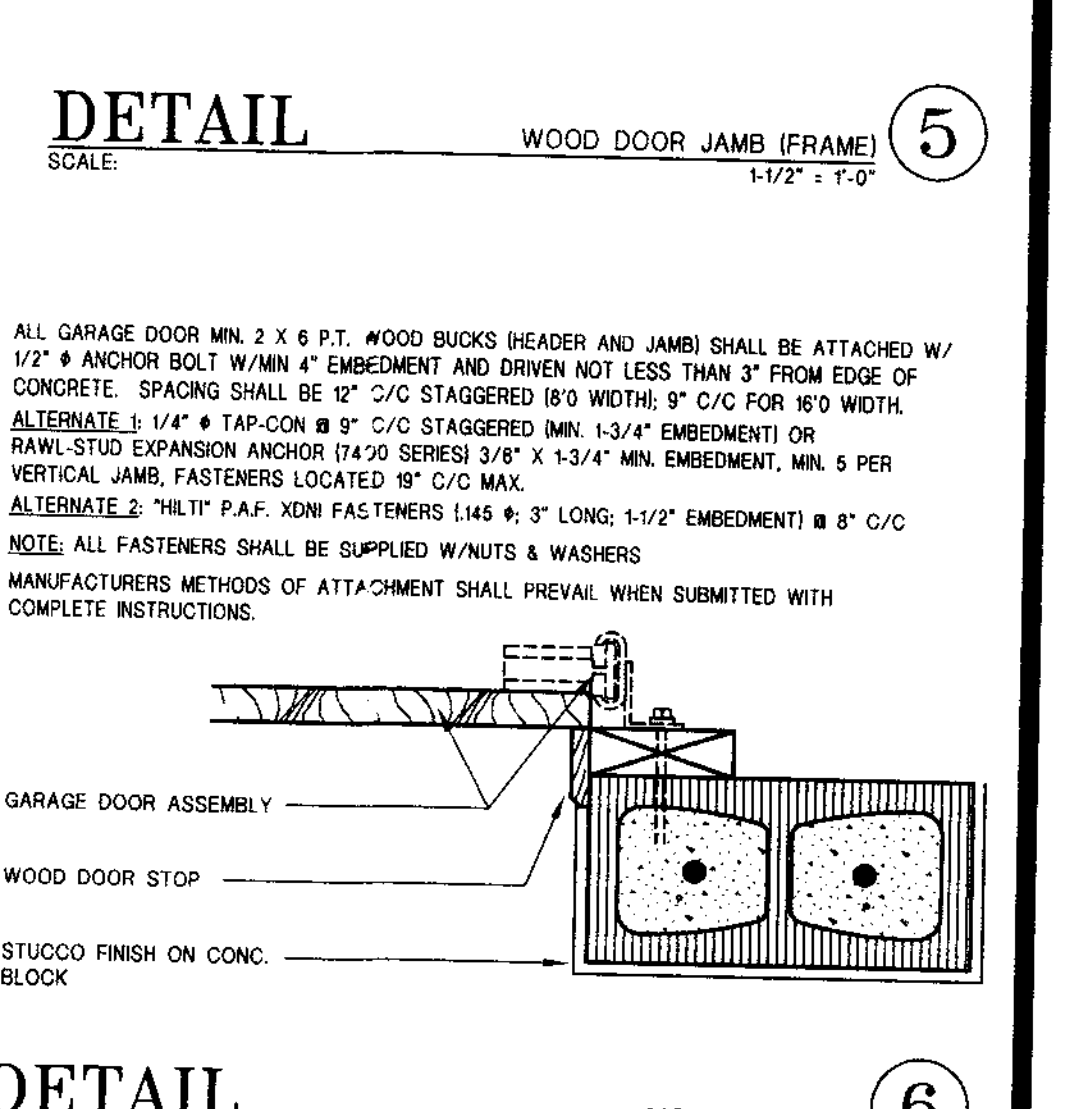
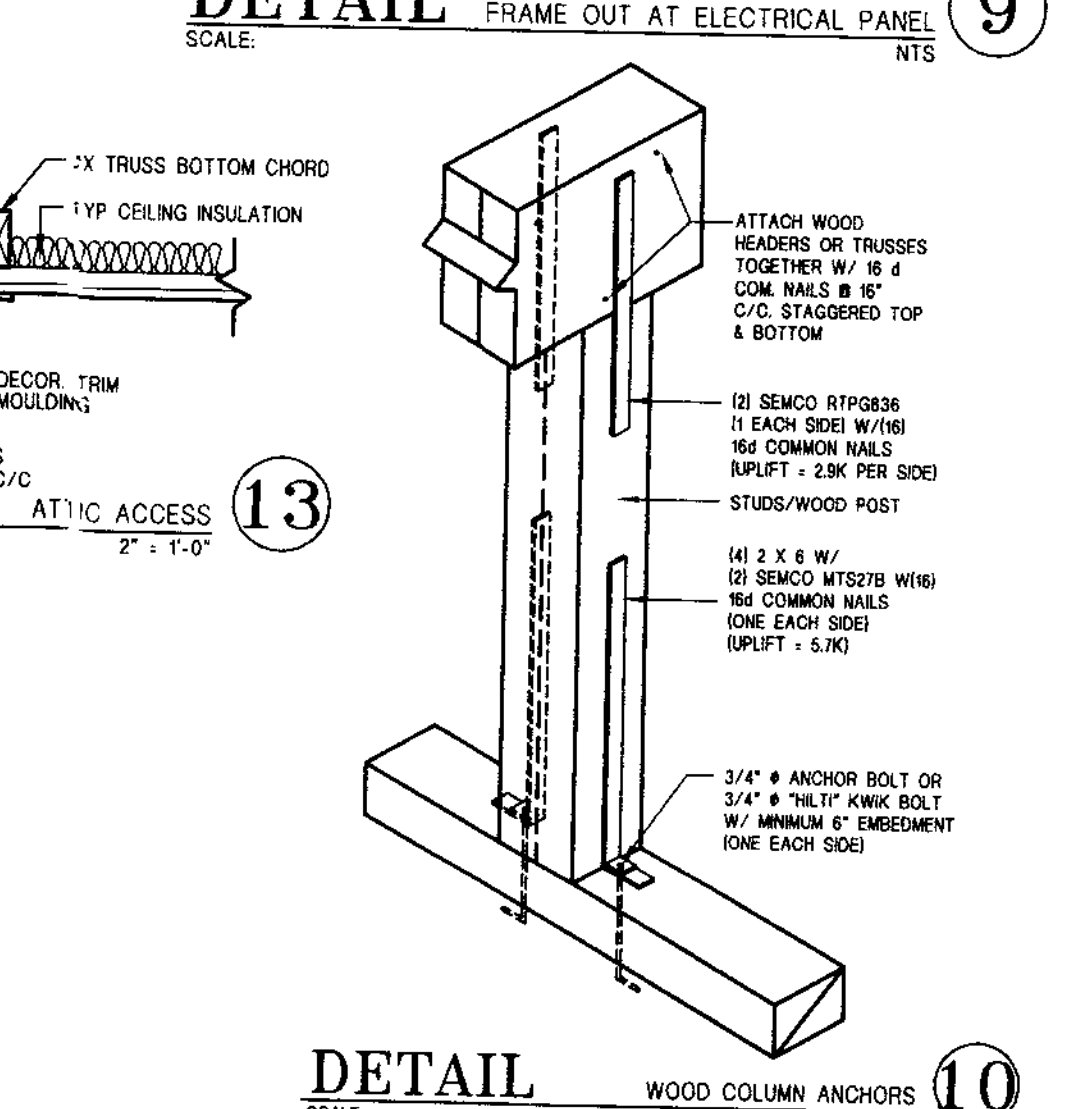
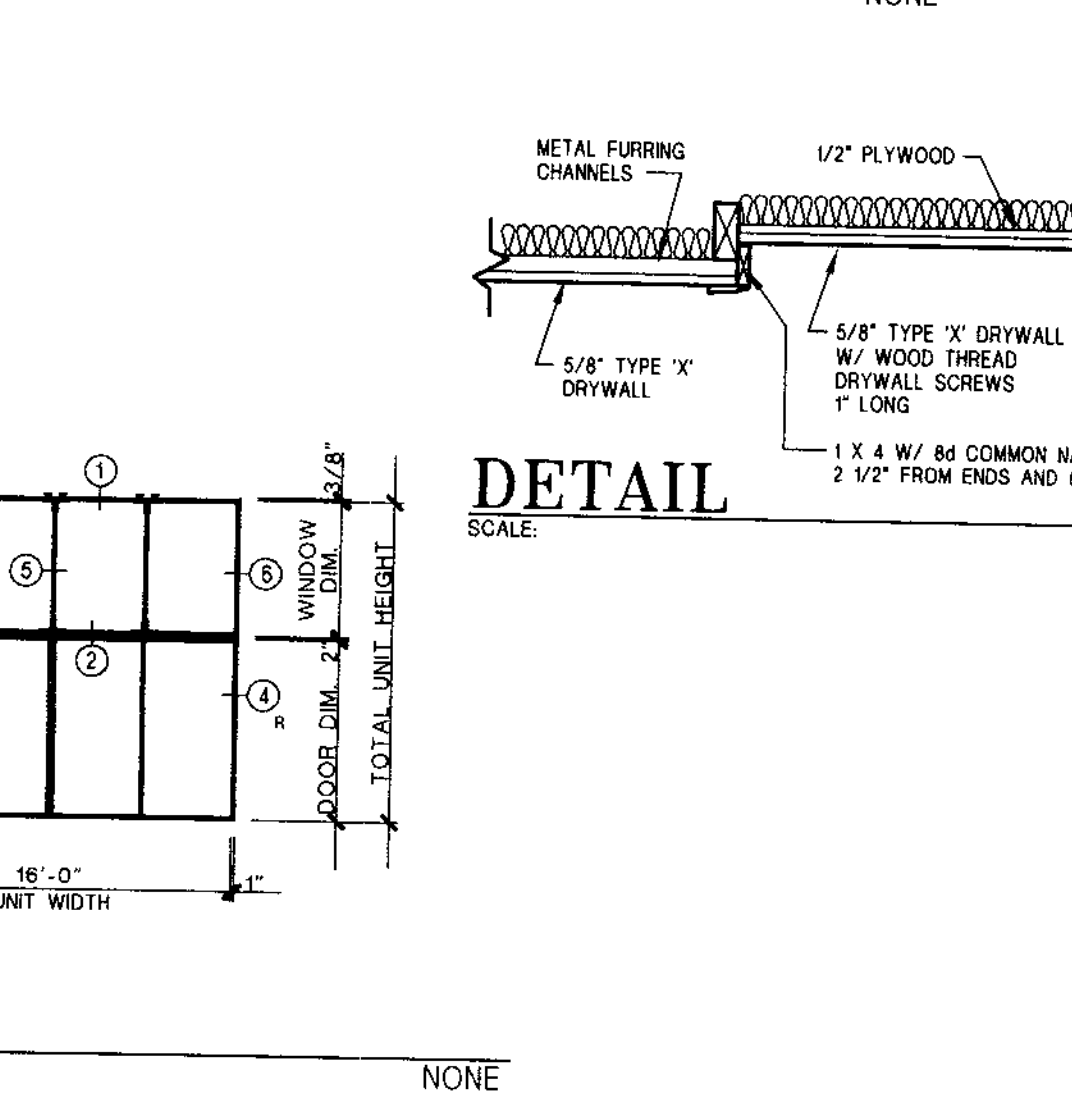
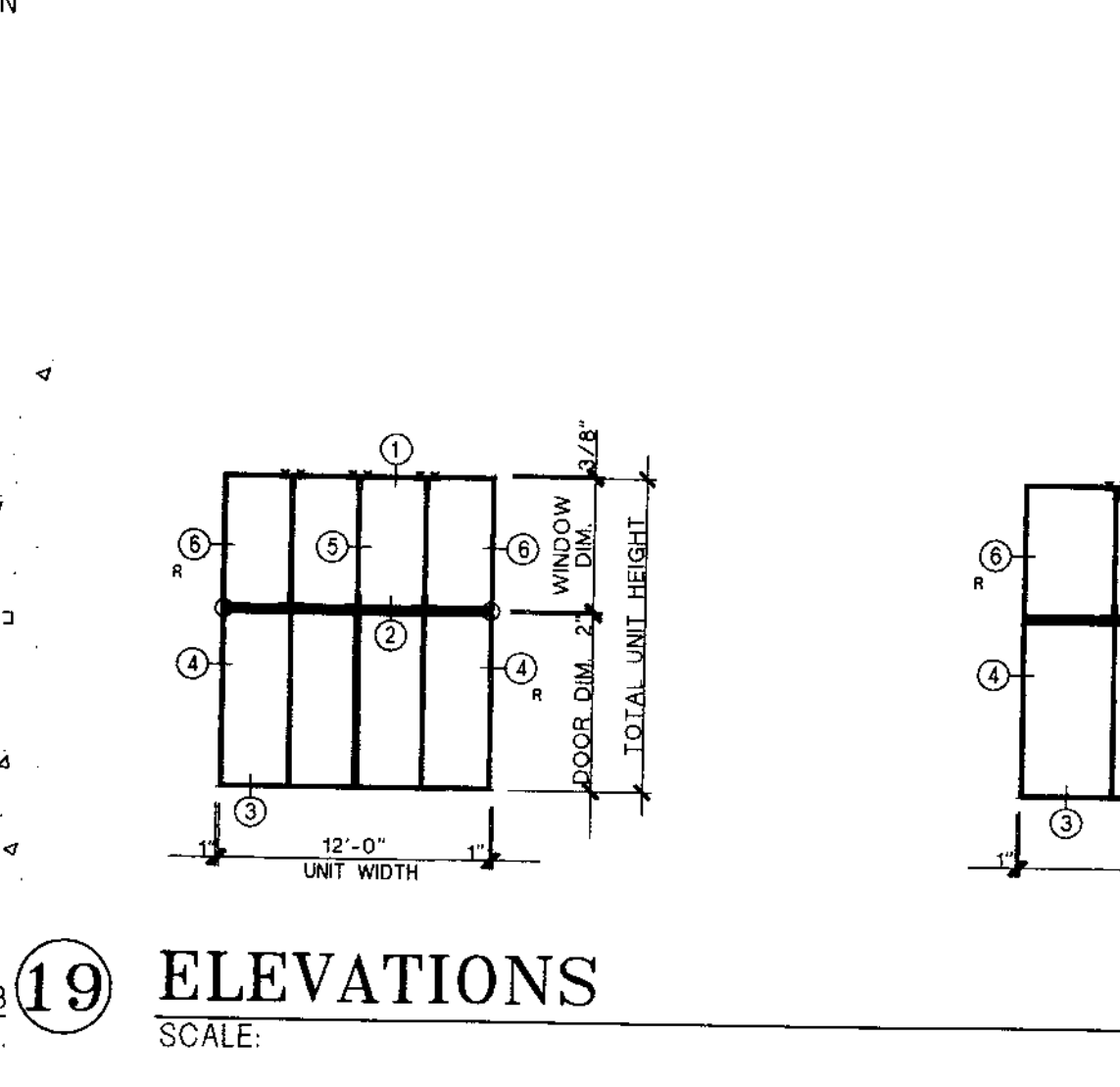
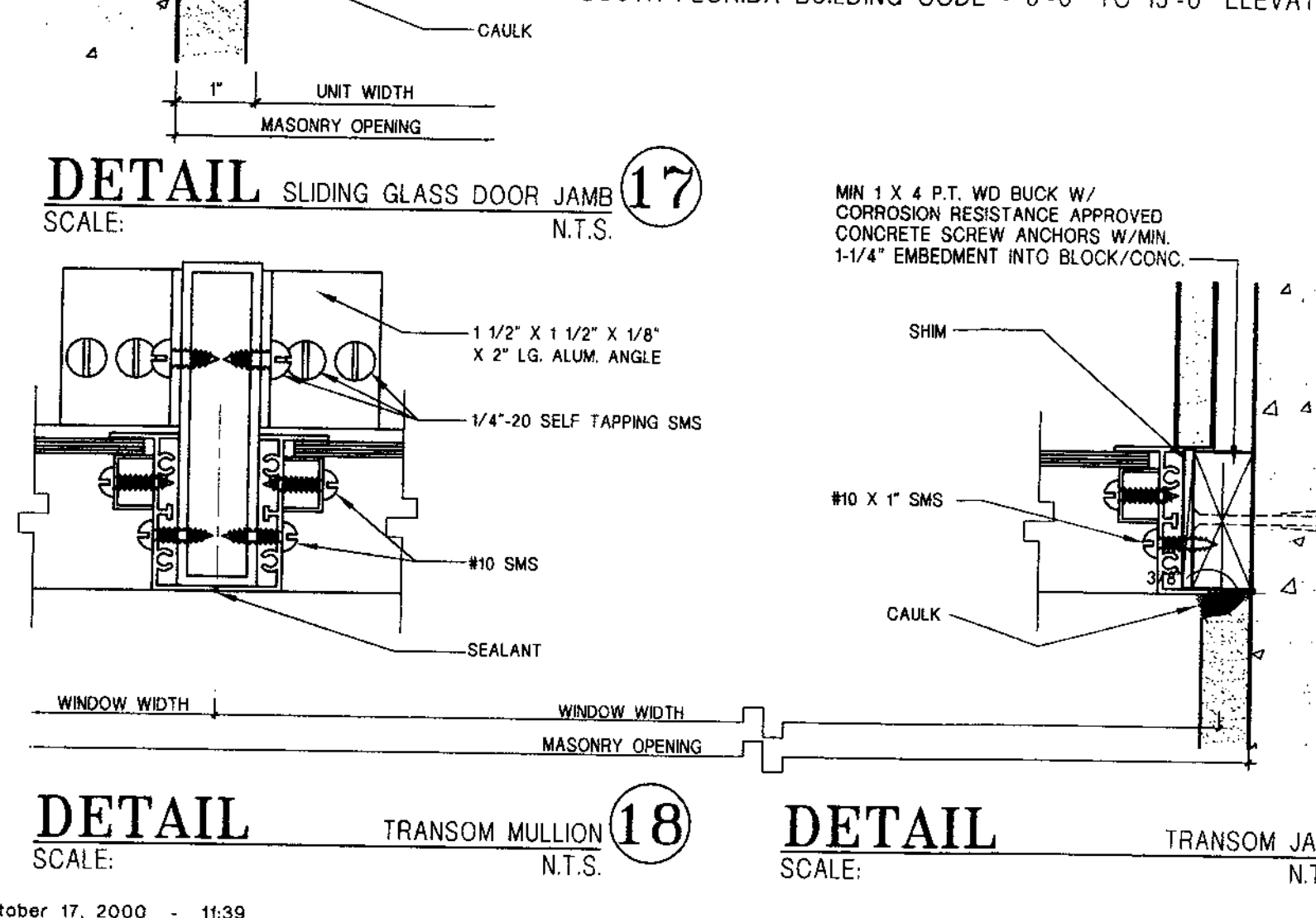
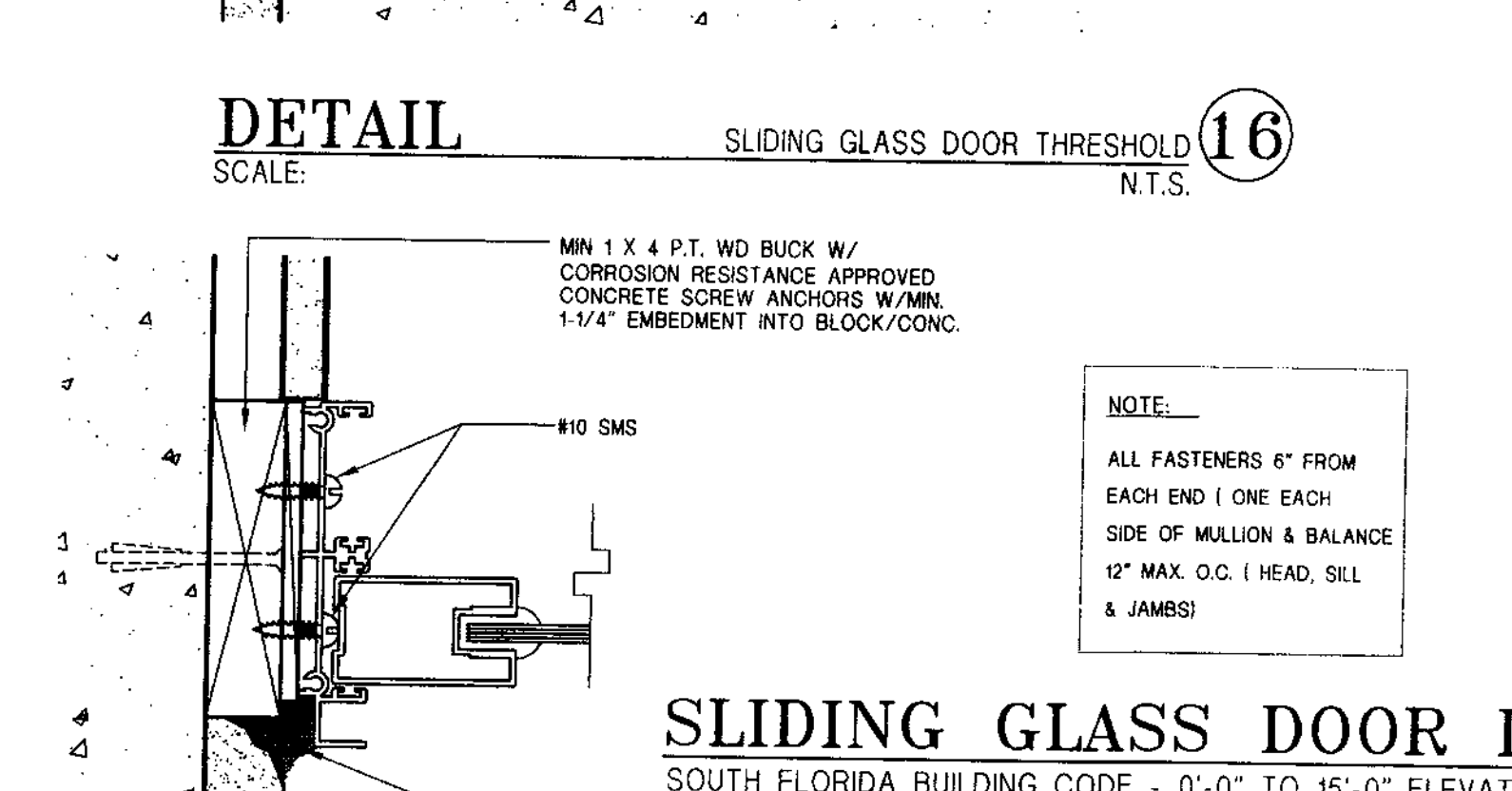
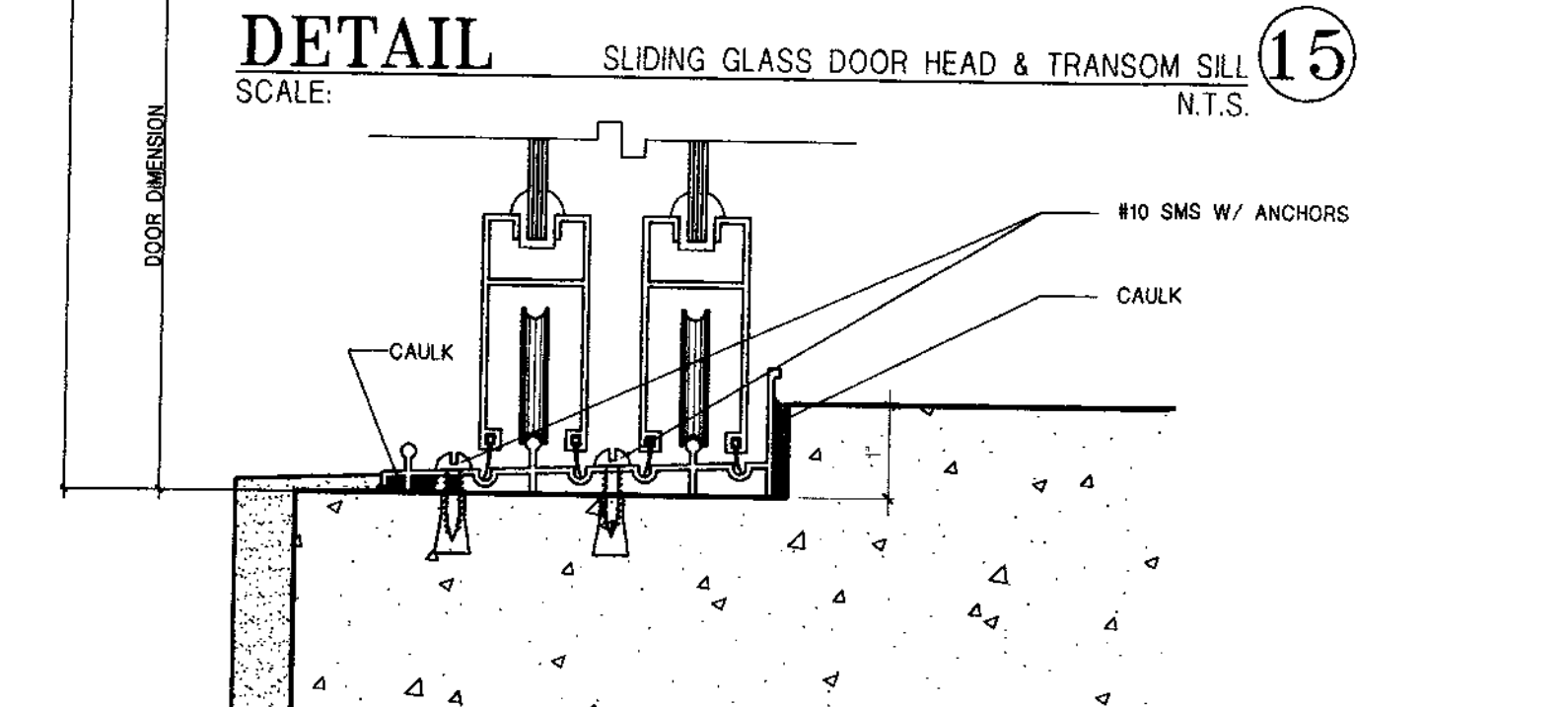
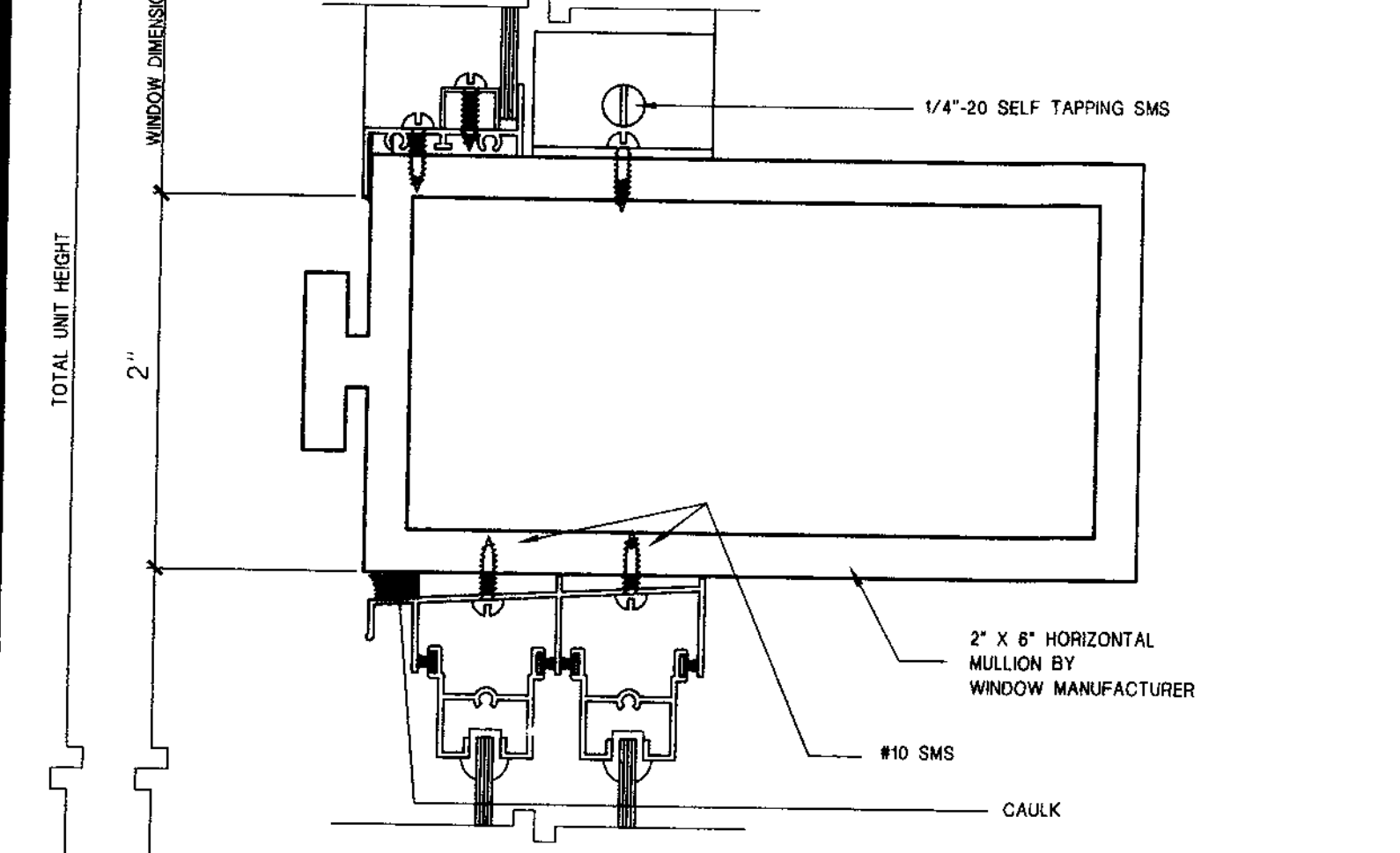
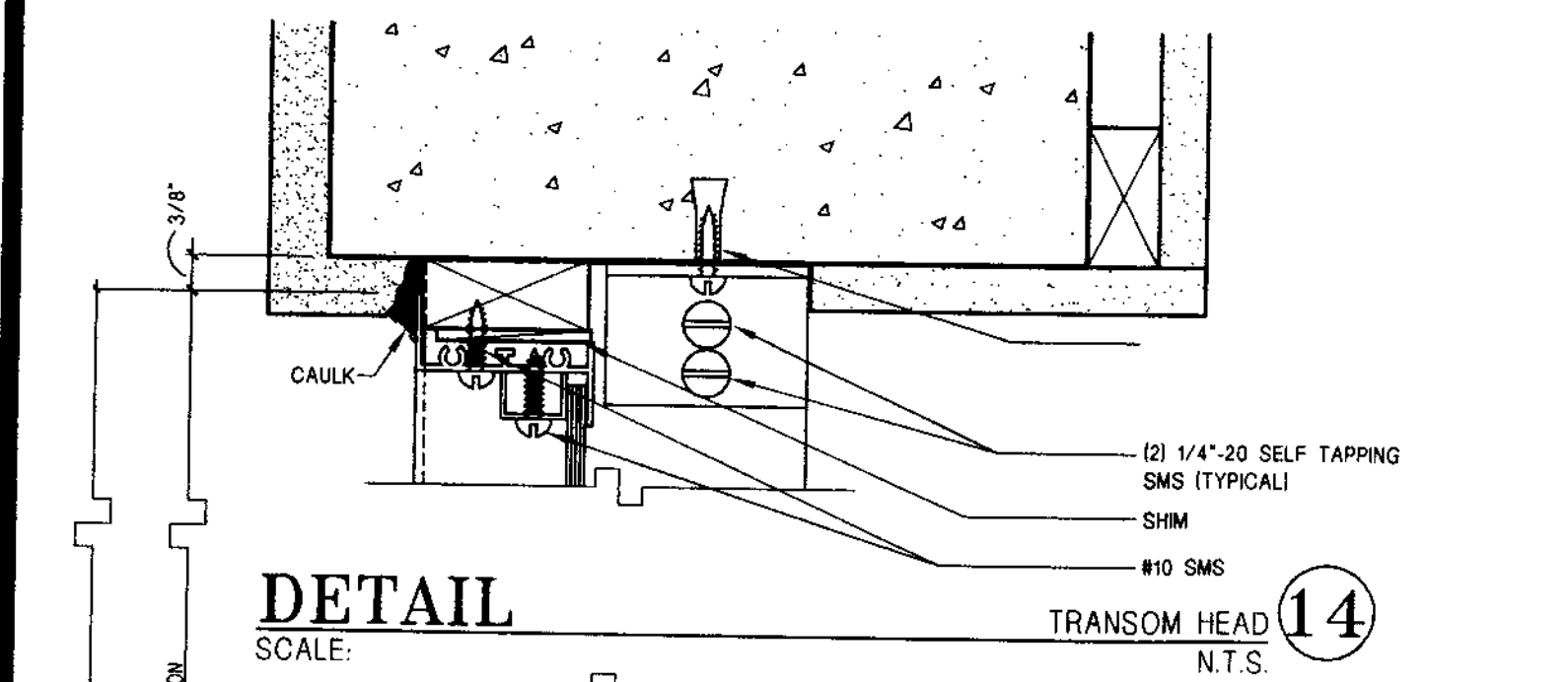
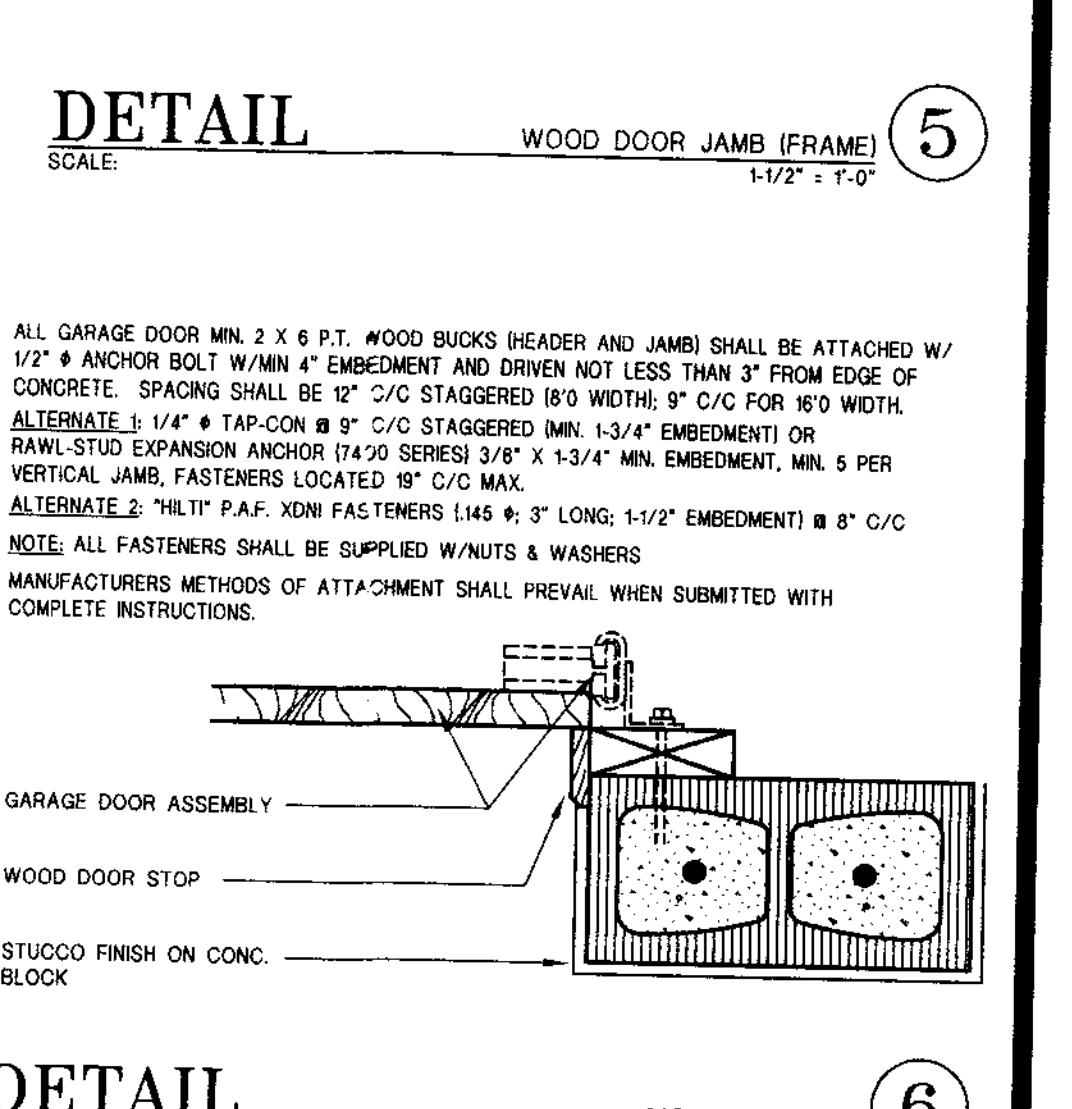
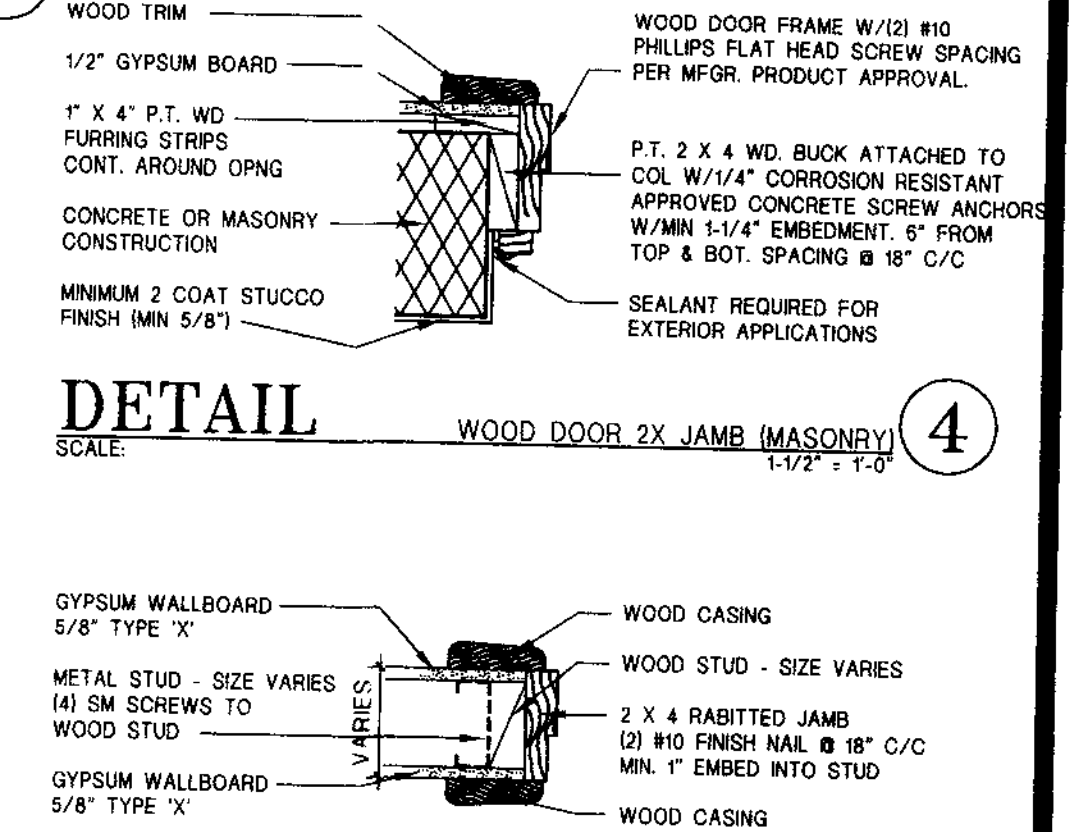
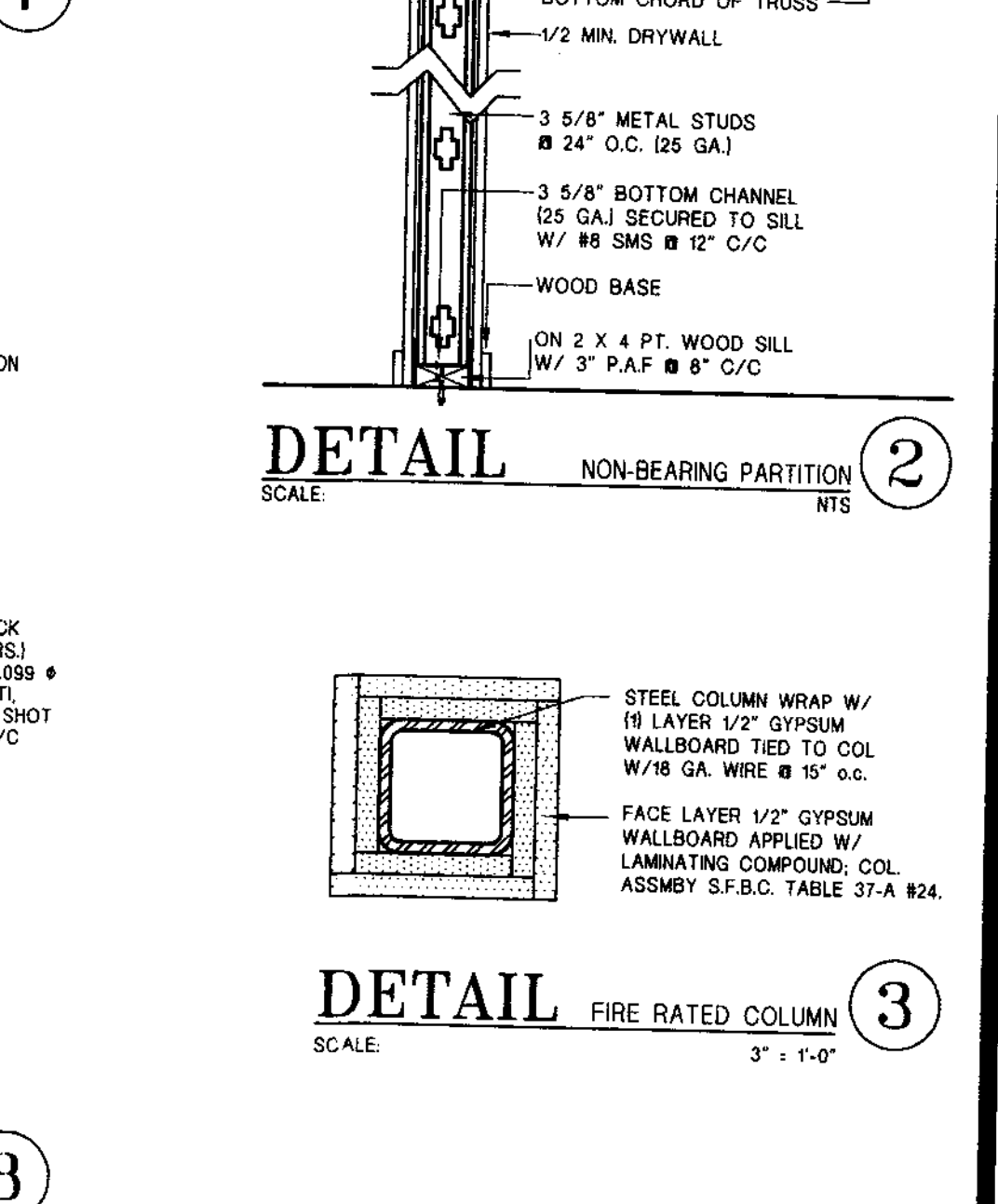
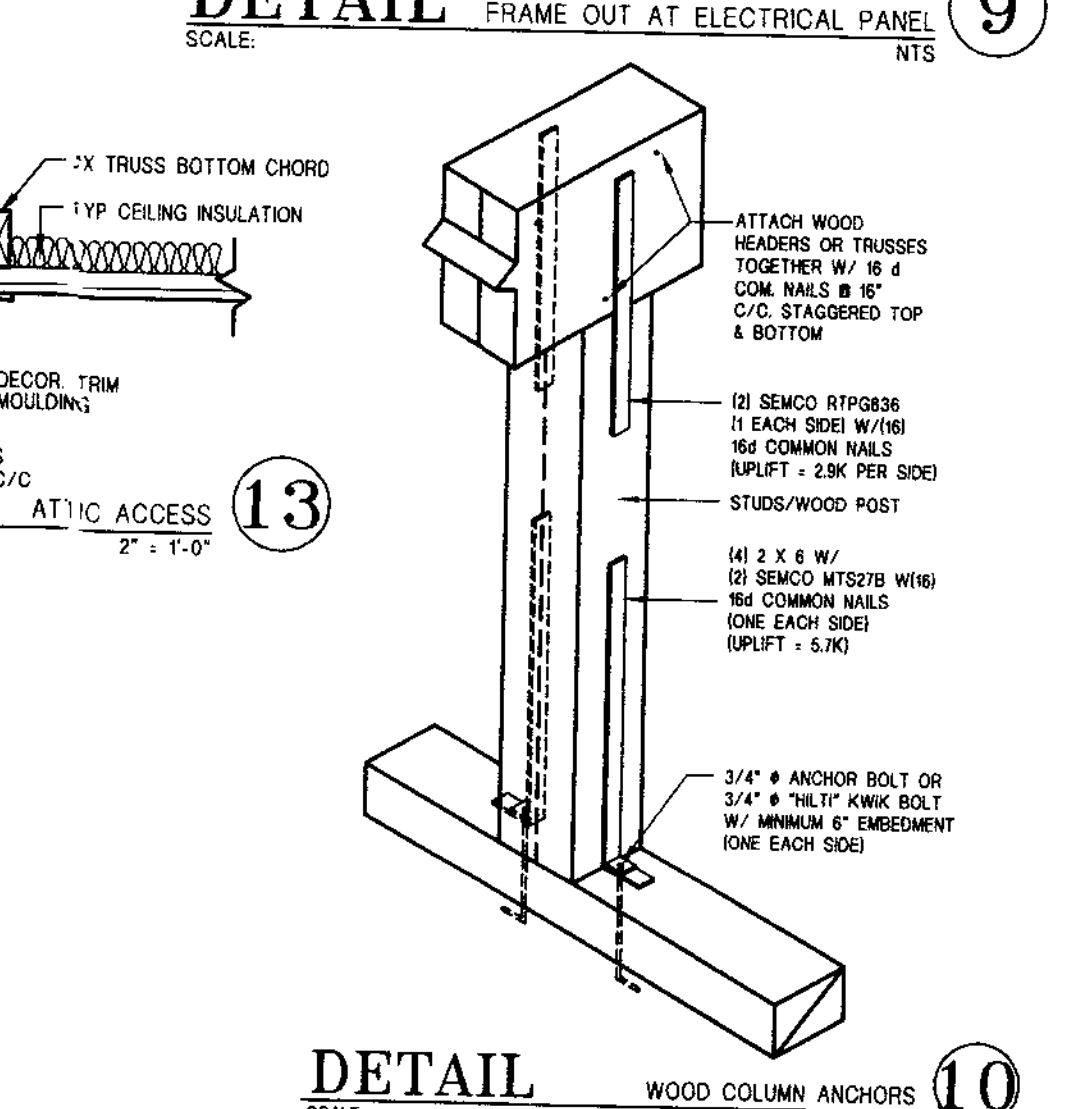
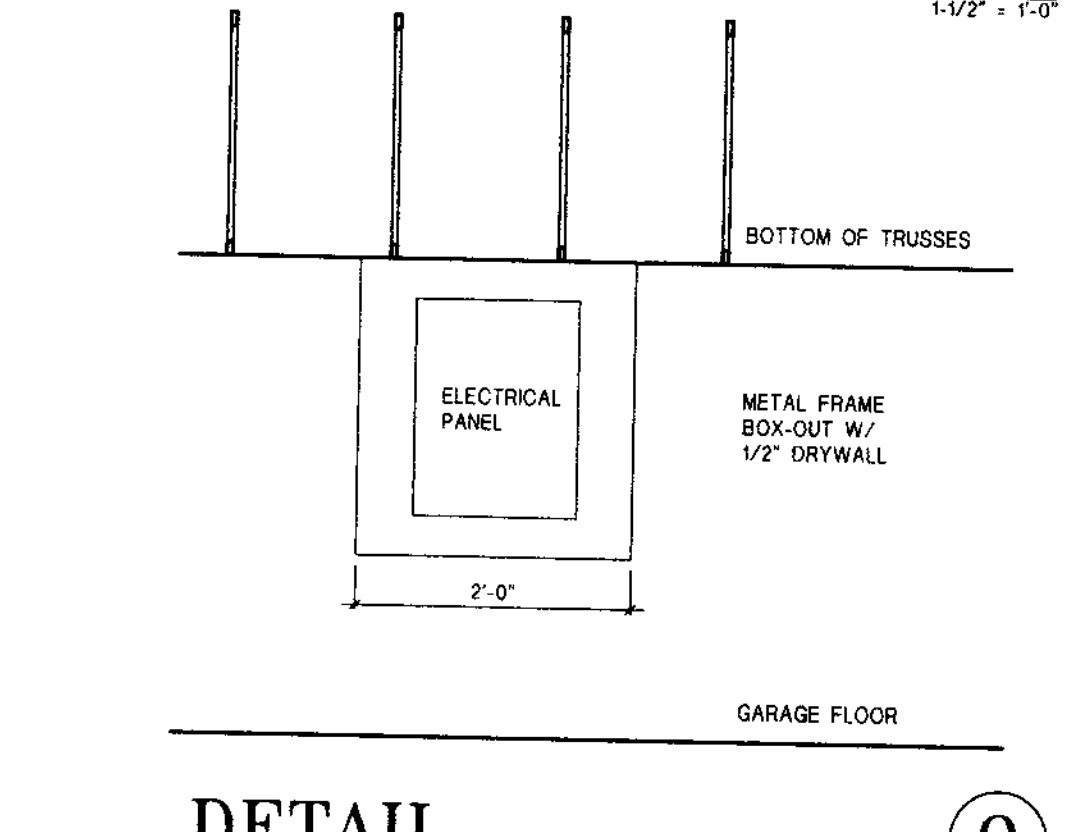
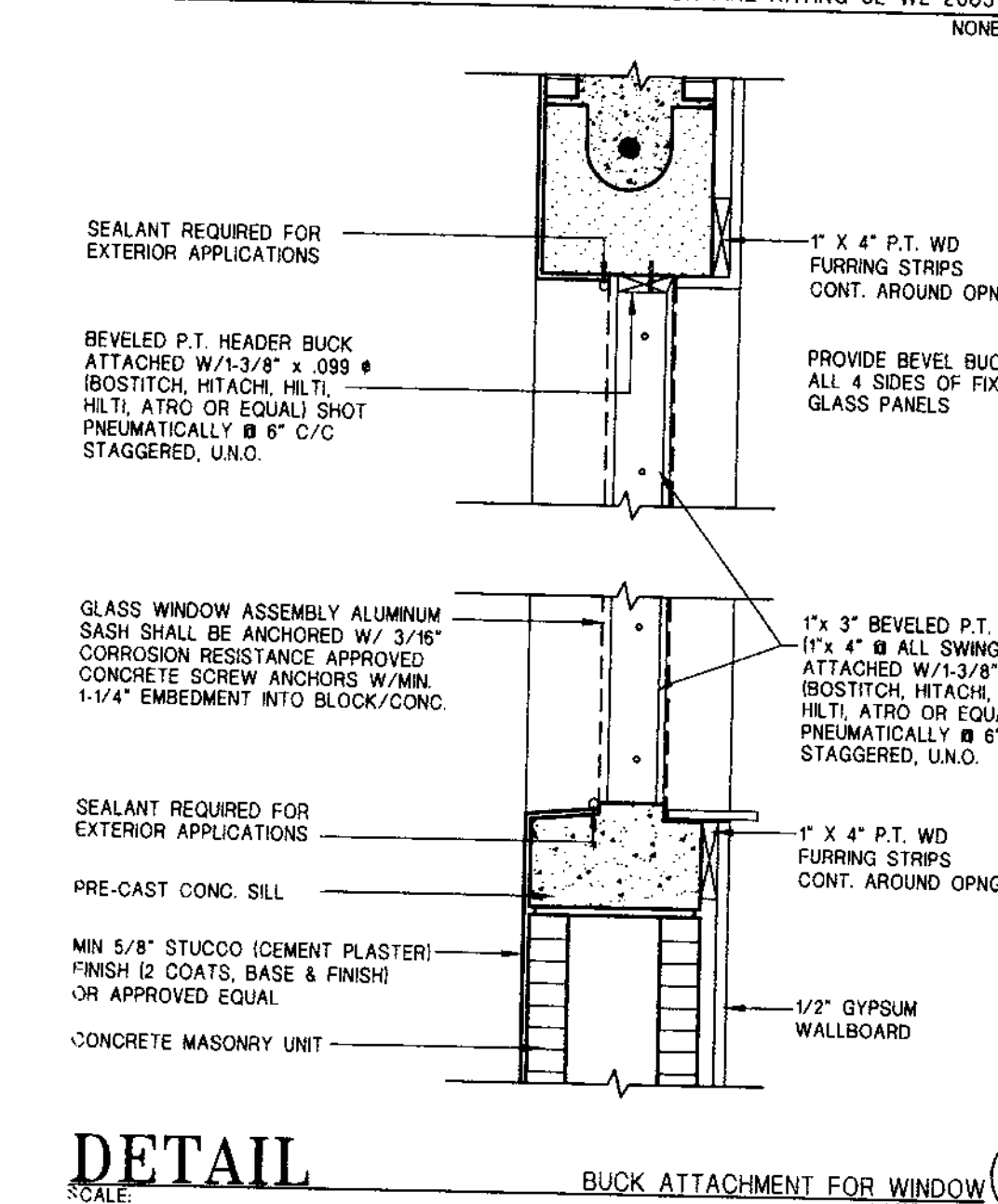
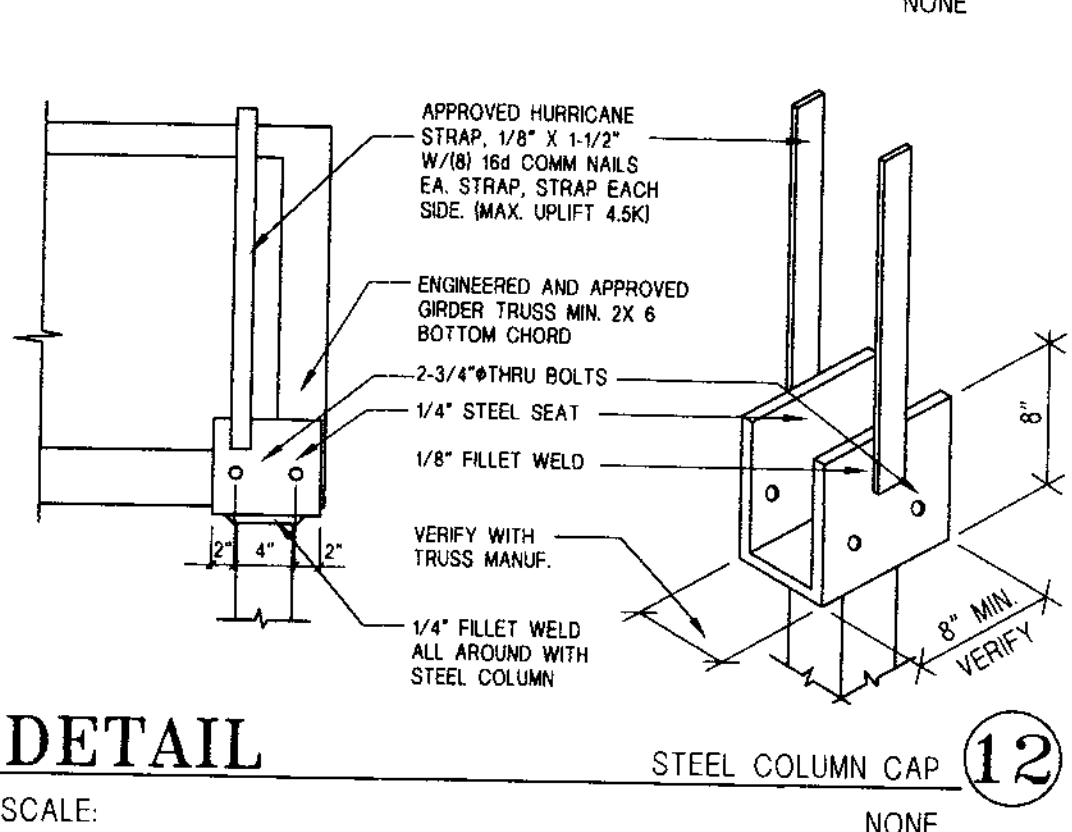
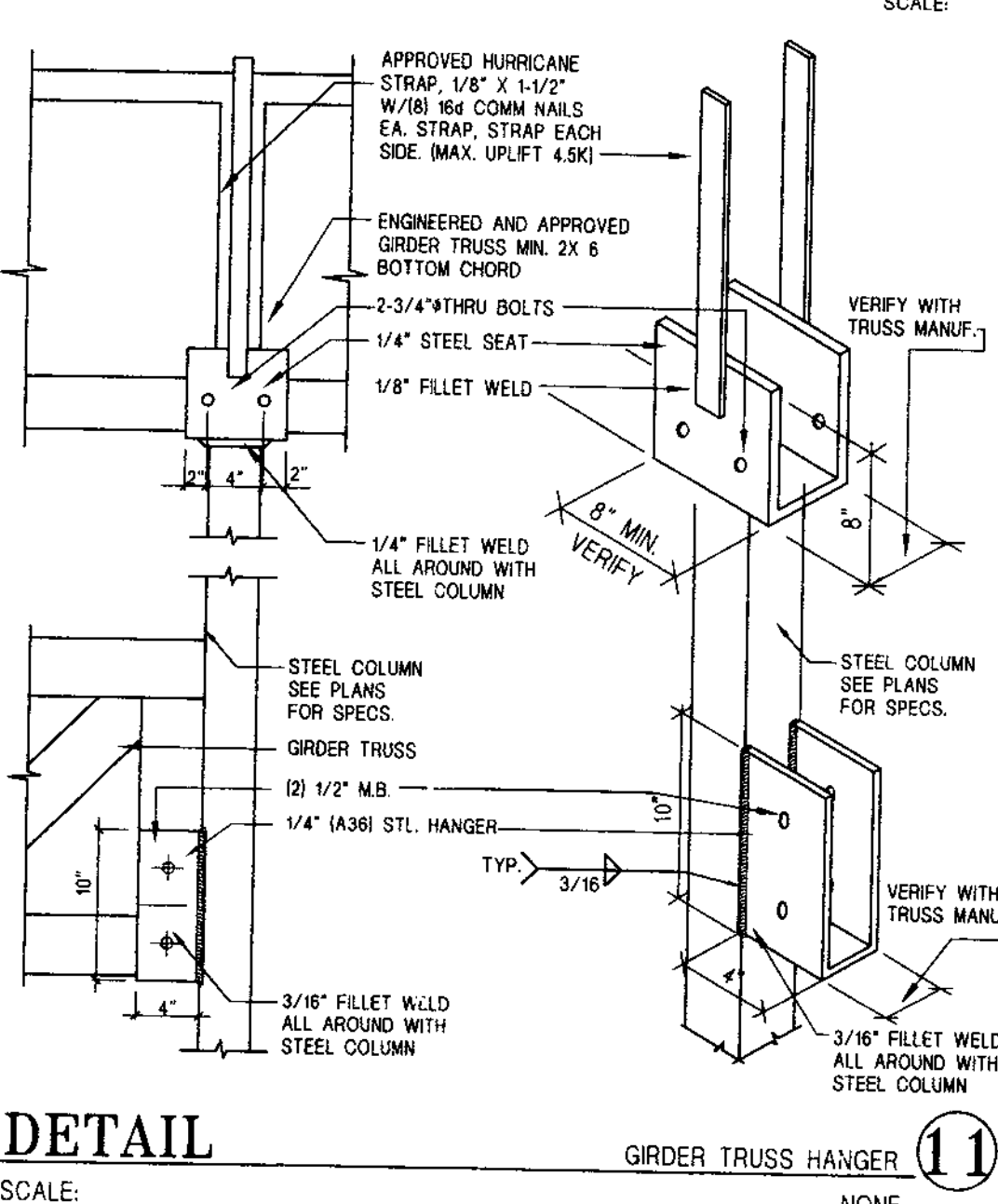
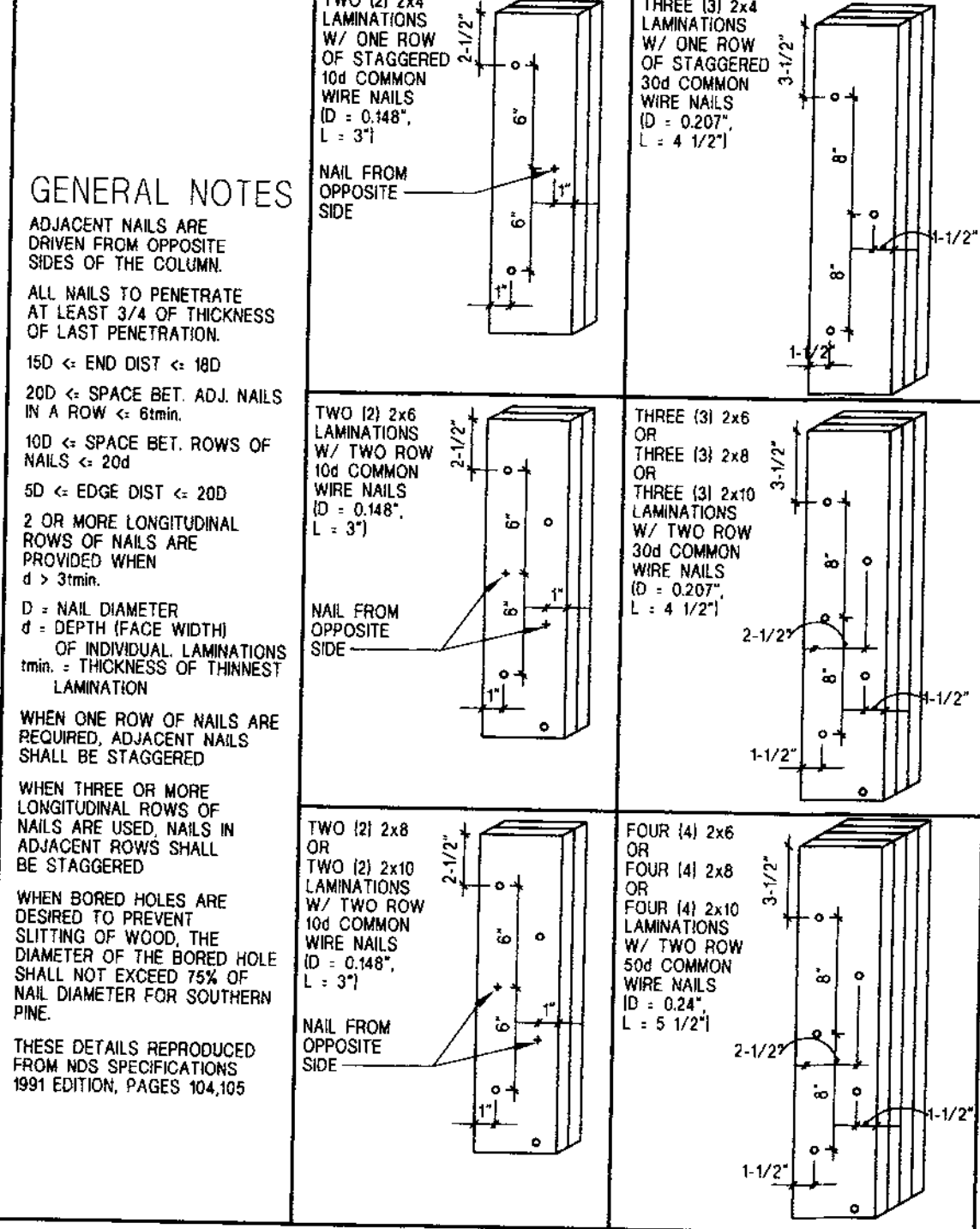
OCT 19 2000

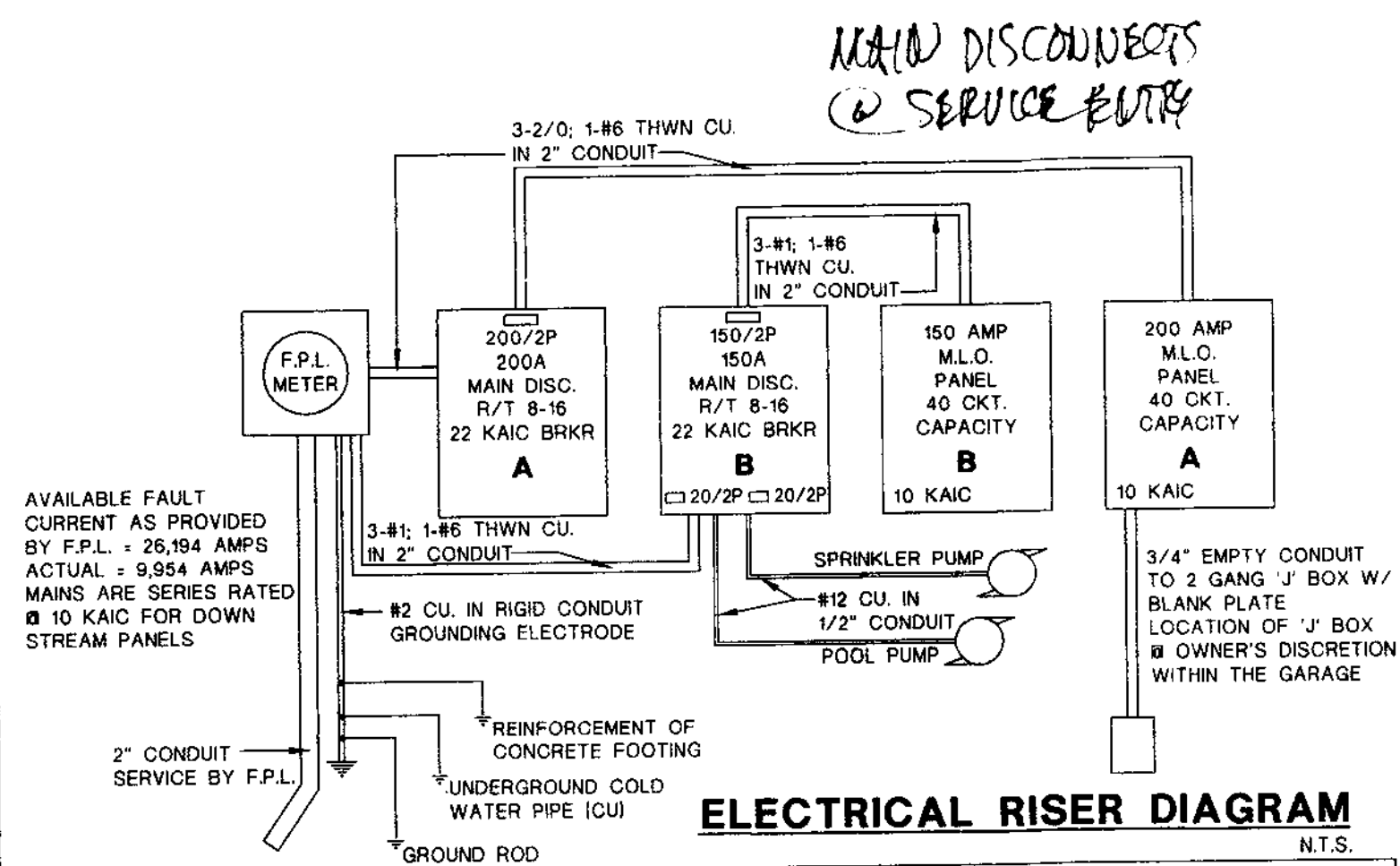
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PLYWOOD ROOF SHEAR DIAPHRAGM SCHEDULE						
LOCATION	PANEL GRADE	FRAMING/ TRUSS SPCG.	MIN. PANEL THICKNESS	NAIL SPACING @ DIAPH. BOUNDARIES	NAIL SPACING @ OTHER PNL. EDGES	COMMON NAIL SIZE
ROOF	APA RATED SHEATHING STRUCT. II, EXT. OR EQUAL	2'-0" MAXIMUM	19/32	4"	6"	8d
				REMARKS		
				UNBLOCKED		



NDS BUILT UP COLUMN NAILING SCHEDULE





ELECTRICAL RISER DIAGRAM

COMPUTED LOAD TABLE - B

CKT. NO.	CKTS.	QTY.	EQUIPMENT	BREAKER	COND.	LOAD
1-15	15		GENERAL LIGHTING	15A/P	14	14,508
(2)	2	1	POOL PUMP	20A/2P	12	1840
(2)	2	1	SPRINKLER PUMP	20A/2P	12	1840
16	1		POOL LIGHT	15A/P	14	100
37-40	4		SPACE			
SUB TOTAL :						(18,286)

FIRST 10,000 WATTS @ 100% = 10,000

REMAINDER 8,286 @ 40% = 3,314

SUB-TOTAL = 13,314 WATTS

TOTAL ESTIMATED CONNECTED LOAD = (13,314) (56) AMPS
240V

SPRINKLER SOLENOID MAY BE SUBSTITUTED FOR PUMP WITH 110V/50A/1 POLE BREAKER
WATTAGE IS LESS THAN SPECIFIED FOR PUMP.

PANEL LOAD CALCULATIONS BASED ON OPTIONAL CALCULATIONS FOR SINGLE FAMILY DWELLINGS (N.E.C. 220-31) 120/240V SINGLE PHASE 60 CYCLE. CONDUCTORS SIZED FOR THIN COPPER.

COMPUTED LOAD TABLE - A

CKT. NO.	CKTS.	QTY.	EQUIPMENT	BREAKER	COND.	LOAD
9,11	2	1	COOKTOP	40A/2P	8	7390
10,12	2	1	DRYER	30A/2P	10	5600
13,15	2	1	WATER HEATER	25A/2P	10	4500
14	1		MICROWAVE/VENT	20A/1P	12	1570
16,18	2	1	DISPOSAL	20A/2P	12	900
			DISHWASHER			1200
17	1		REFRIGERATOR	20A/1P	12	830
19	1		WASHER	20A/1P	12	1500
20-22	3	14	SMALL APPLIANCE	20A/1P	12	4500
23,24	1	7	BATHROOM GFCI'S	20A/1P	12	1610
25	1		GARAGE DOOR OPNR	15A/1P	14	11.6
26,28	2	1	DBL. OVEN	50A/2P	6	9000
37-40	4		SPACE			
SUB TOTAL :						(39,606)

FIRST 10,000 WATTS @ 100% = 10,000

REMAINDER 29,606 @ 40% = 11,802

SUB-TOTAL = 21,802 WATTS

AIR CONDITIONING LOAD TABLE

CKT. NO.	CKTS.	QTY.	EQUIPMENT	BREAKER	COND.	%	LOAD
1,3	2	1	ACCU #1	40A/2P	8	100	(5760)
5,7	2	1	AHU #1	60A/2P	6	100	10150
2,4	2	1	ACCU #2	40A/2P	8	100	(4848)
6,8	2	1	AHU #2	60A/2P	6	100	10150
SUB-TOTAL (LARGEST LOAD) :							(20,300)

TOTAL ESTIMATED CONNECTED LOAD = (42,102) (176) AMPS
240V

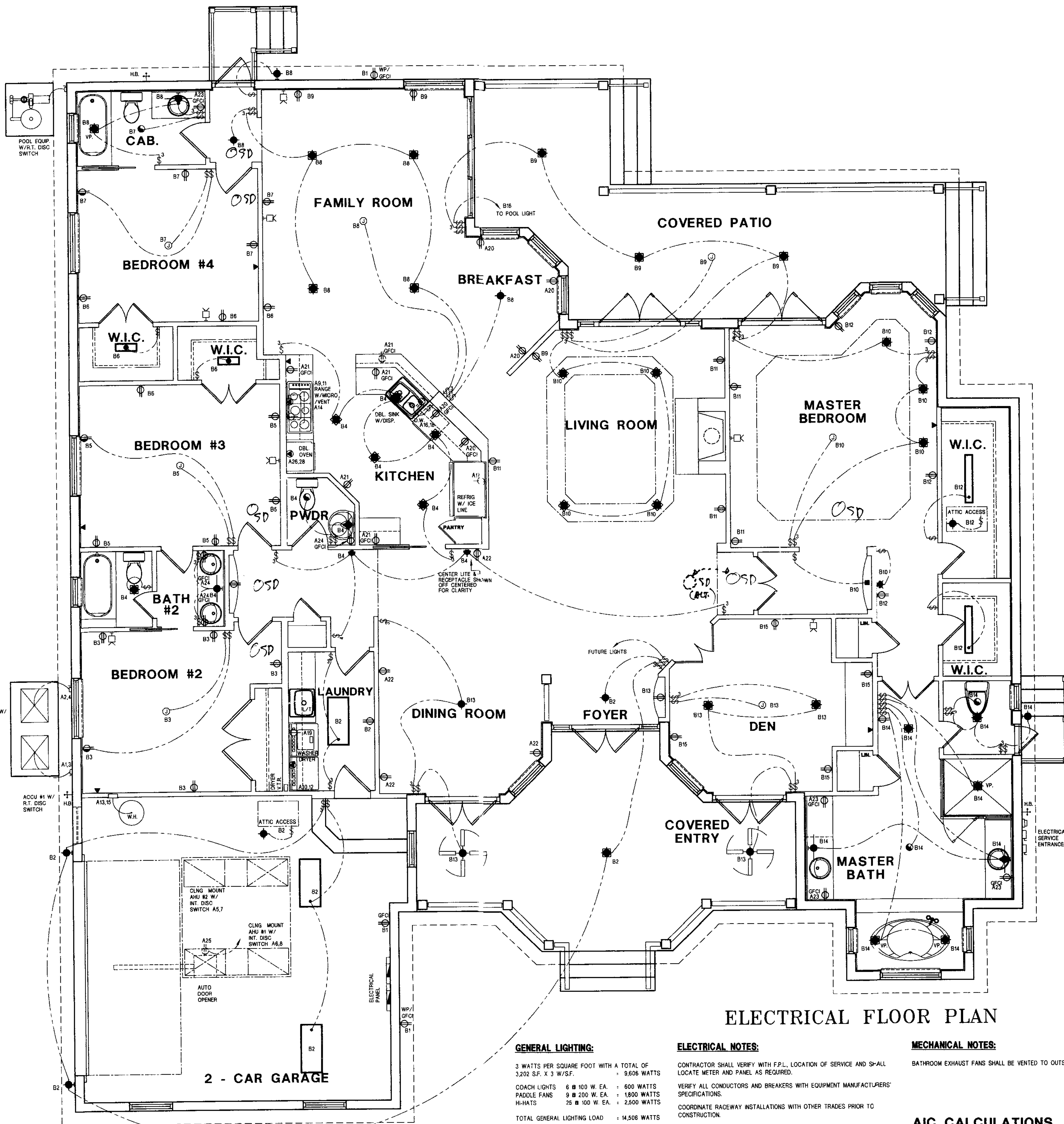
GENERAL LIGHTING LEGEND

CIR. #	RECEPT	LGTS	FANS	PTS	LOCATION
B1	3	-	-	6	EXT. GFCI
B2	-	8	-	10	FOYER, ENTRY, COACH LIGHTS, GARAGE, ATTIC & LAUNDRY
B3	5	-	1	11	BEDROOM #2
B4	-	10	1	11	DINING, BATHROOM #2, HALL, POWDER, KITCHEN
B5	5	-	1	11	BEDROOM #3
B6	4	2	-	10	BEDROOM #2 & #3, FAMILY ROOM & W.I.C.'S
B7	4	-	2	10	BEDROOM #4, CABANA E.F.
B8	-	9	1	10	BREAKFAST, FAMILY ROOM, CABANA LIGHTS, COACH LIGHT
B9	3	3	1	10	FAMILY ROOM, PATIO & LIVING ROOM
B10	-	9	1	10	LIVING ROOM, MASTER BEDROOM, HALL
B11	5	-	-	10	LIVING ROOM, MASTER BEDROOM
B12	4	3	-	11	MASTER BEDROOM, HALL & W.I.C.'S
B13	1	5	3	10	PATIO FANS, FOYER DEN, DINING
B14	1	8	2	12	MASTER BATH
B15	4	-	-	8	DEN

ELECTRICAL SYMBOL LEGEND

SYMBOL	DESCRIPTION
FLUORESCENT TUBE STRIP	
CEILING MOUNTED INCANDESCENT	
WALL MOUNTED INCANDESCENT	
WALL MTD VANTY INCANDESCENT	
CLOSET HEADER LIGHT	
WALL SCONCE	
W-HAT INCANDESCENT W-W INDICATES WALL WASHER	
H-HAT / EXHAUST FAN	
HEAT LAMP / EXHAUST FAN	
EXHAUST FAN	
S.D. SMOKE DETECTOR 10V	
1/2" BOX OPTIONAL W/ FAN	
DUPLEX RECEPTACLE 120V - 12" AFF UNLESS NOTED OTHERWISE	
DUPLEX RECEPTACLE 120V - HEIGHT NOTED AFF	
DUPLEX RECEPTACLE - UNDER CABINET	
FLOOR OUTLET	
SINGLE OUTLET	
SINGLE OUTLET - UNDER CABINET	
RECEPTACLE W/ GROUND FAULT CIRCUIT INTERRUPTER WP-WATERPROOF	
DUPLEX RECEPTACLE W/ TOP 1/2 SWITCH ACTIVATED	
QUAD PLEX RECEPTACLE	
GROUNDING 240V. RECEPTACLE	
GROUNDING 120V. RECEPTACLE	
TELEPHONE OUTLET	
TELEVISION OUTLET	
PUSH BUTTON	
SINGLE SWITCH @ 48" AFF	
MULTIPLE SWITCH @ 48" AFF	
2 X 4 FLUORESCENT FIXTURE W/2 4" TUBES (100W)	
1 X 4 FLUORESCENT FIXTURE W/1 4" TUBES (100W)	
1 X 2 FLUORESCENT FIXTURE W/1 2" TUBES (100W)	
SPEAKER	
FLOOD LIGHT	
MOTION SENSOR	
GLASS BREAK SENSOR	
CEILING FAN SHADING INDICATES LIGHT	

ALL SYMBOLS MAY NOT BE APPLICABLE



ELECTRICAL FLOOR PLAN

GENERAL LIGHTING:

3 WATTS PER SQUARE FOOT WITH A TOTAL OF 3,202 S.F. X 3 W/S.F. = 9,606 WATTS

COACH LIGHTS 6 @ 100 W. EA. = 600 WATTS
PADDLE FANS 9 @ 200 W. EA. = 1,800 WATTS
H-HATS 25 @ 100 W. EA. = 2,500 WATTS

TOTAL GENERAL LIGHTING LOAD = 14,906 WATTS

COACH LIGHTS 6 X 1 = 6 PTS
H-HATS 25 X 1 = 25 PTS
RECEPTACLES 40 X 2 = 80 PTS
LIGHTS 28 X 1 = 28 PTS
FAN/HEAT 11 X 2 = 22 PTS

TOTAL CIRCUIT POINTS = 181 PTS

TOTAL GEN. LGT. CIRCUITS = 14 CIRCUITS

ELECTRICAL NOTES:

CONTRACTOR SHALL VERIFY WITH F.P.L. LOCATION OF SERVICE AND SP-ALL LOCATE METER AND PANEL AS REQUIRED.

VERIFY ALL CONDUCTORS AND BREAKERS WITH EQUIPMENT MANUFACTURERS' SPECIFICATIONS.

COORDINATE RACEWAY INSTALLATIONS WITH OTHER TRADES PRIOR TO CONSTRUCTION.

ALL BRANCH CIRCUITS IN RACEWAY OR NON-METALLIC SHEATHED CABLE.

SD-SMOKE DETECTOR PER S.F.B.C. 312.21(H) ALL DETECTOR INSTALLATIONS IN GROUP H & I OCCUPANCY SHALL BE HARD WIRED TO THE ELECTRICAL DISTRIBUTION SYSTEM AND ALL SUCH DETECTORS SHALL HAVE BATTERY BACKUP. ALL SUCH DETECTORS IN NEW CONSTRUCTION SHALL BE INTERCONNECTED WITHIN INDIVIDUAL LIVING UNITS TO KITCHEN OR BATHROOM LIGHTING CIRCUIT AHEAD OF SWITCH.

MECHANICAL NOTES:

BATHROOM EXHAUST FANS SHALL BE VENTED TO OUTSIDE

AIC CALCULATIONS

F = $\frac{2 \times 47 \times 26,194}{6250 \times 240} = 2,462,236 \div 1,500,000 = 1.64$

M = $\frac{1.64}{1.64} = 2.84 \div 0.38$

AIC = $26,194 \times 0.38 = 9,954$ AMPS

THE CAPTIVA

LOT 22

FOR: A NEW CUSTOM RESIDENCE DR. & MRS. ROBERT CUSHING

BY: FOGLIA CUSTOM HOMES

SEWALL'S MEADOWS MARTIN COUNTY, FLORIDA

HOMES BY ARCHITECT INC.

SEWALL'S POINT, 7430 WILES ROAD, CORAL SPRINGS, FLORIDA 33067

PHONE: (954) 755-0690 FAX: (954) 755-8601

Architect's Plans/Specs/Consultants

Designed by: TRAY

Drawn by: M.J.D.

Checked by: _____

Date: OCTOBER 11, 2000

Scale: 1/4" = 1'-0"

Project #: 00284

Sheet 8

8608

SHUTTERS

TOWN OF SEWALL'S POINT

Date 5-24-07 BUILDING PERMIT NO. 8608
 Building to be erected for Cushing Type of Permit Hurricane
 Applied for by Coastal Shutters (Contractor) Building Fee 120
 Subdivision Sewalls Meadow 22 Block _____ Radon Fee _____
 Address 102 Abbie Ct Impact Fee _____
 Type of structure SFR A/C Fee _____
 Electrical Fee _____
 Plumbing Fee _____
 Roofing Fee _____
 Parcel Control Number: 133841-013-000-00220-
 Amount Paid \$120- Check # 6638 Cash _____ Other Fees () _____
 Total Construction Cost \$ 18665- TOTAL Fees 120-

Signed [Signature]
 Applicant

Signed John Adams
 Town Building Official

PERMIT

☐ BUILDING	☐ ELECTRICAL	☐ MECHANICAL
☐ PLUMBING	☐ ROOFING	☐ POOL/SPA/DECK
☐ DOCK/BOAT LIFT	☐ DEMOLITION	☐ FENCE
☐ SCREEN ENCLOSURE	☐ TEMPORARY STRUCTURE	☐ GAS
☐ FILL	☒ HURRICANE SHUTTERS	☐ RENOVATION
☐ TREE REMOVAL	☐ STEMWALL	☐ ADDITION

INSPECTIONS

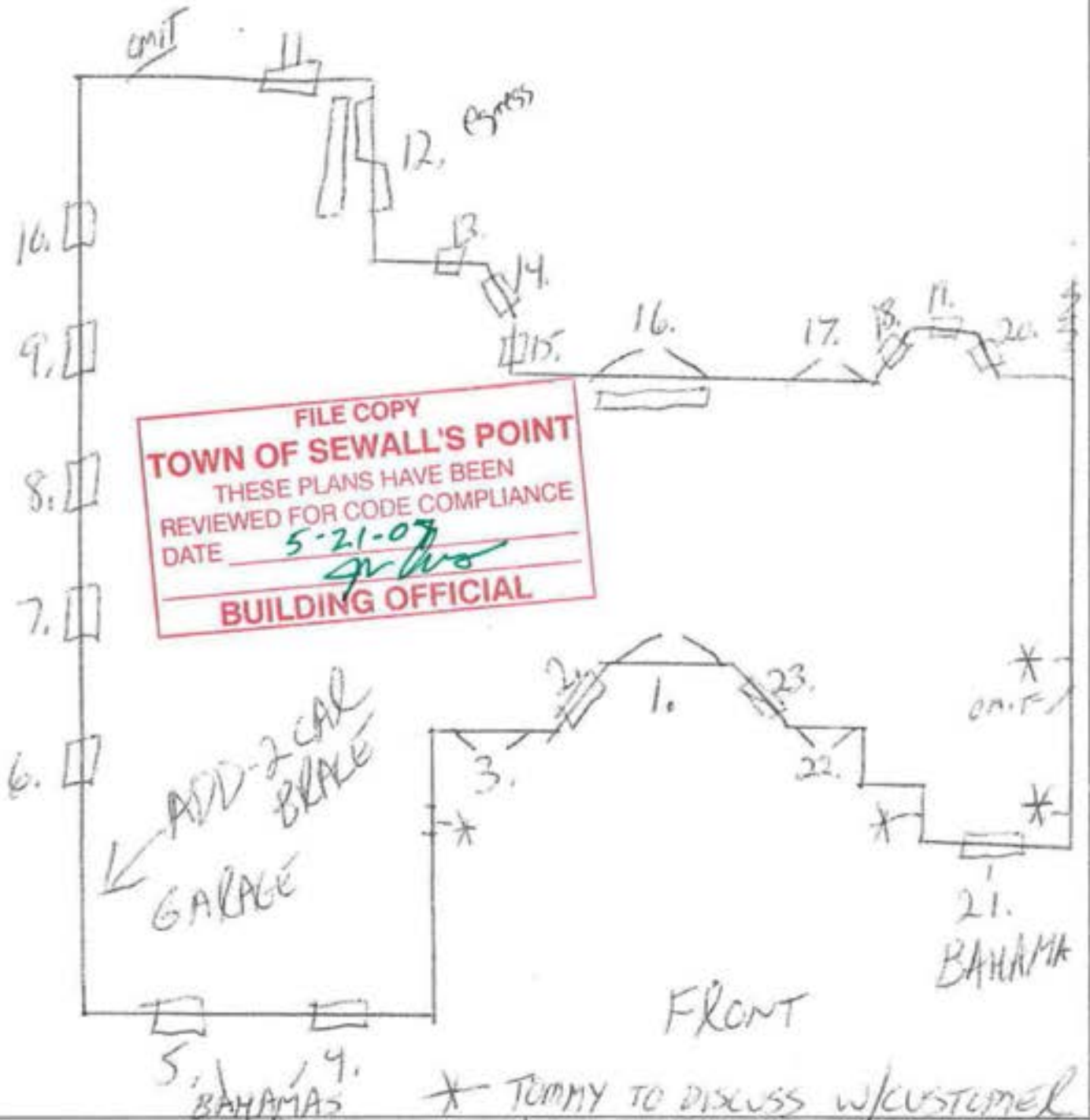
UNDERGROUND PLUMBING	_____	UNDERGROUND GAS	_____
UNDERGROUND MECHANICAL	_____	UNDERGROUND ELECTRICAL	_____
STEMWALL FOOTING	_____	FOOTING	_____
SLAB	_____	TIE BEAM/COLUMNS	_____
ROOF SHEATHING	_____	WALL SHEATHING	_____
TRUSS ENG/WINDOW/DOOR BUCKS	_____	LATH	_____
ROOF TIN TAG/METAL	_____	ROOF-IN-PROGRESS	_____
PLUMBING ROUGH-IN	_____	ELECTRICAL ROUGH-IN	_____
MECHANICAL ROUGH-IN	_____	GAS ROUGH-IN	_____
FRAMING	_____	EARLY POWER RELEASE	_____
FINAL PLUMBING	_____	FINAL ELECTRICAL	_____
FINAL MECHANICAL	_____	FINAL GAS	_____
FINAL ROOF	_____	BUILDING FINAL	_____

DR. CUSHING
CUSTOMER'S NAME

WHITE ACCORDION, WHITE BAHAMAS
PRODUCT LOW-RED-ADJUSTABLE

LAYOUT SHEET

SIZES BELOW ARE APPROXIMATE FOR ESTIMATING PURPOSES ONLY



During installation we must have access in and out of property for good installation. Owner, or someone with authority, must be present during installation to inspect and pay balance due.

I ACKNOWLEDGE AND UNDERSTAND THE ABOVE

[Signature]
CUSTOMER'S SIGNATURE

THIS ORDER IS BEING CUSTOM MADE TO YOUR SPECIFICATIONS AND IS NOT SUBJECT TO CANCELLATION.

SHUTTER SCHEDULE

	APPOX	APPOX	APPOX	#OF		#OF	HEADER	
I.D.	OPENING	SHUTTER	SHUTTER	STORM	ANCHOR	WINDOW BARS	REINF. REQ'D	REMARKS
NO.	SIZE(WXH)	WIDTH	HEIGHT	BARS REQ'D	SPACING	EACH SIDE	YES/NO	
	37"X63"	45"	71"	N/A	12"	N/A	NO	EXAMPLE
1	82 x 98	92 1/2	102 1/2	1/1	6	1/1	NO	
2	40 x 64	46	72					
3	64 x 99	72	103 1/2					
4	40 x 68	42	70					
5	40 x 68	42	70					
6	58 x 65	65	73					
7	28 x 40	33	47 1/2					
8	58 x 65	65	73					
9	58 x 65	65	73					
10	28 x 40	33	47 1/2					
11	58 x 98	65 1/2	105 1/2					
12	110 x 130	121 1/4	134					
13	34 x 98	39 1/2	105 1/2					
14	34 x 98	39 1/2	105 1/2					
15	34 x 98	39 1/2	105 1/2					
16	110 x 130	121 1/4	134					
17	70 x 98	78 1/2	102					
18	28 x 65	33	73					
19	28 x 65	33	73					
20	28 x 65	33	73					
21	56 x 68	59	70					
22	64 x 99	72	103 1/2	✓	✓	✓	✓	
23	40 x 64	46	72	✓	✓	✓	✓	
24								
25								
26								
27								
28								
29								
30								

FLORIDA DEPARTMENT OF Community Affairs



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Product Approval

USER: Public User

[Product Approval Menu](#) >
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 Application Detail

FL #	FL389-R1
Application Type	Revision
Code Version	2004
Application Status	Approved
Comments	
Archived	☐
Product Manufacturer	American Shutter Systems Association, Inc.
Address/Phone/Email	4268 Westroads Drive West Palm Beach, FL 33407 (561) 841-5480 bfeeley@easternmetal.com
Authorized Signature	Bill Feeley bfeeley@easternmetal.com
Technical Representative	
Address/Phone/Email	
Quality Assurance Representative	

Address/Phone/Email

Category

Shutters

Subcategory

Accordion

Compliance Method

Evaluation Report from a Florida Registered Architect or a Licensed Florida Professional Engineer

☐ Evaluation Report - Hardcopy Received

Florida Engineer or Architect Name who developed the Evaluation Report

Walter A., Tillit, Jr. P.E.

Florida License

PE-44167

Quality Assurance Entity

National Accreditation and Management Institute

Validated By

John Henry Kampmann Jr.

Certificate of Independence

Referenced Standard and Year (of Standard)

Standard**Year**

ASTM E-1886

2002

ASTM E-1996

2002

Equivalence of Product Standards Certified By

Sections from the Code

1609.1.4

1609.1.4

Product Approval Method

Method 1 Option D

Date Submitted

08/05/2005

Date Validated 09/23/2005
 Date Pending FBC Approval 09/29/2005
 Date Approved 10/11/2005

Summary of Products

FL #	Model, Number or Name	Description
389.1	Bertha HV Accordion Shutter (Non-HVHZ)- Drwg #05-196	Extruded aluminum interlocking blades retained by tracks (top and bottom) for storm protection
Limits of Use (See Other) Approved for use in HVHZ: Approved for use outside HVHZ: Impact Resistant: Design Pressure: +/- Other: This product shall not be installed within High Velocity Hurricane Zone as defined on section 1620.2 of the Florida Building Code.		Installation Instructions Verified By: Evaluation Reports PTID 389_R1 T CERT INDEPENDENCE 05-0715.01.pdf PTID 389_R1 T Drawing No.05-196.pdf PTID 389_R1 T PROD EVALUATION REPORT 05-0715.01.pdf

[Back](#)
[Next](#)

DCA Administration

Department of Community Affairs
Florida Building Code Online
Codes and Standards

2555 Shumard Oak Boulevard
 Tallahassee, Florida 32399-2100

(850) 487-1824, Suncom 277-1824, Fax (850) 414-8436

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Product Approval Accepts:



GENERAL NOTES:

1. BERTHA H.V. ACCORDION SHUTTER SYSTEM SHOWN ON THIS PRODUCT EVALUATION DOCUMENT (P.E.D.) HAS BEEN VERIFIED FOR COMPLIANCE IN ACCORDANCE WITH THE 2004 EDITION OF THE FLORIDA BUILDING CODE. THIS ACCORDION SHUTTER SYSTEM SHALL NOT BE INSTALLED AT HIGH VELOCITY HURRICANE ZONES. DESIGN WIND LOADS SHALL BE DETERMINED AS PER SECTION 1609 OF THE ABOVE MENTIONED CODE FOR A BASIC WIND SPEED AS REQUIRED BY THE JURISDICTION WHERE SHUTTER WILL BE INSTALLED, AND FOR A DIRECTIONALITY FACTOR $K_d=0.85$, IN ACCORDANCE WITH ASCE 7-98 STANDARD, IN ORDER TO VERIFY THAT ANCHORS ON THIS P.E.D., AS TESTED, WERE NOT OVERSTRESSED, A 33% INCREASE IN ALLOWABLE LOADS FOR WIND LOADS WAS NOT USED IN THEIR ANALYSIS. BERTHA H.V. ACCORDION SHUTTER SYSTEM'S ADEQUACY FOR WIND AND IMPACT LOAD HAS BEEN VERIFIED IN ACCORDANCE WITH SECTION 1609.1.4 OF THE ABOVE MENTIONED CODE AS PER AIL REPORTS #0214.01-03 AND #0715.01-03 AS PER TAS-201, TAS-202 AND TAS-203 PROTOCOLS AND ASTM E-1886 AND E-1998 STANDARDS.

2. BERTHA H.V. ACCORDION PINS (HV SCREWS), USED AT BLADES KNUCKLE AND FOR DIRECT MOUNT CONNECTION TO TRACKS SHALL BE # 14X2.75 AND # 14X1.75 RESPECTIVELY, 410-H7 MINIMUM SERIES STAINLESS STEEL SCREWS WITH 135.0 ksi YIELD STRENGTH AND 180 ksi TENSILE STRENGTH. PINS SHALL BE COATED WITH BERTHA HV DACROSHIELD COATING SYSTEM AS MANUFACTURED BY APPROVED COATING APPLICATORS, REGISTERED WITH AMERICAN SHUTTER SYSTEMS ASSOCIATION. PINS MUST BEAR THE HV MARKING ON THEIR HEAD.

3. ALL ALUMINUM EXTRUSIONS SHALL BE ALUMINUM ASSOCIATION 6063-T5 ALLOY AND TEMPER, WITH $F_y = 25.0$ ksi MINIMUM (UNLESS OTHERWISE NOTED).

4. ALL SCREWS TO BE STAINLESS STEEL 304 OR 316 AISI SERIES OR CORROSION RESISTANT COATED CARBON STEEL AS PER DIN 50018 WITH 50 ksi YIELD STRENGTH AND 90 ksi TENSILE STRENGTH.

5. BOLTS TO BE ALUMINUM ASSOCIATION 2014-T4 ALLOY AND TEMPER, ASTM A-307 GALVANIZED STEEL, OR AISI 304 SERIES STAINLESS STEEL WITH 35 ksi MINIMUM YIELD STRENGTH.

6. SEE SHEETS 10, 11 AND 12 FOR ANCHORS SPECIFICATIONS.

7. THIS BERTHA H.V. ACCORDION SHUTTER SYSTEM IS PATENT PENDING. COMPONENTS OF THIS APPROVAL ARE COVERED, IN WHOLE OR IN PART BY U.S. PATENT ISSUED TO EASTERN METAL SUPPLY, INC.

8. IT SHALL BE THE RESPONSIBILITY OF THE CONTRACTOR TO VERIFY THE SOUNDNESS OF THE STRUCTURE WHERE SHUTTER IS TO BE ATTACHED TO INSURE PROPER ANCHORAGE. CONTRACTOR TO SEAL/CAULK ALL SHUTTER COMPONENT EDGES WHICH REMAIN, THRU FASTENING IN CONTINUOUS CONTACT WITH THE BUILDING TO PREVENT WIND/RAIN INTRUSION.

9. EACH UNIT MUST BEAR A PERMANENT LABEL IN A VISIBLE PLACE WITH WARNING NOTE INSTRUCTING THE TENANT OR HOME OWNER THAT THE H.V. LOCK (W) OR ALTERNATE LOCK (V) MUST BE PROPERLY LOCKED DURING PERIODS OF HURRICANE WARNING. FOR EGRESS CONDITIONS, AN INSIDE HV LOCK (W) (WHERE KEY MUST REMAIN IN THE LOCK), AN INSIDE ALTERNATE LOCK (V), OR AN EGRESS H.V. LOCK (N) SHALL BE USED.

10. SHUTTER'S MANUFACTURER LABEL SHALL BE PLACED ON A READILY VISIBLE LOCATION. ONE LABEL SHALL BE PLACED FOR EVERY OPENING. LABEL SHALL READ AS FOLLOWS:
AMERICAN SHUTTER SYSTEMS ASSOCIATION INC
WEST PALM BEACH, FL
FLORIDA STATEWIDE PRODUCT APPROVED.

11. BERTHA H.V. ACCORDION SHUTTER SYSTEM'S INSTALLATION SHALL COMPLY WITH SPECS INDICATED IN THIS DRAWING PLUS ANY BUILDING AND ZONING REGULATIONS PROVIDED BY THE JURISDICTION WHERE PERMIT IS APPLIED TO.

12. (a) THIS P.E.D. PREPARED BY THIS ENGINEER IS GENERIC AND DOES NOT PROVIDE INFORMATION FOR A SITE SPECIFIC PROJECT; I.e. WHERE THE SITE CONDITIONS DEVIATE FROM THE P.E.D.

(b) CONTRACTOR TO BE RESPONSIBLE FOR THE SELECTION, PURCHASE AND INSTALLATION INCLUDING LIFE SAFETY OF THIS PRODUCT, BASED ON THIS P.E.D., PROVIDED HE/SHE DOES NOT DEVIATE FROM THE CONDITIONS DETAILED ON THIS DOCUMENT. CONSTRUCTION SAFETY AT SITE IS THE CONTRACTOR'S RESPONSIBILITY.

(c) THIS P.E.D. WILL BE CONSIDERED INVALID IF ALTERED BY ANY MEANS.

(d) SITE SPECIFIC PROJECTS SHALL BE PREPARED BY A FLORIDA REGISTERED ENGINEER OR ARCHITECT WHICH WILL BECOME THE ENGINEER OF RECORD (E.O.R.) FOR THE PROJECT AND WHO WILL BE RESPONSIBLE FOR THE PROPER USE OF THE P.E.D. ENGINEER OF RECORD, ACTING AS A DELEGATED ENGINEER TO THE P.E.D. ENGINEER SHALL SUBMIT TO THIS LATTER THE SITE SPECIFIC DRAWINGS FOR REVIEW.

(e) THIS P.E.D. SHALL BEAR THE DATE AND ORIGINAL SEAL AND SIGNATURE OF THE PROFESSIONAL ENGINEER OF RECORD THAT PREPARED IT.

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BERTHA HV™ Accordion Shutter System

REV. NO.	DESCRIPTION	DATE	Drawn by: JAG
1	ISSUE	7/13/05	
2			DRAWING No
3			05-186
4			SHEET
5			OF 1

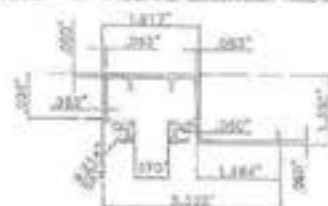


TILECO INC.
TILE TESTING & ENGINEERING COMPANY
4344 N.W. 36th St., Ste. 205, Miramar, Florida 33181
Phone: (305) 371-1525 Fax: (305) 371-1531
e-mail: tileco@tileco.com
88-0004710
WALTON A. TILLOTSON, P.E.
FLORIDA No. 12345

A.S.S.A.
American Shutter Systems
Association, Inc.
4258 Westroads Drive
West Palm Beach, FL 33407



(A) HV BLADE # 1
COVERED UNDER US PATENT # 5458172



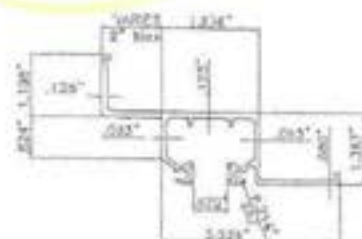
(B) HV CEILING HEADER # 1



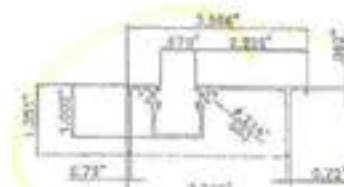
(C) HV 1" (Max.) B/O HEADER/SILL # 1



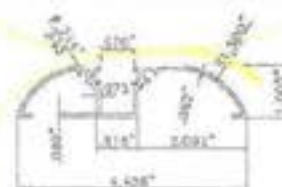
(E) HD CEILING HEADER/SILL # 2



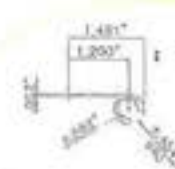
(F) HD 2" (Max.) B/O HEADER/SILL # 2



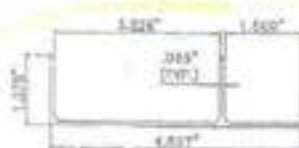
(I) HV ADJ. TRACK



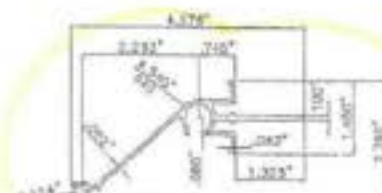
(J) HV WALK-OVER TRACK



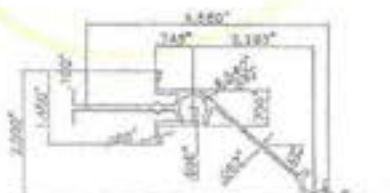
(K) HV 90°/180° STARTER



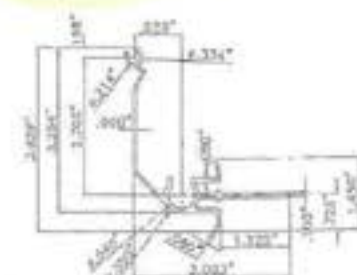
(H) HV ADJ. ADAPTER W/LIP



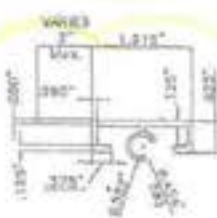
(L) HV CENTERMATE # 1
(SELF MATING)



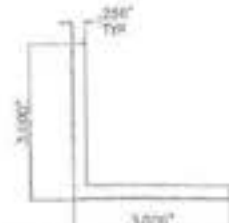
(M) HV FLOATING
CENTERMATE # 2



(X) HV CORNER CENTERMATE # 3



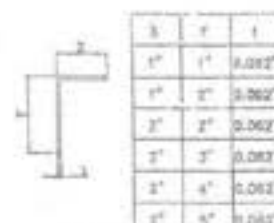
(N) H.V. 2" (Max.) DIRECT MOUNT



(D) 3" FLOOR ANGLE



(O) B.O. WALL ANGLE



(R) SIDE ANGLE



(S) CORNER POST

F.B.C. (Non High Velocity Hurricane Zone)



TITLCO INC.

TITLIT TESTING & ENGINEERING COMPANY
6252 NW 15th St., Ste. 200, Miami, Florida 33150
Phone: (305) 671-1520 Fax: (305) 671-1531
e-mail: titlco@titlco.com
CS-0008719
WALTER A. TITLIT JR. P.E.
FL00054 Lic. # 41087

A.S.S.A.

American Shutter Systems
Association, Inc.
4285 Westroads Drive
West Palm Beach, FL 33407

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BERTHA HV™
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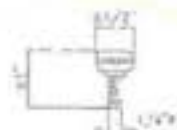
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2			DRAWING NO.
3			05-196
4			SHEET
5			2 OF 1a



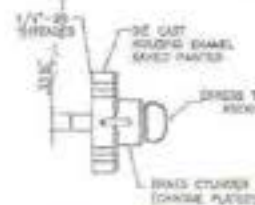
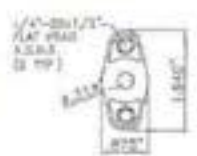
① LOCKING ROD
(SEE NOTES ON SHEET 4)



3/8" #18 ACETAL
THUMBSCREWS

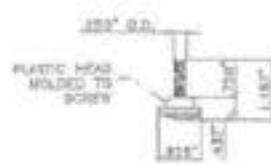


1/4" #20 S.S.
THUMBSCREWS

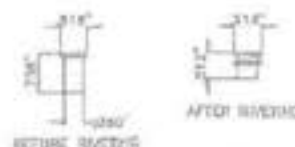


② H.V. LOCK/ EGRESS H.V. LOCK

(SEE NOTES ON SHEET 4)



1/4" #20 x 3/4"
S.S. THUMBSCREW

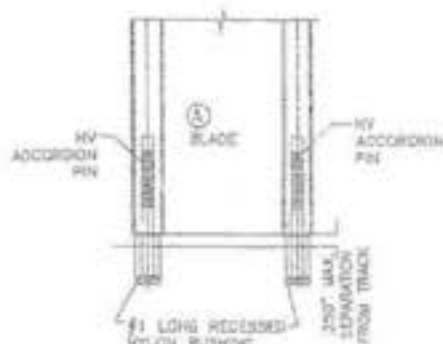
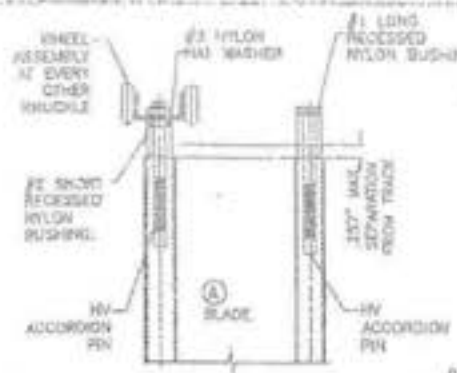


1/4" #20 RIVNUTS

③ ALTERNATE LOCK (TO HV LOCK ②)

(SEE NOTES ON SHEET 4)

*SERIES 9444 ALUMINUM (2024-T3 ALLOY)
LARGE FLANGE THIN SHEET RIVNUT,
PART # 9892, AS MANUFACTURED BY
ADEL CHERRY TEXTURAL PAINTS/CO.,
NEW JERSEY 07064 OR EQUAL.



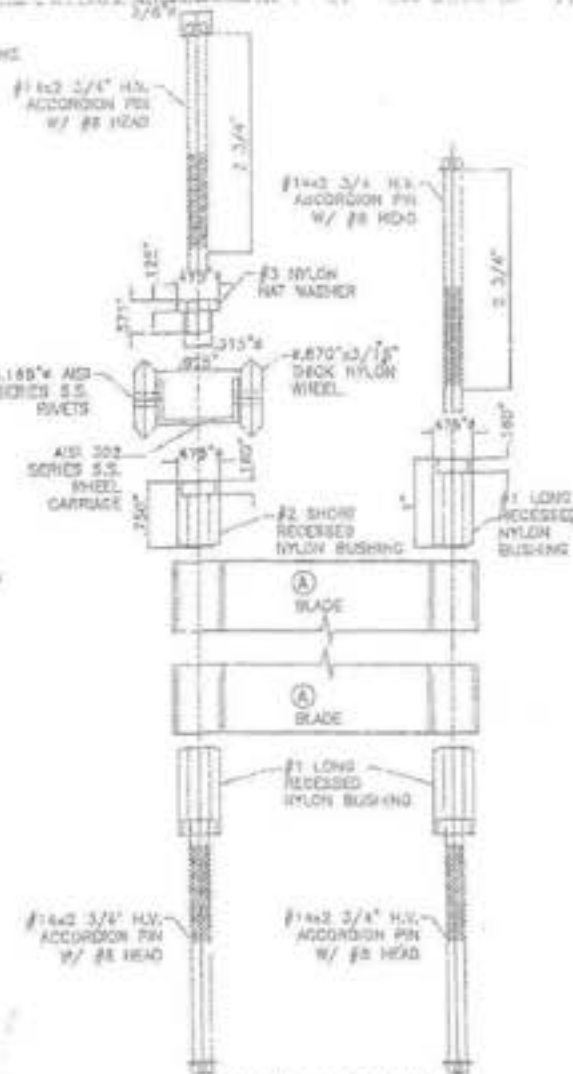
TYPICAL SHUTTER ASSEMBLY

SCALE: 3/8" = 1"



WHEEL CARRIAGE DETAIL

N.T.S.



ASSEMBLY LAYOUT

N.T.S.

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DR-0000710
ERIC A. TILIT & P. L. AUSTIN (L) & SONS



American Shutter Systems
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4260 Westrock Drive
West Palm Beach, FL 33407

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REV. NO.	DESCRIPTION	DATE	Drawn by: J.S.S.
1	As Shown	7/13/05	DATE: 7/13/05
2			DRAWING No
3			05-196
4			SHEET
5			5 OF 16

* NOTES ON LOCKING RODS (U), HV LOCK (W) AND ALTERNATE (V) LOCK

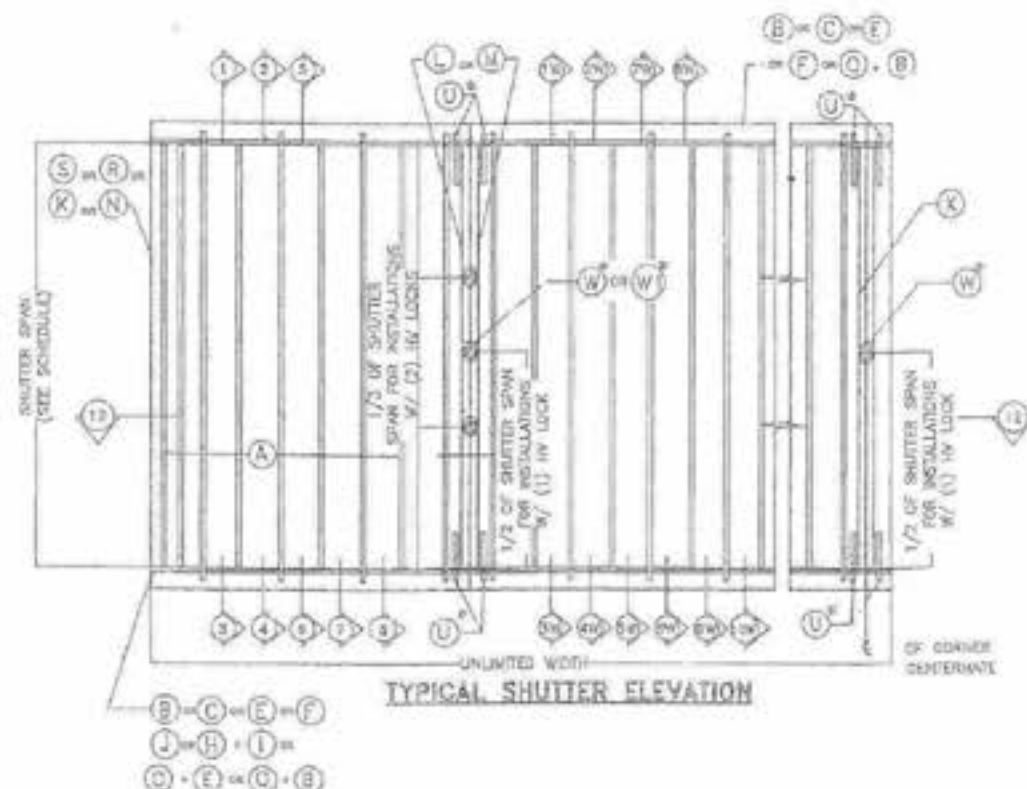
1. MANDATORY CONDITION #1: ONE HV LOCK (W) SHALL BE USED FOR ANY SPAN AND INSTALLATIONS AT ANY ELEVATION. HV LOCK (W) MAY BE USED AS AN INSIDE OR OUTSIDE LOCK, ATTACHED TO (C) OR (E) CENTERMATES W/(2) 1/4"-20 x 1/2" LONG FLAT HEAD S.S. M.S. LOCATION OF LOCK SHALL BE AS INDICATED ON TYPICAL SHUTTER ELEVATION.

2. OPTIONAL CONDITION #1: IF DESIRED, FOR BETTER PERFORMANCE OR FOR SECURITY PURPOSES, UP TO TWO HV LOCKS (W) MAY BE USED AS AN INSIDE OR OUTSIDE LOCK. CONNECTION OF LOCKS TO CENTERMATES (C) OR (E) SHALL BE AS INDICATED ON NOTE (1) ABOVE. LOCATION OF LOCKS SHALL BE AS INDICATED ON TYPICAL SHUTTER ELEVATION.

3. OPTIONAL CONDITION #2: LOCKING RODS (U) ARE NOT REQUIRED AT HURRICANE POSITION, BUT MAY BE USED IF DECIDED TO ENHANCE SHUTTER PERFORMANCE. LOCKING RODS (U) OR AN ALTERNATIVE DEVICE MAY ALSO BE USED AT STACKING POSITION (NON HURRICANE POSITION).

4. MANDATORY CONDITION #2: ONE HV LOCK (W) SHALL BE USED AT CORNER CENTERMATES FOR ANY SPAN AND INSTALLATIONS AT ANY ELEVATION. HV LOCK (W) SHALL BE USED AS AN OUTSIDE OR INSIDE LOCK, ATTACHED TO (C) CORNER CENTERMATE W/(2) 1/4"-20 x 1/2" LONG FLAT HEAD S.S. M.S. TWO LOCKING RODS (U) AT TOP & BOTTOM SHALL BE USED. LOCATION OF LOCK SHALL BE AS INDICATED ON TYPICAL SHUTTER ELEVATION. MAXIMUM SHUTTER SPAN SHALL BE DETERMINED AS PER SCHEDULES SHOWN ON SHEET 5.

5. ALTERNATE TO MANDATORY CONDITION #1 VALID ONLY FOR SYMMETRICAL CENTERMATES: ONE ALTERNATE LOCK (V) MAY BE USED IN LIEU OF HV LOCK (W) FOR ANY SPAN AND INSTALLATIONS AT ANY ELEVATION. ALTERNATE LOCK (V) MAY BE USED AS AN INSIDE OR OUTSIDE LOCK. ATTACHMENT OF 1/4"-20x3/4" S.S. THUMBSCREW TO (C) OR (E) CENTERMATES SHALL BE MADE W/ 1/4"-20x3/4" ALUM. RIVNUT AS SHOWN ON ALTERNATE SECTION 12, SHEET 7.



F.B.C.(Non High Velocity Hurricane Zone)

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REV. NO.	DESCRIPTION	DATE	DESIGNED BY
1	REVISED	7/13/05	
2			
3			
4			
5			
6			

DRAWING No
05-196

SHEET
2 OF 3

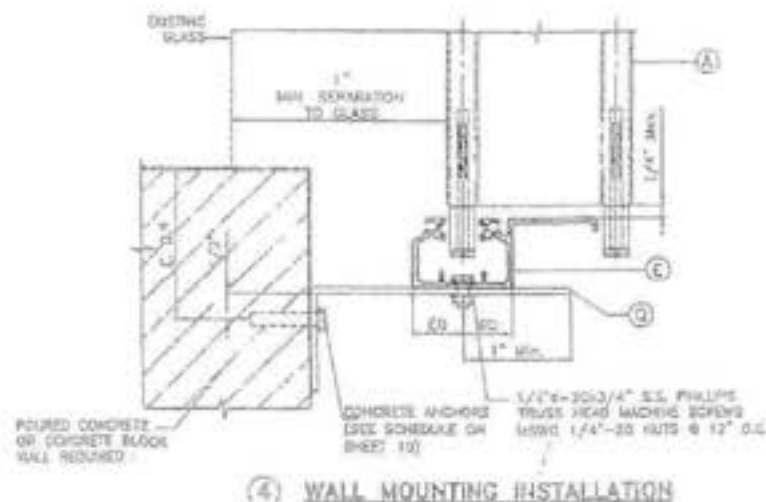
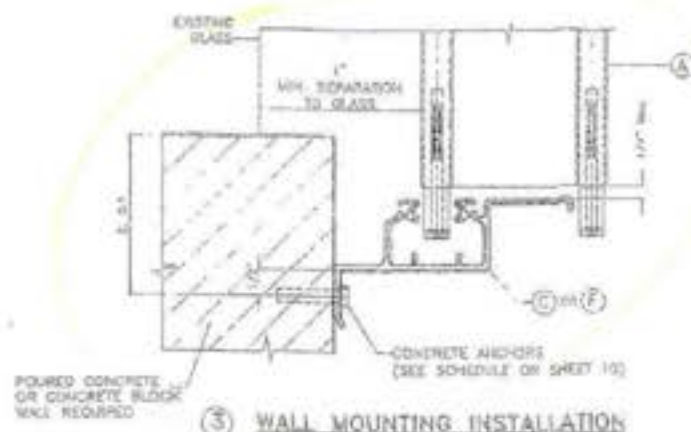
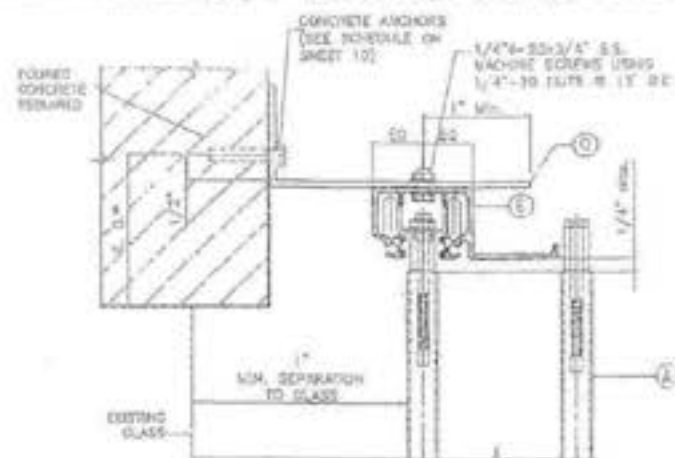
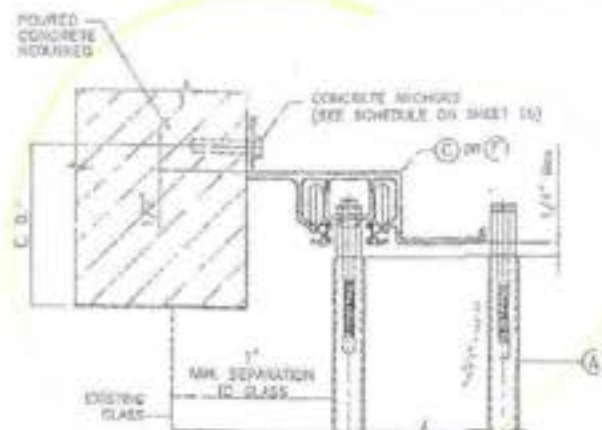


TILIECO INC.

TILLY TESTING & ENGINEERING COMPANY
CORP. 100, 2ND FL. 305 WILSON BLVD., FLORIDA 33118
Phone: (305) 771-1530 Fax: (305) 771-1531
E-MAIL: TILLY@TILIECO.COM
C#-0000718
WILLIAM A. TILLY, P.E.
FLORIDA Lic. # 11118

A.S.S.A.

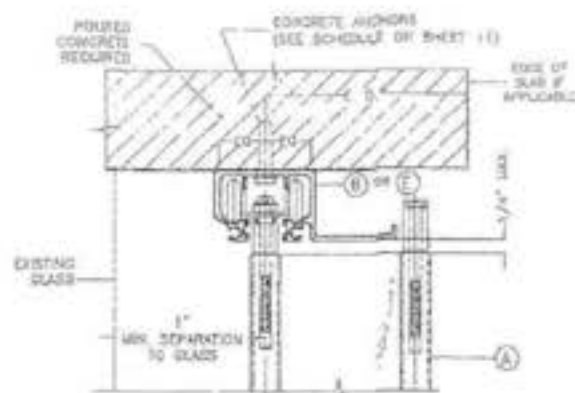
American Shutter Systems
Association, Inc.
4288 Westbrooke Drive
West Palm Beach, FL 33407



NOTES FOR COMPARISON OF SWISS AND

- 1- SEE SHEET 3 FOR TYPICAL SHUTTER ASSEMBLY
2- MOUNTING SECTIONS CAN BE COMBINED
IN ANY WAY TO SUIT ANY INSTALLATION

* E. D. = EDOL DISTANCE
SEE NOTES & SCHEDULE ON SHEET 112



⑤ CEILING MOUNTING INSTALLATION



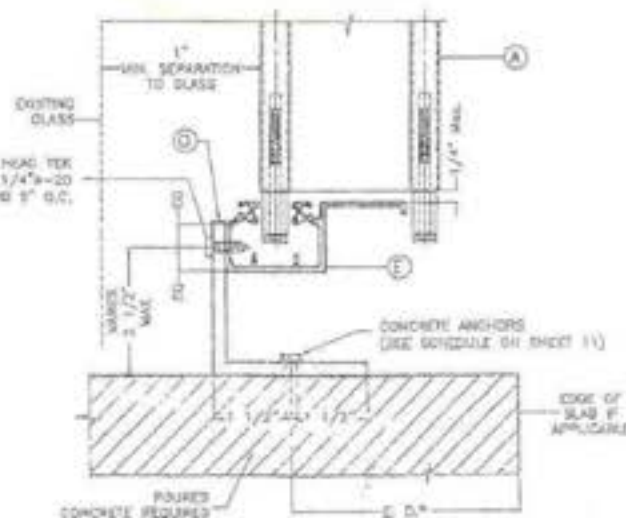
⑥ WALK-OVER MOUNTING INSTALLATION



⑦ FLOOR MOUNTING INSTALLATION †

†1/4" x 1/4" S.S. HEX HEAD TEK
SCREWS @ 8" O.C. OR 1/4" x 20
S.S. W.S. R/NUT @ 8" O.C.

†NOTE:
THIS INSTALLATION SHALL BE
REMOVABLE WHEN PERFORMED
ADJACENT TO AN OPERABLE EXIT
OR ENTRANCE



⑧ FLOOR MOUNTING INSTALLATION †

NOTE FOR CONSTRUCTION OF SECTIONS

- 1- SEE SHEET 3 FOR 1-PANEL SHUTTER ASSEMBLY
- 2- MOUNTING SECTIONS CAN BE COMBINED IN ANY WAY TO SUIT ANY INSTALLATION

* E, D = EDGE DISTANCE
(SEE NOTES & SCHEDULE ON SHEET 11)

INSTALLATION DETAILS

F.B.C. (Non High Velocity Hurricane Zone)

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3300 N.W. 38th St., Ste. 300, Virginia Gardens, Florida 33196
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e-mail: tilieco@tilieco.com
ED-0000712
ENTER A. TILIT & P. E.
FLORIDA EN. # 41187

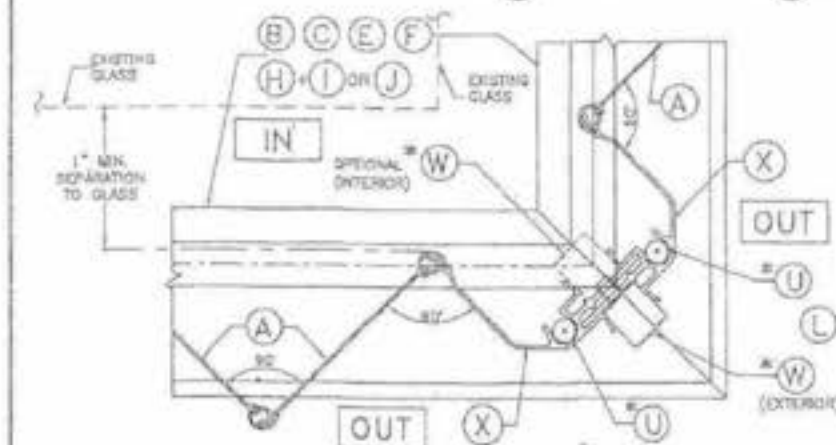
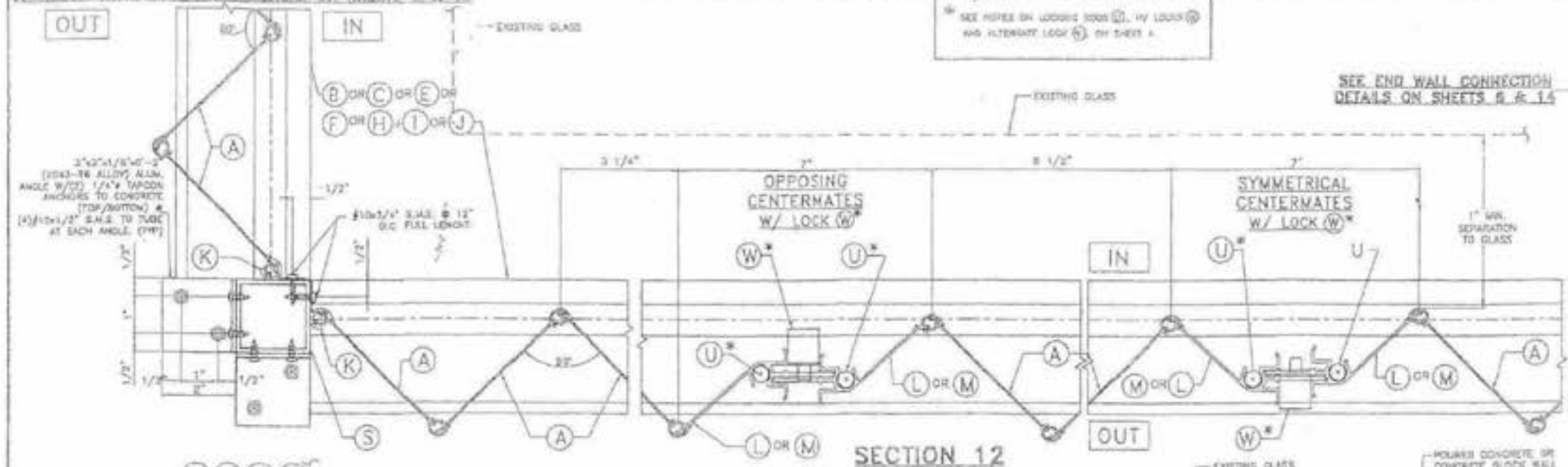


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4288 Westroads Drive
West Palm Beach, FL 33407

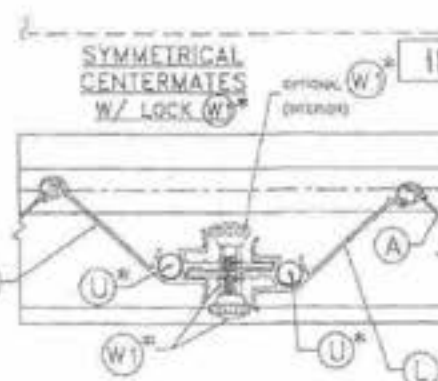
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Accordion Shutter System

REV. NO.	DESCRIPTION	DATE	Drawn by: E.A.S.
1	REV. 05-196	3/10/04	DATE: 7/13/05
2			DRAWING No
3			05-196
4			SHEET
5			5 OF 10

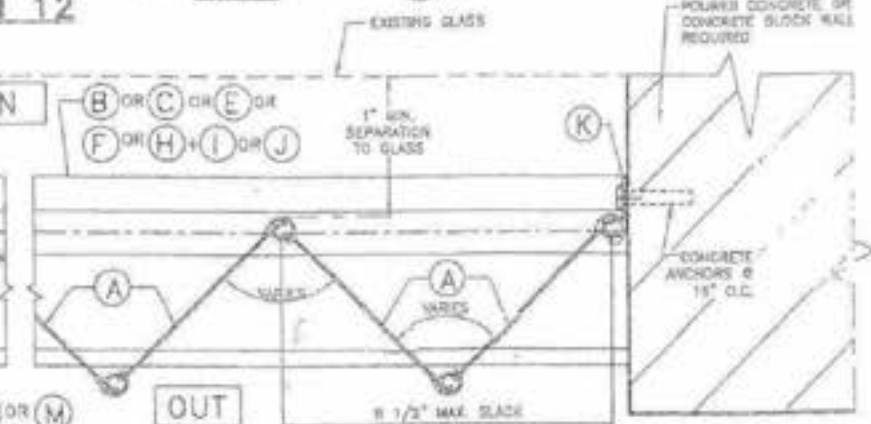
SEE END WALL CONNECTION DETAILS ON SHEETS 8 & 14



SECTION 12 AT CORNER CENTERMATE #3



ALTERNATE SECTION 12



SECTION 12 AT SLACK DETAIL (IF APPLICABLE)

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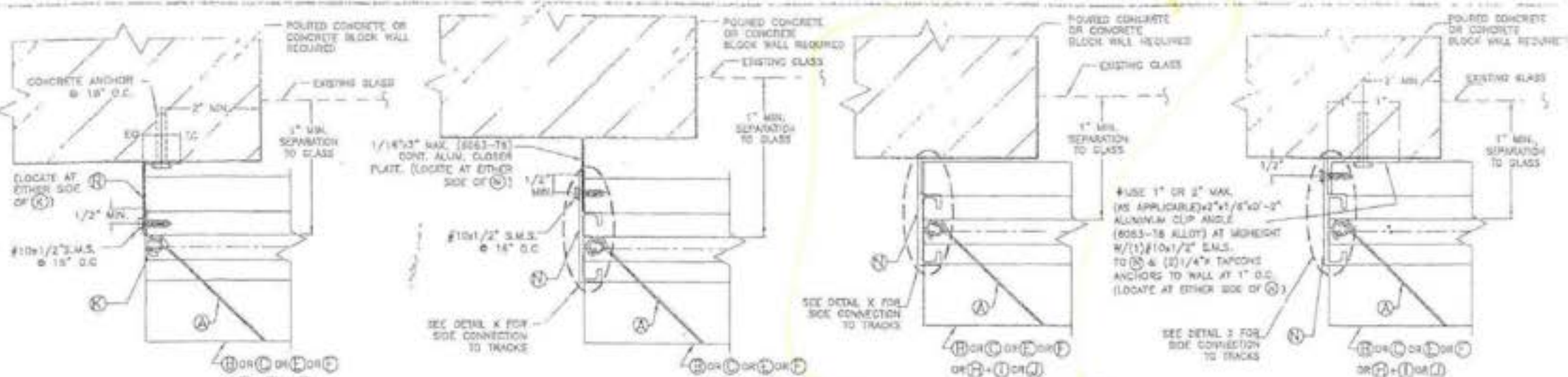
REV. NO.	DESCRIPTION	DATE	Drawn by J.A.S.
1	REV. 05-04	11/15/05	DATE: 7/13/05
2			DRAWING No
3			05-196
4			SHEET
5			7 OF 11



TILLIT CO. INC.

TILLIT TESTING & ENGINEERING COMPANY
6822 N.W. 24th St., Ste. 200, MIAMI SHORES, FLORIDA 33146
Phone: (305) 871-1120 Fax: (305) 871-1521
E-mail: tillit@tillit.com
WALTER A. TILLIT P. E.
FLORIDA LICENSE NO. 48662

A.S.S.A.
American Shutter Systems
Association, Inc.
4258 Westroads Drive
West Palm Beach, FL 33407



END CONNECTION DETAIL ①

END CONNECTION DETAIL ②

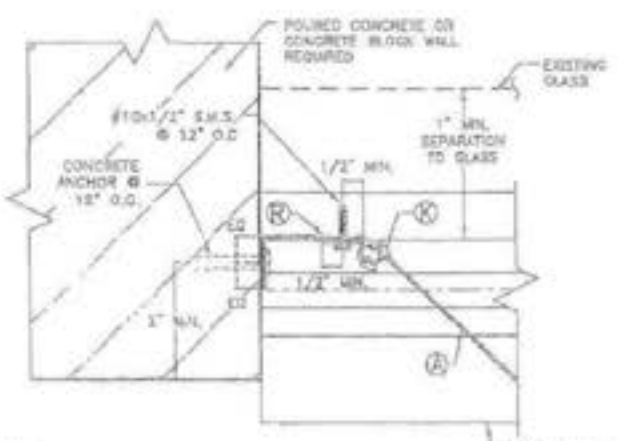
END CONNECTION DETAIL ③

END CONNECTION DETAIL ④

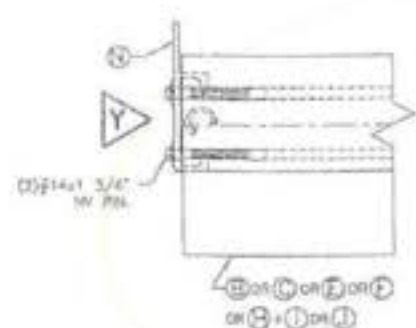
• REQUIRED ONLY FOR SPANS GREATER THAN 12'-0"



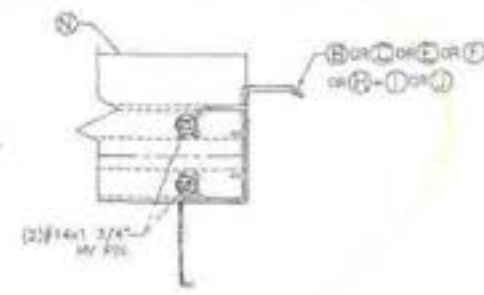
END CONNECTION DETAIL ⑤



END CONNECTION DETAIL ⑥



PLAN



VIEW Y

DETAIL X

F.B.C. (Wan High Velocity Hurricane Zone)

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2355 K.W. 250, PL. 30, 305, WYOMING, FLORIDA 33148
Phone: (305) 671-1500 • Fax: (305) 671-1520
e-mail: TTE@TTECO.com
800-305-8710
WALTER A. TILLIE, P.E.
FLORIDA Lic. # 44192



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4268 Westroads Drive
West Palm Beach, FL 33407

BERTHA HV™
Accordion Shutter System

REV.	DATE	DESCRIPTION	BY	CHKD.	DATE	Drawn by J.A.S.
1						DATE: 7/13/05
2						
3						DRAWING No
4						05-196
5						SHEET
6						8 OF 14

MAXIMUM SHUTTER SPAN
"L+" OR "L-" (ft.) INSTALLATIONS
W/ SYMMETRICAL CENTERMATES,
ONE HV LOCK & NO LOCKING RODS *
(SEE NOTE 1)

MAXIMUM DESIGN LOAD (psf) (+ OR -)	CONCRETE & WOOD INSTALLATIONS &			
	WALL MOUNTING		FLOOR/CEILING MOUNTING	
	L+ (ft.)	L- (ft.)	L+ (ft.)	L- (ft.)
30 OR LESS	15'-0"	15'-0"	15'-0"	15'-0"
35	14'-10"	14'-10"	15'-2"	15'-0"
40	13'-10"	13'-10"	14'-2"	15'-2"
45	13'-11"	13'-11"	13'-4"	14'-3"
50	12'-5"	12'-5"	12'-5"	13'-7"
55	11'-10"	11'-10"	12'-5"	12'-11"
60	11'-4"	11'-4"	11'-7"	12'-4"
65	10'-10"	10'-10"	11'-1"	11'-11"
70	10'-6"	10'-6"	10'-9"	11'-5"
75	10'-1"	10'-1"	10'-4"	11'-1"
80	9'-10"	9'-10"	10'-2"	10'-9"
85	9'-5"	9'-5"	9'-9"	10'-5"
90	9'-3"	9'-3"	9'-5"	10'-1"
95	9'-2"	9'-2"	9'-2"	9'-10"
100	8'-9"	8'-9"	8'-5"	9'-7"
105	8'-7"	8'-7"	8'-4"	9'-4"
110	8'-4"	8'-4"	8'-3"	9'-2"
115	8'-5"	8'-5"	8'-4"	8'-11"
120	8'-5"	8'-5"	8'-2"	8'-9"
125	7'-10"	7'-10"	8'-0"	8'-5"
130	7'-8"	7'-8"	7'-10"	8'-1"
135	7'-7"	7'-7"	7'-8"	7'-9"
140	7'-5"	7'-5"	7'-3"	7'-6"
145	7'-3"	7'-3"	7'-5"	7'-3"
150	7'-2"	7'-2"	7'-4"	7'-0"
155	7'-8"	7'-8"	7'-2"	6'-9"
160	6'-11"	6'-11"	7'-1"	6'-7"
165	6'-10"	6'-10"	7'-0"	6'-4"
170	6'-9"	6'-9"	6'-11"	6'-2"
175	6'-7"	6'-7"	6'-8"	6'-0"
180	6'-6"	6'-6"	6'-8"	5'-10"
185	6'-5"	6'-5"	6'-7"	5'-8"
190	6'-4"	6'-4"	6'-6"	5'-6"
195	6'-3"	6'-3"	6'-5"	5'-5"
200	6'-2"	6'-2"	6'-4"	5'-3"
205	6'-1"	6'-1"	6'-3"	5'-1"

MAXIMUM SHUTTER SPAN
"L+" OR "L-" (ft.)
INSTALLATIONS W/
DEPOSING CENTERMATES,
ONE HV LOCK & NO
LOCKING RODS *
(SEE NOTE 1)

MAXIMUM DESIGN LOAD (psf) (+ OR -)	CONCRETE & WOOD INSTALLATIONS &	
	WALL MOUNTING & FLOOR/CEILING MOUNTING	
	L+ (ft.)	L- (ft.)
30	15'-0"	15'-0"
35	14'-10"	14'-10"
40	13'-10"	13'-10"
45	13'-11"	13'-11"
50	12'-5"	12'-5"
55	11'-10"	11'-10"
60	11'-4"	11'-4"
65	10'-10"	10'-10"
70	10'-6"	10'-6"
75	10'-1"	10'-1"
80	9'-10"	9'-10"
85	9'-5"	9'-5"
90	9'-3"	9'-3"
95	9'-2"	9'-2"
100	8'-9"	8'-9"
105	8'-7"	8'-7"
110	8'-4"	8'-4"
115	8'-5"	8'-5"
120	8'-5"	8'-5"
125	7'-10"	7'-10"
130	7'-8"	7'-8"
135	7'-7"	7'-7"
140	7'-5"	7'-5"
145	7'-3"	7'-3"
150	7'-2"	7'-2"
155	7'-8"	7'-8"
160	6'-11"	6'-11"
165	6'-10"	6'-10"
170	6'-9"	6'-9"
175	6'-7"	6'-7"
180	6'-6"	6'-6"
185	6'-5"	6'-5"
190	6'-4"	6'-4"
195	6'-3"	6'-3"
200	6'-2"	6'-2"
205	6'-1"	6'-1"

INSTALLATION LOADS		
	CONCRETE & MASONRY	WOOD
WALL MOUNTING	1, 2, 3 & 4	1W, 2W, 3W, 4W, 5W & 6W
FLOOR/CEILING MOUNTING	5, 6, 7 & 8	6W, 5W & 10W

A MAXIMUM SHUTTER SPAN "L+" OR "L-" FOR
INSTALLATIONS INTO WOOD SHALL BE LIMITED TO
110 PSF DESIGN LOAD.

† CORNER CENTERMATE #3 MAY BE USED AT ALL
MOUNTING INSTALLATIONS. LOCKING SPECIFICATIONS
SHALL BE AS INDICATED ON MANDATORY CONDITION #2
NOTE #A, SHEET 4 OF 14.

* NOTES:

(1) L+: MAXIMUM ALLOWABLE SPAN FOR A GIVEN
POSITIVE DESIGN LOAD.
L-: MAX. ALLOWABLE SPAN FOR A GIVEN NEGATIVE
DESIGN LOAD.

(2) PROCEDURE TO DETERMINE MAXIMUM SPAN FOR
WALL MOUNTINGS (TOP/BOTTOM) OR FLOOR/CEILING:

GIVEN A POSITIVE DESIGN LOAD, DETERMINE MAXIMUM
SPAN "L+" FROM SCHEDULE.
GIVEN A NEGATIVE DESIGN LOAD, DETERMINE MAXIMUM
SPAN "L-" FROM SCHEDULE.

FINAL MAXIMUM ALLOWABLE SPAN IS EQUAL TO THE
MINIMUM DETERMINED SPAN BETWEEN "L+" AND "L-".

(3) PROCEDURE TO DETERMINE MAXIMUM SPAN FOR
COMBINATIONS W/ BETWEEN WALL MOUNTINGS
W/ FLOOR/CEILING MOUNTINGS:

FOR A GIVEN POSITIVE DESIGN LOAD:

DETERMINE:
L1+= MAX. SPAN FOR WALL MOUNTING INSTALLATIONS.
L2+= MAX. SPAN FOR FLOOR/CEILING MOUNTING
INSTALLATIONS.

FOR A GIVEN NEGATIVE DESIGN LOAD:

DETERMINE:
L1-= MAX. SPAN FOR WALL MOUNTING INSTALLATIONS.
L2-= MAX. SPAN FOR FLOOR/CEILING MOUNTING
INSTALLATIONS.

FINAL MAXIMUM ALLOWABLE SPAN IS EQUAL TO THE
MINIMUM BETWEEN "L1+", "L2+", "L1-" AND "L2-".

(4) GO TO ANCHOR SCHEDULE WITH FINAL MAXIMUM
ALLOWABLE SPAN AND NEGATIVE DESIGN LOAD TO
DETERMINE MAXIMUM ANCHOR SPACING.

F.B.C. (Non High Velocity Hurricane Zone)

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BERTHA HV™ Accordian Shutter System

REV. NO.	DESCRIPTION	DATE	Drawn by J.A.S.
1	AS 19-19A	7/13/05	
2			
3			
4			
5			
6			

DRAWING No
05-196

SHEET
1 OF 14



TILTEC O INC.

TILTEC TESTING & ENGINEERING COMPANY
201 N. 20th St., Ste. 201, VERO BEACH, FLORIDA 3358
Phone: (305) 201-1500 Fax: (305) 201-1531
e-mail: Tilt@Tilt.com
28-000710
WALTER A. TILTE JR., P. E.
FLORIDA No. 7-94167

A.S.S.A.

American Shutter Systems
Association, Inc.
4205 Westroads Drive
West Palm Beach, FL 33407

UNIVERSITY MICROFILMS

TAPER
ALL POINTS SHUT-OUT
WEDGE-BOLT

MAXIMUM DESIGN PRESSURE RATING (p.s.f.) AND CORRESPONDING MAXIMUM ANCHOR SPACING (in.) SCHEDULE FOR A GIVEN MAX. SHUTTER SPAN, DESIGN NEGATIVE LOAD AND A CORRESPONDING MOUNTING TYPE FOR CONCRETE OR CONCRETE BLOCK INSTALLATIONS (SEE MOUNTINGS 1, 2, 3 & 4 ON SHEET 5)

* SEE ANCHOR
SPECIFICATIONS
ON SHEET 11

SHUTTER SPAN (ft)	POURED CONCRETE												POURED CONCRETE/CONCRETE BLOCK											
	WALL MOUNTING 1 W/ (1) HV 1" (Max.) B/D HEADER/ SILL #1				WALL MOUNTING 1 W/ (1) HV 2" (Max.) B/D HEADER/ SILL #2				WALL MOUNTING 2				WALL MOUNTING 3 W/ (1) HV 1" (Max.) B/D HEADER/ SILL #1				WALL MOUNTING 3 W/ (1) HV 2" (Max.) B/D HEADER/ SILL #2				WALL MOUNTING 4			
	NEGATIVE DESIGN LOAD (psf)				NEGATIVE DESIGN LOAD (psf)				NEGATIVE DESIGN LOAD (psf)				NEGATIVE DESIGN LOAD (psf)				NEGATIVE DESIGN LOAD (psf)				NEGATIVE DESIGN LOAD (psf)			
	30	40	50	60	70	80	90	100	110	120	130	140	30	40	50	60	70	80	90	100	110	120	130	140
3'-0"	14	12	10	8	6	5	4	3	2	1	1	1	14	12	10	8	6	5	4	3	2	1	1	1
4'-0"	14	12	10	8	6	5	4	3	2	1	1	1	14	12	10	8	6	5	4	3	2	1	1	1
5'-0"	14	12	10	8	6	5	4	3	2	1	1	1	14	12	10	8	6	5	4	3	2	1	1	1
6'-0"	14	12	10	8	6	5	4	3	2	1	1	1	14	12	10	8	6	5	4	3	2	1	1	1
7'-0"	14	12	10	8	6	5	4	3	2	1	1	1	14	12	10	8	6	5	4	3	2	1	1	1
8'-0"	14	12	10	8	6	5	4	3	2	1	1	1	14	12	10	8	6	5	4	3	2	1	1	1
9'-0"	14	12	10	8	6	5	4	3	2	1	1	1	14	12	10	8	6	5	4	3	2	1	1	1
10'-0"	14	12	10	8	6	5	4	3	2	1	1	1	14	12	10	8	6	5	4	3	2	1	1	1
11'-0"	14	12	10	8	6	5	4	3	2	1	1	1	14	12	10	8	6	5	4	3	2	1	1	1
12'-0"	14	12	10	8	6	5	4	3	2	1	1	1	14	12	10	8	6	5	4	3	2	1	1	1
13'-0"	14	12	10	8	6	5	4	3	2	1	1	1	14	12	10	8	6	5	4	3	2	1	1	1
14'-0"	14	12	10	8	6	5	4	3	2	1	1	1	14	12	10	8	6	5	4	3	2	1	1	1
15'-0"	14	12	10	8	6	5	4	3	2	1	1	1	14	12	10	8	6	5	4	3	2	1	1	1
16'-0"	14	12	10	8	6	5	4	3	2	1	1	1	14	12	10	8	6	5	4	3	2	1	1	1
17'-0"	14	12	10	8	6	5	4	3	2	1	1	1	14	12	10	8	6	5	4	3	2	1	1	1
18'-0"	14	12	10	8	6	5	4	3	2	1	1	1	14	12	10	8	6	5	4	3	2	1	1	1
19'-0"	14	12	10	8	6	5	4	3	2	1	1	1	14	12	10	8	6	5	4	3	2	1	1	1
20'-0"	14	12	10	8	6	5	4	3	2	1	1	1	14	12	10	8	6	5	4	3	2	1	1	1
21'-0"	14	12	10	8	6	5	4	3	2	1	1	1	14	12	10	8	6	5	4	3	2	1	1	1
22'-0"	14	12	10	8	6	5	4	3	2	1	1	1	14	12	10	8	6	5	4	3	2	1	1	1
23'-0"	14	12	10	8	6	5	4	3	2	1	1	1	14	12	10	8	6	5	4	3	2	1	1	1
24'-0"	14	12	10	8	6	5	4	3	2	1	1	1	14	12	10	8	6	5	4	3	2	1	1	1
25'-0"	14	12	10	8	6	5	4	3	2	1	1	1	14	12	10	8	6	5	4	3	2	1	1	1
26'-0"	14	12	10	8	6	5	4	3	2	1	1	1	14	12	10	8	6	5	4	3	2	1	1	1
27'-0"	14	12	10	8	6	5	4	3	2	1	1	1	14	12	10	8	6	5	4	3	2	1	1	1
28'-0"	14	12	10	8	6	5	4	3	2	1	1	1	14	12	10	8	6	5	4	3	2	1	1	1
29'-0"	14	12	10	8	6	5	4	3	2	1	1	1	14	12	10	8	6	5	4	3	2	1	1	1
30'-0"	14	12	10	8	6	5	4	3	2	1	1	1	14	12	10	8	6	5	4	3	2	1	1	1

F.B.C. (Non High Velocity Hurricane Zone)

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BERTHA HV™
Accordion Shutter System

REV.	DESCRIPTION	DATE	Drawn by J.A.S.
1	REV 01-000	1/13/03	DATE: 7/13/03
2			DRAWING NO
3			05-196
4			SHEET
5			10 OF 14



P.E. SEAL/SIGNATURE/DATE

TILT CO INC.

TILT TESTING & ENGINEERING COMPANY
4000 N.W. 20th St., Ste. 200, Vero Beach, FL 32980
Phone: (305) 771-1010, Fax: (305) 771-1051
E-mail: tilt@tiltco.com
OR-0000718
WALTER A. TILT & F. E.
FLORIDA Lic. # 51162

A.S.S.A.
American Shutter Systems
Association, Inc.
4266 Westroads Drive
West Palm Beach, FL 33407

ANCHOR
TAPCON
ALL POINTS SOLID-SET
WEDGE-BOLT

MAXIMUM DESIGN PRESSURE RATING (p.s.f.) AND CORRESPONDING MAXIMUM ANCHOR SPACING (in.) SCHEDULE FOR A GIVEN MAX. SHUTTER SPAN, DESIGN NEGATIVE LOAD AND A CORRESPONDING MOUNTING TYPE FOR CONCRETE INSTALLATIONS
(SEE MOUNTINGS 5, 6, 7, A & B ON SHEET 4).

MAXIMUM SPAN (ft)	POURED CONCRETE																															
	CEILING MOUNTING 5								FLOOR MOUNTING 6								FLOOR MOUNTING 7								FLOOR MOUNTING 8							
	NEGATIVE DESIGN LOAD (psf)								NEGATIVE DESIGN LOAD (psf)								NEGATIVE DESIGN LOAD (psf)								NEGATIVE DESIGN LOAD (psf)							
	30	40	60	75	90	120	150	205	30	40	60	75	90	120	150	205	30	40	60	75	90	120	150	205	30	40	60	75	90	120	150	205
3'-0" OR LESS	14	12	10	8	6	5	4	3	14	12	10	8	6	5	4	3	14	12	10	8	6	5	4	3	14	12	10	8	6	5	4	3
4'-0"	14	12	10	8	6	5	4	3	14	12	10	8	6	5	4	3	14	12	10	8	6	5	4	3	14	12	10	8	6	5	4	3
5'-0"	14	12	10	8	6	5	4	3	14	12	10	8	6	5	4	3	14	12	10	8	6	5	4	3	14	12	10	8	6	5	4	3
6'-0"	14	12	10	8	6	5	4	3	14	12	10	8	6	5	4	3	14	12	10	8	6	5	4	3	14	12	10	8	6	5	4	3
7'-0"	14	12	10	8	6	5	4	3	14	12	10	8	6	5	4	3	14	12	10	8	6	5	4	3	14	12	10	8	6	5	4	3
8'-0"	14	12	10	8	6	5	4	3	14	12	10	8	6	5	4	3	14	12	10	8	6	5	4	3	14	12	10	8	6	5	4	3
9'-0"	14	12	10	8	6	5	4	3	14	12	10	8	6	5	4	3	14	12	10	8	6	5	4	3	14	12	10	8	6	5	4	3
10'-0"	14	12	10	8	6	5	4	3	14	12	10	8	6	5	4	3	14	12	10	8	6	5	4	3	14	12	10	8	6	5	4	3
11'-0"	14	12	10	8	6	5	4	3	14	12	10	8	6	5	4	3	14	12	10	8	6	5	4	3	14	12	10	8	6	5	4	3
12'-0"	14	12	10	8	6	5	4	3	14	12	10	8	6	5	4	3	14	12	10	8	6	5	4	3	14	12	10	8	6	5	4	3
13'-0"	14	12	10	8	6	5	4	3	14	12	10	8	6	5	4	3	14	12	10	8	6	5	4	3	14	12	10	8	6	5	4	3
14'-0"	14	12	10	8	6	5	4	3	14	12	10	8	6	5	4	3	14	12	10	8	6	5	4	3	14	12	10	8	6	5	4	3
15'-0"	14	12	10	8	6	5	4	3	14	12	10	8	6	5	4	3	14	12	10	8	6	5	4	3	14	12	10	8	6	5	4	3
16'-0"	14	12	10	8	6	5	4	3	14	12	10	8	6	5	4	3	14	12	10	8	6	5	4	3	14	12	10	8	6	5	4	3

NOTES ON ANCHOR REQUIREMENTS: POURED CONCRETE OR CONCRETE BLOCK SUBSTRATES

1. ANCHORS TO WALL SHALL BE AS FOLLOWS:

(A) TO EXISTING POURED CONCRETE (MIN. $f'_c = 3,000$)

- 1/4" TAPCON ANCHORS AS MANUFACTURED BY L.T.C. BANGS/ RED HEAD

- 1/4" x 3/4" ALL POINTS SOLID-SET ANCHORS AS DISTRIBUTED BY ALL POINTS SCREW, BOLT & SPECIALTY COMPANY

- 1/4" WEDGE-BOLT ANCHORS AS MANUFACTURED BY FORDS FASTENING, INC.

NOTES:

A.1) MINIMUM EMBEDMENT INTO POURED CONCRETE OF TAPCON ANCHORS IS 1 3/4", AND 2" FOR WEDGE-BOLT ANCHORS.

A.2) MINIMUM EMBEDMENT OF 1/4" x 3/4" ALL POINTS SOLID-SET ANCHORS SHALL BE 7/8" INTO THE POURED CONCRETE. NO EMBEDMENT INTO STUCCO SHALL BE PERMITTED. 1/4" #20 U.S. MACHINE SCREW USED SHALL BE 1 1/2" LONG MINIMUM SHOULD STUCCO EXIST, AND 1" MINIMUM FOR WALLS WITH NO STUCCO.

A.3) IN CASE THAT PRECAST STONE, PRECAST CONCRETE PANELS, PAVES OR ANY VENEER BE FOUND ON THE EXISTING WALL OR FLOOR, ANCHORS SHALL BE LONG ENOUGH TO REACH THE MAIN STRUCTURE BEHIND SAID WALL, FINISHES, ANCHORAGE SHALL BE AS INDICATED ON NOTES A.1) & A.2) ABOVE. FOR INSTALLATIONS ON VINYL SIDING OR EPS CONSULT THE ENGINEER.

(B) TO EXISTING CONCRETE BLOCK WALL:

- 1/4" TAPCON ANCHORS AS MANUFACTURED BY L.T.C. BANGS/ RED HEAD

- 1/4" x 3/4" ALL POINTS SOLID-SET ANCHORS AS DISTRIBUTED BY ALL POINTS SCREW, BOLT & SPECIALTY COMPANY

- 1/4" WEDGE-BOLT W/ 1/4" BLOCK-PLUS ANCHORS AS MANUFACTURED BY FORDS FASTENING, INC.

NOTES:

B.1) MINIMUM EMBEDMENT INTO CONCRETE BLOCK OF TAPCON & WEDGE-BOLT ANCHORS IS 1 1/4".

B.2) MINIMUM EMBEDMENT OF 1/4" x 3/4" ALL POINTS SOLID-SET ANCHORS SHALL BE ENTIRELY EMBEDDED INTO THE CONCRETE BLOCK. NO EMBEDMENT INTO STUCCO SHALL BE PERMITTED. 1/4" #20 U.S. MACHINE SCREW USED SHALL BE 1 1/2" LONG MINIMUM SHOULD STUCCO EXIST, AND 1" MINIMUM FOR WALLS WITH NO STUCCO.

B.3) IN CASE THAT PRECAST STONE, PRECAST CONCRETE PANELS, PAVES OR ANY VENEER BE FOUND ON THE EXISTING WALL OR FLOOR, ANCHORS SHALL BE LONG ENOUGH TO REACH THE MAIN STRUCTURE BEHIND SAID WALL, FINISHES, ANCHORAGE SHALL BE AS INDICATED ON NOTES B.1) & B.2) ABOVE. FOR INSTALLATIONS ON VINYL SIDING OR EPS CONSULT THE ENGINEER.

(C) ANCHORS SHALL BE INSTALLED FOLLOWING ALL OF THE RECOMMENDATIONS AND SPECIFICATIONS OF THE ANCHOR'S MANUFACTURER.

(D) SEE SCHEDULE BUGH FOR EDGE DISTANCE (ED) VERSUS SPACING RELATIONSHIP FOR ANCHORS.

EDGE DISTANCE VERSUS SPACING FOR ANCHORS

MINIMUM ANCHOR SPACINGS ARE VALID FOR 3 1/2" EDGE DISTANCE FOR 1/4" TAPCON AND SOLID SET AND WEDGE-BOLT FOR E. D. LESS THAN THE ABOVE MENTIONED. REDUCE ANCHOR SPACING BY MULTIPLYING SPACING SHOWN ON SCHEDULE BY THE FOLLOWING FACTORS. (NOTE: MIN. E. D. FOR ALL POINT SOLID SET ANCHORS IS 3"). REDUCED SPACING OBTAINED USING FACTOR SHALL NOT BE LESS THAN MINIMUM SPACING INDICATED FOR EACH ANCHOR TYPE.

ACTUAL E. D.	FACTOR		
	WEDGE-BOLT	1/4" TAPCON	SOLID SET
3"	0.83	0.83	0.78
2 1/2"	0.66	0.66	-
2"	0.50	0.50	-

F.B.C. (Non High Velocity Hurricane Zone)

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BERTHA HV™
Accordion Shutter System

REV. NO.	DESCRIPTION	DATE	Drawn By: J.S.S.
1	AS SHOWN	DATE: 7/12/05	
2			DRAWING No
3			05-196
4			SHEET
5			11 OF 14



TILTECO TESTING & ENGINEERING COMPANY
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FLORIDA No. 00000000

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**MAXIMUM DESIGN PRESSURE RATING (p.s.f.) AND CORRESPONDING MAXIMUM ANCHOR* SPACING (in.) SCHEDULE FOR
A GIVEN MAX. SHUTTER SPAN, NEGATIVE DESIGN LOAD AND A CORRESPONDING MOUNTING TYPE.**

(SEE MOUNTINGS ON SHEETS 13 & 14).

MAXIMUM SPAN (ft)	SUBSTRATE																																					
	SOUTHERN PINE No. 2 W/ MIN. G=0.55, DOUGLAS FIR W/ MIN. G=0.50												SPRUCE PINE FIR NORTH W/ MIN. G=0.43												SPRUCE PINE FIR SOUTH W/ MIN. G=0.36												1/2" MIN. CDX PLYWOOD	
	WALL MOUNTINGS 1W, 2W, 3W & 6W						CEILING MOUNT 8W FLOOR MOUNTINGS 9W & 10W						WALL MOUNTINGS 1W, 2W, 3W & 6W						CEILING MOUNT 8W FLOOR MOUNTINGS 9W & 10W						WALL MOUNTINGS 1W, 2W, 3W & 6W						CEILING MOUNT 8W FLOOR MOUNTINGS 9W & 10W						WALL MOUNTINGS 4W & 5W	
	NEGATIVE DESIGN LOAD (psf)						NEGATIVE DESIGN LOAD (psf)						NEGATIVE DESIGN LOAD (psf)						NEGATIVE DESIGN LOAD (psf)						NEGATIVE DESIGN LOAD (psf)						NEGATIVE DESIGN LOAD (psf)						NEGATIVE DESIGN LOAD (psf)	
	30	40	60	75	90	105	30	40	60	75	90	105	30	40	60	75	90	105	30	40	60	75	90	105	30	40	60	75	90	105	30	40	60	75	90	105		
3'-0" OR LESS	12	10	10	9	8	8	8	8	6	6	6	6	12	10	10	9	8	8	8	8	6	6	6	6	6	6	6	6	6	6	6	5	4	4	4	3		
4'-0"	12	10	10	9	8	8	8	8	6	6	6	6	12	10	10	9	8	8	8	8	6	6	6	6	6	6	6	6	6	6	5	4	4	4	3			
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6'-0"	12	10	10	9	8	8	8	8	6	6	6	6	12	10	10	9	8	8	8	8	6	6	6	6	6	6	6	6	5	4	4	4	3					
7'-0"	12	10	10	9	8	8	8	8	6	6	6	6	12	10	10	9	8	8	8	8	6	6	6	6	6	6	6	5	4	4	3.5	3	3	3	3			
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16'-0"	12	10	10	9	8	8	8	8	6	6	6	6	12	10	10	9	8	8	8	8	6	6	6	6	6	6	6	5	4	4	3	3	3	3	3			

+ NOTES ON ANCHORS REQUIREMENTS: WOOD SUBSTRATES.

1. ANCHORS TO WALL, FLOOR OR SOLID WOOD JOIST SHALL BE AS FOLLOWS:

- 1/4" TYPICAL ANCHORS, AS MANUFACTURED BY L.T.V. BUILDING MATERIALS COMPANY AS INDICATED AT EACH SECTION ON SHEETS 13 & 14.
EXCEPTION: SEE NOTE 2 BELOW.

2. ANCHORS TO HOLLOW CEILING SHALL BE AS PER MOUNTING TYPE TW, SHEET 14. AND SHALL CONSIST OF LAG BOLDS AS PER NDS 1987 SPECIFICATIONS.

3. SUBSTRATE SHALL CONSIST OF THE FOLLOWING:

- SOUTHERN PINE No. 2 W/ G=0.55 (N.D.S.)
- DOUGLAS FIR W/ G=0.50 (N.D.S.)
- SPRUCE PINE FIR NORTH W/ G=0.43 (N.D.S.)
- SPRUCE PINE FIR SOUTH W/ G=0.36 (N.D.S.)
- 1/2" CDX PLYWOOD (1988 APA)

4. ANCHORS SHALL BE INSTALLED FOLLOWING ALL OF THE RECOMMENDATIONS AND SPECIFICATIONS OF THE ANCHORS MANUFACTURER.

F.B.C. (Non High Velocity Hurricane Zone)

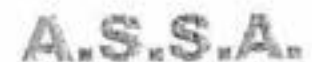
© 2003 EASTERN METAL SUPPLY, INC.

**BERTHA HV™
Accordion Shutter System**

REV. NO.	DESCRIPTION	DATE	Drawn by J.A.S.
1	NO. 1000	7/13/05	DATE: 7/13/05
2			DRAWING No
3			05-196
4			SHEET
5			12 OF 14



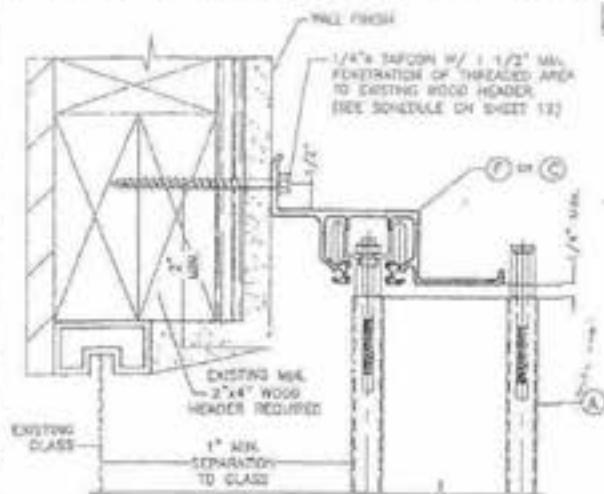
TILT TESTING & ENGINEERING COMPANY
4258 W. 30th St., Ste. 300, Miramar, Florida 33181
Phone: (305) 671-1530, Fax: (305) 671-1531
E-Mail: info@tiltco.com
813-000-7115
WALTER A. TILLY, Jr., P.E.
FLORIDA Lic. # 94751



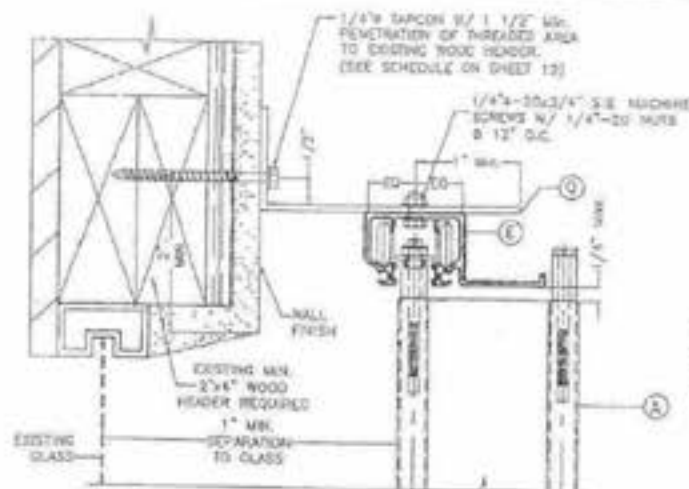
**American Shutter Systems
Association, Inc.**
4258 Westroads Drive
West Palm Beach, FL 33407

INSTALLATION DETAILS ON EXISTING WOOD BUILDINGS

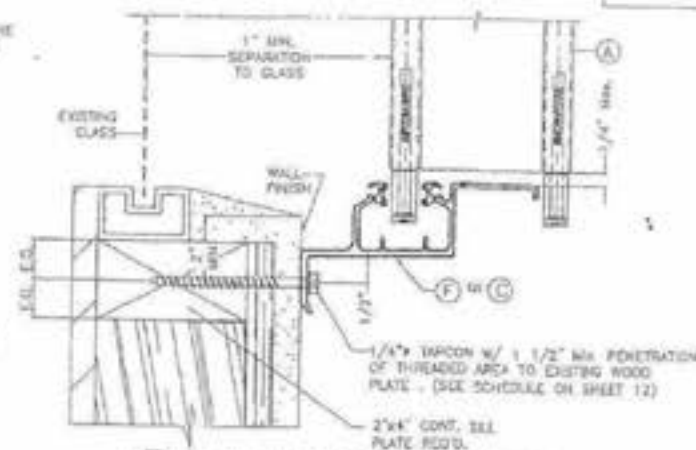
NOTE:
1-SEE SHEET 3 FOR TYPICAL SHUTTER
ASSEMBLY
2-MOUNTING SECTIONS CAN BE
COMBINED IN ANY WAY TO SUIT ANY
INSTALLATION.



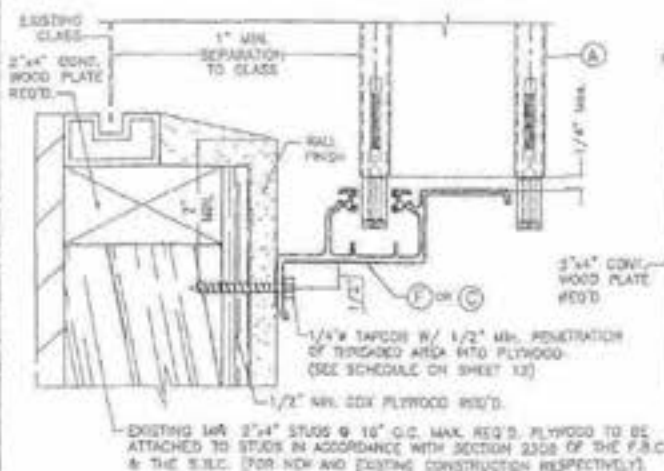
1W WALL CONNECTION AT TOP



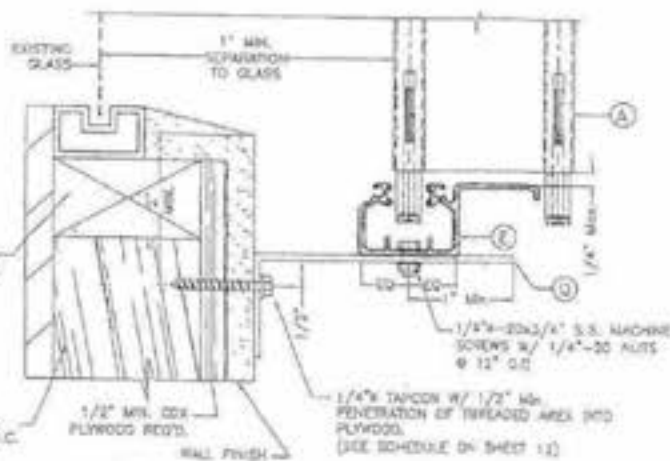
2W WALL CONNECTION AT TOP



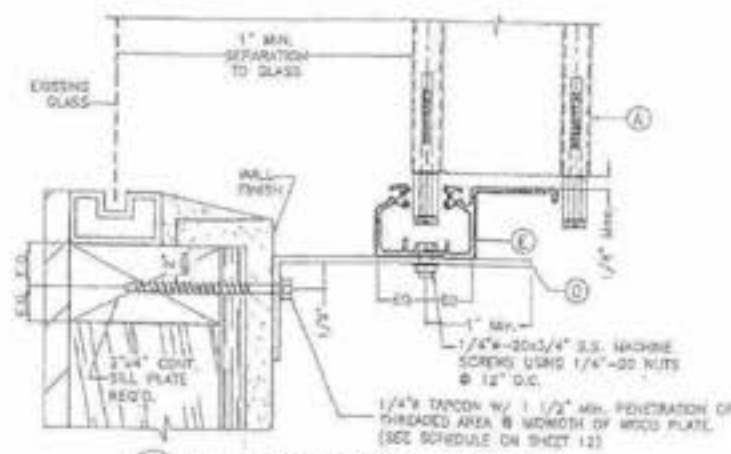
3W WALL CONNECTION AT BOTTOM



4W WALL CONNECTION AT BOTTOM



5W WALL CONNECTION AT BOTTOM



6W WALL CONNECTION AT BOTTOM

F.B.C. (Max High Velocity Hurricane Zone)

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TILECO INC.
TULLY TESTING & ENGINEERING COMPANY
2825 S.W. 38th St., Suite 300, Virginia Beach, Virginia 23462
Phone: (757) 571-1530, Fax: (757) 571-1531
www.tileco.com
BB-0004718
WALTER A. TULL, P.E.
FLORIDA Lic. # 12518

A.S.S.A.
American Shutter Systems
Association, Inc.
4265 Westroads Drive
West Palm Beach, FL 33407

BERTHA HV™
Accordion Shutter System

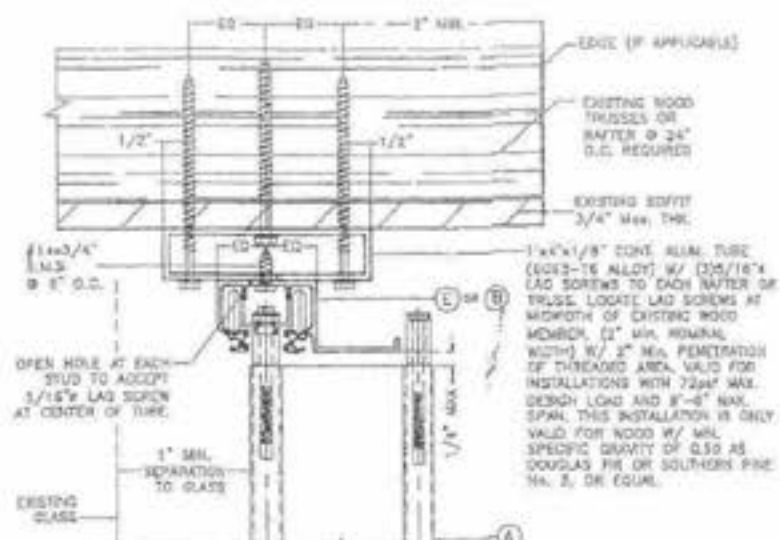
REV. NO.	DESCRIPTION	DATE	Drawn By/Checked By
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2			
3			
4			
5			
6			

DRAWING NO.

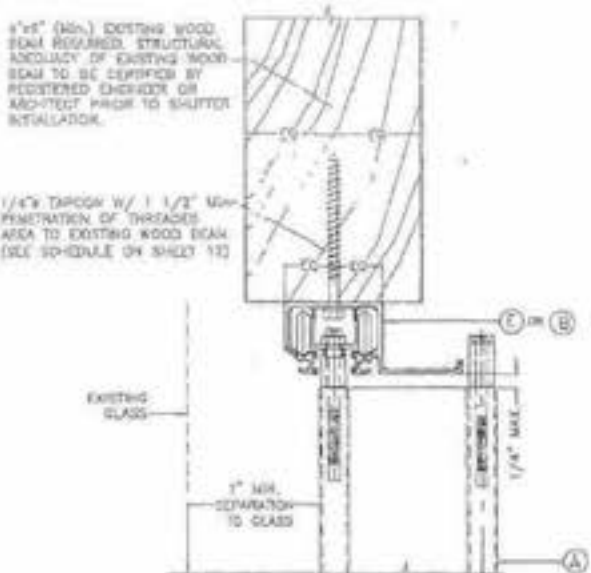
05-196

SHEET

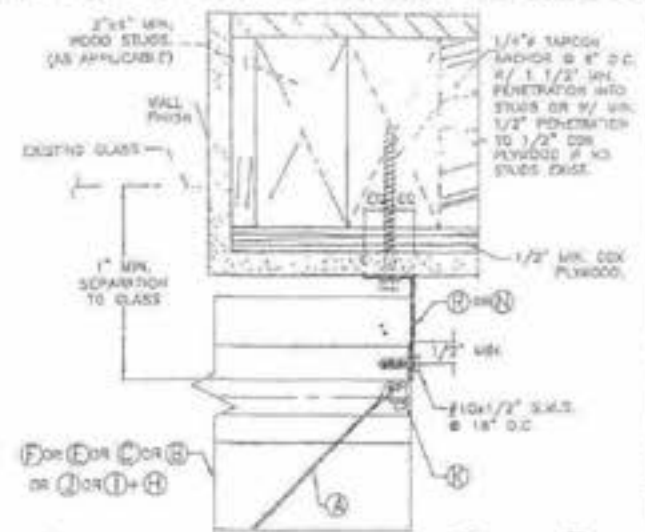
13 OF 14



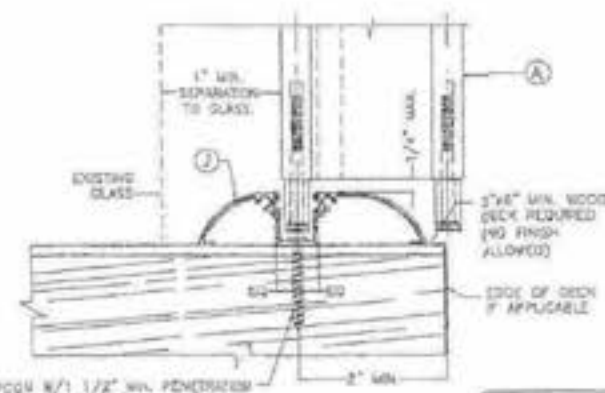
7W HOLLOW CEILING MOUNTING INSTALLATION



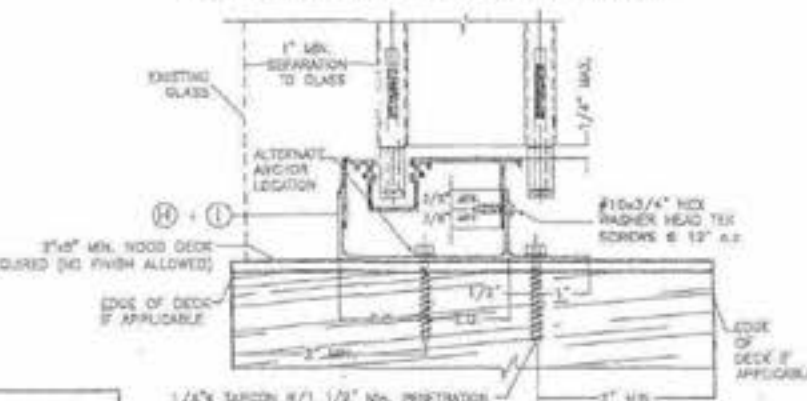
8W SOFFIT MOUNTING INSTALLATION



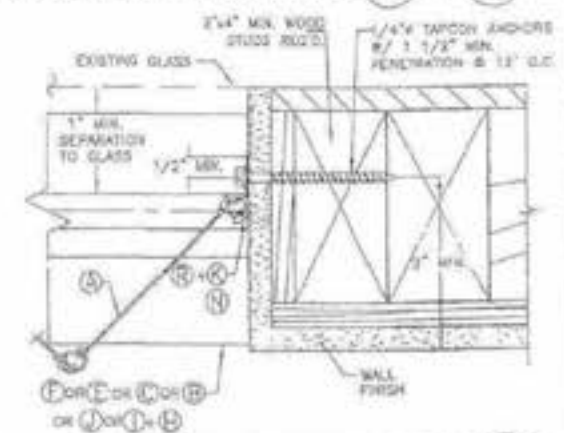
END CONNECTION DETAIL 1W & 4W



9W FLOOR CONNECTION



10W FLOOR CONNECTION



END CONNECTION DETAIL 5W

NOTES:
1-SEE SHEET 3 FOR TYPICAL SHUTTER ASSEMBLY
2-MOUNTING SECTIONS CAN BE COMBINED
IN ANY WAY TO SUIT ANY INSTALLATION.

F.B.C. (Non High Velocity Hurricane Zone)

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JOHN A. TILLEY & SONS
P.E. SEAL/SIGNATURE/DATE

TILTECO INC.
TILIT TESTING & ENGINEERING COMPANY
4500 N.W. 30th St., Ft. Lauderdale, Florida 33309
Phone: (954) 371-1520 Fax: (954) 371-1521
e-mail: tiliteco@aol.com
OR-2008712
WALTER A. TILIT, J.E.E.
FLORIDA Lic. # 44197

A.S.S.A.
American Shutter Systems
Association, Inc.
4255 Westroads Drive
West Palm Beach, FL 33407

BERTHA HV™ Accordion Shutter System			
REV. NO.	DESCRIPTION	DATE	Drawn By: J.A.S.
1	REV. 01-000	1/1/00	DATE: 7/13/03
2			DRAWING NO.
3			05-196
4			SHEET
5			14 OF 16
6			

BUILDING'S PEAK ROOF HEIGHT vs DESIGN LOAD FOR COMPONENTS & CLADDING (P.S.F.) FOR ASCE 7-02 - PEAK ROOF HEIGHT LESS THAN OR EQUAL TO 60 FEET *

BASIC WIND SPEED = 140 mph CATEGORY II BUILDINGS (I=1.00)
RESIDENTIAL, APARTMENT BUILDINGS & COMMERCIAL BUILDINGS (RETAIL, WHOLESALE)

DESIGN LOADS INDICATED BELOW SHALL ONLY BE USED FOR THE DETERMINATION OF POSITIVE OR NEGATIVE PRESSURES FOR A GIVEN OPENING OR OPENINGS OF A BUILDING WHERE SAID OPENINGS ARE SURROUNDED BY OTHER OPENINGS LOCATED WITHIN THE LOWEST 60 FEET OF BUILDING, AND WHICH ARE PROTECTED AGAINST WIND BORNE DEBRIS BY MEAN OF IMPACT RESISTANT GLASS OR ADEQUATE HURRICANE SHUTTERS. ***

DESIGN LOADS INDICATED BELOW SHALL ONLY BE USED FOR THE DETERMINATION OF POSITIVE OR NEGATIVE PRESSURES FOR A GIVEN OPENING OR OPENINGS OF A BUILDING WHERE SAID OPENINGS ARE SURROUNDED BY OTHER OPENINGS LOCATED WITHIN THE LOWEST 60 FEET OF BUILDING, AND WHICH ARE NOT PROTECTED AGAINST WIND BORNE DEBRIS BY MEAN OF IMPACT RESISTANT GLASS OR ADEQUATE HURRICANE SHUTTERS. ***

PEAK ROOF HEIGHT (ft)	1** LESS THAN 7'-0"		1** GREATER OR EQUAL THAN 7'-0" AND LESS THAN 12'-0"		1** GREATER OR EQUAL THAN 12'-0"		1** LESS THAN 7'-0"		1** GREATER OR EQUAL THAN 7'-0" AND LESS THAN 12'-0"		1** GREATER OR EQUAL THAN 12'-0"	
	POSITIVE	NEGATIVE	POSITIVE	NEGATIVE	POSITIVE	NEGATIVE	POSITIVE	NEGATIVE	POSITIVE	NEGATIVE	POSITIVE	NEGATIVE
0' - 15'	42.7	57.3	40.5	52.4	37.8	48.8	58.1	70.8	53.9	65.8	51.2	60.2
> 15' - 20'	45.4	60.8	43.0	55.7	40.1	49.7	59.8	75.0	57.3	69.9	54.4	63.9
> 20' - 25'	47.8	63.7	45.1	58.4	42.1	52.1	62.5	78.6	60.0	73.3	57.0	67.0
> 25' - 30'	49.4	66.2	46.8	60.7	43.7	54.1	64.3	81.7	62.4	76.2	59.2	69.8
> 30' - 40'	52.5	70.3	49.8	64.4	46.4	57.5	68.0	86.8	66.3	80.9	62.9	74.0
> 40' - 50'	55.0	73.7	52.2	67.5	48.7	60.3	72.3	91.0	69.4	84.8	65.9	77.5
> 50' - 60'	57.2	76.8	54.2	70.2	50.8	62.6	75.1	94.5	72.2	88.1	68.5	80.8

BUILDING'S PEAK ROOF HEIGHT vs DESIGN LOAD FOR COMPONENTS & CLADDING (P.S.F.) FOR ASCE 7-02 - PEAK ROOF HEIGHT GREATER THAN 60 FEET *

BASIC WIND SPEED = 140 mph CATEGORY II BUILDINGS (I=1.00)
RESIDENTIAL, APARTMENT BUILDINGS & COMMERCIAL BUILDINGS (RETAIL, WHOLESALE)

DESIGN LOADS INDICATED BELOW SHALL ONLY BE USED FOR THE DETERMINATION OF POSITIVE OR NEGATIVE PRESSURES FOR A GIVEN OPENING OR OPENINGS OF A BUILDING WHERE SAID OPENINGS ARE SURROUNDED BY OTHER OPENINGS LOCATED WITHIN THE LOWEST 60 FEET OF BUILDING, AND WHICH ARE PROTECTED AGAINST WIND BORNE DEBRIS BY MEAN OF IMPACT RESISTANT GLASS OR ADEQUATE HURRICANE SHUTTERS. ***

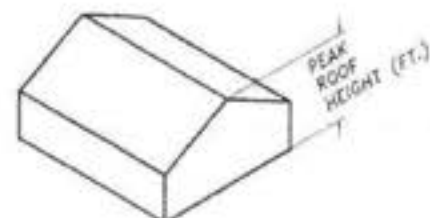
DESIGN LOADS INDICATED BELOW SHALL ONLY BE USED FOR THE DETERMINATION OF POSITIVE OR NEGATIVE PRESSURES FOR A GIVEN OPENING OR OPENINGS OF A BUILDING WHERE SAID OPENINGS ARE SURROUNDED BY OTHER OPENINGS LOCATED WITHIN THE LOWEST 60 FEET OF BUILDING, AND WHICH ARE NOT PROTECTED AGAINST WIND BORNE DEBRIS BY MEAN OF IMPACT RESISTANT GLASS OR ADEQUATE HURRICANE SHUTTERS. ***

PEAK ROOF HEIGHT (ft)	1** LESS THAN 12'-0"		1** GREATER OR EQUAL THAN 12'-0"		1** LESS THAN 12'-0"		1** GREATER OR EQUAL THAN 12'-0"	
	POSITIVE	NEGATIVE	POSITIVE	NEGATIVE	POSITIVE	NEGATIVE	POSITIVE	NEGATIVE
> 60' - 70'	54.1	68.1	49.2	65.3	72.6	117.7	67.7	103.8
> 70' - 80'	55.8	102.0	50.8	87.7	74.7	123.0	69.6	106.7
> 80' - 90'	57.0	104.5	51.8	89.8	76.6	124.1	71.4	108.4
> 90' - 100'	58.3	106.9	53.0	91.9	78.3	126.8	73.0	111.9
> 100' - 120'	60.6	111.7	55.1	95.5	81.3	131.8	75.8	116.3
> 120' - 140'	62.8	114.7	56.9	98.7	84.0	136.2	78.3	120.1
> 140' - 160'	64.4	118.0	58.5	101.5	86.4	140.0	80.8	123.5
> 160' - 180'	66.0	121.0	60.0	104.0	88.6	143.6	82.6	126.6
> 180' - 200'	67.5	123.7	61.3	106.3	90.6	146.8	84.4	129.5
> 200' - 250'	70.7	129.6	64.3	111.5	94.9	153.8	88.5	135.7
> 250' - 300'	73.5	134.7	66.8	115.8	98.6	158.9	92.0	141.0
> 300' - 350'	75.9	139.1	69.0	119.8	101.9	165.1	95.0	145.8
> 350' - 400'	78.1	143.1	71.0	123.7	104.8	169.8	97.7	149.8
> 400' - 450'	80.0	146.7	72.7	126.1	107.4	174.1	100.2	153.8
> 450' - 500'	81.8	150.0	74.4	128.0	109.8	178.0	102.4	157.0

* INDICATED PRESSURES CORRESPOND TO THE MAINWIND ZONE AND INTERIOR ZONE CONDITIONS AND FOR ALL ROOF SLOPES.

** L = STORM PANEL LENGTH, ACCORDING SHUTTER, BLADE LENGTH OR BAHAMA SHUTTER HEIGHT

*** PRODUCTS WHICH HAVE A MAIN-STATE COUNTY OR A F.E.C. APPROVAL



BUILDING SCHEMATIC

NOTES:

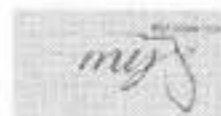
- 1.) THIS SCHEDULE, FORMS AND DETAILS AND LOAD SCHEDULE, PREPARED BY THE ENGINEER, BASED ON ASCE 7-02, SECTION 6.5.5.1, FOR A BASIC WIND SPEED OF 140 MPH AND APPLICABLE WINDWAVE FACTORS & APPROPRIATE, IS SOLELY GENERAL AND DOES NOT PROVIDE INFORMATION FOR A SPECIFIC PROJECT.
- 2.) THE WIND LOAD SCHEDULE MUST BE REVIEWED, SIGNED AND DATED BY THE REGISTERED PROFESSIONAL ENGINEER, THE BUILDING PERMIT.
- 3.) CONTRACTOR OR PERMIT HOLDER TO BE RESPONSIBLE FOR THE SELECTION OF DESIGN AND LOAD WIND SPEED ON THE SPECIFIC WIND LOAD SCHEDULE.
- 4.) THE WIND LOAD SCHEDULE SHALL BE ENDORSED WITH A SIGNATURE BY THE OWNER.
- 5.) DESIGN AND LOAD SCHEDULES BY THE CONTRACTOR OR PERMIT HOLDER FROM THIS WIND LOAD SCHEDULE SHALL BE INDICATED ON A SEPARATE DOCUMENT PREPARED BY THE CONTRACTOR OR PERMIT HOLDER (e.g., SPECIFICATIONS, OPENING DETAILS, ETC.).

2004 FLORIDA BUILDING CODE (WITH HIGH VELOCITY HURRICANE ZONES)

ASCE 7-02 / WIND LOAD SCHEDULES STORM PANELS, ACCORDION & BAHAMA SHUTTERS						AS BUILT SCALE
WIND LOAD SCHEDULE						DATE
BASIC WIND SPEED = 140 mph EXPOSURE C, K _z = 0.85						02 - 080
USE IN	DESCRIPTION	DATE	REV IN	DESCRIPTION	DATE	DRAWING NO.
1	ASCE 7-02	02/01/04	2	ASCE 7-02	02/01/04	5-001
3	ASCE 7-02	02/01/04	4	ASCE 7-02	02/01/04	5-001

TILECO INC.
TILE TESTING & ENGINEERING COMPANY
1001 W. 10th St., Suite 100, Fort Lauderdale, FL 33304
(954) 581-1111
FAX: (954) 581-1112
WWW.TILECO.COM

Community Affairs



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Product Approval
USER Table User

Product Approval Menu | Product or Application Search | Application List | Application Detail

- BUILDING CODES
- FLORIDA COMMUNITY TRUST
- FROM PUNCH FLOOR

- SEAS
- FREQUENTLY ASKED QUESTIONS
- ABOUT OUR SECRETARY
- E-MAIL THE SECRETARY
- INSPECTOR GENERAL
- WEB ASSISTANCE
- CONTACT US
- OUR LOGO
- DCA EMPLOYEE SERVICES

FI	FL2668-R1
Application Type	Revision
Code Version	2004
Application Status	Approved
Comments	
Archived	☐
Product Manufacturer	Town and Country Industries, Inc.
Address Phone Email	400 West McNab Road Ft. Lauderdale, FL 33309 (954) 970-7700 townj@tc-alum.com
Authorized Signature	Vladimir Knezevich vjk@knezevich.com
Technical Representative	Thomas B. Johnston
Address Phone Email	400 west McNab Rd. Ft. Lauderdale, FL 33309 (954) 970-9999 townj@tc-alum.com
Quality Assurance Representative	
Address Phone Email	
Category	Shutters
Subcategory	Bahama
Compliance Method	Evaluation Report from a Florida Registered Architect or a Licensed Florida Professional Engineer ☐ Evaluation Report - Hardcopy Received
Florida Engineer or Architect Name who developed the Evaluation Report	John W. Knezevich
Florida License	PE-41961
Quality Assurance Entity	National Accreditation and Management Institute
Validated By	ORLANDO L. BLANCO, P.E.
Certificate of Independence	

Reference(Standard and Year of Standard)	Standard	Year
	ASTM E1886 and E1996	2002
	ASTM E330	2002

Equivalence of Product Standards
Certified By:

Sections from the Code:

Product Approval Method Method 2 Option B

Date Submitted 9/23/2005
Date Validated 9/23/2005
Date Pending FBC Approval 10/1/2005
Date Approved 10/11/2005

Summary of Products

FL #	Model, Number or Name	Description
2668.1	Bahama Shutter	Aluminum Bahama Shutter with operable blades
Limits of Use: Approved for use in HVHZ: Approved for use outside HVHZ: Impact Resistant: Design Pressure: + Other: Maximum allowable design pressure = +/- 30 PSF for 6'-11" span shutter, and maximum allowable design pressure = +/- 37.5 PSF for 8'-0" span shutter. The Bahama Shutter does not provide impact protection in the open position or in the closed position with open louver blades. To provide impact protection, the shutter must be in the closed position and the louver blades must be locked closed. Maximum Bahama Shutter projection = 8'-0". See reference drawing for allowable angles of operation for varying projections. Maximum Bahama Shutter louver blade span = 3'-2". This product is NOT suitable for installation in the High Velocity Hurricane Zone. This product may only be installed on concrete, hollow concrete block or wood substrates. For all other conditions site specific design shall be by this office or our delegated engineer.		Installation Instructions Verified By: Evaluation Reports PTID 2668_R1_T 05-353 DWGs_Bahama.pdf PTID 2668_R1_T 05-355 Report_Bahama.pdf

[Back](#) [Next](#)

DCA Administration
Department of Community Affairs
Florida Building Code Online
Codes and Standards
2333 Shumard Oak Boulevard
Tallahassee, Florida 32399-2100
1830J 407-1824, Bureau 777-1824, Fax 1830J 474-8456
© 2000-2005 The State of Florida. All rights reserved. Copyright and Disclaimer
Product Approval Accepts:



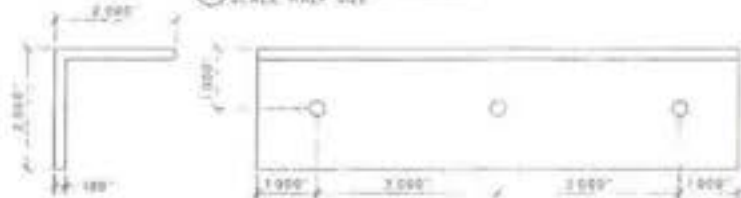


GENERAL NOTES:

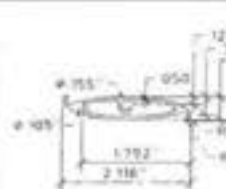
1. THESE PRODUCT EVALUATION DOCUMENTS REPRESENT A SHUTTER SYSTEM DESIGNED AND TESTED IN ACCORDANCE WITH THE FLORIDA BUILDING CODE 2004.
2. THIS SHUTTER SYSTEM HAS BEEN TESTED FOR LARGE MISSILE IMPACT RESISTANCE AND CYCLIC PRESSURE LOADING IN CONFORMANCE WITH ASTM E1996-02 AND E1996-03 STANDARDS AND FOR UNIFORM STATIC AIR PRESSURE IN CONFORMANCE WITH ASTM E1996-03 STANDARD. REFERENCED TEST REPORT IS REPORT NO. E2023-0210-04 & NO. E2023-0217-04, PREPARED BY HURRICANE TESTING LABORATORY.
3. NO INCREASE IN ALLOWABLE STRESS HAS BEEN USED IN THE DESIGN OF THIS PRODUCT.
4. DETERMINE THE POSITIVE AND NEGATIVE DESIGN LOADS TO USE WHEN REFERENCING THESE DOCUMENTS IN ACCORDANCE WITH THE GOVERNING CODE AND GOVERNING WIND VELOCITY. FOR WIND LOAD CALCULATIONS IN ACCORDANCE WITH ASCE 7-99, A DIRECTIONALITY FACTOR OF $R_e = 0.85$ MAY BE USED.
5. THESE PRODUCT EVALUATION DOCUMENTS ARE PREPARED BY THE PRODUCT ENGINEER AND ARE GENERIC. THEY DO NOT INCLUDE INFORMATION PREPARED FOR A SPECIFIC SITE.
6. USE OF THESE APPROVAL DOCUMENTS SHALL COMPLY WITH CHAPTER 4105.33 OF THE FLORIDA ADMINISTRATIVE CODE.
7. THESE PRODUCT EVALUATION DOCUMENTS ARE SUITABLE TO BE APPLIED BY THE CONTRACTOR PROVIDED THE CONTRACTOR DOES NOT DEVIATE FROM THE CONDITIONS SET FORTH HEREIN AND THE CONTRACTOR VERIFIES THE EXISTING STRUCTURE IS CAPABLE OF SUPPORTING THE SUPERIMPOSED LOADS.
8. ALTERATIONS OR ADDITIONS TO THIS DOCUMENT ARE NOT PERMITTED.
9. WHEN THE SITE CONDITIONS DEVIATE FROM THESE PRODUCT EVALUATION DOCUMENTS, SITE SPECIFIC DOCUMENTS SHALL BE PREPARED BY A FLORIDA REGISTERED ENGINEER OR ARCHITECT. IF THE DEVIATING SITE SPECIFIC DOCUMENTS ARE PREPARED BY A DELEGATED FLORIDA REGISTERED ENGINEER OR ARCHITECT, SAID DOCUMENTS SHALL BEAR THE DATE, SIGNATURE AND ORIGINAL SEAL OF THE DELEGATED ENGINEER OR ARCHITECT AND SHALL BE SUBMITTED TO THE PRODUCT ENGINEER FOR REVIEW.
10. COMPONENTS SHALL BE LABELED WITH A MINIMUM OF ONE LABEL PER DRAWING OR THE MANUFACTURER AND ITS LABEL SHALL PROVIDE A CERTIFICATE OF COMPLIANCE WHERE LABELS ARE USED. THE LABEL SHALL READ AS FOLLOWS:
 JOHN & COUNTRY INDUSTRIES, INC.
 11 LAUDERDALE, FL
 ASTM LARGE MISSILE IMPACT RESISTANT
11. MAXIMUM ALLOWABLE DESIGN PRESSURE = ± 5.0 PSF FOR 6'-11" MAX. SPANS
 ± 3.75 PSF FOR 8'-0" MAX. SPANS.
12. ALL BOLTS AND WASHERS SHALL BE GALVANIZED OR STAINLESS STEEL WITH A MINIMUM TENSILE STRENGTH OF 60 KSI U.G.N.
13. ALL EXTRUSIONS SHALL BE 6063-T6 ALUMINUM ALLOY, U.G.N.
14. BLADES MUST BE CLOSED & LOCKED FOR WIND AND IMPACT PROTECTION.



10. "2" BAR - PROFILE & LAYOUT
SCALE: HALF SIZE



12. ANGLE - PROFILE & LAYOUT
SCALE: HALF SIZE



1. BLADE
SCALE: HALF SIZE



5. JAMB SNAP
SCALE: HALF SIZE



2. FEMALE MULLION
SCALE: HALF SIZE



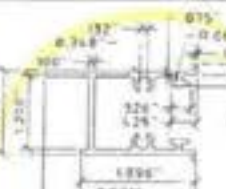
6. HEADER
SCALE: HALF SIZE



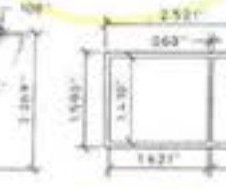
3. MALE MULLION
SCALE: HALF SIZE



7. HEADER HINGE
SCALE: HALF SIZE



4. JAMB
SCALE: HALF SIZE



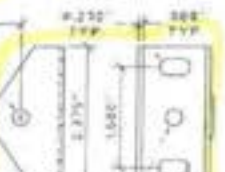
8. SILL
SCALE: HALF SIZE



9. ANGLE
SCALE: HALF SIZE



11. ANGLE
SCALE: HALF SIZE



13. CAMEL BACK HINGE
SCALE: HALF SIZE



23. ALT. CAMEL BACK HINGE
SCALE: HALF SIZE



18. PIN - 3/8" Ø
SCALE: HALF SIZE



17. PIN - 1/4" Ø
SCALE: HALF SIZE



21. TENSION RING
SCALE: FULL SIZE



19. GALV. STEEL RING PLATE
INSERT FOR MULLION
SCALE: FULL SIZE



20. TAB STOP
SCALE: FULL SIZE



22. ALT. TAB STOP
SCALE: FULL SIZE



14. ALUMINUM PIPE
SCALE: HALF SIZE



15. ALUMINUM PIPE
SCALE: HALF SIZE

PIPE SCHEDULE			
SHUTTER SPAN (L)	1	01	02
6'-0"	0.038	0.073	0.119
8'-0"	0.050	0.099	0.151

FLORIDA BUILDING CODE, 2004

Thomson-Tomasetti

BAHAMA SHUTTER

John & Country

05-355

Date: 4/27/07



**Town of Sewall's Point
BUILDING PERMIT APPLICATION**

Permit Number: _____

OWNER/TITLEHOLDER NAME ROBERT CUSHING Phone (Day) 772-283-5016 (Fax) _____

Job Site Address: 102 ABILE CT. City: SEWALLS POINT State: FL Zip: 34996

Legal Desc. Property (Subd/Lot/Block) LOT 22 SEWALLS MEADOW (PDN # 52) Parcel Number: 13-38.41-013.000.002200

Owner Address (if different): _____ City: _____ State: _____ Zip: _____

Description of Work To Be Done: add accordion shutters to 20 openings, Bahama's for 3 openings

WILL OWNER BE THE CONTRACTOR?:

YES ☐ NO ☒

COST AND VALUES:

Estimated Cost of Construction or Improvements: \$ 18,665⁰⁰
(Notice of Commencement needed over \$2500)

Estimated Fair Market Value prior to improvement: \$ _____

Is improvement cost 50% or more of Fair Market Value? YES ☐ NO ☐

Method of Determining Fair Market Value: _____

(If no, fill out the Contractor & Subcontractor sections below)

(If yes, Owner Builder Affidavit must accompany application)

CONTRACTOR/Company: COASTAL SHUTTERS OF THE TC Phone: 772-335-9198 Fax: 772-335-7077

Street: 1386 SE HUFFMAN Rd City: PORT ST LUCIE State: FL Zip: 34952

State Registration Number: _____ State Certification Number: _____ Martin County License Number: CSCL131149646

SUBCONTRACTOR INFORMATION:

Electrical: _____ State: _____ License Number: _____

Mechanical: _____ State: _____ License Number: _____

Plumbing: _____ State: _____ License Number: _____

Roofing: _____ State: _____ License Number: _____

ARCHITECT _____ Lic.#: _____ Phone Number: _____

Street: _____ City: _____ State: _____ Zip: _____

ENGINEER _____ Lic.#: _____ Phone Number: _____

Street: _____ City: _____ State: _____ Zip: _____

AREA SQUARE FOOTAGE - SEWER - ELECTRIC Living: _____ Garage: _____ Covered Patio: _____ Screened Porch: _____

Carport: _____ Total Under Roof _____ Wood Deck: _____ Accessory Building: _____

NOTICE: In addition to the requirements of this permit, there may be additional restrictions applicable to this property that may be found in the public records of this county, and there may be additional permits required from other governmental entities such as water management districts, state agencies, or federal agencies.

CODE EDITIONS IN EFFECT AT TIME OF APPLICATION: Florida Building Code (Structural, Mechanical, Plumbing, Gas): 2004
National Electrical Code: 2002 Florida Energy Code: 2004 Florida Accessibility Code: 2004 Florida Fire Code 2004

I HEREBY CERTIFY THAT THE INFORMATION I HAVE FURNISHED ON THIS APPLICATION IS TRUE AND CORRECT TO THE BEST OF MY KNOWLEDGE AND I AGREE TO COMPLY WITH ALL APPLICABLE CODES, LAWS AND ORDINANCES DURING THE BUILDING PROCESS.

OWNER OR AGENT SIGNATURE (required)
[Signature]
State of Florida, County of Martin
This the 27th day of April, 2007
by Robert Cushing who is personally

known to me or produced
as Identification C25E-762-42-140-0
[Signature] Notary Public
My Commission Expires 4/2/10
Seal

CONTRACTOR SIGNATURE (required)
[Signature]
On State of Florida, County of Martin
This the 27th day of April, 2007
by James C. Koorich who is personally

known to me or produced
as Identification [Signature]
My Commission Expires 4/2/10



PERMIT APPLICATIONS VALID 30 DAYS FROM APPROVAL NOTIFICATION - PLEASE PICK UP YOUR PERMIT PROMPTLY



Martin County, Florida **Laurel Kelly, C.F.A**

Site Provided by...
 governmax.com T1.14

Summary

print Owner 4 of 5

Parcel Info

Summary

Land

Residential

Improvement

Commercial

Image

Sales & Transfers

Assessments →

Taxes →

Parcel Map →

Full Legal →

Parcel ID 13-38-41-013-000-00220-0
Unit Address 102 ABBIE CT

Serial ID 119126
Index Order Owner
Commercial 0
Residential 1

Summary

Property Location 102 ABBIE CT
Tax District 2200 Sewall's Point
Account # 119126
Land Use 101 0100 Single Family
Neighborhood 120300
Acres 0.577

Legal Description

Property Information
 LOT 22 SEWALL'S MEADOW (PB 14 PG 32)

Did call -
 this is to
 be pulled
 in Sewall's
 point
 Jim

Search By

Parcel ID

Owner

Address

Account #

Use Code

Legal Description

Neighborhood

Sales

Map →

Owner Information

Owner Information
CUSHING, ROBERT B & PEARL C

Mail Information

102 ABBIE COURT
 STUART FL 34996

Assessment Info

Front Ft. 0.00

Market Land Value \$340,000

Market Impr Value \$529,540

Market Total Value \$869,540

Site Functions

Property Search

Contact Us

On-Line Help

County Home

Site Home

County Login

Recent Sale

Sale Amount \$552,100

Sale Date 9/14/2001

Book/Page 1582 0288

Print | Back to List | << First < Previous Next > Last >>

Legal disclaimer / Privacy Statement

Data updated on 04/19/2007

Powered by
MANATRON

CERTIFICATE OF INSURANCE

The company indicated below certifies that the insurance afforded by the policy or policies numbered and described below is in force as of the effective date of this certificate. This Certificate of Insurance does not amend, extend, or otherwise alter the terms and Conditions of Insurance coverage contained in any policy numbered and described below.

CERTIFICATE HOLDER:

TOWN OF SEWALLS POINT
1 SOUTH SEWALLS POINT ROAD
SEWALLS POINT, FL 34996

INSURED:

COASTAL SHUTTERS OF THE
TREASURE COAST INC
1386 SE HUFFMAN RD
PORT ST LUCIE, FL 34952-3354

TYPE OF INSURANCE LIABILITY	POLICY NUMBER & ISSUING CO.	POLICY EFF. DATE	POLICY EXP. DATE	LIMITS OF LIABILITY (*LIMITS AT INCEPTION)
☒ Liability and Medical Expense	77-AC-710645-3001 NATIONWIDE MUTUAL INSURANCE CO.	05-23-07	05-23-08	Any One Occurrence..... \$ 2,000,000
☒ Personal and Advertising Injury				Any One Person/Org \$ 2,000,000
☒ Medical Expenses				ANY ONE PERSON \$ 5,000
☒ Fire Legal Liability				Any One Fire or Explosion \$ 100,000
				General Aggregate* \$ 2,000,000
				Prod/Comp Ops Aggregate* \$ 2,000,000
☐ Other Liability				
AUTOMOBILE LIABILITY	77-BA-710645-3002	05-23-07	05-23-08	
☒ BUSINESS AUTO	NATIONWIDE MUTUAL INSURANCE CO.			Bodily Injury (Each Person) \$
☒ Owned				(Each Accident) \$
☒ Hired				Property Damage (Each Accident) \$
☒ Non-Owned				Combined Single Limit \$ 500,000
EXCESS LIABILITY				Each Occurrence \$
☐ Umbrella Form				Prod/Comp Ops/Disease Aggregate* \$
☐ Workers' Compensation and				STATUTORY LIMITS
☐ Employers' Liability				BODILY INJURY/ACCIDENT ... \$
				Bodily Injury by Disease EACH EMPLOYEE \$
				Bodily Injury by Disease POLICY LIMIT \$

DESCRIPTION OF OPERATIONS/LOCATIONS
VEHICLES/RESTRICTIONS/SPECIAL ITEMS

Effective Date of Certificate: 05-23-2007
Date Certificate Issued: 05-21-2007

Authorized Representative: Atlantic Ins Partners, Inc.
Countersigned at: 963 CENTRAL PARKWAY
Stuart, FL 34994

AC# 2652295

STATE OF FLORIDA

DEPARTMENT OF BUSINESS AND PROFESSIONAL REGULATION
CONSTRUCTION INDUSTRY LICENSING BOARD

SEQ# L06071100705

DATE	BATCH NUMBER	LICENSE NBR
07/11/2006	058087220	SCC131149646

The SPECIALTY STRUCTURE CONTRACTOR
Named below IS CERTIFIED
Under the provisions of Chapter 489 FS.
Expiration date: AUG 31, 2008

KOERICK, JAMES CHARLES JR
COASTAL SHUTTERS OF THE TREASURE COAST INC
1386 SE HUFFMAN ROAD
PORT SAINT LUCIE FL 34952

JEB BUSH
GOVERNOR

DISPLAY AS REQUIRED BY LAW

SIMONE MARSTILLER
SECRETARY

2006-2007

ST. LUCIE COUNTY OCCUPATIONAL LICENSE

BOB DAVIS, CPA, CGFO, CFC, ST. LUCIE COUNTY TAX COLLECTOR

1799-17994033

EXPIRES SEP 30, 2007

FACILITIES
OR
MACHINES

ROOMS

SEATS

EMPLOYEES 1-10

TYPE OF BUSINESS 1799-SPECIALTY CONTRACTOR

BUSINESS LOCATION 1386 SE Hoffman Rd
City of Pt St Lucie

NAME James Charles Koerick Jr SCC131149646
MAILING Coastal Shutter of the TC Inc
ADDRESS Koerick, James C
1386 SE Hoffman Rd
Port St Lucie FL 34952

X RENEWAL
NEW LICENSE
TRANSFER-
ORIGINAL TAX 11.80

AMOUNT
PENALTY
COLLECTION COST
TOTAL 11.80

Please see back for additional information

P00000012111

PAID 08/15/2006 99-20060815-087307 11.80

TO BE COMPLETED WHEN CONSTRUCTION VALUE EXCEEDS \$2500.00

PERMIT # _____

TAX FOLIO # 13-38-41-013-000-00220-0**NOTICE OF COMMENCEMENT**STATE OF FLORIDACOUNTY OF SEWALL'S POINT

THE UNDERSIGNED HEREBY GIVES NOTICE THAT IMPROVEMENT WILL BE MADE TO CERTAIN REAL PROPERTY, AND IN ACCORDANCE WITH CHAPTER 713, FLORIDA STATUTES, THE FOLLOWING INFORMATION IS PROVIDED IN THIS NOTICE OF COMMENCEMENT.

LEGAL DESCRIPTION OF PROPERTY (INCLUDE STREET ADDRESS IF AVAILABLE):LOT 22 SEWALL'S MEADOW (PB 14 PG 32)**GENERAL DESCRIPTION OF IMPROVEMENT:** adding shutters TO 23 openings**OWNER:** ROBERT B CUSHING**ADDRESS:** 102 ABBIE CT. SEWALL'S POINT, FL. 34996**PHONE #:** 72-283-5016**FAX #:** _____**CONTRACTOR:** COASTAL SHUTTERS OF THE TREASURE COAST**ADDRESS:** 1386 SE HUFFMAN ROAD. PORT ST LUCIE, FL 34952**PHONE #:** 72-335-9198**FAX #:** 72-335-7077**SURETY COMPANY (IF ANY):** _____**ADDRESS:** _____**PHONE #:** _____**FAX #:** _____**BOND AMOUNT:** _____**LENDER:** _____**ADDRESS:** _____**PHONE #:** _____**FAX #:** _____STATE OF FLORIDA
HARTIN COUNTY

THIS IS TO CERTIFY THAT THE
FOREGOING _____ PAGES IS A TRUE
AND CORRECT COPY OF THE ORIGINAL
MARSHA EWING CLERK

BY [Signature] D.C.
DATE 5.17.07



PERSONS WITHIN THE STATE OF FLORIDA DESIGNATED BY OWNER UPON WHOM NOTICES OR OTHER DOCUMENTS MAY BE SERVED AS PROVIDED BY SECTION 713.13(1)(A)7, FLORIDA STATUTES:

NAME: _____**ADDRESS:** _____**PHONE #:** _____**FAX #:** _____

IN ADDITION TO HIMSELF, OWNER DESIGNATES

OF _____ TO RECEIVE A COPY OF THE LIENOR'S NOTICE AS PROVIDED IN SECTION 713.13(1)(B), FLORIDA STATUTES.

PHONE #: _____**FAX #:** _____**EXPIRATION DATE OF NOTICE OF COMMENCEMENT:** _____

THE EXPIRATION DATE IS ONE (1) YEAR FROM THE DATE OF RECORDING UNLESS A DIFFERENT DATE IS SPECIFIED ABOVE.

SIGNATURE OF OWNER

SWORN TO AND SUBSCRIBED BEFORE ME THIS 3rd May 2007 BY Robert Cushing

PERSONALLY KNOWN

PRODUCED ID

TYPE OF ID C2276242400

NOTARY SIGNATURE

/data/gnd/bnd/bldg_forms/Noc.aw

12/01/99

INSTR # 2013938 OR BK 02248 PG 1898 RECD 05/17/2007 12:30:39 PM
Pg 1898 (1 of 1)
MARSHA EWING MARTIN COUNTY DEPUTY CLERK L. Wood



A/C #2 15 ✓ 45 ✓
A/C #1 30 ✓ 30 ✓

Date of Issuance

TOWN OF SEWALL'S POINT

One South Sewall's Point Road
Sewall's Point, Florida 34996
(772) 287-2455

CORRECTION NOTICE

ADDRESS: 102 ABBIE CT

I have this day inspected this structure and these premises and have found the following violations of the City, County, and/or State laws governing same.

SHUTTERS

PAPERWORK IS NOT USABLE -
DIRECT CONTRACTOR TO REPLACE
w/ NEW & PROTECT FROM
WEATHER & RESCHEDULE
INSPECTION

You are hereby notified that no work shall be concealed upon these premises until the above violations are corrected. When corrections have been made, call for an inspection.

DATE: 6/8

INSPECTOR

DO NOT REMOVE THIS TAG

COMMENTS:

OR:

COMMENTS:

USE

OR:

COMMENTS:

USE

OR:

COMMENTS:

OR:

COMMENTS:

OR:

COMMENTS:

OR:

COMMENTS:

INSPECTOR:

PERMIT

8613

PERMIT

8614

3

PERMIT

8611

2

PERMIT

8616

2

PERMIT

8608

4

PERMIT

7819

1

PERMIT

0027

5

OTHER:

3 TUSCAN
MASTERPIECE

INSULATION PASS

TOWN OF SEWALL'S POINT

Building Department - Inspection Log

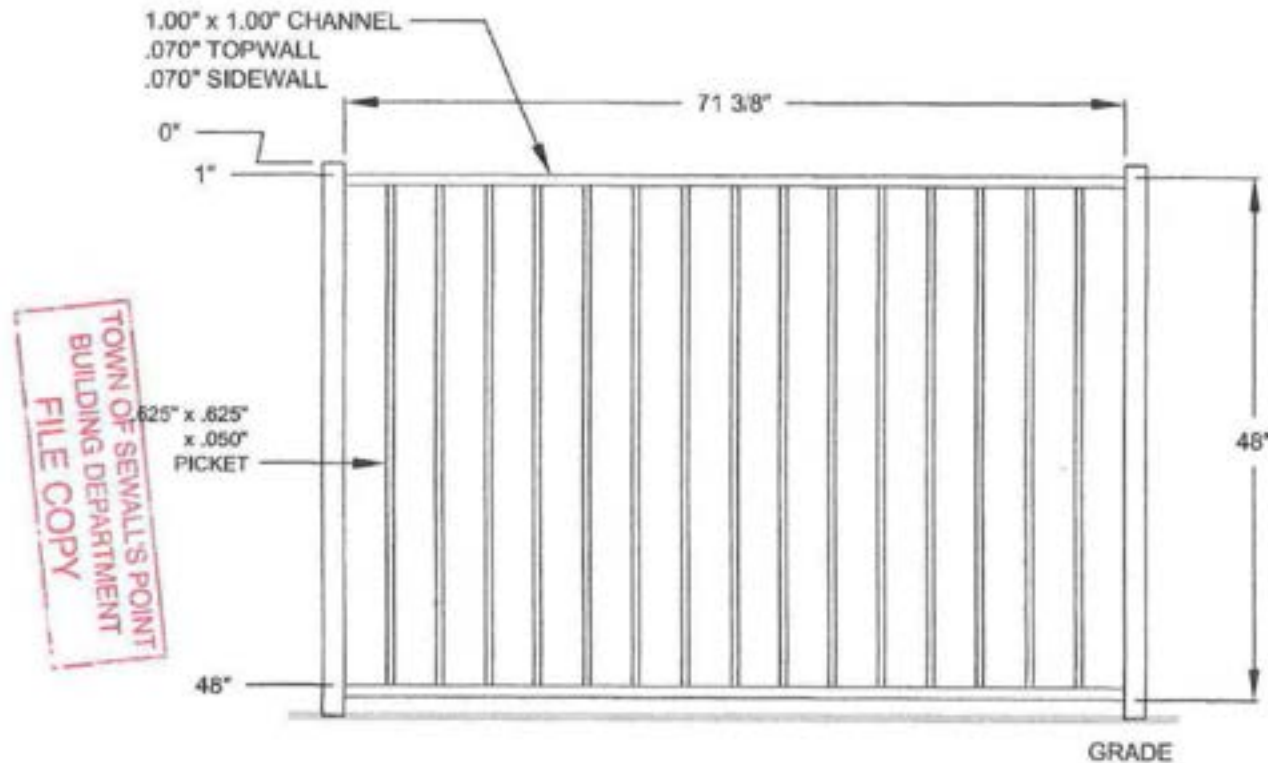
Date of Inspection: ☐ Mon ☐ Wed ☒ Fri 7-20, 2007 Page 1 of 1

PERMIT	OWNER/ADDRESS/CONTR.	INSPECTION TYPE	RESULTS	NOTES/COMMENTS:
3608	Cushing	Final	PASS	CLOSE
1	102 Abbie Ct Coastal Shutters			INSPECTOR: <i>[Signature]</i>
8647	Tompeck	Final	PASS	CLOSE
3	20 Emarieta Coastal Shutters			INSPECTOR: <i>[Signature]</i>
7682	Olsen	Final-dock	FAIL	
4	19 N River Rd O/B	repair		INSPECTOR: <i>[Signature]</i>
8401	Olsen	Final-secured	PASS	CLOSE
4	19 N River Rd 1001 (Olsen)			INSPECTOR: <i>[Signature]</i>
8638	McKinley	Final-	PASS	CLOSE
2	48 Rio Vista Monterey Glass			INSPECTOR: <i>[Signature]</i>
8528	Masterpiece Sys.	Dry in	PASS	
	S. mandalay Masterpiece Bldg.			INSPECTOR: <i>[Signature]</i>
PERMIT	OWNER/ADDRESS/CONTR.	INSPECTION TYPE	RESULTS	NOTES/COMMENTS:
				INSPECTOR:
OTHER:				

10178
FENCE

71.375" PANEL SERIES-A
2-CH 48" RESIDENTIAL FLUSH BTM

STEPHENS PIPE & STEEL (MEL)	DATE: ~~6-20-12~~ 7-31-12
PO/JOB NAME:	SWING: PANEL /white
QUOTE NO: Q10061102	HINGE TYPE: self closing
COLOR: -- white	DAYLIGHT OPENING:
APPROVAL SIGNATURE (REQUIRED):	



* Gate with childproof MagnaLatch.



TOWN OF SEWALL'S POINT BUILDING DEPARTMENT
One S. Sewall's Point Road
Sewall's Point, Florida 34996
Tel 772-287-2455 Fax 772-220-4765

BUILDING PERMIT CARD

THIS CARD MUST BE POSTED IN A CONSPICUOUS PLACE IN PLAIN VIEW FROM THE STREET PRIOR TO BEGINNING ANY WORK

A FINAL INSPECTION IS REQUIRED FOR ALL PERMITS

PERMIT NUMBER:	10178	DATE ISSUED:	AUGUST 2, 2012
SCOPE OF WORK:	FENCE		
CONTRACTOR:	OB		
PARCEL CONTROL NUMBER:	133841013000-002200	SUBDIVISION	SEWALLS MEADOW - 1.22
CONSTRUCTION ADDRESS:	102 ABBIE CT		
OWNER NAME:	CUSHING		
QUALIFIER:	OB	CONTACT PHONE NUMBER:	283-5016

WARNING TO OWNER: YOUR FAILURE TO RECORD A NOTICE OF COMMENCEMENT MAY RESULT IN YOUR PAYING TWICE FOR IMPROVEMENTS TO YOUR PROPERTY. IF YOU INTEND TO OBTAIN FINANCING, CONSULT WITH YOUR LENDER OR AN ATTORNEY BEFORE RECORDING YOUR NOTICE OF COMMENCEMENT. A CERTIFIED COPY OF THE RECORDED NOTICE OF COMMENCEMENT MUST BE SUBMITTED TO THE BUILDING DEPARTMENT PRIOR TO THE FIRST REQUESTED INSPECTION.

NOTICE: IN ADDITION TO THE REQUIREMENTS OF THIS PERMIT, THERE MAY BE ADDITIONAL RESTRICTIONS APPLICABLE TO THIS PROPERTY THAT MAY BE FOUND IN PUBLIC RECORDS OF THIS COUNTY, AND THERE MAY BE ADDITIONAL PERMITS REQUIRED FROM OTHER GOVERNMENTAL ENTITIES SUCH AS WATER MANAGEMENT DISTRICTS, STATE AGENCIES, OR FEDERAL AGENCIES.

24 HOUR NOTICE REQUIRED FOR INSPECTIONS - ALL CONSTRUCTION DOCUMENTS MUST BE AVAILABLE ON SITE
CALL 287-2455 - 8:00AM TO 4:00PM INSPECTIONS: 9:00AM TO 3:00PM - MONDAY THROUGH FRIDAY

INSPECTIONS

UNDERGROUND PLUMBING _____
UNDERGROUND MECHANICAL _____
STEM-WALL FOOTING _____
SLAB _____
ROOF SHEATHING _____
TIE DOWN /TRUSS ENG _____
WINDOW/DOOR BUCKS _____
ROOF DRY-IN/METAL _____
PLUMBING ROUGH-IN _____
MECHANICAL ROUGH-IN _____
FRAMING _____
FINAL PLUMBING _____
FINAL MECHANICAL _____
FINAL ROOF _____

UNDERGROUND GAS _____
UNDERGROUND ELECTRICAL _____
FOOTING _____
TIE BEAM/COLUMNS _____
WALL SHEATHING _____
INSULATION _____
LATH _____
ROOF TILE IN-PROGRESS _____
ELECTRICAL ROUGH-IN _____
GAS ROUGH-IN _____
METER FINAL _____
FINAL ELECTRICAL _____
FINAL GAS _____
BUILDING FINAL _____

ALL RE-INSPECTION FEES AND ADDITIONAL INSPECTION REQUESTS WILL BE CHARGED TO THE PERMIT HOLDER. THE CONTRACTOR OR OWNER /BUILDER MUST SCHEDULE A FINAL INSPECTION. FAILURE TO RECEIVE A SUCCESSFUL FINAL INSPECTION WILL RESULT IN PERMIT RENEWAL FEES, FINES, AND OR DENIAL OF FUTURE BUILDING PERMITS TO THE CONTRACTOR OR OWNER /BUILDER.

Town of Sewall's Point

BUILDING PERMIT APPLICATION

Permit Number:

10178

Date: 7-31-12

OWNER/LESSEE NAME: Robert & Pearl Cushing

Phone (Day) 772-283-5016 (Fax)

Job Site Address: 102 Abbie Court

City: Stuart

State: FL

Zip: 34996

Legal Description: LOT 22 Sewall's Meadow

Parcel Control Number: 13-38-41-013-000-00220-0

Fee Simple Holder Name:

Address:

City:

State:

Zip:

Telephone:

*SCOPE OF WORK (PLEASE BE SPECIFIC):

Fence

WILL OWNER BE THE CONTRACTOR?

(If yes, Owner/Builder questionnaire must accompany application)

YES ☒

NO ☐

Has a Zoning Variance ever been granted on this property?

YES ☐

(YEAR)

NO ☐

(Must include a copy of all variance approvals with application)

COST AND VALUES: (Required on ALL permit applications)

Estimated Value of Improvements: \$ 1873.00

(Notice of Commencement required when over \$2500 prior to first inspection, \$7,500 on HVAC change out)

Is subject property located in flood hazard area? VE10 AE9 AE8 X

FOR ADDITIONS, REMODELS AND RE-ROOF APPLICATIONS ONLY:

Estimated Fair Market Value prior to improvement: \$

(Fair Market Value of the Primary Structure only, Minus the land value)

PRIVATE APPRAISALS MUST BE SUBMITTED WITH PERMIT APPLICATION

Construction Company:

Phone:

Fax:

Qualifiers name:

Street:

City:

State:

Zip:

State License Number:

OR: Municipality:

License Number:

LOCAL CONTACT:

Phone Number:

DESIGN PROFESSIONAL:

Fla. License#

Street:

City:

State:

Zip:

Phone Number:

AREAS SQUARE FOOTAGE: Living:

Garage:

Covered Patios/Porches:

Enclosed Storage:

Carport:

Total under Roof

Elevated Deck:

Enclosed area below BFE*

* Enclosed non-habitable areas below the Base Flood Elevation greater than 300 sq. ft. require a Non-Conversion Covenant Agreement.

CODE EDITIONS IN EFFECT THIS APPLICATION: Florida Building Code (Structural, Mechanical, Plumbing, Existing, Gas): 2010

National Electrical Code: 2008, Florida Energy Code: 2010, Florida Accessibility Code: 2010, Florida Fire Prevention Code: 2010

WARNINGS TO OWNERS AND CONTRACTORS:

1. YOUR FAILURE TO RECORD A NOTICE OF COMMENCEMENT MAY RESULT IN YOUR PAYING TWICE FOR IMPROVEMENTS TO YOUR PROPERTY. WHEN FINANCING, CONSULT WITH YOUR LENDER OR AN ATTORNEY BEFORE RECORDING YOUR NOTICE OF COMMENCEMENT. A NOTICE OF COMMENCEMENT MUST BE RECORDED AND POSTED ON THE JOB SITE BEFORE THE FIRST INSPECTION.
2. IT IS YOUR RESPONSIBILITY TO DETERMINE IF YOUR PROPERTY IS ENCUMBERED BY ANY DEED RESTRICTIONS. SOME RESTRICTIONS APPLICABLE TO THIS PROPERTY MAY BE FOUND IN THE PUBLIC RECORDS OF MARTIN COUNTY OR THE TOWN OF SEWALL'S POINT. THERE MAY BE ADDITIONAL PERMITS REQUIRED FROM OTHER GOVERNMENTAL ENTITIES SUCH AS WATER MANAGEMENT DISTRICTS, STATE AGENCIES, OR FEDERAL AGENCIES.
3. BUILDING PERMITS FOR SINGLE FAMILY RESIDENCES AND SUBSTANTIAL IMPROVEMENTS TO SINGLE FAMILY RESIDENCES ARE VALID FOR A PERIOD OF 24 MONTHS. RENEWAL FEES WILL BE ASSESSED AFTER 24 MONTHS PER TOWN ORDINANCE 50-95.
4. THIS PERMIT WILL BECOME NULL AND VOID IF THE WORK AUTHORIZED BY THIS PERMIT IS NOT COMMENCED WITHIN 180 DAYS, OR IF WORK IS SUSPENDED OR ABANDONED FOR A PERIOD OF 180 DAYS AT ANY TIME AFTER THE WORK IS COMMENCED. ADDITIONAL FEES WILL BE ASSESSED ON ANY PERMIT THAT BECOMES NULL AND VOID. REF. FBC 2007 SECT. 105.4.1, 105.4.1.1 - .5.

*****A FINAL INSPECTION IS REQUIRED ON ALL BUILDING PERMITS*****

AFFIDAVIT: APPLICATION IS HEREBY MADE TO OBTAIN A PERMIT TO DO THE WORK AS SPECIFICALLY INDICATED ABOVE. I CERTIFY THAT NO WORK OR INSTALLATION HAS COMMENCED PRIOR TO THE ISSUANCE OF A PERMIT AND THAT THE INFORMATION I HAVE FURNISHED ON THIS APPLICATION IS TRUE AND CORRECT TO THE BEST OF MY KNOWLEDGE. I AGREE TO COMPLY WITH ALL APPLICABLE CODES, LAWS, AND ORDINANCES OF THE TOWN OF SEWALL'S POINT DURING THE BUILDING PROCESS.

OWNER/AGENT/LESSEE - NOTARIZED SIGNATURE:

X

State of Florida, County of: Martin

On This the 1 day of Aug 2012

by Dr. Robert Cushing who is personally

known to me or produced

As identification:

My Commission Expires: Feb 11, 2014

CONTRACTOR/LICENSEE NOTARIZED SIGNATURE:

X

State of Florida, County of:

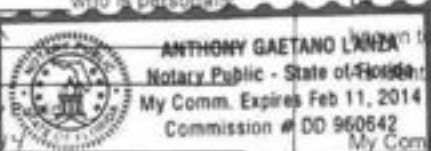
On This the day of 20

by who is personally

known to me or produced

As identification:

My Commission Expires:



SINGLE FAMILY PERMIT APPLICATIONS MUST BE ISSUED WITHIN 30 DAYS OF APPROVAL NOTIFICATION (FBC 105.3.4) ALL OTHER APPLICATIONS WILL BE CONSIDERED ABANDONED AFTER 180 DAYS (FBC 105.3.2) - PLEASE PICK UP YOUR PERMIT PROMPTLY!

**Martin County, Florida
Laurel Kelly, C.F.A***generated on 8/1/2012 10:52:26 AM EDT***Summary**

Parcel ID	Account #	Unit Address	Market Total Value	Website Updated
13-38-41-013-000-00220-0	119126	102 ABBIE COURT, SEWALL'S POINT	\$537,440	7/28/2012

Owner Information

Owner(Current)	CUSHING ROBERT B & PEARL C
Owner/Mail Address	102 ABBIE CT STUART FL 34996
Sale Date	9/14/2001
Document Book/Page	1582 0288
Document No.	JMB
Sale Price	552100

Location/Description

Account #	119126	Map Page No.	SP-05
Tax District	2200	Legal Description	LOT 22 SEWALL'S MEADOW (PB 14 PG 32)
Parcel Address	102 ABBIE COURT, SEWALL'S POINT		
Acres	.5770		

Parcel Type

Use Code	0100 Single Family
Neighborhood	120300 Sewall's Meadow

Assessment Information

Market Land Value	\$242,000
Market Improvement Value	\$295,440
Market Total Value	\$537,440



TOWN OF SEWALL'S POINT BUILDING DEPARTMENT

One S. Sewall's Point Road
Sewall's Point, Florida 34996
Tel 772-287-2455 Fax 772-220-4765

OWNER/BUILDER QUESTIONNAIRE AND DISCLOSURE STATEMENT
MUST BE COMPLETED AND REVIEWED PRIOR TO PERMIT ISSUANCE

APPLICABLE ONLY TO OWNER-OCCUPIED SINGLE FAMILY RESIDENCES
AND COMMERCIAL IMPROVEMENTS LESS THAN \$75,000 IN VALUE

NOTICE: FLORIDA STATUTE 489 REQUIRING CONSTRUCTION TO BE DONE ONLY BY LICENSED CONTRACTORS PROVIDES AN EXEMPTION FROM LICENSING FOR A PROPERTY OWNER WHO ACTS AS HIS/HER OWN CONTRACTOR UNDER SPECIFIC CONDITIONS. ANSWERS TO THE FOLLOWING QUESTIONS ARE ESSENTIAL TO DETERMINE IF THOSE STATE QUALIFICATIONS ARE SATISFIED BY AN OWNER/BUILDER APPLICANT.

ALL QUESTIONS MUST BE ANSWERED. IF A QUESTION DOES NOT APPLY, INDICATE BY WRITING "N/A"

Owner/Builder Applicant Name: Dr. Robert B. Cushing

Site address of the proposed building work: 102 Abbie Ct. Sewall's Point, FL 34996

Name of legal title owner of the address above: Dr. Robert B. Cushing

Describe the scope of work for the proposed new construction: Fence around Pool -

Name of Architect of Record: NA Structural Engineer of Record: NA

Who will supervise the trade work to meet the applicable code? Owner

What provisions have you made for Liability and Property Damage Insurance? Homeowners Ins

What provisions exist for withholding Social Security and Federal Income Taxes, as required by Federal Law, from wages paid to people you hire who are not licensed? Self

What previous Owner/Builder improvements have you done in the State of Florida?

Location: None Scope of Work Done: _____ Year: _____

Location: _____ Scope of Work Done: _____ Year: _____

What code books do you have available for reference? Building: NA

Electric: _____ Plumbing: _____ HVAC: _____

Other: _____

I have internet access and will view The Florida Building code at www.floridabuilding.org YES ☒ NO ☐

Do you understand that as the permit holder you are liable for following all Local, County, State and Federal codes, laws and requirements, and you are also liable for anyone injured on the construction site? Yes (yes/no)

Have you consulted with your Homeowner's Insurance Agent? NA Lender? _____ Attorney? _____

In order to assure your success in this project, please signify your awareness that the function of the building department is to issue you a building permit and verify code compliance through plan review and the inspection process. I am aware that town staff is not obligated to offer supervision, design or instructional advice prior or during my project. NA (Initials).



TOWN OF SEWALL'S POINT BUILDING DEPARTMENT
One S. Sewall's Point Road
Sewall's Point, Florida 34996
Tel 772-287-2455 Fax 772-220-4765

OWNER/BUILDER DISCLOSURE STATEMENT

NOTICE: STATE LAW REQUIRES THAT ALL PERMITTING AGENCIES PROVIDE INDIVIDUALS SUBMITTING APPLICATIONS FOR OWNER/BUILDER PERMITS THE FOLLOWING INFORMATION:

1. THE TOWN OF SEWALL'S POINT CODE OF LAWS AND ORDINANCES REQUIRES THAT ANY PERSON DESIRING TO ENGAGE IN BUSINESS AS A CONTRACTOR IN THE TOWN OF SEWALL'S POINT BE A HOLDER OF A CERTIFICATE OF COMPETENCY.
2. FLORIDA STATUTES 489.103 (7) ALLOWS YOU, AS A PROPERTY OWNER, AN EXEMPTION TO CONSTRUCT OR IMPROVE A SINGLE FAMILY DWELLING AND ACCESSORY-USE STRUCTURES ON SAID PROPERTY FOR YOUR OWN USE OR OCCUPANCY WITHOUT HAVING A CERTIFICATE OF COMPETENCY.
3. AS AN OWNER/BUILDER, YOU MUST PHYSICALLY PERFORM OR MATERIALLY SUPERVISE ALL CONSTRUCTION/IMPROVEMENTS SPECIFIED ON YOUR OWNER/BUILDER PERMIT AND YOU ARE TOTALLY RESPONSIBLE FOR ALL ACTIVITIES ASSOCIATED THEREWITH. OWNER/BUILDERS WHO WISH TO DO ELECTRICAL OR PLUMBING WORK MUST PASS A SHORT OPEN BOOK QUIZ ADMINISTERED BY THE BUILDING DEPARTMENT.
4. IF YOU DO NOT PHYSICALLY PERFORM A SPECIFIC PHASE OF SAID CONSTRUCTION/IMPROVEMENT, BUT CHOOSE TO SUB-CONTRACT IT, THEN SUCH CONSTRUCTION/IMPROVEMENT MUST BE SUB-CONTRACTED TO A LOCALLY LICENSED OR STATE CERTIFIED CONTRACTOR.
5. YOU MAY NOT HIRE AN UNLICENSED INDIVIDUAL WHO IN ANY MANNER ACTS IN THE CAPACITY OF A GENERAL CONTRACTOR, THAT IS, ONE WHO FULFILLS YOUR DUTIES UNDER PARAGRAPH (3) ABOVE.
6. UNDER AN OWNER/BUILDER PERMIT YOU MAY NOT CONSTRUCT A BUILDING WHICH YOU INTEND TO SELL OR LEASE. THE SALE OR LEASE, OR OFFERING FOR SALE OR LEASE, OF ANY SUCH STRUCTURE BY THE OWNER-BUILDER WITHIN 1 YEAR AFTER COMPLETION OF SAME CREATES A PRESUMPTION THAT THE CONSTRUCTION WAS UNDERTAKEN FOR PURPOSES OF SALE OR LEASE WHICH IS A VIOLATION OF THIS EXEMPTION.
7. THIS EXEMPTION SHALL NOT APPLY TO ANY PERSON WHO HAS LEASED, SOLD, OR OFFERED FOR SALE MORE THAN 1 STRUCTURE BUILT UNDER AN OWNER-BUILDER PERMIT IN ANY JURISDICTION WITHIN THE 5 YEARS IMMEDIATELY PRECEDING THE APPLICATION FOR A PERMIT.
8. THERE MUST BE A THIRTY-SIX (36) MONTH PERIOD BETWEEN THE ISSUANCE OF A CERTIFICATE OF OCCUPANCY FOR THE INITIAL DWELLING AND THE SUBMITTAL OF AN OWNER/BUILDER APPLICATION FOR A SUBSEQUENT DWELLING. NO OTHER BUILDING PERMIT FOR A HOME SHALL BE ISSUED TO THAT OWNER/BUILDER, THE OWNER/BUILDER'S SPOUSE OR ANY MEMBER OF THE OWNER/BUILDER'S IMMEDIATE FAMILY UNDER EIGHTEEN YEARS OF AGE UNTIL THREE YEARS AFTER THE HOME BUILT UNDER THE FIRST BUILDING PERMIT HAS RECEIVED A CERTIFICATE OF OCCUPANCY.
9. ALL CONSTRUCTION IMPROVEMENTS PERFORMED UNDER YOUR OWNER/BUILDER PERMIT MUST BE IN STRICT COMPLIANCE WITH FLORIDA STATE STATUTE SECTION 489, SEWALL'S POINTS CODE OF LAWS AND ORDINANCES, ALL BUILDING & ZONING CODES AND REGULATIONS PERTAINING TO SINGLE FAMILY DWELLINGS, ACCESSORY USE STRUCTURES AS APPLICABLE.
10. YOUR OWNER/BUILDER EXEMPTION DOES NOT WAIVE ANY PERMIT CONDITIONS OR REQUIREMENTS OR WAIVE ANY PORTION OR PORTIONS OF ANY APPLICABLE BUILDING OR SWIMMING POOL CODES OR TOWN ORDINANCES.
11. ALL CONSTRUCTION/IMPROVEMENTS PERFORMED UNDER YOUR OWNER/BUILDING PERMIT MUST ALSO BE IN STRICT COMPLIANCE WITH ALL APPLICABLE ZONING REGULATIONS (QUESTIONS REGARDING ZONING REGULATIONS SHOULD BE DIRECTED TO THE TOWN OF SEWALL'S POINT AT 772-287-2455.)



TOWN OF SEWALL'S POINT BUILDING DEPARTMENT

One S. Sewall's Point Road
Sewall's Point, Florida 34996
Tel 772-287-2455 Fax 772-220-4765

12. YOU MUST BE FAMILIAR WITH AND CALL FOR THE REQUIRED INSPECTIONS OF ALL CONSTRUCTION/IMPROVEMENTS PERFORMED UNDER YOUR OWNER/BUILDER PERMIT.

13. AS AN OWNER/BUILDER YOU MUST VERIFY THAT ALL INDIVIDUALS OR FIRMS ENGAGED IN CONSTRUCTION/IMPROVEMENTS ACTIVITIES UNDER YOUR OWNER/BUILDER PERMIT ARE PROPERLY LICENSED AS REQUIRED BY STATE LAW OR LOCAL ORDINANCE.

14. ANY PERSON WORKING ON YOUR BUILDING WHO IS NOT LICENSED MUST WORK UNDER YOUR DIRECT SUPERVISION AND MUST BE EMPLOYED BY YOU, WHICH MEANS THAT YOU MUST DEDUCT F.I.C.A. AND WITHHOLDING TAX AND PROVIDE WORKERS' COMPENSATION FOR THAT EMPLOYEE, ALL AS PRESCRIBED BY LAW.

15 I, AS AN OWNER/BUILDER, IN CONSIDERATION OF A BUILDING PERMIT ISSUED BY SEWALL'S POINT, FLORIDA, AGREE TO INDEMNIFY AND HOLD HARMLESS SAID SEWALL'S POINT, FLORIDA, ITS OFFICERS AND AGENTS FROM ANY AND ALL CLAIMS, DAMAGES, OR EXPENSES THAT SEWALL'S POINT MAY BE LIABLE FOR WHICH ARISE FROM THE CONSTRUCTION/IMPROVEMENTS ACCOMPLISHED IN CONNECTION WITH SAID BUILDING PERMIT.

I HEREBY ACKNOWLEDGE THAT I HAVE THOROUGHLY READ AND COMPLETELY UNDERSTAND THE PRECEDING PAGE OF THE OWNER/BUILDER DISCLOSURE STATEMENT.

ON THIS 1 DAY OF August 2014
PROPERTY ADDRESS 102 Abbie Ct. Sewalls Pt FL 32996
CITY Seaside STATE FL ZIP 32996

SIGNATURE OF OWNER/BUILDER

SWORN TO AND SUBSCRIBED BEFORE ME THIS 1 DAY OF Aug 2012

BY Robert Cushing

PERSONALLY KNOWN _____

OR PRODUCED ID ☒

TYPE/OF ID FLDL#C252-762 18 40-0

Valerie Meyer
NOTARY SIGNATURE



R4101.17.1 Outdoor swimming pools. Outdoor swimming pools shall be provided with a barrier complying with R4101.17.1.1 through R4101.17.1.14.

R4101.17.1.1 The top of the barrier shall be at least 48 inches (1219 mm) above grade measured on the side of the barrier which faces away from the swimming pool. The maximum vertical clearance between grade and the bottom of the barrier shall be 2 inches (51 mm) measured on the side of the barrier which faces away from the swimming pool. Where the top of the pool structure is above grade the barrier may be at ground level or mounted on top of the pool structure. Where the barrier is mounted on top of the pool structure, the maximum vertical clearance between the top of the pool structure and the bottom of the barrier shall be 4 inches (102 mm).

R4101.17.1.2 The barrier may not have any gaps, openings, indentations, protrusions, or structural components that could allow a young child to crawl under, squeeze through, or climb over the barrier as herein described below. One end of a removable child barrier shall not be removable without the aid of tools. Openings in any barrier shall not allow passage of a 4-inch-diameter (102 mm) sphere.

R4101.17.1.3 Solid barriers which do not have openings shall not contain indentations or protrusions except for normal construction tolerances and tooled masonry joints.

R4101.17.1.4 Where the barrier is composed of horizontal and vertical members and the distance between the tops of the horizontal members is less than 45 inches (1143 mm), the horizontal members shall be located on the swimming pool side of the fence. Spacing between vertical members shall not exceed 13/4 inches (44 mm) in width. Where there are decorative cutouts within vertical members, spacing within the cutouts shall not exceed 13/4 inches (44 mm) in width.

R4101.17.1.5 Where the barrier is composed of horizontal and vertical members and the distance between the tops of the horizontal members is 45 inches (1143 mm) or more, spacing between vertical members shall not exceed 4 inches (102 mm). Where there are decorative cutouts within vertical members, spacing within the cutouts shall not exceed 13/4 inches (44 mm) in width.

R4101.17.1.6 Maximum mesh size for chain link fences shall be a 2 1/4 inch square (57 mm) unless the fence is provided with slats fastened at the top or bottom which reduce the openings to no more than 1 3/4 inches (44 mm).

R4101.17.1.7 Where the barrier is composed of diagonal members, the maximum opening formed by the diagonal members shall be no more than 1 3/4 inches (44 mm).

R4101.17.1.8 Access gates, when provided, shall be self-closing and shall comply with the requirements of Sections R4101.17.1.1 through R4101.17.1.7 and shall be equipped with a self-latching locking device located on the pool side of the gate. Where the device release is located no less than 54 inches (1372 mm) from the bottom of the gate, the device release mechanism may be located on either side of the gate and so placed that it cannot be reached by a young child over the top or through any opening or gap from the outside. Gates that provide access to the swimming pool must open outward away from the pool. The gates and barrier shall have no opening greater than 1/2 inch (12.7 mm) within 18 inches (457 mm) of the release mechanism.



TOWN OF SEWALL'S POINT BUILDING DEPARTMENT
One S. Sewall's Point Road
Sewall's Point, Florida 34996
Tel 772-287-2455 Fax 772-2204765

FENCE and or POOL BARRIER CHECKLIST

A document review will be performed on the following items prior to the submittal of a permit application. Failure to submit these items will result in the application package returned to the applicant until the deficient documents are included. This review sheet must accompany the application submittal.

Please make sure you have ALL required copies before submitting permit application

1 Copy Completed permit application

2 Copies Survey or site plan showing the following:

- All existing structures on property
- Location of proposed fence
- Setbacks from the fence to property lines
- Height & type of fence
- Location of all easements
- Street & house number on site plans

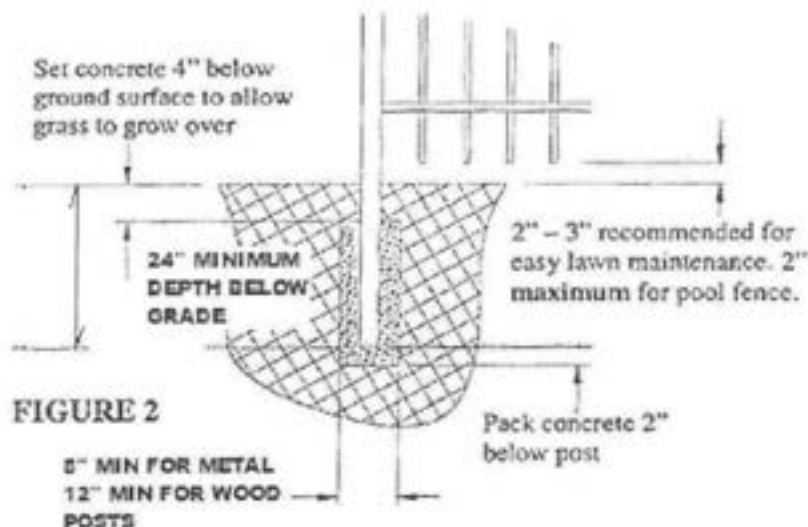
4' ALUMINUM -

DO NOT SUBMIT PREVIOUSLY STAMPED SITE PLANS

2 Copies support post footer sketch indicating size of footers. Fences to be used as a Pool Barrier (other than chain link fence) must include an accurate sketch or drawing indicating barrier requirement compliance.

2 Copies, if fence crosses any easement, Easement agreement from all utility Companies are required. Agreement form included in permit package.

Typical Fence Footer



TOWN OF SEWALLS POINT

BUILDING DEPARTMENT - INSPECTION LOG

Date of Inspection ☐ Mon ☐ Tue ☐ Wed ☒ Thur ☐ Fri 8-9-12 Page 1 of 1

PERMIT #	OWNER/ADDRESS/CONTRACTOR	INSPECTION TYPE	RESULTS	COMMENTS
9904	Howley			
	14 CRANES NEST	FINAL REMODEL	PASS	CLOSE
	O/B			INSPECTOR
PERMIT #	OWNER/ADDRESS/CONTRACTOR	INSPECTION TYPE	RESULTS	COMMENTS
10148	BOTWITNK			
	27 EMALITA WAY	ROOF FINAL	PASS	CLOSE
	CARDINAL			INSPECTOR <i>AF</i>
PERMIT #	OWNER/ADDRESS/CONTRACTOR	INSPECTION TYPE	RESULTS	COMMENTS
10131	ANTONELLI			
	10 ISLAND RD	WINDOW	PASS	CLOSE
	HAMILTON	FINAL		INSPECTOR <i>AF</i>
PERMIT #	OWNER/ADDRESS/CONTRACTOR	INSPECTION TYPE	RESULTS	COMMENTS
10178	CUSHING			
	102 ADDIE CT	FENCE FINAL	PASS	CLOSE
	O/B			INSPECTOR <i>AF</i>
PERMIT #	OWNER/ADDRESS/CONTRACTOR	INSPECTION TYPE	RESULTS	COMMENTS
		PROTECTOR TREE		NEED
	11 PRM		NG	ARTIST OPINION
				INSPECTOR
PERMIT #	OWNER/ADDRESS/CONTRACTOR	INSPECTION TYPE	RESULTS	COMMENTS
				INSPECTOR
PERMIT #	OWNER/ADDRESS/CONTRACTOR	INSPECTION TYPE	RESULTS	COMMENTS
				INSPECTOR

10604

A/C CHANGE OUT

Town of Sewall's Point
BUILDING PERMIT APPLICATION

Date: 09-19-2013

Permit Number: 10604

OWNER/LESSEE NAME: Robert & Pearl Peshing Phone (Day) 283-5016 (Fax) _____
Job Site Address: 102 Abbie Court City: Stuart State: FL Zip: 34996
Legal Description: 13-38-41-017-000-0000-0 Parcel Control Number: Sewall's Point
Fee Simple Holder Name: James Address: _____
City: _____ State: _____ Zip: _____ Telephone: _____

***SCOPE OF WORK (PLEASE BE SPECIFIC):** Replace HVAC system

WILL OWNER BE THE CONTRACTOR?

(If yes, Owner Builder questionnaire must accompany application)

YES _____ NO X

Has a Zoning Variance ever been granted on this property?

YES _____ (YEAR) _____ NO _____

(Must include a copy of all variance approvals with application)

COST AND VALUES: (Required on ALL permit applications)

Estimated Value of Improvements: \$ 6460.00

(Notice of Commencement required when over \$2500 prior to first inspection, \$7,500 on HVAC change out)

Is subject property located in flood hazard area? VE10 AE8 AE8 X

FOR ADDITIONS, REMODELS AND RE-ROOF APPLICATIONS ONLY:

Estimated Fair Market Value prior to improvement: \$ _____

(Fair Market Value of the Primary Structure only, Minus the land value)

PRIVATE APPRAISALS MUST BE SUBMITTED WITH PERMIT APPLICATION

Construction Company: Stephen K. Denny, Inc Phone: 743-9554 Fax: 746-2581

Qualifiers name: Steve Sanders Street: 406 Commerce Way City: Jupiter State: FL Zip: 33458

State License Number: PAC 1813800 OR: Municipality: _____ License Number: _____

LOCAL CONTACT: Deborah Davis-Acker Phone Number: 561-743-9554

DESIGN PROFESSIONAL: NA Fla. License# _____

Street: _____ City: _____ State: _____ Zip: _____ Phone Number: _____

AREAS SQUARE FOOTAGE: Living: 3280 Garage: _____ Covered Patios/ Porches: _____ Enclosed Storage: _____

Carport: _____ Total under Roof _____ Elevated Deck: SEP 20 2013 Enclosed area below BFE*: _____

* Enclosed non-habitable areas below the Base Flood Elevation greater than 300 sq. ft. require a Non-Conversion Covenant Agreement.

CODE EDITIONS IN EFFECT THIS APPLICATION: Florida Building Code (Structural, Mechanical, Plumbing, Existing, Gas): 2010
National Electrical Code: 2008, Florida Energy Code: 2010, Florida Accessibility Code: 2010, Florida Fire Prevention Code: 2010

WARNINGS TO OWNERS AND CONTRACTORS:

1. YOUR FAILURE TO RECORD A NOTICE OF COMMENCEMENT MAY RESULT IN YOUR PAYING TWICE FOR IMPROVEMENTS TO YOUR PROPERTY. WHEN FINANCING, CONSULT WITH YOUR LENDER OR AN ATTORNEY BEFORE RECORDING YOUR NOTICE OF COMMENCEMENT. A NOTICE OF COMMENCEMENT MUST BE RECORDED AND POSTED ON THE JOB SITE BEFORE THE FIRST INSPECTION.
2. IT IS YOUR RESPONSIBILITY TO DETERMINE IF YOUR PROPERTY IS ENCUMBERED BY ANY DEED RESTRICTIONS. SOME RESTRICTIONS APPLICABLE TO THIS PROPERTY MAY BE FOUND IN THE PUBLIC RECORDS OF MARTIN COUNTY OR THE TOWN OF SEWALL'S POINT. THERE MAY BE ADDITIONAL PERMITS REQUIRED FROM OTHER GOVERNMENTAL ENTITIES SUCH AS WATER MANAGEMENT DISTRICTS, STATE AGENCIES, OR FEDERAL AGENCIES.
3. BUILDING PERMITS FOR SINGLE FAMILY RESIDENCES AND SUBSTANTIAL IMPROVEMENTS TO SINGLE FAMILY RESIDENCES ARE VALID FOR A PERIOD OF 24 MONTHS. RENEWAL FEES WILL BE ASSESSED AFTER 24 MONTHS PER TOWN ORDINANCE 50-95.
4. THIS PERMIT WILL BECOME NULL AND VOID IF THE WORK AUTHORIZED BY THIS PERMIT IS NOT COMMENCED WITHIN 180 DAYS, OR IF WORK IS SUSPENDED OR ABANDONED FOR A PERIOD OF 180 DAYS AT ANY TIME AFTER THE WORK IS COMMENCED. ADDITIONAL FEES WILL BE ASSESSED ON ANY PERMIT THAT BECOMES NULL AND VOID. REF. FBC 2007 SECT. 105.4.1, 105.4.1.1 - .5.

******* A FINAL INSPECTION IS REQUIRED ON ALL BUILDING PERMITS *******

AFFIDAVIT: APPLICATION IS HEREBY MADE TO OBTAIN A PERMIT TO DO THE WORK AS SPECIFICALLY INDICATED ABOVE. I CERTIFY THAT NO WORK OR INSTALLATION HAS COMMENCED PRIOR TO THE ISSUANCE OF A PERMIT AND THAT THE INFORMATION I HAVE FURNISHED ON THIS APPLICATION IS TRUE AND CORRECT TO THE BEST OF MY KNOWLEDGE. I AGREE TO COMPLY WITH ALL APPLICABLE CODES, LAWS, AND ORDINANCES OF THE TOWN OF SEWALL'S POINT DURING THE BUILDING PROCESS.

OWNER /AGENT/LESSEE - NOTARIZED SIGNATURE:

X [Signature]
State of Florida, County of: Florida
On This the 19 day of September, 2013
by Robert Peshing who is personally
known to me or produced [Signature]
As identification: [Signature]

Notary Public

My Commission Expires: _____

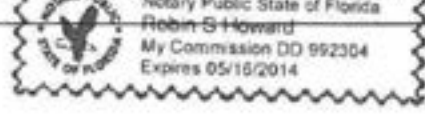
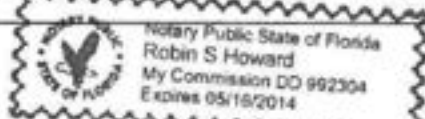
CONTRACTOR/LICENSEE NOTARIZED SIGNATURE:

X [Signature]
State of Florida, County of: _____
On This the 19 day of Sept, 2013
by Steve Sanders who is personally
known to me or produced _____
As identification: [Signature]

Notary Public

My Commission Expires: _____

SINGLE FAMILY PERMIT APPLICATIONS MUST BE ISSUED WITHIN 30 DAYS OF APPROVAL. NOTIFICATION (FBC 105.3.4) ALL OTHER APPLICATIONS WILL BE CONSIDERED ABANDONED AFTER 180 DAYS (FBC 105.3.2) - PLEASE PICK UP YOUR PERMIT PROMPTLY!





Martin County, Florida

Laurel Kelly, C.F.A

Site Provided by...
governmax.com L13

Summary

Tabs

Summary

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[Sales](#)
[Taxes →](#)
[NEW: Navigator](#)
[Parcel Map →](#)
[Notice of Prop.](#)
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Functions

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[County Login](#)

Parcel ID	Account #	Unit Address	Market Total Value	Webs Update
13-38-41-013-000-00220-0	119126	102 ABBIE COURT, SEWALL'S POINT	\$608,670	9/14/2

Owner Information	
Owner(Current)	CUSHING ROBERT B & PEARL C
Owner/Mail Address	102 ABBIE CT STUART FL 34996
Sale Date	9/14/2001
Document Book/Page	1582 0288
Document No.	JMB
Sale Price	552100

Account #	Location/Description	Map Page No.	SP-01
119126			
Tax District	2200	Legal Description	LOT 22 SEWALL'S MEADOW (PB 1 32)
Parcel Address	102 ABBIE COURT, SEWALL'S POINT		
Acres	.5770		

Parcel Type	
Use Code	0100 Single Family
Neighborhood	120300 Sewall's Meadow

Assessment Information	
Market Land Value	\$247,500
Market Improvement Value	\$361,170
Market Total Value	\$608,670

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TOWN OF SEWALL'S POINT BUILDING DEPARTMENT
One S. Sewall's Point Road
Sewall's Point, Florida 34996
Tel 772-287-2455 Fax 772-220-4765

BUILDING PERMIT CARD

THIS CARD MUST BE POSTED IN A CONSPICUOUS PLACE IN PLAIN VIEW FROM THE STREET PRIOR TO BEGINNING ANY WORK

A FINAL INSPECTION IS REQUIRED FOR ALL PERMITS

PERMIT NUMBER:	10604	DATE ISSUED:	SEPTEMBER 24, 2013
SCOPE OF WORK:	AC CHANGEOUT		
CONTRACTOR:	STEPHEN K DENNY INC		
PARCEL CONTROL NUMBER:	133841013-000-002200	SUBDIVISION	SEWALLS MEADOW - L 22
CONSTRUCTION ADDRESS:	102 ABBIE CT		
OWNER NAME:	CUSHING		
QUALIFIER:	STEVE SAUNDERS	CONTACT PHONE NUMBER:	561-743-9554

WARNING TO OWNER: YOUR FAILURE TO RECORD A NOTICE OF COMMENCEMENT MAY RESULT IN YOUR PAYING TWICE FOR IMPROVEMENTS TO YOUR PROPERTY. IF YOU INTEND TO OBTAIN FINANCING, CONSULT WITH YOUR LENDER OR AN ATTORNEY BEFORE RECORDING YOUR NOTICE OF COMMENCEMENT. A CERTIFIED COPY OF THE RECORDED NOTICE OF COMMENCEMENT MUST BE SUBMITTED TO THE BUILDING DEPARTMENT PRIOR TO THE FIRST REQUESTED INSPECTION.

NOTICE: IN ADDITION TO THE REQUIREMENTS OF THIS PERMIT, THERE MAY BE ADDITIONAL RESTRICTIONS APPLICABLE TO THIS PROPERTY THAT MAY BE FOUND IN PUBLIC RECORDS OF THIS COUNTY, AND THERE MAY BE ADDITIONAL PERMITS REQUIRED FROM OTHER GOVERNMENTAL ENTITIES SUCH AS WATER MANAGEMENT DISTRICTS, STATE AGENCIES, OR FEDERAL AGENCIES.

24 HOUR NOTICE REQUIRED FOR INSPECTIONS - ALL CONSTRUCTION DOCUMENTS MUST BE AVAILABLE ON SITE
CALL 287-2455 - 8:00AM TO 4:00PM INSPECTIONS: 9:00AM TO 3:00PM - MONDAY THROUGH FRIDAY

INSPECTIONS

UNDERGROUND PLUMBING	_____	UNDERGROUND GAS	_____
UNDERGROUND MECHANICAL	_____	UNDERGROUND ELECTRICAL	_____
STEM-WALL FOOTING	_____	FOOTING	_____
SLAB	_____	TIE BEAM/COLUMNS	_____
ROOF SHEATHING	_____	WALL SHEATHING	_____
TIE DOWN /TRUSS ENG	_____	INSULATION	_____
WINDOW/DOOR BUCKS	_____	LATH	_____
ROOF DRY-IN/METAL	_____	ROOF TILE IN-PROGRESS	_____
PLUMBING ROUGH-IN	_____	ELECTRICAL ROUGH-IN	_____
MECHANICAL ROUGH-IN	_____	GAS ROUGH-IN	_____
FRAMING	_____	METER FINAL	_____
FINAL PLUMBING	_____	FINAL ELECTRICAL	_____
FINAL MECHANICAL	_____	FINAL GAS	_____
FINAL ROOF	_____	BUILDING FINAL	_____

ALL RE-INSPECTION FEES AND ADDITIONAL INSPECTION REQUESTS WILL BE CHARGED TO THE PERMIT HOLDER. THE CONTRACTOR OR OWNER /BUILDER MUST SCHEDULE A FINAL INSPECTION. FAILURE TO RECEIVE A SUCCESSFUL FINAL INSPECTION WILL RESULT IN PERMIT RENEWAL FEES, FINES, AND OR DENIAL OF FUTURE BUILDING PERMITS TO THE CONTRACTOR OR OWNER /BUILDER.

BUILDING DEPARTMENT - INSPECTION LOG

☒ Mon

☐ Tue

☐ Wed☐ Thur☐ Fri

1-13-14

Page

of

1

PERMIT #	OWNER/ADDRESS/CONTRACTOR	INSPECTION TYPE	RESULTS	COMMENTS
0604	CUSIANG	FINAL A/C		
	102 ABBIE CT		PASS	CLOSE
	S. K Denny A/C			INSPECTOR <i>JD</i>
PERMIT #	OWNER/ADDRESS/CONTRACTOR	INSPECTION TYPE	RESULTS	COMMENTS
	GARY	DRIVEWAY		
	RIO VISTA			
				INSPECTOR
PERMIT #	OWNER/ADDRESS/CONTRACTOR	INSPECTION TYPE	RESULTS	COMMENTS
	MANDRAJ d RIVER	LIGHT POLE		LANTERN
				INSPECTOR
PERMIT #	OWNER/ADDRESS/CONTRACTOR	INSPECTION TYPE	RESULTS	COMMENTS
				INSPECTOR
PERMIT #	OWNER/ADDRESS/CONTRACTOR	INSPECTION TYPE	RESULTS	COMMENTS
				INSPECTOR
PERMIT #	OWNER/ADDRESS/CONTRACTOR	INSPECTION TYPE	RESULTS	COMMENTS
				INSPECTOR
PERMIT #	OWNER/ADDRESS/CONTRACTOR	INSPECTION TYPE	RESULTS	COMMENTS
				INSPECTOR



TOWN OF SEWALL'S POINT BUILDING DEPARTMENT

One S. Sewall's Point Road
Sewall's Point, Florida 34996
Tel 772-287-2455 Fax 772-2204765

Air Conditioning Change out Affidavit

Residential ☒ Commercial ☐

Package Unit ☐ Yes ☒ No (Use Condenser side of form below for equipment listing)

Duct Replacement ☐ Yes ☒ No - Refrigerant line replacement ☐ Yes ☒ No

Flushing Existing Refrigerant lines ☐ Yes ☒ No - Adding Refrigerant Drier ☐ Yes ☒ No

Rooftop A/C Stand Installation ☐ Yes ☒ No - Curb Installation ☐ Yes ☒ No

Smoke Detector in Supply (over 2000 CFM) ☐ Yes ☒ No

One form required for each A/C system installed

REPLACEMENT SYSTEM COMPONENTS

Air handler: Mfg: Lennox Model# 0556

Volts 240 CFM's Heat Strip 4 Kw

Min. Circuit Amps 35 Wire gauge

Max. Breaker size Min. Breaker size

Ref. line size: Liquid 3/8 Suction 3/4

Refrigerant type 410A

Location: Existing ☒ New ☐

Attic/Garage/Closet (specify)

Access:

Condenser: Mfg: Lennox Model# K21-036

Volts SEER/EER 19 BTU's

Min. Circuit Amps 35 Wire gauge

Max. Breaker size Min. Breaker size

Ref. line size: Liquid 3/8 Suction 3/4

Refrigerant type 410A

Location: Existing ☒ New ☐

Left/Right/Rear/Front/Roof ground

Condensate Location

NOTE: CONTRACTOR MUST SUPPLY A PROPER LADDER IF REQUIRED FOR INSPECTION

EXISTING SYSTEM COMPONENTS

Air handler: Mfg: Trane Model# TW1024

Volts 240 CFM's Heat Strip 5 Kw

Min. Circuit Amps 35 Wire gauge

Max. Breaker size Min. Breaker size

Ref. line size: Liquid Suction

Refrigerant type R-22

Location: Ext. ☒ New ☐

Attic/Garage/Closet (specify) attic

Access:

Condenser: Mfg: Trane Model# TT1024

Volts SEER/EER 11A BTU's

Min. Circuit Amps Wire gauge

Max. Breaker size 35 Min. Breaker size

Ref. line size: Liquid 3/8 Suction 3/4

Refrigerant type R-22

Location: Ext. ☒ New ☐

Left/Right/Rear/Front/Roof ground

Condensate Location

Certification:

I hereby certify that the information entered on this form accurately represents the equipment installed and further that this equipment is considered matched as required by FBC - R (N) 1107 & 1108

Signature

Date 09-19-2013



TOWN OF SEWALL'S POINT BUILDING DEPARTMENT
One S. Sewall's Point Road
Sewall's Point, Florida 34996
Tel 772-287-2455 Fax 772-2204765

Air Conditioning Change out Affidavit

Residential ☒ Commercial _____

Package Unit _____ Yes ☒ No (Use Condenser side of form below for equipment listing)

Duct Replacement _____ Yes ☒ No - Refrigerant line replacement _____ Yes ☒ No

Flushing Existing Refrigerant lines _____ Yes ☒ No - Adding Refrigerant Drier _____ Yes ☒ No

Rooftop A/C Stand Installation _____ Yes ☒ No - Curb Installation _____ Yes ☒ No

Smoke Detector in Supply (over 2000 CFM) _____ Yes ☒ No

One form required for each A/C system installed

REPLACEMENT SYSTEM COMPONENTS

Air handler: Mfg: Lennox Model# 10132MV-1

Volts 240 CFM's _____ Heat Strip 4 Kw _____

Min. Circuit Amps 35 Wire gauge _____

Max. Breaker size _____ Min. Breaker size _____

Ref. line size: Liquid 3/8 Suction 3/4

Refrigerant type 410 A

Location: Existing ☒ New _____

Attic/Garage/Closet (specify) _____

Access: _____

Condenser: Mfg: Lennox Model# XCA1-036

Volts _____ SEER/EER 19 BTU's _____

Min. Circuit Amps 35 Wire gauge _____

Max. Breaker size _____ Min. Breaker size _____

Ref. line size: Liquid 3/8 Suction 3/4

Refrigerant type 410 A

Location: Existing ☒ New _____

Left/Right/Rear/Front/Roof ground

Condensate Location _____

NOTE: CONTRACTOR MUST SUPPLY A PROPER LADDER IF REQUIRED FOR INSPECTION

EXISTING SYSTEM COMPONENTS

Air handler: Mfg: Trane Model# TW024

Volts 240 CFM's _____ Heat Strip 5 Kw _____

Min. Circuit Amps 35 Wire gauge _____

Max. Breaker size _____ Min. Breaker size _____

Ref. line size: Liquid _____ Suction _____

Refrigerant type R-22

Location: Ext. ☒ New _____

Attic/Garage/Closet (specify) attic

Access: _____

Condenser: Mfg: Trane Model# TTN024

Volts _____ SEER/EER 11A BTU's _____

Min. Circuit Amps _____ Wire gauge _____

Max. Breaker size 35 Min. Breaker size _____

Ref. line size: Liquid 3/8 Suction 3/4

Refrigerant type R-22

Location: Ext. ☒ New _____

Left/Right/Rear/Front/Roof ground

Condensate Location _____

Certification:

I hereby certify that the information entered on this form accurately represents the equipment installed and further that this equipment is considered matched as required by FBC - R (N)1107 & 1108

Signature

Date

[Signature]

09-19-2013



TOWN OF SEWALL'S POINT BUILDING DEPARTMENT

One S. Sewall's Point Road
Sewall's Point, Florida 34996
Tel 772-287-2455 Fax 772-2204765

Air Conditioning Change out AffidavitResidential ☒ Commercial ☐Package Unit ☐ Yes ☒ No (Use Condenser side of form below for equipment listing)Duct Replacement ☐ Yes ☒ No - Refrigerant line replacement ☐ Yes ☒ NoFlushing Existing Refrigerant lines ☐ Yes ☒ No - Adding Refrigerant Drier ☐ Yes ☒ NoRooftop A/C Stand Installation ☐ Yes ☒ No - Curb Installation ☐ Yes ☒ NoSmoke Detector in Supply (over 2000 CFM) ☐ Yes ☒ NoOne form required for each A/C system installedREPLACEMENT SYSTEM COMPONENTSAir handler: Mfg: Lennox Model# 10632muVolts 240 CFM's Heat Strip 4 Kw Min. Circuit Amps 35 Wire gauge Max. Breaker size Min. Breaker size Ref. line size: Liquid 3/8 Suction 3/4Refrigerant type 410 ALocation: Existing ☒ New ☐Attic/Garage/Closet (specify) located above garageAccess: (need ladder)Condenser: Mfg: Lennox Model# 1021-036Volts SEER/EER 19 BTU's Min. Circuit Amps 35 Wire gauge Max. Breaker size Min. Breaker size Ref. line size: Liquid 3/8 Suction 3/4Refrigerant type 410 ALocation: Existing ☒ New ☐Left/Right/Rear/Front/Roof groundCondensate Location **NOTE: CONTRACTOR MUST SUPPLY A PROPER LADDER IF REQUIRED FOR INSPECTION**EXISTING SYSTEM COMPONENTSAir handler: Mfg: Trane Model# TTN024Volts 240 CFM's Heat Strip 5 Kw Min. Circuit Amps 35 Wire gauge Max. Breaker size Min. Breaker size Ref. line size: Liquid Suction Refrigerant type R-22Location: Ext. ☒ New ☐Attic/Garage/Closet (specify) atticAccess: Condenser: Mfg: Trane Model# TTN024Volts SEER/EER 11A BTU's Min. Circuit Amps Wire gauge Max. Breaker size 35 Min. Breaker size Ref. line size: Liquid 3/8 Suction 3/4Refrigerant type R-22Location: Ext. ☒ New ☐Left/Right/Rear/Front/Roof groundCondensate Location Certification:

I hereby certify that the information entered on this form accurately represents the equipment installed and further that this equipment is considered matched as required by FBC - R (N) 1107 & 1108

[Signature]
Signature

09-19-2013
Date

Project Information

For: CUSHING RESIDENCE
102 ABBIE COURT, STUART, FL 34996

Notes:

Design Information

Weather: W Palm Beach, FL, US

Winter Design Conditions

Outside db 47 °F
Inside db 70 °F
Design TD 23 °F

Summer Design Conditions

Outside db 90 °F
Inside db 75 °F
Design TD 15 °F
Daily range L
Relative humidity 50 %
Moisture difference 59 gr/lb

Heating Summary

Structure 15810 Btuh
Ducts 4248 Btuh
Central vent (0 cfm) 0 Btuh
Humidification 0 Btuh
Piping 0 Btuh
Equipment load 20058 Btuh

Sensible Cooling Equipment Load Sizing

Structure 25229 Btuh
Ducts 6912 Btuh
Central vent (0 cfm) 0 Btuh
Blower 0 Btuh
Use manufacturer's data n
Rate/swing multiplier 0.95
Equipment sensible load 30598 Btuh

Infiltration

Method Simplified
Construction quality Tight
Fireplaces 0

	Heating	Cooling
Area (ft ²)	1221	1221
Volume (ft ³)	12210	12210
Air changes/hour	0.16	0.08
Equiv. AVF (cfm)	33	16

Latent Cooling Equipment Load Sizing

Structure 647 Btuh
Ducts 1893 Btuh
Central vent (0 cfm) 0 Btuh
Equipment latent load 2540 Btuh
Equipment total load 33139 Btuh
Req. total capacity at 0.70 SHR 3.6 ton

Heating Equipment Summary

Make
Trade
Model
AHRI ref
Efficiency 80 AFUE
Heating input 0 MBtuh
Heating output 0 Btuh
Temperature rise 0 °F
Actual air flow 1187 cfm
Air flow factor 0.059 cfm/Btuh
Static pressure 0 in H₂O
Space thermostat

Cooling Equipment Summary

Make Lennox
Trade XC21 SERIES
Cond XC21-036-230-10
Coil CBX32MV-036*++TDR
AHRI ref 5991578
Efficiency 13.5 EER, 19 SEER
Sensible cooling 24920 Btuh
Latent cooling 10680 Btuh
Total cooling 35600 Btuh
Actual air flow 1187 cfm
Air flow factor 0.037 cfm/Btuh
Static pressure 0 in H₂O
Load sensible heat ratio 0.93

Calculations approved by ACCA to meet all requirements of Manual J 8th Ed.

CLIPS



Florida Department of
Business & Professional
Regulation

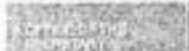
BCIS Home Log In User Registration Hot Topics Submit Surcharge Stats & Facts Publications FBC Staff BCIS Site Map Links Search



Product Approval
USER: Public User

Online Product Approval

Product Approval Menu > Product or Application Search > Application List > Application Detail



FL #	FL15750
Application Type	New
Code Version	2010
Application Status	Approved
Comments	
Archived	
Product Manufacturer	RM Enterprises, Inc.
Address/Phone/Email	352 Tall Pines Rd. Suite A-1 West Palm Beach, FL 33413 (561) 682-1225 raymanucy@aol.com
Authorized Signature	Ray Manucy raymanucy@aol.com
Technical Representative	
Address/Phone/Email	
Quality Assurance Representative	
Address/Phone/Email	
Category	Structural Components
Subcategory	Anchors
Compliance Method	Evaluation Report from a Florida Registered Architect or a Licensed Florida Professional Engineer - Evaluation Report - Hardcopy Received
Florida Engineer or Architect Name who developed the Evaluation Report	Denis K. Solano
Florida License	PE-PE-56902
Quality Assurance Entity	Keystone Certifications, Inc.
Quality Assurance Contract Expiration Date	12/31/2016
Validated By	Steven M. Ulrich, PE Validation Checklist - Hardcopy Received
Certificate of Independence	FL15750_RQ_COI_Certification_of_Independence.pdf
Referenced Standard and Year (of Standard)	
Equivalence of Product Standards	
Certified By	
Sections from the Code	1609 1620

TABLE # 1. CLIP SELECTION (GROUND APPLICATION)					
RM-1"x4" / 1"x6" 8" GALV. STEEL CLIP TIE-DOWN SCHEDULE FOR GROUND APPLICATION ONLY					
MAXIMUM SURFACE AREA OF UNITS LARGEST FACE	MAXIMUM UNITS WIDTH	MAXIMUM UNITS HEIGHT	CORRELATION DETAIL APPLICABLE TO EACH SUBSTRATE CONDITION		
			CLIP AT 1" CORRELATION OF 8" CLIP	CLIP AT 1" CORRELATION OF 8" CLIP	CLIP AT 1" CORRELATION OF 8" CLIP
10-50 FT	32" MAX	48" MAX	SEE DETAIL "D" ON 5-2	NA	NA

TABLE # 2. CLIP SELECTION (GROUND & ROOF TOP APPLICATIONS H<15FT)					
RM-FB-2x4 & FB-2x6 GALV. STEEL CLIP TIE-DOWN SCHEDULE FOR GROUND & ROOF TOP APPLICATIONS (H<15FT)					
MAXIMUM SURFACE AREA OF UNITS LARGEST FACE	MAXIMUM UNITS WIDTH	MAXIMUM UNITS HEIGHT	CORRELATION DETAIL APPLICABLE TO EACH SUBSTRATE CONDITION		
			CLIP AT 1" CORRELATION OF 8" CLIP	CLIP AT 1" CORRELATION OF 8" CLIP	CLIP AT 1" CORRELATION OF 8" CLIP
10-50 FT	48" MAX	48" MAX	SEE DETAIL "D" ON 5-2	SEE DETAIL "C" ON 5-3	SEE DETAIL "D" ON 5-3

NOTE: 1" MAX UNIT HEIGHT

TABLE # 3. CONNECTION TO BASE SUBSTRATE (GROUND & ROOF TOP APPLICATIONS H<15 FT)	
BASE ANCHOR SCHEDULE FOR RM-FB-2x4 & FB-2x6 TIE DOWN CLIPS	
SUBSTRATE	ANCHOR TYPE
ALUMINUM (1/2" THICK, MAX 12" MAX. ALUM)	1/2" DIA. 30 GALV. KNUX-FLUX SELF DRILLING SCREWS BY HULT. SEE DETAIL "D" ON 5-2.
STEEL (1/2" THICK, MAX 12" MAX. ALUM)	1/2" DIA. 30 GALV. KNUX-FLUX SELF DRILLING SCREWS BY HULT. SEE DETAIL "D" ON 5-2.
CONCRETE (4" THICK, MAX, 3000 PSI MAX)	1/2" DIA. 30 GALV. KNUX-FLUX SELF DRILLING SCREWS BY HULT. SEE DETAIL "D" ON 5-2.

TABLE # 4. CONN. TO CONC. BASE SUBSTRATE (GROUND APPLICATIONS ONLY)	
ANCHOR SCHEDULE FOR CONC. SUBSTRATE APPLICATIONS RM-1"x4" & 1"x6" 8" CLIPS	
SUBSTRATE	ANCHOR TYPE
CONCRETE (4" THICK, MAX, 3000 PSI MAX)	1/2" DIA. 30 GALV. KNUX-FLUX SELF DRILLING SCREWS BY HULT. SEE DETAIL "D" ON 5-2.

TABLE # 5			
WIND DESIGN CRITERIA BASED ON BLDG HEIGHT & WIND VELOCITY.			
(H) HEIGHT FROM GRADE TO TOP OF ROOF LEVEL OR UNIT BASE			
WIND VELOCITIES (V)	V=140 MPH	V=170 MPH	
LOCATION	ASCE-10	ASCE-10	
BLIND	LATERAL	65.9 PSF (41.54 PSF)	77.35 PSF (46.41 PSF)
	UP/LIFT	50.57 PSF (30.34 PSF)	NA
COASTAL (1)	LATERAL	104	94.0 PSF (56.4 PSF)
	UP/LIFT	104	NA

1) COASTAL CONSIDERATIONS APPLY TO BUILDING LOCATED WITHIN 1 MILE OF THE SHORE LINE FOR A DISTANCE UP TO 660 FT OR AS GOVERNED BY LOCAL ORDINANCES.

GENERAL NOTES:

- THIS PRODUCT HAS BEEN DESIGNED AND SHALL BE FABRICATED IN ACCORDANCE WITH THE REQUIREMENTS OF THE 2010 F.B.C. & ASCE 7-10 USING WIND PRESSURES AS SHOWN ON TABLE #5 FOR D.W. AND FACTORED BY 1.5 WHEN APPLICABLE. SEE STAND SELECTION PROTOCOL ON THIS SHEET.
- LIMITATION OF USE:
 - GROUND APPLICATION: USE RM-FB-2x4 & FB-2x6 AND RM-1"x4" & RM-1"x6" AT COASTAL & BLIND GROUND APPLICATIONS WITH WIND VELOCITIES V<170 mph. SEE GROUND APPLICATION DETAILS ON 5-2.
 - ROOF TOP APPLICATION: USE RM-FB-2x4 & FB-2x6 AT ROOF TOP APPLICATIONS LIMITED TO BLDG ROOF HEIGHTS <15 FT LOCATED ON BLIND AREAS ONLY. WIND VELOCITIES V<140 mph. SEE TYPICAL ROOF TOP CONNECTION DETAILS "B", "C", "D" ON 5-3.
- MAXIMUM & MINIMUM DIMENSIONS AND MINIMUM WEIGHT OF MECHANICAL UNIT SHALL CONFORM TO SPECIFICATIONS SHOWN HEREIN ON THE TYP. MECH. UNIT TIE DOWN CONNECTIONS ON 5-2 AND CONNECTION DETAILS ON 5-3.
- ALLOWABLE DESIGN PRESSURES USED TO DETERMINE CAPACITY OF CLIPS AS LISTED HEREIN IN TABLE #6 AND DETERMINED THROUGH RATIONALLY ANALYSIS. CLIPS SHALL BE TESTED TO CHECK FOR CONSISTENCY IN EACH CALCULATION PERFORMED.
- ALL CONCRETE SPECIFIED HEREIN IS NOT PART OF THIS CERTIFICATION, AS A MINIMUM, ALL CONCRETE SHALL BE STRUCTURAL CONCRETE 4" MAX. THICK AND SHALL HAVE MINIMUM COMPRESSIVE STRENGTH OF 3000 PSI, UNLESS NOTED OTHERWISE.
- ALL STEEL CLIPS SHALL BE A500 50S STEEL (GRADE 50 W/ F_y=50 KSI OR BETTER). ALL STEEL MEMBER SHALL BE PROTECTED AGAINST CORROSION BY AN APPROVED COAT OF PAINT, ENAMEL, OR OTHER APPROVED PROTECTION IN ACCORDANCE WITH THE 2010 F.B.C. SECTION 2203.2 AND 2220 600-RATED COATING REQUIRED FOR ALL COASTAL INSTALLATIONS.
- SEE ANCHOR SCHEDULES TABLE #3, & TABLE #4 FOR ANCHOR REQUIREMENTS. ALL FASTENERS SHALL HAVE APPROPRIATE CORROSION PROTECTION TO PREVENT ELECTROLYSIS.
- THE CONTRACTOR IS RESPONSIBLE TO INSULATE ALL MEMBERS FROM DISSIMILAR MATERIALS TO PREVENT ELECTROLYSIS.
- THE ADEQUACY OF ANY EXISTING STRUCTURE TO WITHSTAND SUPERIMPOSED LOADS SHALL BE VERIFIED BY THE ON SITE DESIGN PROFESSIONAL AND IS NOT INCLUDED IN THIS CERTIFICATION. CONTRACTOR SHALL HIRE THE SERVICES OF FLORIDA PROFESSIONAL ENGINEER.
- WATERPROOFING OF EXISTING SUBSTRATE SHALL BE THE FULL RESPONSIBILITY OF THE CONTRACTOR.

DESIGN CRITERIA:

- WIND CALC AS PER ASCE 7-10.
- V=170 mph / 140 mph. SEE LIMITATION OF USE ON GENERAL NOTES #2.
- G=0.50
- GCF=3.3 (LATERAL)
- GCF=1.5 (UP/LIFT)

STAND SELECTION PROTOCOL:

- DETERMINE WHETHER YOUR UNIT WILL BE LOCATED ON A GROUND CONCRETE SLAB, GROUND MOUNTED ALUM./STEEL/CONC. STAND OR OVER A ROOF IN A MAX. HEIGHT, <15 FT. SEE TABLE #1 & #2 FOR APPLICABLE CLIP SELECTION BASED ON UNIT SIZE & TYPE OF ALUM./STEEL/CONC. SUBSTRATE.
- FOR GROUND CONC. SLAB APPLICATIONS ONLY YOU MAY SELECT ONE OF THE TWO TIE DOWN CLIP PATTERN AS SHOWN ON 5-2 DETAIL "D" & "E" BASED ON THE TYPE OF CLIP SELECTED ON STEP #1.
- FOR ROOF TOP AND GROUND MOUNTED STAND APPLICATION, SELECT THE CORRESPONDING APPLICABLE DETAIL "B", "C" OR "D" FROM TABLE #2. (SEE ALSO INSTALLATION DETAIL ON 5-3).

CLIP TIE-DOWN SCHEDULE NOTES:

- TIE DOWN CLIPS SHALL BE FASTENED TO MECH. HOLDING UNIT WITH (1) OR MAXIMUM (2) GALV. KNUX-FLUX SELF DRILLING SCREWS BY HULT. SEE DETAILS ON 5-3.
- MECH. HOLDING UNIT SHALL CONFORM TO THE FOLLOWING:
 - ALUM. HOLDING UNITS SHALL BE 5052-H32 MAX. ALUM. SHEET WITH F_y=30 KSI, 0.008" MIN. THICKNESS (1/64")
 - STEEL HOLDING UNITS SHALL BE 33 KSI MIN. STEEL, GRADE 42 GA. W/ F_y=40 KSI
- RM-FB-2x4 & FB-2x6 TIE DOWN CLIPS SHALL NOT BE USED FOR BLDG HEIGHTS IN EXCESS OF 15 FT. SEE LIMITATION OF USE IN GENERAL NOTE #2.
- RM-1"x4" & 1"x6" 8" TIE DOWN CLIPS SHALL BE USED ONLY ON GROUND APPLICATIONS WITH WIND VELOCITIES V<170 mph. SEE LIMITATION OF USE IN GENERAL NOTE #2.

Denis K. Solano
P.E. NRE

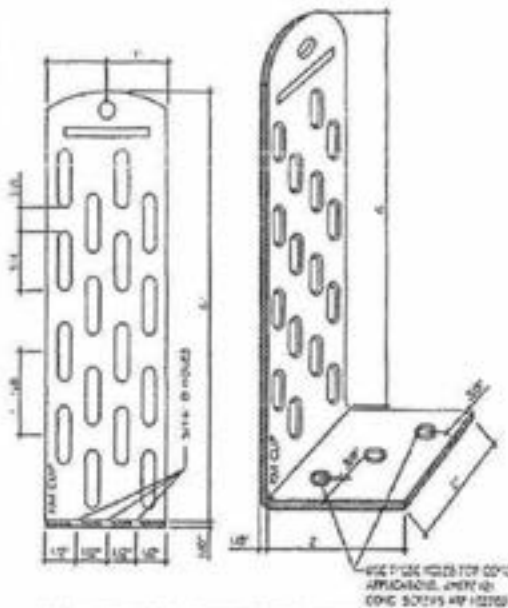
CONSULTANTS:

Robert Bruckner Partnership, Inc.
352 TALL PINES ROAD, SUITE 4-1
WEST PALM BEACH, FLORIDA 33413
Tel: 561-833-1111
Fax: 561-833-1112
www.bruckner.com

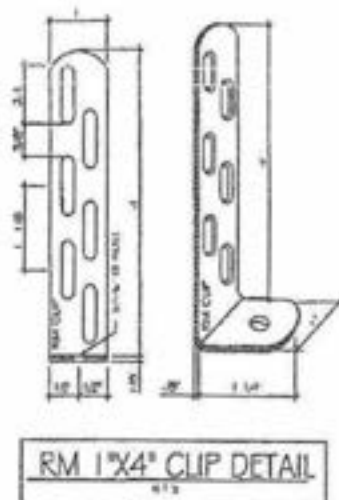
By: R.M. ENTERPRISES OF THE SE, INC.
352 TALL PINES ROAD, SUITE 4-1
WEST PALM BEACH, FLORIDA 33413
Tel: 561-833-1111
Fax: 561-833-1112
www.bruckner.com

As of: 6/20/20
Rev'd: 06/20
Drawn: 6/20/20
Checked: 6/20/20
Project: 1111

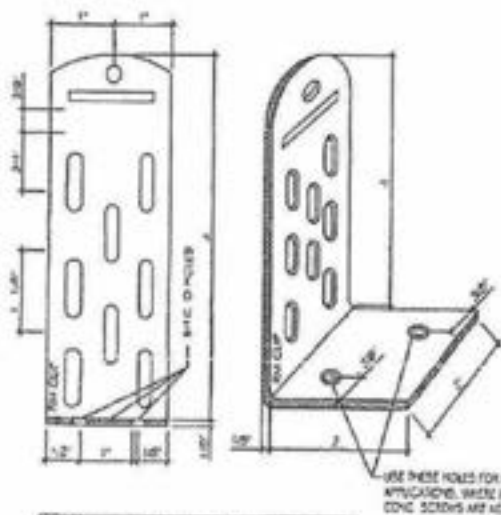
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1 of 3



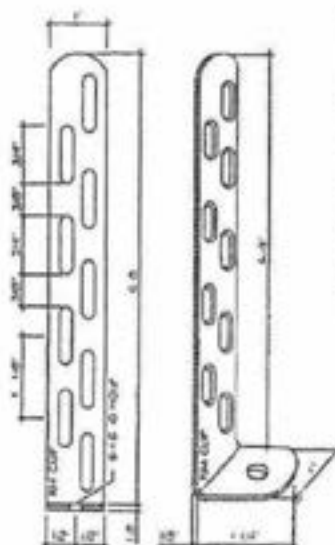
RM FB2X6 CLIP DETAIL
N.T.S.



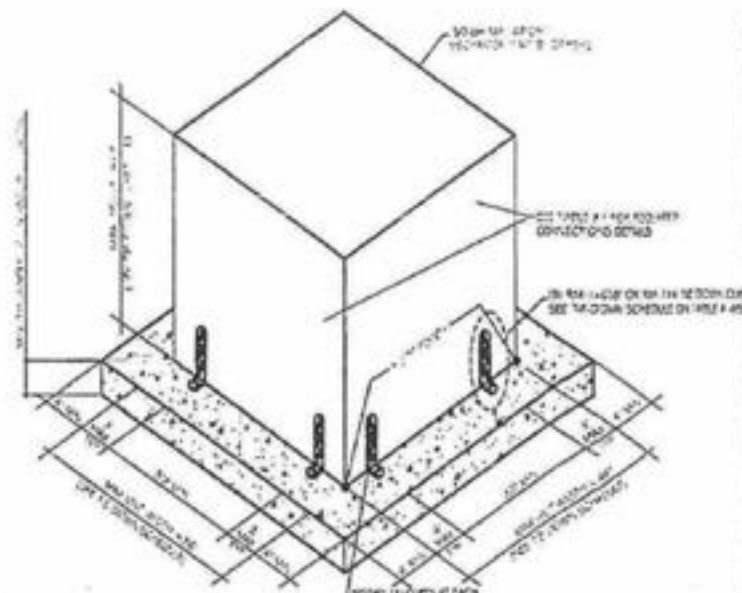
RM 1\"/>



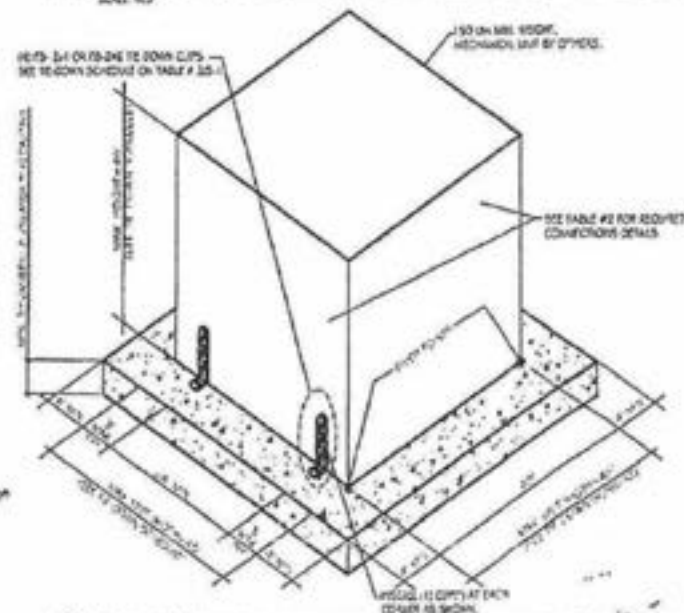
RM FB2X4 CLIP DETAIL
N.T.S.



RM 1\"/>



1 TYP. MECHANICAL UNIT TIE-DOWN ISOMETRIC FOR RM CLIPS:
RM-(1\"/>



2 TYP. MECHANICAL UNIT TIE-DOWN ISOMETRIC FOR RM CLIPS:
FB-(2\"/>

Denis K. Solano
P.E. 56902 S.I. 2016

CONSULTANTS:

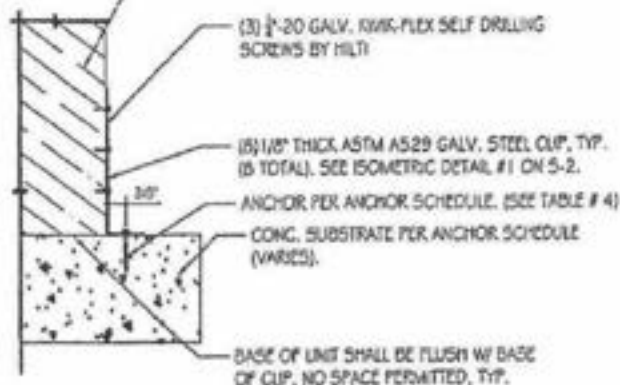
Solano Structural Partners, Inc.
332 TALL PINES ROAD SUITE A-1
WEST PALM BEACH, FLORIDA 33413
TEL: 561-833-1111 FAX: 561-833-1112
WWW.SOLANOSTRUCTURAL.COM

CLIENT:
R.M. ENTERPRISES OF THE SE, INC.
332 TALL PINES ROAD SUITE A-1
WEST PALM BEACH, FLORIDA 33413

DATE: 07-22-15
SCALE: 20'-0\"/>

SHEET NO:
2
2 of 3

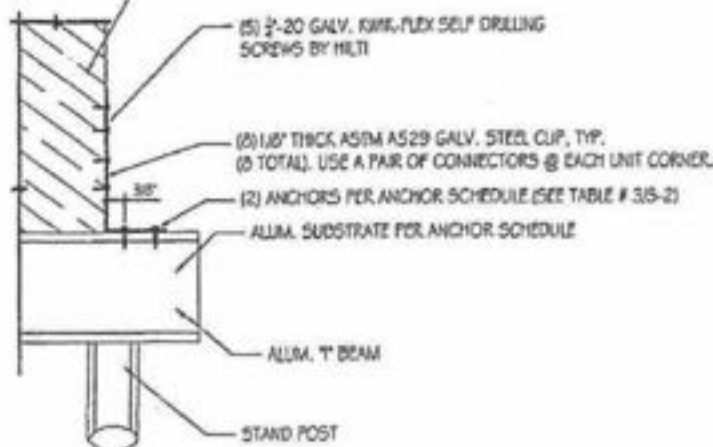
MECH UNIT BY OTHERS (MAX. UNIT DIMS: 30"X30"X48").
ALUMINUM HOUSING UNITS SHALL BE 5052-H32 ALLOY
MIN. ALUM. SHEET W/ $F_{ty}=30$ KSI, 16 GA. THICKNESS,
STEEL HOUSING UNITS SHALL BE 33 KSI MIN. STEEL, GRADE 33,
22 GA MIN. ($t=0.0299$). MIN. UNIT WEIGHT = 150 LB.



A 1" TIE-DOWN CLIP ANCHOR DETAIL
TO CONC. (GROUND APPLICATION ONLY).

SCALE: 3" = 1'

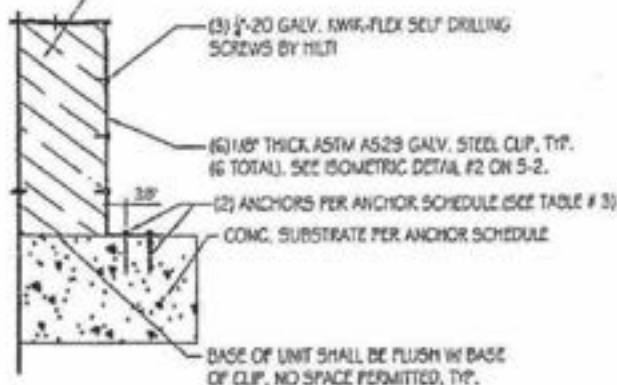
MECH UNIT BY OTHERS (MAX. UNIT DIMS: 30"X30"X48").
ALUMINUM HOUSING UNITS SHALL BE 5052-H32 ALLOY
MIN. ALUM. SHEET W/ $F_{ty}=30$ KSI, 16 GA. THICKNESS,
STEEL HOUSING UNITS SHALL BE 33 KSI MIN. STEEL, GRADE 33,
22 GA MIN. ($t=0.0299$). MIN. UNIT WEIGHT = 150 LB.



C 2" TIE-DOWN CLIP ANCHOR DETAIL TO ALUM. SUBSTRATE
FOR GROUND APPLICATION AND BLDG HEIGHT ≤ 15 FT.

SCALE: 3" = 1'

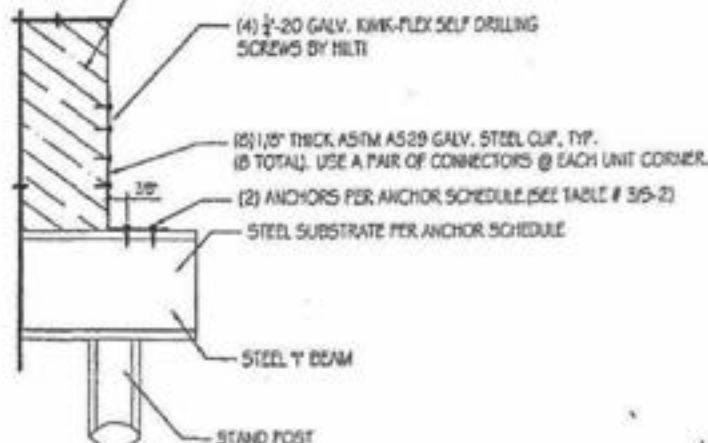
MECH UNIT BY OTHERS (MAX. UNIT DIMS: 30"X30"X48").
ALUMINUM HOUSING UNITS SHALL BE 5052-H32 ALLOY
MIN. ALUM. SHEET W/ $F_{ty}=30$ KSI, 16 GA. THICKNESS,
STEEL HOUSING UNITS SHALL BE 33 KSI MIN. STEEL, GRADE 33,
22 GA MIN. ($t=0.0299$). MIN. UNIT WEIGHT = 150 LB.



B 2" TIE-DOWN CLIP ANCHOR DETAIL TO CONC. SUBSTRATE.

SCALE: 3" = 1'

MECH UNIT BY OTHERS (MAX. UNIT DIMS: 30"X30"X48").
ALUMINUM HOUSING UNITS SHALL BE 5052-H32 ALLOY
MIN. ALUM. SHEET W/ $F_{ty}=30$ KSI, 16 GA. THICKNESS,
STEEL HOUSING UNITS SHALL BE 33 KSI MIN. STEEL, GRADE 33,
22 GA MIN. ($t=0.0299$). MIN. UNIT WEIGHT = 150 LB.



D 2" TIE-DOWN CLIP ANCHOR DETAIL TO STEEL SUBSTRATE
FOR GROUND APPLICATION AND BLDG HEIGHT ≤ 15 FT.

SCALE: 3" = 1'

CONSULTANTS:



R.M. ENTERPRISES OF THE SE, INC.
352 TALL PINES ROAD SUITE 4-1
WEST PALM BEACH, FLORIDA 33413
MECHANICAL UNIT STEEL TIE-DOWN CLIPS

Rev: 0-20-2
Date: 12-2
By: A-1
Check: P-2
Project No:

3
3 of 3

Denis K. Solano
P.E. 56702 S.I. 2046

Certificate of Product Ratings

AHRI Certified Reference Number: 5991578

Date: 9/19/2013

Product: Split System: Air-Cooled Condensing Unit, Coil with Blower

Outdoor Unit Model Number: XC21-036-230-10

Indoor Unit Model Number: CBX32MV-036*+TDR

Manufacturer: LENNOX INDUSTRIES, INC.

Trade/Brand name: XC21 SERIES

Manufacturer responsible for the rating of this system combination is LENNOX INDUSTRIES, INC.

Rated as follows in accordance with AHRI Standard 210/240-2008 for Unitary Air-Conditioning and Air-Source Heat Pump Equipment and subject to verification of rating accuracy by AHRI-sponsored, independent, third party testing:

Cooling Capacity (Btuh): 35600

EER Rating (Cooling): 13.50

SEER Rating (Cooling): 19.00

FootNote 11 - The AHRI 210/240 certified EER ratings are calculated under the same methodology as the EER ratings at T1 conditions of ISO 5151:2010 and ISO 13253:2011.

* Ratings followed by an asterisk (*) indicate a voluntary rerate of previously published data, unless accompanied with a VAS, which indicates an involuntary rerate.

DISCLAIMER

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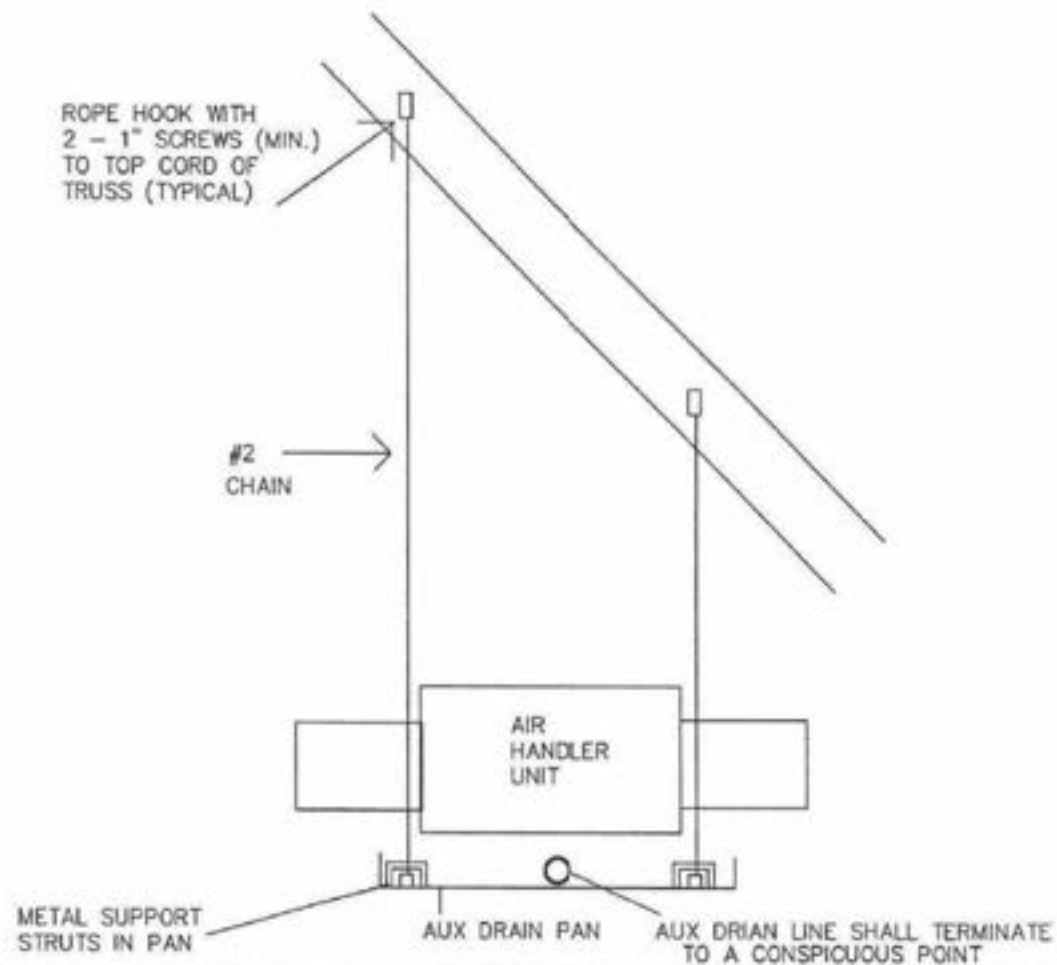
CERTIFICATE VERIFICATION

The information for the model cited on this certificate can be verified at www.ahridirectory.org, click on "Verify Certificate" link and enter the AHRI Certified Reference Number and the date on which the certificate was issued, which is listed above, and the Certificate No., which is listed below.



Air-Conditioning, Heating,
and Refrigeration Institute

HORIZONTAL AIR HANDLER DETAIL



REVISIONS

ALL-CONCRETE DRIVING FOR

STAPLETON K. GENTRY, INC.
(903) 344-0000
107 107 STREET
ARLINGTON, TEXAS 76010



406 Commerce Way
Jupiter, FL 33458

743-9554
Lic# CAC-1813800

stephen k. denny, inc.
AIR CONDITIONING & POOL HEATING
Sales & Service
1-561-743-9554 phone
1-561-746-2581 fax

July 8, 2013

Re: Duct Certification at 102 Abbie Court, Stuart FL 34996

I do hereby certify that the all accessible (a minimum of 30 inches clearance) joints and seams in the Air Distribution System have been inspected and sealed where needed using reinforced mastic per Section 101.4.7.1.1 of the Florida Building Code, Energy Conservation at the above referenced location.

Sincerely,

Steve Sanders
Qualifier for stephen k. denny, inc.
License # CAC1813800

SDS/hd



TOWN OF SEWALL'S POINT BUILDING DEPARTMENT
One S. Sewall's Point Road
Sewall's Point, Florida 34996
Tel: 772-287-2455 Fax 772-220-4765

FLORIDA ENERGY CONSERVATION CODE

Mandatory Duct Inspection Certification for HVAC change-out

For use when part of the duct and/or HVAC system has been replaced (Section 101.4.7.1.1 & FS 553.912)

Owner: Robert + Pearl Rushing Contractor name: Stephen K. Denny, Inc
Street address: 102 Abbie Court Jurisdiction: _____
City: Stuart Permit No.: _____
Zip: 34996 Final inspection date: _____

I certify that I have inspected the duct work associated with the HVAC unit referenced by the permit listed above and found it complies with the requirements of Section 101.4.7.1.1 as indicated below:

- ☒ Where needed, the existing ducts have been sealed using reinforced mastic or code-approved equivalent.
☐ Ducts are located within conditioned space. (Section 101.4.7.1.1 exception 1)
☐ The joints or seams are already sealed with fabric and mastic (Section 101.4.7.1.1 exception 2)
☐ System was tested (see below) and repairs were made as necessary – (Section 101.4.7.1.1 exception 3)

Signature: Steve Sanders Date: 09-19-2013

Printed Name: Steve Sanders

Contractor License #: CAC 1813800

I certified I have tested the replaced air distribution system(s) referenced by the permit listed above at a pressure differential of 25 Pascals (0.10 in. w.c.).

Signature: _____ Date: _____

Printed Name: _____

TREE

TOWN OF SEWALL'S POINT

APPLICATION FOR TREE REMOVAL, RELOCATION, REPLACEMENT

1/17/01 SCHED
10SP. ✓
APPR: 1/17
ISSUED 1/17



Permit # 0399
Date Issued 1/17/01

This application shall include a written statement giving reasons for removal, relocation or replacement and a site plan which shall include the dimensional location on a survey, scale drawing, or aerial photograph, superimposed with lot lines to scale, of all existing or proposed structures, improvements and site uses, location of affected trees identified with an estimated size and number, etc.

Owner REGULA CONTRACTING CORP. Address 7418 WILSON BL. GAITHERSBURG Phone 954-755-8808

Contractor REGULA CONTRACTING Address " " " Phone " " "

Number of trees to be removed (list kinds of trees) 102 ABBIE CANY.

(1) 24" OAK & (1) TREE CLUSTER

Number of trees to be relocated within 30 days (no fee) (list kinds of trees):

(INCIDENT TO DEVELOPMENT - BPN 5228 1/16/01)

Number of trees to be replaced (list kinds of trees):

Permit Fee \$ 15.00 (~~\$25.00~~ - first tree plus \$10.00 - each additional tree - not to exceed \$100.00. \$15.00)

(No permit fee for trees which are relocated on property or lie within a utility easement & are required to be removed in order to provide utility service, nor for a tree which is dead, diseased, injured or hazardous to life or property.)

Plans approved as submitted ✓ Plans approved as marked " "

Permit good for one year. Fee for renewal of expired permit is \$5.00

Signature of applicant [signature] Date submitted 1-16-01

Approved by Building Inspector [signature] Date 1/17/01

Approved by Building Commissioner " " Date " "

Completed " " Date " " Checked by " "

FEE

THE FOLLOWING TREES MAY BE REMOVED OR DESTROYED WITHOUT ~~OBSTRUCTING~~ BRAZILIAN PEPPER, FLORIDA HOLLY TREE, AUSTRALIAN PINE AND STRANGLER FIG. FOR THE PURPOSE OF THIS PERMIT, A TREE IS DEFINED AS ANY SELF-SUPPORTING WOODY OR FIBROUS PERENNIAL PLANT WHICH HAS A MINIMUM HEIGHT OF TWELVE (12) FEET.

THE FOLLOWING TREES MUST BE REMOVED BEFORE CONSTRUCTION BEGINS: BRAZILIAN PEPPER, FLORIDA HOLLY TREE, AUSTRALIAN PINE AND MELALEUCA?

TOWN OF SEWALL'S POINT

Building Department - Inspection Log

Date of Inspection: ☐ Mon ☒ Wed ☐ Fri 1/17/01 ~~7 2000~~ Page 2 of 2

PERMIT	OWNER/ADDRESS/CONTR.	INSPECTION TYPE	RESULTS	REMARKS
✓ 5075	VASQUES	T/T # MTL	Passed	1/17
5 (6)	82 S. SEWALL'S POINT RD GROZA (536-7563 x7)			
PERMIT	OWNER/ADDRESS/CONTR.	INSPECTION TYPE	RESULTS	REMARKS
5 T/R	FOGLIA	FIELD VERIF.	Passed	BPN 5228 (1/16/01)
(5)	102 ABBIE CT		Ok to remove as per plan	
	FOGLIA			
PERMIT	OWNER/ADDRESS/CONTR.	INSPECTION TYPE	RESULTS	REMARKS
5214	Rohloff	Shedding	Passed	1/17
(9)	20 Rivermore		Partial	west slope
	RTS			
PERMIT	OWNER/ADDRESS/CONTR.	INSPECTION TYPE	RESULTS	REMARKS
PERMIT	OWNER/ADDRESS/CONTR.	INSPECTION TYPE	RESULTS	REMARKS
PERMIT	OWNER/ADDRESS/CONTR.	INSPECTION TYPE	RESULTS	REMARKS

OTHER: _____

INSPECTOR (Name/Signature): _____

FILE

TOWN OF SEWALL'S POINT, FLORIDA

Date 1/17/01 17 TREE REMOVAL PERMIT No 0399
APPLIED FOR BY FOGLIA CONTRACTING CORP (Contractor or Owner)
Owner 102 ABBA COURT
Sub-division _____, Lot _____, Block _____
Kind of Trees (1) "cypress" ; (1) oak
No. Of Trees: REMOVE 2
No. Of Trees: RELOCATE -0- WITHIN 30 DAYS (NO FEE)
No. Of Trees: REPLACE -0- WITHIN 30 DAYS
REMARKS SEE SURVEY SKETCH FOR LOCATION
Signed, [Signature] Applicant
Signed, [Signature] Town Clerk
FEE \$ 15.00 CASH
RECEIVED WRP IF 1/17/01
REMOVAL IN ORDER
TO CONSTRUCTION
PN 5228 (1/16/01)
S.V.D.
BEN OTT

TOWN OF SEWALL'S POINT

Call 287-2455 - 8:00 A.M.-12:00 Noon for Inspection

WORK HOURS 8:00 A.M. - 5:00 P.M. - NO SUNDAY WORK.

TREE REMOVAL PERMIT

RE: ORDINANCE 103

OK to remove as per plan
1/17/01
Add'l. tree removal permit issued
5/23/01 to remove
dead oak tree
5/23/01

PROJECT DESCRIPTION _____

REMARKS _____

TOWN OF SEWALL'S POINT

Building Department - Inspection Log

Date of Inspection: ☐ Mon ☒ Wed ☐ Fri MAY 23, 2001; Page 1 of 2.

PERMIT	OWNER/ADDRESS/CONTR.	INSPECTION TYPE	RESULTS	NOTES/COMMENTS:
✓ 5318	Koenke	Deck	Passed	
S (6)	66 S. Sewall's Pt Rd. Advantage Pools			INSPECTOR: J 5/23
PERMIT	OWNER/ADDRESS/CONTR.	INSPECTION TYPE	RESULTS	NOTES/COMMENTS:
✓ 5143	GILFORD	TIE BM/COL	Passed	
N (7)	85 N. SEWALL'S POINT RD SCOTT HOLMES (JERRY)	(Highway) 201-9689		INSPECTOR: J 5/23
PERMIT	OWNER/ADDRESS/CONTR.	INSPECTION TYPE	RESULTS	NOTES/COMMENTS:
✓ 5344	HEURY	SHEATHING	Passed	EARLY AS POSSIBLE 900
S (1)	B E. HIGH POINT HEATON REFR. CO.	(North slope 'DAN' 287-0116)		INSPECTOR: J 5/23
PERMIT	OWNER/ADDRESS/CONTR.	INSPECTION TYPE	RESULTS	NOTES/COMMENTS:
✓ T/R	FOGLIA	FIELD VERIF.	Passed	SUPPL. TO PN 0399
S (2)	102 ABBIE COURT FOGLIA CONST.			Oak tree dead INSPECTOR: J 5/23
PERMIT	OWNER/ADDRESS/CONTR.	INSPECTION TYPE	RESULTS	NOTES/COMMENTS:
✓ 5378	COMBS	ROOFING -	Passed	
S (4)	MANDALAY ISLAND CARPS & HUFF	FINAL		INSPECTOR: J 5/23
PERMIT	OWNER/ADDRESS/CONTR.	INSPECTION TYPE	RESULTS	NOTES/COMMENTS:
✓ 5374	PRAWNBROKER	A/C UNIT - ^{ROOF}	Postponed	due to rain
N (8)	3754 SE OCEAN SUPERIOR REFRIG.	FINAL 201-1340-GREG	Will call + reschedule	INSPECTOR: J 5/23
PERMIT	OWNER/ADDRESS/CONTR.	INSPECTION TYPE	RESULTS	NOTES/COMMENTS:
✓ T/R	REILLY	FIELD VERIF.	Passed	dead tree
S (5)	785 SEWALL'S POINT RD. FERRY LAWN SERVICE			INSPECTOR: J 5/23

OTHER: _____

TOWN OF SEWALL'S POINT, FLORIDA

Date 5/23/01 ☒ TREE REMOVAL PERMIT No 0449
APPLIED FOR BY Paglia Contr. Co. 62 Abbie Ct. (Contractor or Owner)
Owner _____

Sub-division _____, Lot _____, Block _____

Kind of Trees Oak

No. Of Trees: REMOVE 1

No. Of Trees: RELOCATE _____ WITHIN 30 DAYS (NO FEE)

No. Of Trees: REPLACE _____ WITHIN 30 DAYS

REMARKS Decayed tree to be removed

Signed, Sign. on file Applicant
Signed, [Signature] Town Clerk
Bldg. Insp.

FEE \$ 0.-

TOWN OF SEWALL'S POINT

Call 287-2455 - 8:00 A.M.-12:00 Noon for Inspection

WORK HOURS 8:00 A.M. - 5:00 P.M.—NO SUNDAY WORK.

TREE REMOVAL PERMIT

RE: ORDINANCE 103

PROJECT DESCRIPTION _____

REMARKS _____

Original tree removal permit
issued 1/17/01

TOWN OF SEWALL'S POINT

APPLICATION FOR TREE REMOVAL, RELOCATION, REPLACEMENT

5/23/01 SCHED. (NSP)



Permit # 0449

Date Issued 5/23/01

102 ABBIE COURT

This application shall include a written statement giving reasons for removal, relocation or replacement and a site plan which shall include the dimensional location on a survey, scale drawing, or aerial photograph, superimposed with lot lines to scale, of all existing or proposed structures, improvements and site uses, location of affected trees identified with an estimated size and number, etc.

Owner FEENIA CONTRACTING CORP. Address 7428 WILKS RD Phone 954-255-8808Contractor " Address " Phone "Number of trees to be removed (list kinds of trees) (1) OAK APPROX 24"Ø

Number of trees to be relocated within 30 days (no fee) (list kinds of trees):

Number of trees to be replaced (list kinds of trees):

TREE IS DEAD (NO FEE)Permit Fee \$ 0 (~~\$25.00~~ first tree plus \$10.00 - each additional tree - not to exceed \$100.00. \$15.00)

(No permit fee for trees which are relocated on property or lie within a utility easement & are required to be removed in order to provide utility service, nor for a tree which is dead, diseased, injured or hazardous to life or property.)

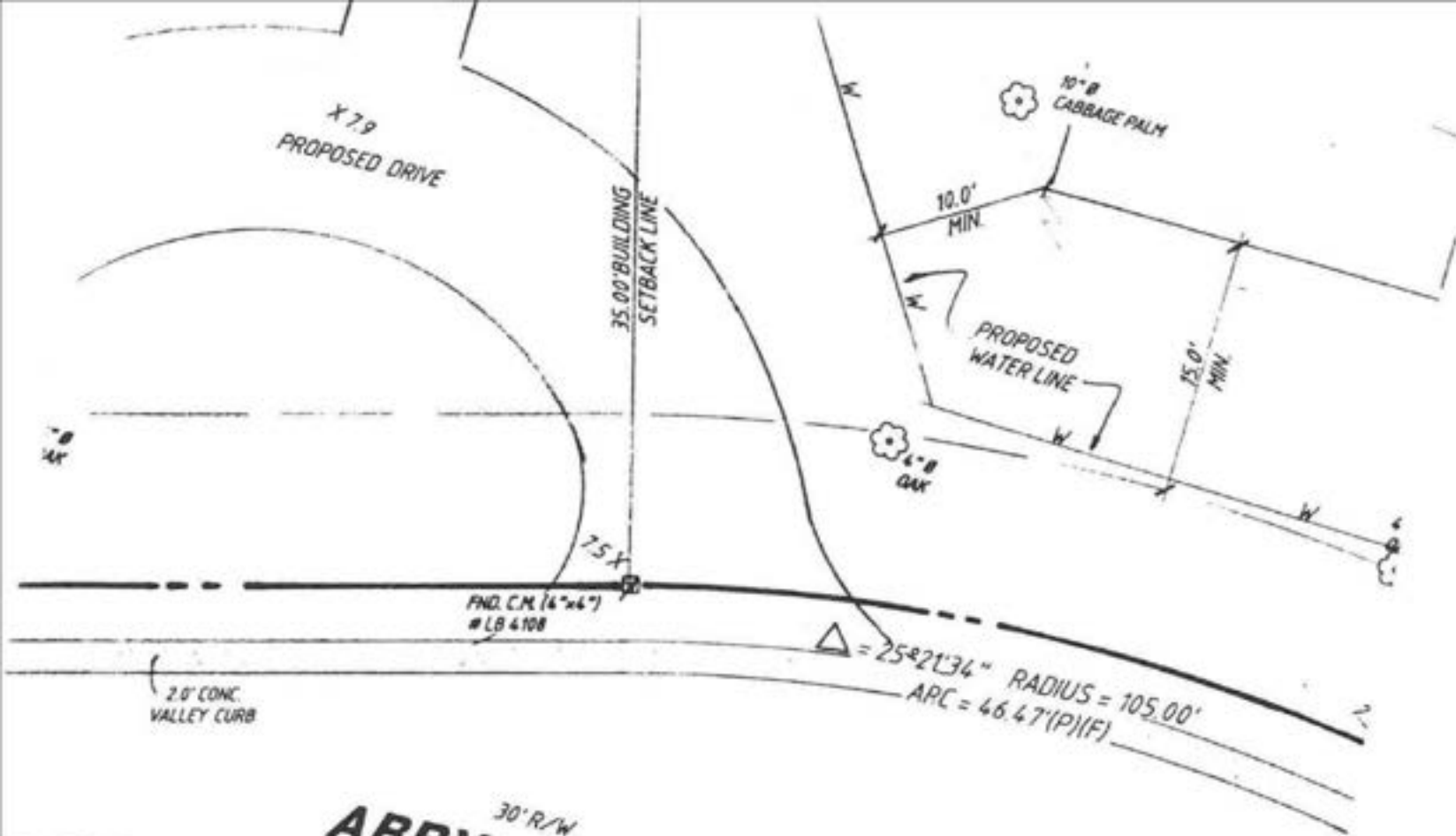
Plans approved as submitted ✓ Plans approved, as marked "

Permit good for one year. Fee for renewal of expired permit is \$5.00

Signature of applicant [signature] - AGENT Date submitted 5-21-01Approved by Building Inspector [signature] Date 5/23/01Approved by Building Commissioner " Date "Completed " Date " Checked by "

THE FOLLOWING TREES MAY BE REMOVED OR DESTROYED WITHOUT ~~OBTAINING A PERMIT~~ PERMIT. BRAZILIAN PEPPER, FLORIDA HOLLY TREE, AUSTRALIAN PINE AND STRANGLER FIG. FOR THE PURPOSE OF THIS PERMIT, A TREE IS DEFINED AS ANY SELF-SUPPORTING WOODY OR FIBROUS PERENNIAL PLANT WHICH HAS A MINIMUM HEIGHT OF TWELVE (12) FEET.

THE FOLLOWING TREES MUST BE REMOVED BEFORE CONSTRUCTION BEGINS: BRAZILIAN PEPPER, FLORIDA HOLLY TREE, AUSTRALIAN PINE AND MELALEUCA?



ABBY COURT
30' R/W

NOTE: SEE TR P0399
FOR FULL SITE SURVEY.

SYMBOLS:

SURVEYOR'S NOTE

1. - SURVEY OF DESCRIPTION AS FURNISHED
2. - LANDS SHOWN HEREON WERE NOT ABSI AND/OR RIGHTS-OF-WAY OF RECORD B

8608

SHUTTERS

TOWN OF SEWALL'S POINT

Date 5-24-07 BUILDING PERMIT NO. 8608
 Building to be erected for Cushing Type of Permit Hurricane
 Applied for by Coastal Shutters (Contractor) Building Fee 120
 Subdivision Sewalls Meadow 22 Block _____ Radon Fee _____
 Address 102 Abbie Ct Impact Fee _____
 Type of structure SFR A/C Fee _____
 Parcel Control Number: _____ Electrical Fee _____
133841-013-000-00220- Plumbing Fee _____
 Amount Paid \$120- Check # 6638 Cash _____ Other Fees () _____
 Total Construction Cost \$ 18665- TOTAL Fees 120-

Signed [Signature]
 Applicant

Signed John Adams
 Town Building Official

PERMIT

☐ BUILDING	☐ ELECTRICAL	☐ MECHANICAL
☐ PLUMBING	☐ ROOFING	☐ POOL/SPA/DECK
☐ DOCK/BOAT LIFT	☐ DEMOLITION	☐ FENCE
☐ SCREEN ENCLOSURE	☐ TEMPORARY STRUCTURE	☐ GAS
☐ FILL	☒ HURRICANE SHUTTERS	☐ RENOVATION
☐ TREE REMOVAL	☐ STEMWALL	☐ ADDITION

INSPECTIONS

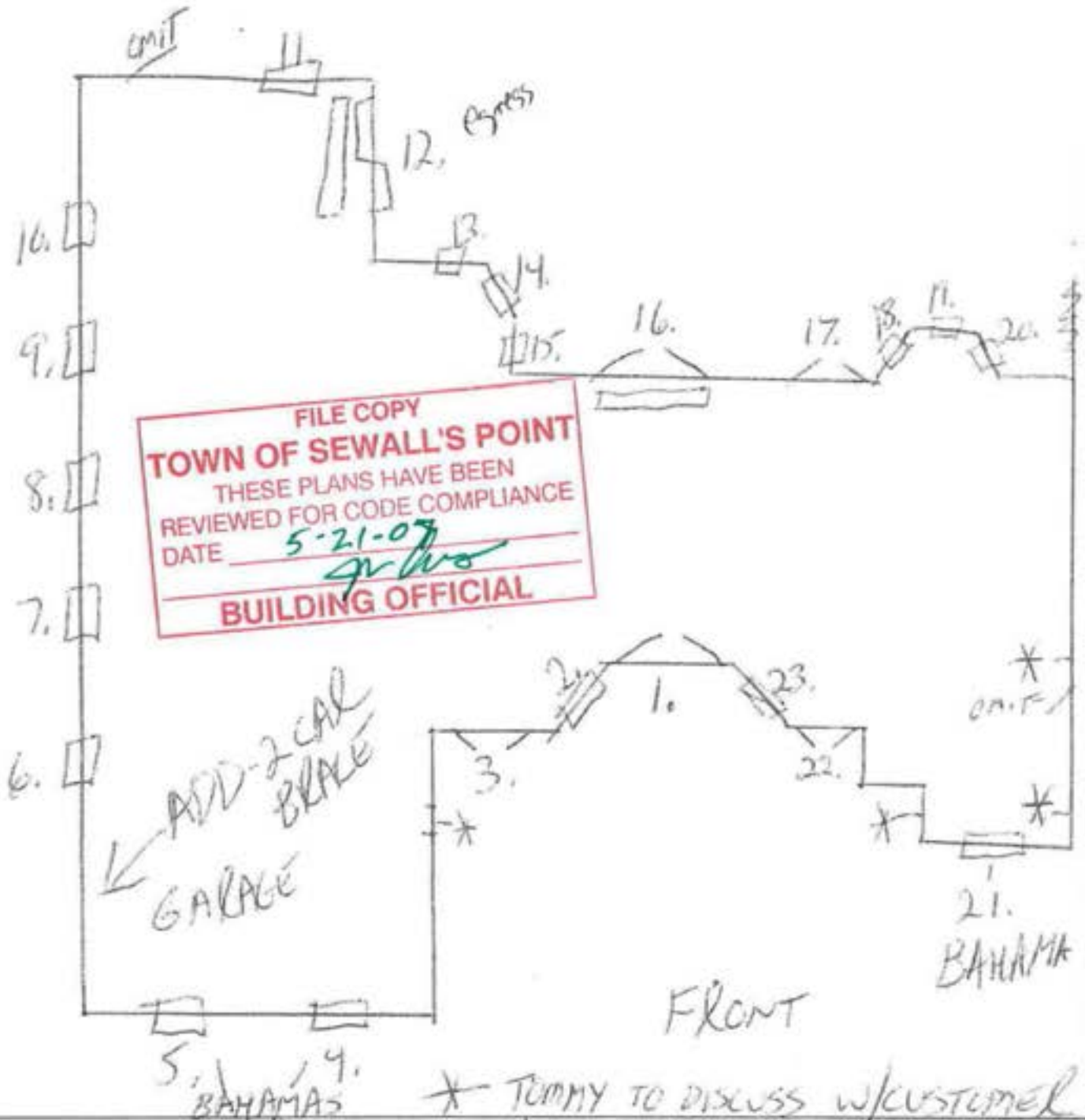
UNDERGROUND PLUMBING	_____	UNDERGROUND GAS	_____
UNDERGROUND MECHANICAL	_____	UNDERGROUND ELECTRICAL	_____
STEMWALL FOOTING	_____	FOOTING	_____
SLAB	_____	TIE BEAM/COLUMNS	_____
ROOF SHEATHING	_____	WALL SHEATHING	_____
TRUSS ENG/WINDOW/DOOR BUCKS	_____	LATH	_____
ROOF TIN TAG/METAL	_____	ROOF-IN-PROGRESS	_____
PLUMBING ROUGH-IN	_____	ELECTRICAL ROUGH-IN	_____
MECHANICAL ROUGH-IN	_____	GAS ROUGH-IN	_____
FRAMING	_____	EARLY POWER RELEASE	_____
FINAL PLUMBING	_____	FINAL ELECTRICAL	_____
FINAL MECHANICAL	_____	FINAL GAS	_____
FINAL ROOF	_____	BUILDING FINAL	_____

DR. CUSHING
CUSTOMER'S NAME

WHITE ACCORDIAN, WHITE PLYWOOD
PRODUCT LOW-RED-ADJUSTABLE

LAYOUT SHEET

SIZES BELOW ARE APPROXIMATE FOR ESTIMATING PURPOSES ONLY



During installation we must have access in and out of property for good installation. Owner, or someone with authority, must be present during installation to inspect and pay balance due.

I ACKNOWLEDGE AND UNDERSTAND THE ABOVE

[Signature]
CUSTOMER'S SIGNATURE

THIS ORDER IS BEING CUSTOM MADE TO YOUR SPECIFICATIONS AND IS NOT SUBJECT TO CANCELLATION.

SHUTTER SCHEDULE

	APPOX	APPOX	APPOX	#OF		#OF	HEADER	
I.D.	OPENING	SHUTTER	SHUTTER	STORM	ANCHOR	WINDOW BARS	REINF. REQ'D	REMARKS
NO.	SIZE(WXH)	WIDTH	HEIGHT	BARS REQ'D	SPACING	EACH SIDE	YES/NO	
	37"X63"	45"	71"	N/A	12"	N/A	NO	EXAMPLE
1	82 x 98	92 1/2	102 1/2	1/1	6	1/1	NO	
2	40 x 64	46	72					
3	64 x 99	72	103 1/2					
4	40 x 68	42	70					
5	40 x 68	42	70					
6	58 x 65	65	73					
7	28 x 40	33	47 1/2					
8	58 x 65	65	73					
9	58 x 65	65	73					
10	28 x 40	33	47 1/2					
11	58 x 98	65 1/2	105 1/2					
12	110 x 130	121 1/4	134					
13	34 x 98	39 1/2	105 1/2					
14	34 x 98	39 1/2	105 1/2					
15	34 x 98	39 1/2	105 1/2					
16	110 x 130	121 1/4	134					
17	70 x 98	78 1/2	102					
18	28 x 65	33	73					
19	28 x 65	33	73					
20	28 x 65	33	73					
21	56 x 68	59	70					
22	64 x 99	72	103 1/2	✓	✓	✓	✓	
23	40 x 64	46	72	✓	✓	✓	✓	
24								
25								
26								
27								
28								
29								
30								

FLORIDA DEPARTMENT OF Community Affairs



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Product Approval

USER: Public User

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 Application Detail

FL #	FL389-R1
Application Type	Revision
Code Version	2004
Application Status	Approved
Comments	
Archived	☐
Product Manufacturer	American Shutter Systems Association, Inc.
Address/Phone/Email	4268 Westroads Drive West Palm Beach, FL 33407 (561) 841-5480 bfeeley@easternmetal.com
Authorized Signature	Bill Feeley bfeeley@easternmetal.com
Technical Representative	
Address/Phone/Email	
Quality Assurance Representative	

Address/Phone/Email

Category

Shutters

Subcategory

Accordion

Compliance Method

Evaluation Report from a Florida Registered Architect or a Licensed Florida Professional Engineer

☐ Evaluation Report - Hardcopy Received

Florida Engineer or Architect Name who developed the Evaluation Report

Walter A., Tillit, Jr. P.E.

Florida License

PE-44167

Quality Assurance Entity

National Accreditation and Management Institute

Validated By

John Henry Kampmann Jr.

Certificate of Independence

Referenced Standard and Year (of Standard)

Standard**Year**

ASTM E-1886

2002

ASTM E-1996

2002

Equivalence of Product Standards Certified By

Sections from the Code

1609.1.4

1609.1.4

Product Approval Method

Method 1 Option D

Date Submitted

08/05/2005

Date Validated 09/23/2005
 Date Pending FBC Approval 09/29/2005
 Date Approved 10/11/2005

Summary of Products

FL #	Model, Number or Name	Description
389.1	Bertha HV Accordion Shutter (Non-HVHZ)- Drwg #05-196	Extruded aluminum interlocking blades retained by tracks (top and bottom) for storm protection
Limits of Use (See Other) Approved for use in HVHZ: Approved for use outside HVHZ: Impact Resistant: Design Pressure: +/- Other: This product shall not be installed within High Velocity Hurricane Zone as defined on section 1620.2 of the Florida Building Code.		Installation Instructions Verified By: Evaluation Reports PTID 389_R1 T CERT INDEPENDENCE 05-0715.01.pdf PTID 389_R1 T Drawing No.05-196.pdf PTID 389_R1 T PROD EVALUATION REPORT 05-0715.01.pdf

[Back](#)
[Next](#)

DCA Administration

Department of Community Affairs
Florida Building Code Online
Codes and Standards

2555 Shumard Oak Boulevard
 Tallahassee, Florida 32399-2100

(850) 487-1824, Suncom 277-1824, Fax (850) 414-8436

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Product Approval Accepts:



GENERAL NOTES:

1. BERTHA H.V. ACCORDION SHUTTER SYSTEM SHOWN ON THIS PRODUCT EVALUATION DOCUMENT (P.E.D.) HAS BEEN VERIFIED FOR COMPLIANCE IN ACCORDANCE WITH THE 2004 EDITION OF THE FLORIDA BUILDING CODE. THIS ACCORDION SHUTTER SYSTEM SHALL NOT BE INSTALLED AT HIGH VELOCITY HURRICANE ZONES. DESIGN WIND LOADS SHALL BE DETERMINED AS PER SECTION 1609 OF THE ABOVE MENTIONED CODE FOR A BASIC WIND SPEED AS REQUIRED BY THE JURISDICTION WHERE SHUTTER WILL BE INSTALLED, AND FOR A DIRECTIONALITY FACTOR $K_d=0.85$, IN ACCORDANCE WITH ASCE 7-98 STANDARD, IN ORDER TO VERIFY THAT ANCHORS ON THIS P.E.D., AS TESTED, WERE NOT OVERSTRESSED, A 33% INCREASE IN ALLOWABLE LOADS FOR WIND LOADS WAS NOT USED IN THEIR ANALYSIS. BERTHA H.V. ACCORDION SHUTTER SYSTEM'S ADEQUACY FOR WIND AND IMPACT LOAD HAS BEEN VERIFIED IN ACCORDANCE WITH SECTION 1609.1.4 OF THE ABOVE MENTIONED CODE AS PER AIL REPORTS #0214.01-03 AND #0715.01-03 AS PER TAS-201, TAS-202 AND TAS-203 PROTOCOLS AND ASTM E-1886 AND E-1998 STANDARDS.

2. BERTHA H.V. ACCORDION PINS (HV SCREWS), USED AT BLADES KNUCKLE AND FOR DIRECT MOUNT CONNECTION TO TRACKS SHALL BE # 14X2.75 AND # 14X1.75 RESPECTIVELY, 410-H7 MINIMUM SERIES STAINLESS STEEL SCREWS WITH 135.0 ksi YIELD STRENGTH AND 180 ksi TENSILE STRENGTH. PINS SHALL BE COATED WITH BERTHA HV DACROSHIELD COATING SYSTEM AS MANUFACTURED BY APPROVED COATING APPLICATORS, REGISTERED WITH AMERICAN SHUTTER SYSTEMS ASSOCIATION. PINS MUST BEAR THE HV MARKING ON THEIR HEAD.

3. ALL ALUMINUM EXTRUSIONS SHALL BE ALUMINUM ASSOCIATION 6063-T5 ALLOY AND TEMPER, WITH $F_y = 25.0$ ksi MINIMUM (UNLESS OTHERWISE NOTED).

4. ALL SCREWS TO BE STAINLESS STEEL 304 OR 316 AISI SERIES OR CORROSION RESISTANT COATED CARBON STEEL AS PER DIN 5018 WITH 50 ksi YIELD STRENGTH AND 90 ksi TENSILE STRENGTH.

5. BOLTS TO BE ALUMINUM ASSOCIATION 2014-T4 ALLOY AND TEMPER, ASTM A-307 GALVANIZED STEEL, OR AISI 304 SERIES STAINLESS STEEL WITH 35 ksi MINIMUM YIELD STRENGTH.

6. SEE SHEETS 10, 11 AND 12 FOR ANCHORS SPECIFICATIONS.

7. THIS BERTHA H.V. ACCORDION SHUTTER SYSTEM IS PATENT PENDING. COMPONENTS OF THIS APPROVAL ARE COVERED, IN WHOLE OR IN PART BY U.S. PATENT ISSUED TO EASTERN METAL SUPPLY, INC.

8. IT SHALL BE THE RESPONSIBILITY OF THE CONTRACTOR TO VERIFY THE SOUNDNESS OF THE STRUCTURE WHERE SHUTTER IS TO BE ATTACHED TO INSURE PROPER ANCHORAGE. CONTRACTOR TO SEAL/CAULK ALL SHUTTER COMPONENT EDGES WHICH REMAIN, THRU FASTENING IN CONTINUOUS CONTACT WITH THE BUILDING TO PREVENT WIND/RAIN INTRUSION.

9. EACH UNIT MUST BEAR A PERMANENT LABEL IN A VISIBLE PLACE WITH WARNING NOTE INSTRUCTING THE TENANT OR HOME OWNER THAT THE H.V. LOCK (W) OR ALTERNATE LOCK (V) MUST BE PROPERLY LOCKED DURING PERIODS OF HURRICANE WARNING. FOR EGRESS CONDITIONS, AN INSIDE HV LOCK (W) (WHERE KEY MUST REMAIN IN THE LOCK), AN INSIDE ALTERNATE LOCK (V), OR AN EGRESS H.V. LOCK (N) SHALL BE USED.

10. SHUTTER'S MANUFACTURER LABEL SHALL BE PLACED ON A READILY VISIBLE LOCATION. ONE LABEL SHALL BE PLACED FOR EVERY OPENING. LABEL SHALL READ AS FOLLOWS:
AMERICAN SHUTTER SYSTEMS ASSOCIATION INC
WEST PALM BEACH, FL
FLORIDA STATEWIDE PRODUCT APPROVED.

11. BERTHA H.V. ACCORDION SHUTTER SYSTEM'S INSTALLATION SHALL COMPLY WITH SPECS INDICATED IN THIS DRAWING PLUS ANY BUILDING AND ZONING REGULATIONS PROVIDED BY THE JURISDICTION WHERE PERMIT IS APPLIED TO.

12. (a) THIS P.E.D. PREPARED BY THIS ENGINEER IS GENERIC AND DOES NOT PROVIDE INFORMATION FOR A SITE SPECIFIC PROJECT; I.e. WHERE THE SITE CONDITIONS DEVIATE FROM THE P.E.D.

(b) CONTRACTOR TO BE RESPONSIBLE FOR THE SELECTION, PURCHASE AND INSTALLATION INCLUDING LIFE SAFETY OF THIS PRODUCT, BASED ON THIS P.E.D., PROVIDED HE/SHE DOES NOT DEVIATE FROM THE CONDITIONS DETAILED ON THIS DOCUMENT. CONSTRUCTION SAFETY AT SITE IS THE CONTRACTOR'S RESPONSIBILITY.

(c) THIS P.E.D. WILL BE CONSIDERED INVALID IF ALTERED BY ANY MEANS.

(d) SITE SPECIFIC PROJECTS SHALL BE PREPARED BY A FLORIDA REGISTERED ENGINEER OR ARCHITECT WHICH WILL BECOME THE ENGINEER OF RECORD (E.O.R.) FOR THE PROJECT AND WHO WILL BE RESPONSIBLE FOR THE PROPER USE OF THE P.E.D. ENGINEER OF RECORD, ACTING AS A DELEGATED ENGINEER TO THE P.E.D. ENGINEER SHALL SUBMIT TO THIS LATTER THE SITE SPECIFIC DRAWINGS FOR REVIEW.

(e) THIS P.E.D. SHALL BEAR THE DATE AND ORIGINAL SEAL AND SIGNATURE OF THE PROFESSIONAL ENGINEER OF RECORD THAT PREPARED IT.

F.B.C. (Non High Velocity Hurricane Zone)

© 2003 EASTERN METAL SUPPLY, INC.

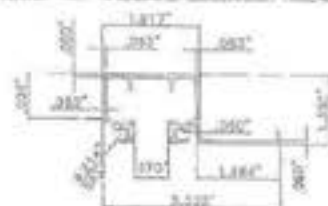
BERTHA HV™ Accordion Shutter System

REV. NO.	DESCRIPTION	DATE	Drawn by: JAG
1	ISSUE	7/13/05	
2			DRAWING No
3			05-186
4			SHEET
5			OF 1





(A) HV BLADE # 1
COVERED UNDER US PATENT # 5458172



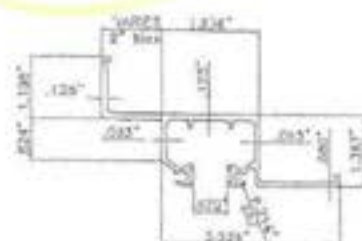
(B) HV CEILING HEADER # 1



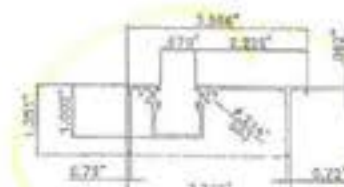
(C) HV 1" (Max.) B/O HEADER/SILL # 1



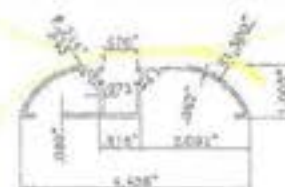
(E) HD CEILING HEADER/SILL # 2



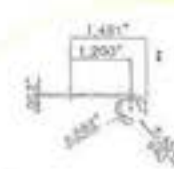
(F) HD 2" (Max.) B/O HEADER/SILL # 2



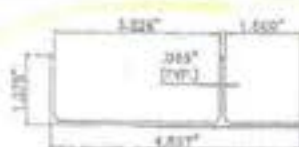
(I) HV ADJ. TRACK



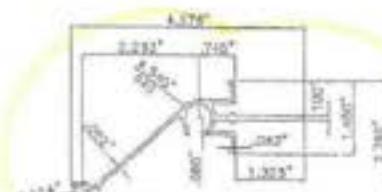
(J) HV WALK-OVER TRACK



(K) HV 90°/180° STARTER



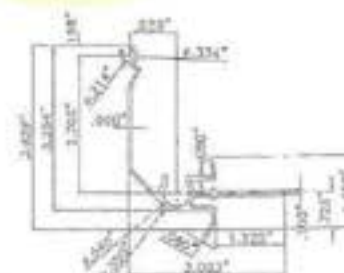
(H) HV ADJ. ADAPTER W/LIP



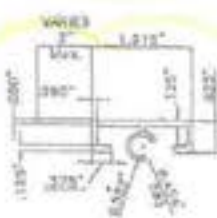
(L) HV CENTERMATE # 1
(SELF MATING)



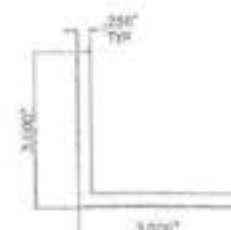
(M) HV FLOATING
CENTERMATE # 2



(X) HV CORNER CENTERMATE # 3



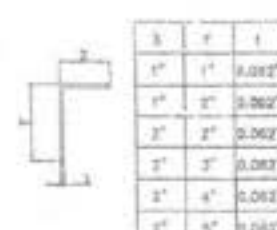
(N) H.V. 2" (Max.) DIRECT MOUNT



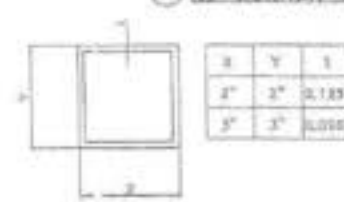
(D) 3" FLOOR ANGLE



(O) B.O. WALL ANGLE



(R) SIDE ANGLE



(S) CORNER POST

F.B.C. (Non High Velocity Hurricane Zone)



TILTECO INC.

TILLET TESTING & ENGINEERING COMPANY
6252 NW 15th St., Ste. 200, Miami, Florida 33150
Phone: (305) 871-1520 Fax: (305) 871-1531
e-mail: Tilt@ttilteco.com
CS-0008719
WALTER A. TILLET JR. P. E.
FL00054 Lic. # 41087

A.S.S.A.

American Shutter Systems
Association, Inc.
4285 Westroads Drive
West Palm Beach, FL 33407

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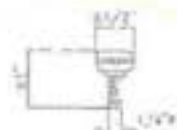
REV. NO.	DESCRIPTION	DATE	Drawn by: J.A.S.
1	NO. 05-196	7/13/05	DATE: 7/13/05
2			DRAWING NO.
3			05-196
4			SHEET
5			2 OF 1a



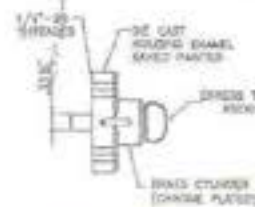
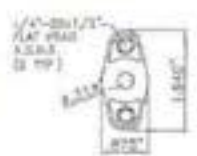
① LOCKING ROD
(SEE NOTES ON SHEET 4)



3/8\"/>

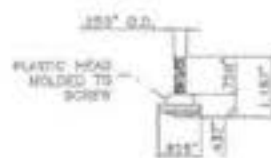


1/4\"/>

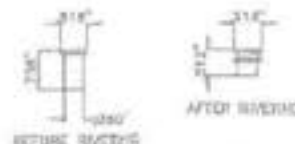


② H.V. LOCK/ EGRESS H.V. LOCK

(SEE NOTES ON SHEET 4)



1/4\"/>

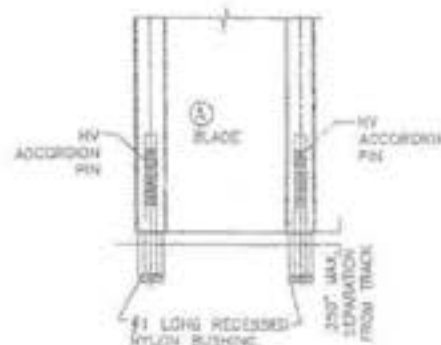
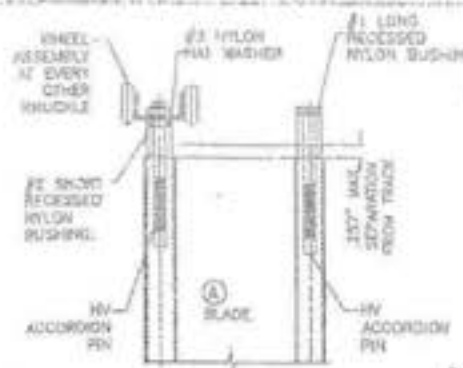


1/4\"/>

③ ALTERNATE LOCK (TO HV LOCK ②)

(SEE NOTES ON SHEET 4)

*SERIES 9444 ALUMINUM (2014-T3 ALLOY)
LARGE FLANGE THIN SHEET RIVNUT,
PART # 9892, AS MANUFACTURED BY
ADEL CHERRY TEXTURAL PAINTS/COAT,
NEW JERSEY 07064 OR EQUAL.



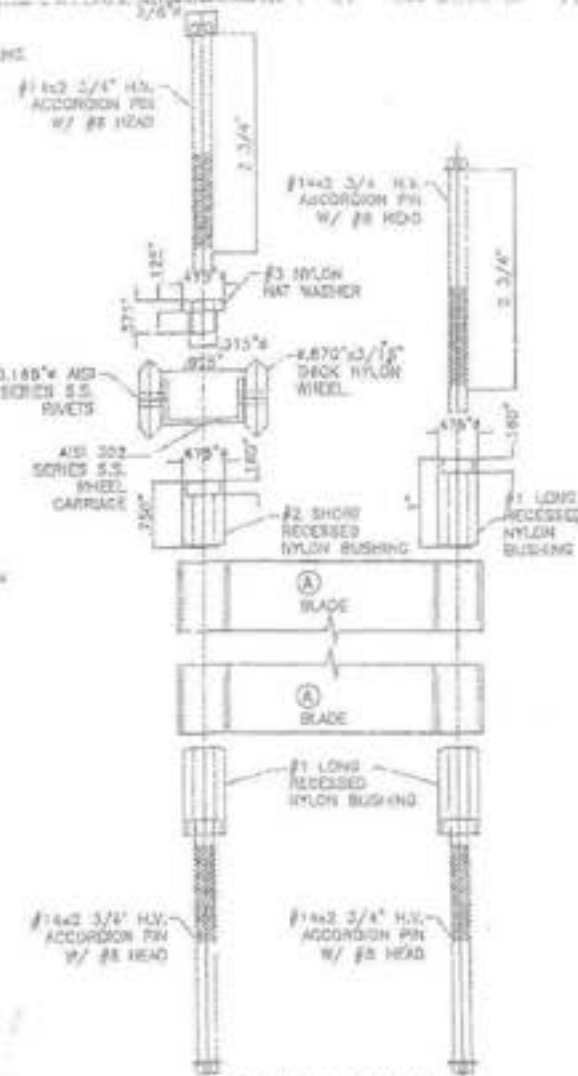
TYPICAL SHUTTER ASSEMBLY

SCALE: 3/8\"/>



WHEEL CARRIAGE DETAIL

N.T.S.



ASSEMBLY LAYOUT

N.T.S.

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KIMBERLY A. TILIT A.P. 1
FLORIDA (IN) 5 6410



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1	As Shown	7/13/05	DATE: 7/13/05
2			DRAWING No
3			05-196
4			SHEET
5			5 OF 16

* NOTES ON LOCKING RODS (U), HV LOCK (W) AND ALTERNATE (V) LOCK

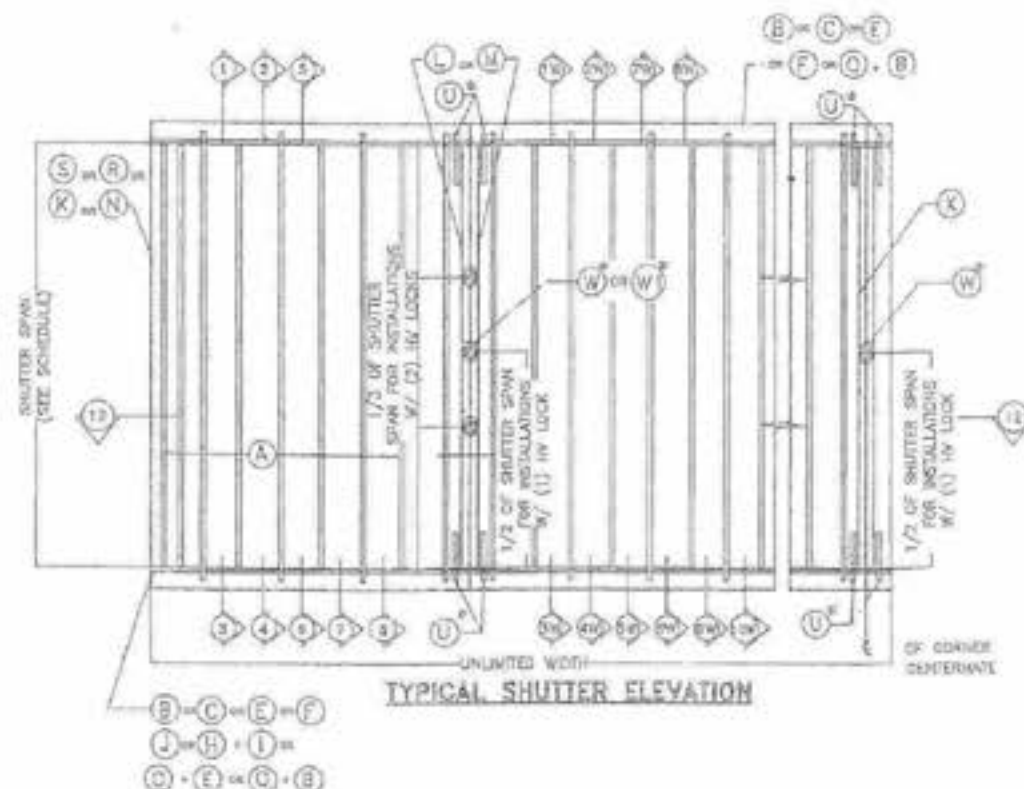
1. MANDATORY CONDITION #1: ONE HV LOCK (W) SHALL BE USED FOR ANY SPAN AND INSTALLATIONS AT ANY ELEVATION. HV LOCK (W) MAY BE USED AS AN INSIDE OR OUTSIDE LOCK, ATTACHED TO (C) OR (E) CENTERMATES W/(2) 1/4"-20 x 1/2" LONG FLAT HEAD S.S. M.S. LOCATION OF LOCK SHALL BE AS INDICATED ON TYPICAL SHUTTER ELEVATION.

2. OPTIONAL CONDITION #1: IF DESIRED, FOR BETTER PERFORMANCE OR FOR SECURITY PURPOSES, UP TO TWO HV LOCKS (W) MAY BE USED AS AN INSIDE OR OUTSIDE LOCK. CONNECTION OF LOCKS TO CENTERMATES (C) OR (E) SHALL BE AS INDICATED ON NOTE (1) ABOVE. LOCATION OF LOCKS SHALL BE AS INDICATED ON TYPICAL SHUTTER ELEVATION.

3. OPTIONAL CONDITION #2: LOCKING RODS (U) ARE NOT REQUIRED AT HURRICANE POSITION, BUT MAY BE USED IF DECIDED TO ENHANCE SHUTTER PERFORMANCE. LOCKING RODS (U) OR AN ALTERNATIVE DEVICE MAY ALSO BE USED AT STACKING POSITION (NON HURRICANE POSITION).

4. MANDATORY CONDITION #2: ONE HV LOCK (W) SHALL BE USED AT CORNER CENTERMATES FOR ANY SPAN AND INSTALLATIONS AT ANY ELEVATION. HV LOCK (W) SHALL BE USED AS AN OUTSIDE OR INSIDE LOCK, ATTACHED TO (C) CORNER CENTERMATE W/(2) 1/4"-20 x 1/2" LONG FLAT HEAD S.S. M.S. TWO LOCKING RODS (U) AT TOP & BOTTOM SHALL BE USED. LOCATION OF LOCK SHALL BE AS INDICATED ON TYPICAL SHUTTER ELEVATION. MAXIMUM SHUTTER SPAN SHALL BE DETERMINED AS PER SCHEDULES SHOWN ON SHEET 5.

5. ALTERNATE TO MANDATORY CONDITION #1 VALID ONLY FOR SYMMETRICAL CENTERMATES: ONE ALTERNATE LOCK (V) MAY BE USED IN LIEU OF HV LOCK (W) FOR ANY SPAN AND INSTALLATIONS AT ANY ELEVATION. ALTERNATE LOCK (V) MAY BE USED AS AN INSIDE OR OUTSIDE LOCK. ATTACHMENT OF 1/4"-20x3/4" S.S. THUMBSCREW TO (C) OR (E) CENTERMATES SHALL BE MADE W/ 1/4"-20x3/4" ALUM. RIVNUT AS SHOWN ON ALTERNATE SECTION 12, SHEET 7.



TYPICAL SHUTTER ELEVATION

F.B.C.(Non High Velocity Hurricane Zone)

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REV. NO.	DESCRIPTION	DATE	DESIGNED BY
1	REVISED	7/13/05	
2			
3			
4			
5			
6			

DRAWING No
05-196

SHEET
2 OF 3

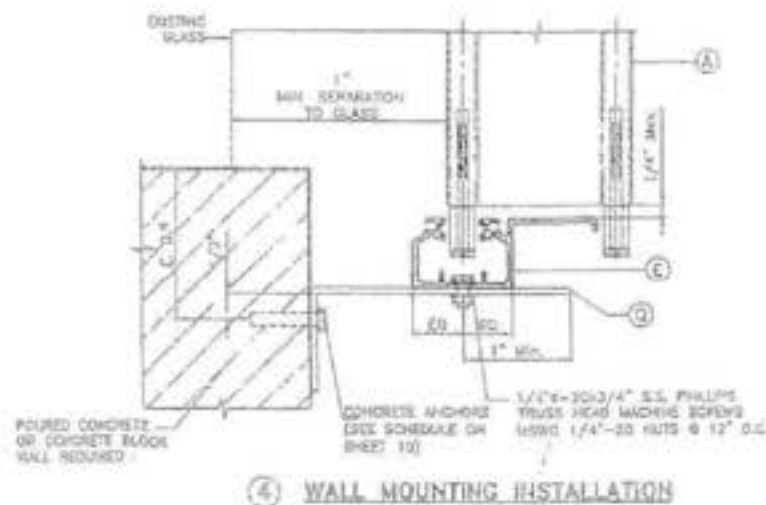
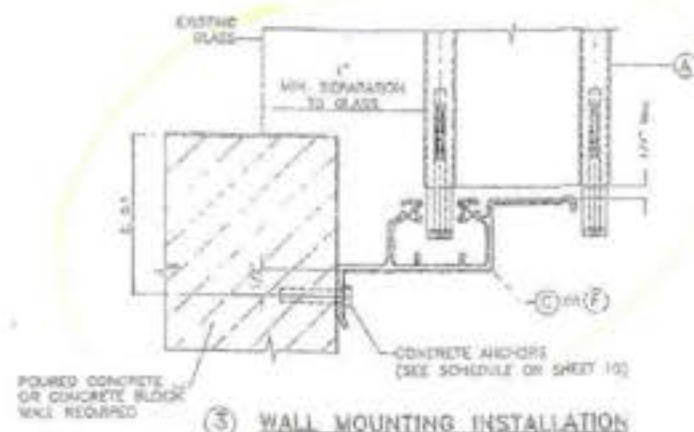
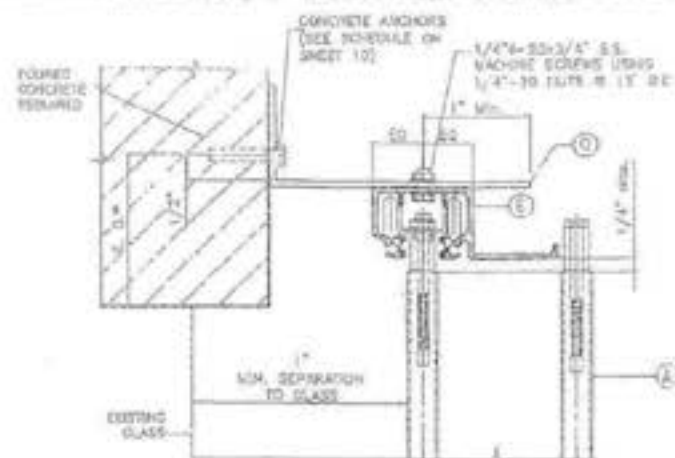
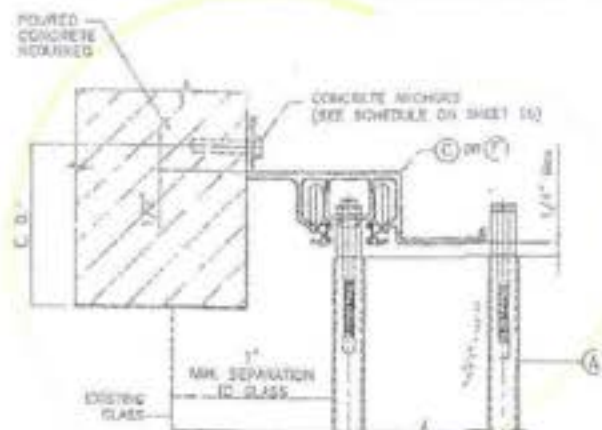


ILIECO INC.

ILIE TESTING & ENGINEERING COMPANY
1001 E. 1st St., Suite 305, Miami Beach, Florida 33138
Phone: (305) 771-1530, Fax: (305) 771-1531
E-MAIL: info@ilietesting.com
C#-0000718
MILAN A. ILIE, P.E.
FLORIDA Lic. # 44107

A.S.S.A.

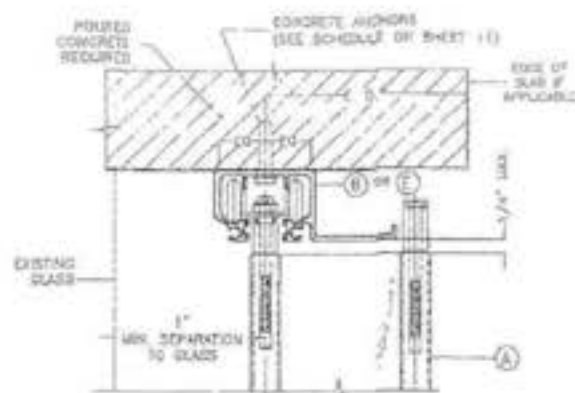
American Shutter Systems
Association, Inc.
4288 Westbrooke Drive
West Palm Beach, FL 33407



NOT FOR COMPARISON OF STUDENTS

- 1- SEE SHEET 2 FOR OPTICAL SHUTTER ASSEMBLY
2- MOUNTING SECTIONS CAN BE COMBINED
IN ANY WAY TO SUIT ANY INSTALLATION

* E. D. = EDOL DISTANCE
(SEE NOTES & SCHEDULE ON SHEET 11)



⑤ CEILING MOUNTING INSTALLATION



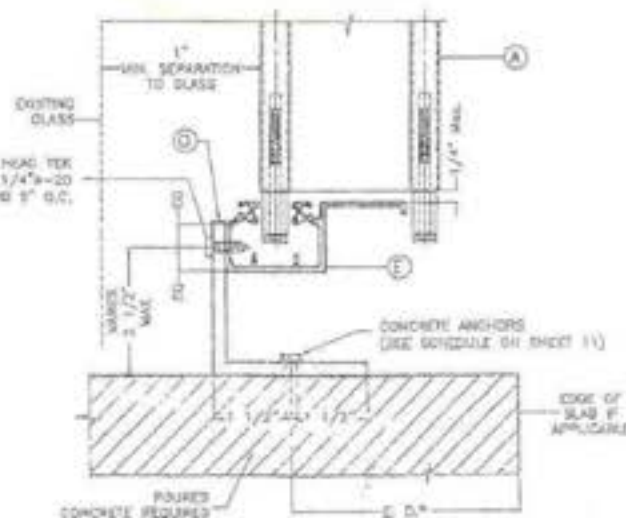
⑥ WALK-OVER MOUNTING INSTALLATION



⑦ FLOOR MOUNTING INSTALLATION †

† 1/4" O.D. HEX HEAD TEK
SCREWS @ 8" O.C. OR 1/4" x 20
S.S. W.S. R/ NAT @ 8" O.C.

NOTE:
THIS INSTALLATION SHALL BE
REMOVABLE WHEN PERFORMED
ADJACENT TO AN OPERABLE EXIT
OR ENTRANCE



⑧ FLOOR MOUNTING INSTALLATION †

NOTE FOR CONSTRUCTION OF SECTIONS

- 1- SEE SHEET 3 FOR 1-PANEL SHUTTER ASSEMBLY
- 2- MOUNTING SECTIONS CAN BE COMBINED IN ANY WAY TO SUIT ANY INSTALLATION

* E, D = EDGE DISTANCE
(SEE NOTES & SCHEDULE ON SHEET 11)

INSTALLATION DETAILS

F.B.C. (Non High Velocity Hurricane Zone)

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e-mail: tilieco@tilieco.com
ED-0000712
ENTER A. TILIT & P. E.
FLORIDA EN. # 41187

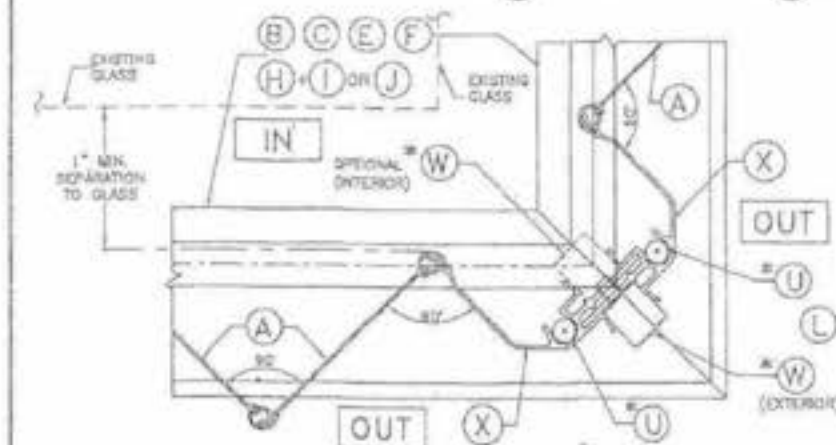
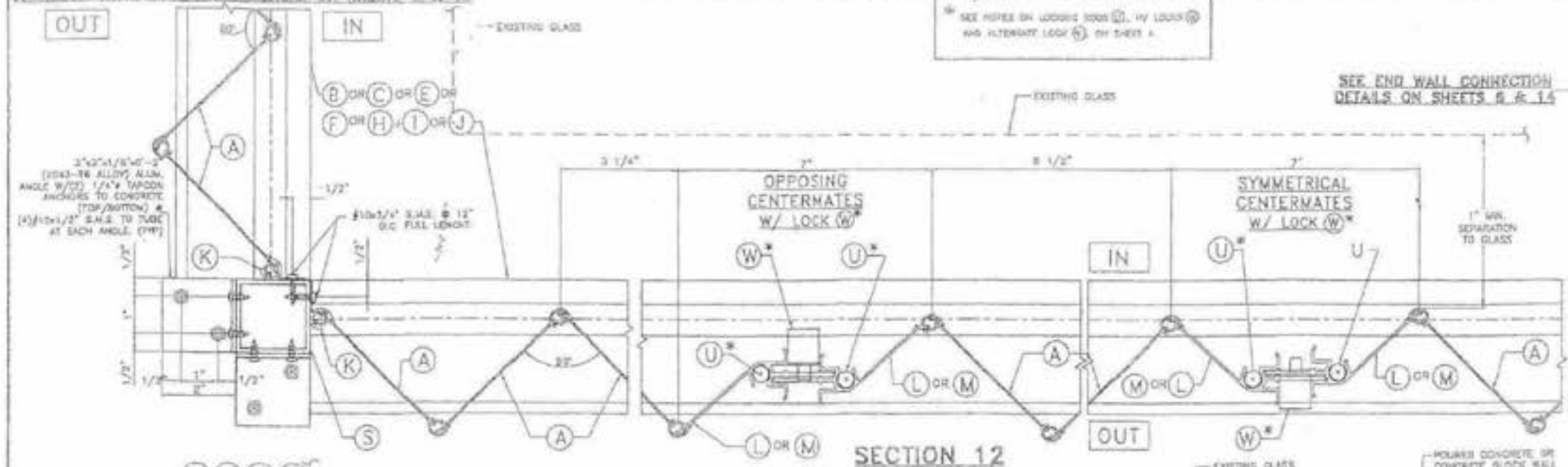


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4288 Westroads Drive
West Palm Beach, FL 33407

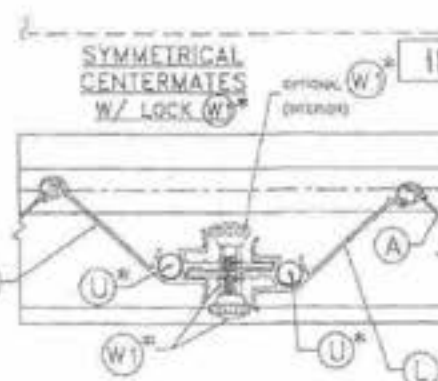
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REV. NO.	DESCRIPTION	DATE	Drawn by: E.A.S.
1	REV. 01-01	3/10/04	DATE: 7/13/05
2			DRAWING No
3			05-196
4			SHEET
5			5 OF 10

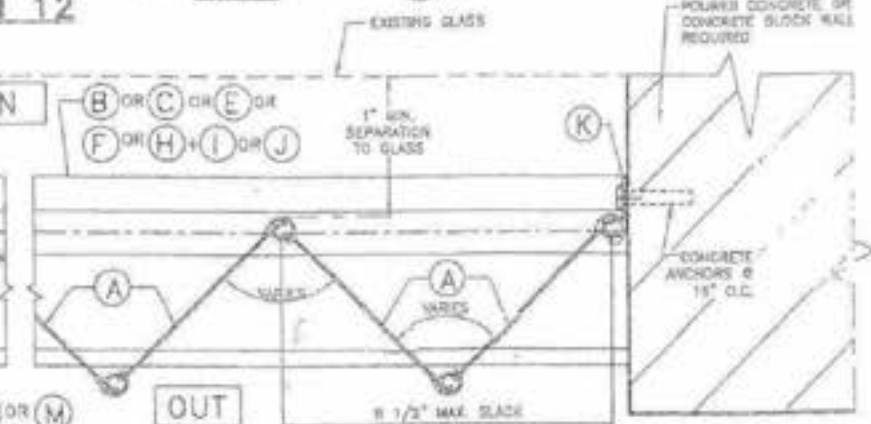
SEE END WALL CONNECTION DETAILS ON SHEETS 8 & 14



SECTION 12 AT CORNER CENTERMATE #3



ALTERNATE SECTION 12



SECTION 12 AT SLACK DETAIL (IF APPLICABLE)

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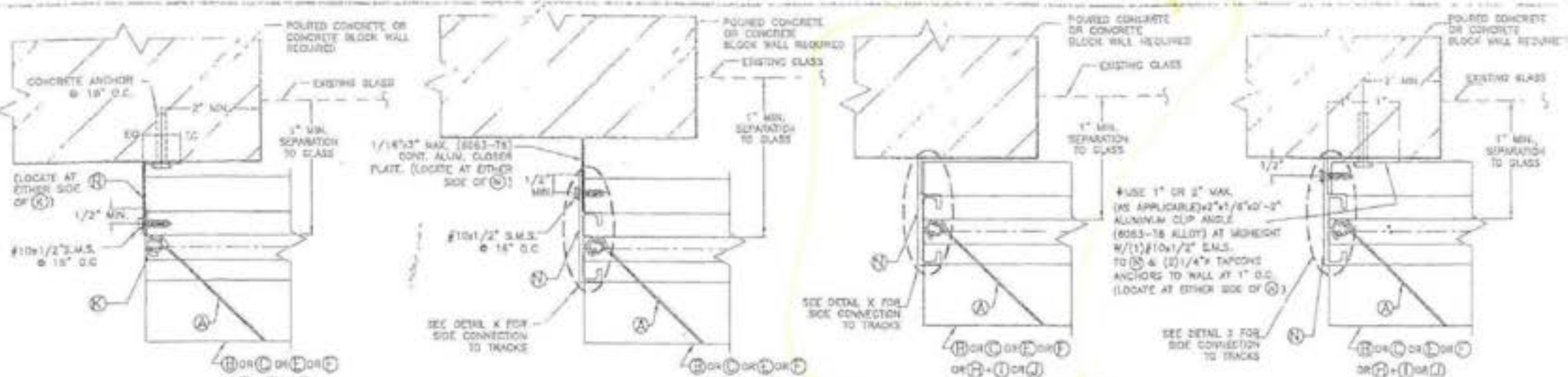
REV. NO.	DESCRIPTION	DATE	Drawn by J.A.S.
1	REV. 05-04	11/15/05	DATE: 7/13/05
2			DRAWING No
3			05-196
4			SHEET
5			7 OF 11



TILLIT CO. INC.

TILLIT TESTING & ENGINEERING COMPANY
6822 N.W. 24th St., Ste. 200, MIAMI GARDENS, FLORIDA 33146
Phone: (305) 871-1120 Fax: (305) 871-1521
E-mail: tillit@tillit.com
408-008713
WALTER A. TILLIT II, P. E.
FLORIDA LIC. # 48142

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American Shutter Systems
Association, Inc.
4258 Westroads Drive
West Palm Beach, FL 33407



END CONNECTION DETAIL ①

END CONNECTION DETAIL ②

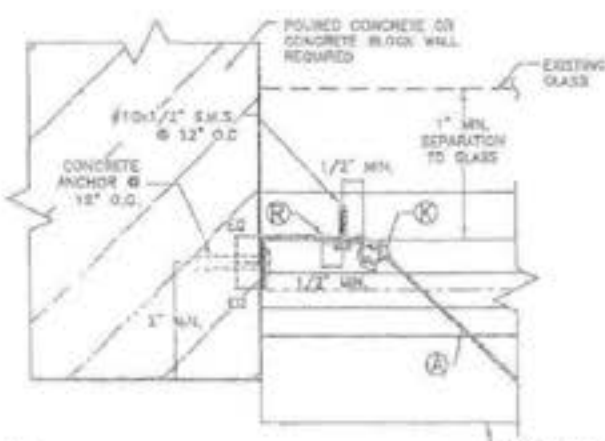
END CONNECTION DETAIL ③

END CONNECTION DETAIL ④

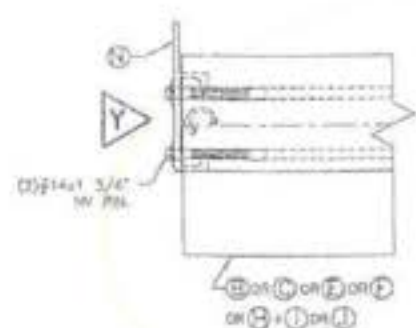
• REQUIRED ONLY FOR SPANS GREATER THAN 12'-0"



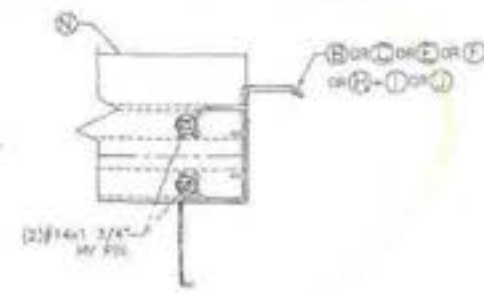
END CONNECTION DETAIL ⑤



END CONNECTION DETAIL ⑥



PLAN



VIEW Y

DETAIL X

F.B.C. (Wan High Velocity Hurricane Zone)

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WALTER A. TILTE, P.E.
FLORIDA Lic. # 14192

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REV.	DESCRIPTION	DATE	Drawn by J.A.S.
1	REV. 02-01	11/11/00	DATE: 7/13/05
2			
3			
4			
5			
6			

DRAWING No
05-196
SHEET
8 OF 14

MAXIMUM SHUTTER SPAN
"L+" OR "L-" (ft.) INSTALLATIONS
W/ SYMMETRICAL CENTERMATES,
ONE HV LOCK & NO LOCKING RODS *
(SEE NOTE 1)

MAXIMUM DESIGN LOAD (psf) (+ OR -)	CONCRETE & WOOD INSTALLATIONS &			
	WALL MOUNTING		FLOOR/CEILING MOUNTING	
	L+ (ft.)	L- (ft.)	L+ (ft.)	L- (ft.)
30 OR LESS	15'-0"	15'-0"	15'-0"	15'-0"
35	14'-10"	14'-10"	15'-2"	15'-0"
40	13'-10"	13'-10"	14'-2"	15'-2"
45	13'-11"	13'-11"	13'-4"	14'-3"
50	12'-5"	12'-5"	12'-5"	13'-7"
55	11'-10"	11'-10"	12'-5"	12'-11"
60	11'-4"	11'-4"	11'-7"	12'-4"
65	10'-10"	10'-10"	11'-1"	11'-11"
70	10'-6"	10'-6"	10'-9"	11'-5"
75	10'-1"	10'-1"	10'-4"	11'-1"
80	9'-10"	9'-10"	10'-2"	10'-9"
85	9'-5"	9'-5"	9'-9"	10'-5"
90	9'-3"	9'-3"	9'-5"	10'-1"
95	9'-2"	9'-2"	9'-2"	9'-10"
100	8'-9"	8'-9"	8'-5"	9'-7"
105	8'-7"	8'-7"	8'-4"	9'-4"
110	8'-4"	8'-4"	8'-3"	9'-2"
115	8'-5"	8'-5"	8'-4"	8'-11"
120	8'-5"	8'-5"	8'-2"	8'-9"
125	7'-10"	7'-10"	8'-0"	8'-5"
130	7'-8"	7'-8"	7'-10"	8'-1"
135	7'-7"	7'-7"	7'-8"	7'-9"
140	7'-5"	7'-5"	7'-3"	7'-6"
145	7'-3"	7'-3"	7'-5"	7'-3"
150	7'-2"	7'-2"	7'-4"	7'-0"
155	7'-8"	7'-8"	7'-2"	6'-9"
160	6'-11"	6'-11"	7'-1"	6'-7"
165	6'-10"	6'-10"	7'-0"	6'-4"
170	6'-9"	6'-9"	6'-11"	6'-2"
175	6'-7"	6'-7"	6'-8"	6'-0"
180	6'-6"	6'-6"	6'-8"	5'-10"
185	6'-5"	6'-5"	6'-7"	5'-8"
190	6'-4"	6'-4"	6'-6"	5'-6"
195	6'-3"	6'-3"	6'-5"	5'-5"
200	6'-2"	6'-2"	6'-4"	5'-3"
205	6'-1"	6'-1"	6'-3"	5'-1"

MAXIMUM SHUTTER SPAN
"L+" OR "L-" (ft.)
INSTALLATIONS W/
DEPOSING CENTERMATES,
ONE HV LOCK & NO
LOCKING RODS *
(SEE NOTE 1)

MAXIMUM DESIGN LOAD (psf) (+ OR -)	CONCRETE & WOOD INSTALLATIONS &	
	WALL MOUNTING & FLOOR/CEILING MOUNTING	
	L+ (ft.)	L- (ft.)
30	15'-0"	15'-0"
35	14'-10"	14'-10"
40	13'-10"	13'-10"
45	13'-11"	13'-11"
50	12'-5"	12'-5"
55	11'-10"	11'-10"
60	11'-4"	11'-4"
65	10'-10"	10'-10"
70	10'-6"	10'-6"
75	10'-1"	10'-1"
80	9'-10"	9'-10"
85	9'-5"	9'-5"
90	9'-3"	9'-3"
95	9'-2"	9'-2"
100	8'-9"	8'-9"
105	8'-7"	8'-7"
110	8'-4"	8'-4"
115	8'-5"	8'-5"
120	8'-5"	8'-5"
125	7'-10"	7'-10"
130	7'-8"	7'-8"
135	7'-7"	7'-7"
140	7'-5"	7'-5"
145	7'-3"	7'-3"
150	7'-2"	7'-2"
155	7'-8"	7'-8"
160	6'-11"	6'-11"
165	6'-10"	6'-10"
170	6'-9"	6'-9"
175	6'-7"	6'-7"
180	6'-6"	6'-6"
185	6'-5"	6'-5"
190	6'-4"	6'-4"
195	6'-3"	6'-3"
200	6'-2"	6'-2"
205	6'-1"	6'-1"

INSTALLATION LOADS		
	CONCRETE & MASONRY	WOOD
WALL MOUNTING	1, 2, 3 & 4	1W, 2W, 3W, 4W, 5W & 6W
FLOOR/CEILING MOUNTING	5, 6, 7 & 8	6W, 5W & 10W

A MAXIMUM SHUTTER SPAN "L+" OR "L-" FOR
INSTALLATIONS INTO WOOD SHALL BE LIMITED TO
110 PSF DESIGN LOAD.

† CORNER CENTERMATE #3 MAY BE USED AT ALL
MOUNTING INSTALLATIONS. LOCKING SPECIFICATIONS
SHALL BE AS INDICATED ON MANDATORY CONDITION #2
NOTE #A, SHEET 4 OF 14.

* NOTES:

(1) L+: MAXIMUM ALLOWABLE SPAN FOR A GIVEN
POSITIVE DESIGN LOAD.
L-: MAX. ALLOWABLE SPAN FOR A GIVEN NEGATIVE
DESIGN LOAD.

(2) PROCEDURE TO DETERMINE MAXIMUM SPAN FOR
WALL MOUNTING (TOP/BOTTOM) OR FLOOR/CEILING:

GIVEN A POSITIVE DESIGN LOAD, DETERMINE MAXIMUM
SPAN "L+" FROM SCHEDULE.
GIVEN A NEGATIVE DESIGN LOAD, DETERMINE MAXIMUM
SPAN "L-" FROM SCHEDULE.

FINAL MAXIMUM ALLOWABLE SPAN IS EQUAL TO THE
MINIMUM DETERMINED SPAN BETWEEN "L+" AND "L-".

(3) PROCEDURE TO DETERMINE MAXIMUM SPAN FOR
COMBINATIONS W/ BETWEEN WALL MOUNTING
W/ FLOOR/CEILING MOUNTING:

FOR A GIVEN POSITIVE DESIGN LOAD:

DETERMINE:
L1 = MAX. SPAN FOR WALL MOUNTING INSTALLATIONS.
L2 = MAX. SPAN FOR FLOOR/CEILING MOUNTING
INSTALLATIONS.

FOR A GIVEN NEGATIVE DESIGN LOAD:

DETERMINE:
L1 = MAX. SPAN FOR WALL MOUNTING INSTALLATIONS.
L2 = MAX. SPAN FOR FLOOR/CEILING MOUNTING
INSTALLATIONS.

FINAL MAXIMUM ALLOWABLE SPAN IS EQUAL TO THE
MINIMUM BETWEEN "L1+", "L2+", "L1-" AND "L2-".

(4) GO TO ANCHOR SCHEDULE WITH FINAL MAXIMUM
ALLOWABLE SPAN AND NEGATIVE DESIGN LOAD TO
DETERMINE MAXIMUM ANCHOR SPACING.

F.B.C. (Non High Velocity Hurricane Zone)

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BERTHA HV™ Accordian Shutter System

REV. NO.	DESCRIPTION	DATE	Drawn by J.A.S.
1	AS 19-19A	7/13/05	
2			
3			
4			
5			
6			

DRAWING No
05-196

SHEET
1 OF 14



TILTECO INC.

TILTE TESTING & ENGINEERING COMPANY
201 N. 20th St., Ste. 301, VERO BEACH, FLORIDA 33408
Phone: (305) 671-1500 • Fax: (305) 671-1531
e-mail: Tilt@Tiltco.com
© 2003 TILTECO
WALTER A. TILTE JR., P. E.
FLORIDA Lic. # 44167

A.S.S.A.
American Shutter Systems
Association, Inc.
4205 Westroads Drive
West Palm Beach, FL 33407

UNIVERSITY MICROFILMS

TAPER
ALL POINTS SHUT-OUT
WEDGE-BOLT

MAXIMUM DESIGN PRESSURE RATING (p.s.f.) AND CORRESPONDING MAXIMUM ANCHOR SPACING (in.) SCHEDULE FOR A GIVEN MAX. SHUTTER SPAN, DESIGN NEGATIVE LOAD AND A CORRESPONDING MOUNTING TYPE FOR CONCRETE OR CONCRETE BLOCK INSTALLATIONS (SEE MOUNTINGS 1, 2, 3 & 4 ON SHEET 5)

* SEE ANCHOR
SPECIFICATIONS
ON SHEET 11

SHUTTER SPAN (ft)	POURED CONCRETE												POURED CONCRETE/CONCRETE BLOCK											
	WALL MOUNTING 1 W/ (1) HV 1" (Max.) B/D HEADER/ SILL #1				WALL MOUNTING 1 W/ (1) HV 2" (Max.) B/D HEADER/ SILL #2				WALL MOUNTING 2				WALL MOUNTING 3 W/ (1) HV 1" (Max.) B/D HEADER/ SILL #1				WALL MOUNTING 3 W/ (1) HV 2" (Max.) B/D HEADER/ SILL #2				WALL MOUNTING 4			
	NEGATIVE DESIGN LOAD (psf)				NEGATIVE DESIGN LOAD (psf)				NEGATIVE DESIGN LOAD (psf)				NEGATIVE DESIGN LOAD (psf)				NEGATIVE DESIGN LOAD (psf)				NEGATIVE DESIGN LOAD (psf)			
	30	40	50	60	70	80	90	100	110	120	130	140	30	40	50	60	70	80	90	100	110	120	130	140
3'-0"	14	12	10	8	6	5	4	3	2	1	1	1	14	12	10	8	6	5	4	3	2	1	1	1
4'-0"	14	12	10	8	6	5	4	3	2	1	1	1	14	12	10	8	6	5	4	3	2	1	1	1
5'-0"	14	12	10	8	6	5	4	3	2	1	1	1	14	12	10	8	6	5	4	3	2	1	1	1
6'-0"	14	12	10	8	6	5	4	3	2	1	1	1	14	12	10	8	6	5	4	3	2	1	1	1
7'-0"	14	12	10	8	6	5	4	3	2	1	1	1	14	12	10	8	6	5	4	3	2	1	1	1
8'-0"	14	12	10	8	6	5	4	3	2	1	1	1	14	12	10	8	6	5	4	3	2	1	1	1
9'-0"	14	12	10	8	6	5	4	3	2	1	1	1	14	12	10	8	6	5	4	3	2	1	1	1
10'-0"	14	12	10	8	6	5	4	3	2	1	1	1	14	12	10	8	6	5	4	3	2	1	1	1
11'-0"	14	12	10	8	6	5	4	3	2	1	1	1	14	12	10	8	6	5	4	3	2	1	1	1
12'-0"	14	12	10	8	6	5	4	3	2	1	1	1	14	12	10	8	6	5	4	3	2	1	1	1
13'-0"	14	12	10	8	6	5	4	3	2	1	1	1	14	12	10	8	6	5	4	3	2	1	1	1
14'-0"	14	12	10	8	6	5	4	3	2	1	1	1	14	12	10	8	6	5	4	3	2	1	1	1
15'-0"	14	12	10	8	6	5	4	3	2	1	1	1	14	12	10	8	6	5	4	3	2	1	1	1
16'-0"	14	12	10	8	6	5	4	3	2	1	1	1	14	12	10	8	6	5	4	3	2	1	1	1
17'-0"	14	12	10	8	6	5	4	3	2	1	1	1	14	12	10	8	6	5	4	3	2	1	1	1
18'-0"	14	12	10	8	6	5	4	3	2	1	1	1	14	12	10	8	6	5	4	3	2	1	1	1
19'-0"	14	12	10	8	6	5	4	3	2	1	1	1	14	12	10	8	6	5	4	3	2	1	1	1
20'-0"	14	12	10	8	6	5	4	3	2	1	1	1	14	12	10	8	6	5	4	3	2	1	1	1

F.B.C. (Non High Velocity Hurricane Zone)

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P.E. SEAL/SIGNATURE/DATE

TILTECO INC.

TILT TESTING & ENGINEERING COMPANY
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Phone: (305) 571-1010 Fax: (305) 571-1051
E-mail: tilt@tilteco.com
OR - 0000718
WALTER A. TILT & SONS
FLORIDA Div. # 01102

A.S.S.A.

American Shutter Systems
Association, Inc.
4266 Westroads Drive
West Palm Beach, FL 33407

BERTHA HV™
Accordion Shutter System

REV.	DESCRIPTION	DATE	Drawn by J.A.S.
1	REV 01-000	1/13/03	DATE: 7/13/03
2			DRAWING NO
3			05-196
4			SHEET
5			10 OF 14

ANCHOR
TAPCON
ALL POINTS SQUAD-BOLT
WEDGE-BOLT

MAXIMUM DESIGN PRESSURE RATING (p.s.f.) AND CORRESPONDING MAXIMUM ANCHOR SPACING (in.) SCHEDULE FOR A GIVEN MAX. SHUTTER SPAN, DESIGN NEGATIVE LOAD AND A CORRESPONDING MOUNTING TYPE FOR CONCRETE INSTALLATIONS
(SEE MOUNTINGS 5, 6, 7, 8 ON SHEET 4)

SHUTTER SPAN (ft)	POURED CONCRETE																															
	CEILING MOUNTING 5								FLOOR MOUNTING 6								FLOOR MOUNTING 7								FLOOR MOUNTING 8							
	NEGATIVE DESIGN LOAD (psf)								NEGATIVE DESIGN LOAD (psf)								NEGATIVE DESIGN LOAD (psf)								NEGATIVE DESIGN LOAD (psf)							
	30	40	60	75	90	120	150	205	30	40	60	75	90	120	150	205	30	40	60	75	90	120	150	205	30	40	60	75	90	120	150	205
3'-0" OR LESS	14	12	10	8	6	5	4	3	14	12	10	8	6	5	4	3	14	12	10	8	6	5	4	3	14	12	10	8	6	5	4	3
4'-0"	14	12	10	8	6	5	4	3	14	12	10	8	6	5	4	3	14	12	10	8	6	5	4	3	14	12	10	8	6	5	4	3
5'-0"	14	12	10	8	6	5	4	3	14	12	10	8	6	5	4	3	14	12	10	8	6	5	4	3	14	12	10	8	6	5	4	3
6'-0"	14	12	10	8	6	5	4	3	14	12	10	8	6	5	4	3	14	12	10	8	6	5	4	3	14	12	10	8	6	5	4	3
7'-0"	14	12	10	8	6	5	4	3	14	12	10	8	6	5	4	3	14	12	10	8	6	5	4	3	14	12	10	8	6	5	4	3
8'-0"	14	12	10	8	6	5	4	3	14	12	10	8	6	5	4	3	14	12	10	8	6	5	4	3	14	12	10	8	6	5	4	3
9'-0"	14	12	10	8	6	5	4	3	14	12	10	8	6	5	4	3	14	12	10	8	6	5	4	3	14	12	10	8	6	5	4	3
10'-0"	14	12	10	8	6	5	4	3	14	12	10	8	6	5	4	3	14	12	10	8	6	5	4	3	14	12	10	8	6	5	4	3
11'-0"	14	12	10	8	6	5	4	3	14	12	10	8	6	5	4	3	14	12	10	8	6	5	4	3	14	12	10	8	6	5	4	3
12'-0"	14	12	10	8	6	5	4	3	14	12	10	8	6	5	4	3	14	12	10	8	6	5	4	3	14	12	10	8	6	5	4	3
13'-0"	14	12	10	8	6	5	4	3	14	12	10	8	6	5	4	3	14	12	10	8	6	5	4	3	14	12	10	8	6	5	4	3
14'-0"	14	12	10	8	6	5	4	3	14	12	10	8	6	5	4	3	14	12	10	8	6	5	4	3	14	12	10	8	6	5	4	3
15'-0"	14	12	10	8	6	5	4	3	14	12	10	8	6	5	4	3	14	12	10	8	6	5	4	3	14	12	10	8	6	5	4	3
16'-0"	14	12	10	8	6	5	4	3	14	12	10	8	6	5	4	3	14	12	10	8	6	5	4	3	14	12	10	8	6	5	4	3

NOTES ON ANCHOR REQUIREMENTS: POURED CONCRETE OR CONCRETE BLOCK SUBSTRATES

1. ANCHORS TO WALL SHALL BE AS FOLLOWS:

(A) TO EXISTING POURED CONCRETE (MIN. $f'_c = 3,000$)

- 1/4" TAPCON ANCHORS AS MANUFACTURED BY L.T.C. BANGS/ RED HEAD

- 1/4" x 3/4" ALL POINTS SOLID-SET ANCHORS AS DISTRIBUTED BY ALL POINTS SQUAD-BOLT & SPECIALTY COMPANY

- 1/4" WEDGE-BOLT ANCHORS AS MANUFACTURED BY FORDS FASTENING, INC.

NOTES:

A.1) MINIMUM EMBEDMENT INTO POURED CONCRETE OF TAPCON ANCHORS IS 1 1/4", AND 2" FOR WEDGE-BOLT ANCHORS.

A.2) MINIMUM EMBEDMENT OF 1/4" x 3/4" ALL POINTS SOLID-SET ANCHORS SHALL BE 7/8" INTO THE POURED CONCRETE. NO EMBEDMENT INTO STUCCO SHALL BE PERMITTED. 1/4" #20 U.S. MACHINE SCREW USED SHALL BE 1 1/2" LONG MINIMUM SHOULD STUCCO EXIST, AND 1" MINIMUM FOR WALLS WITH NO STUCCO.

A.3) IN CASE THAT PRECAST STONE, PRECAST CONCRETE PANELS, PAVES OR ANY VENEER BE FOUND ON THE EXISTING WALL OR FLOOR, ANCHORS SHALL BE LONG ENOUGH TO REACH THE MAIN STRUCTURE BEHIND SAID WALL, FINISHES, ANCHORAGE SHALL BE AS INDICATED ON NOTES A.1) & A.2) ABOVE. FOR INSTALLATIONS ON VINYL SIDING OR EPS CONSULT THE ENGINEER.

(B) TO EXISTING CONCRETE BLOCK WALL:

- 1/4" TAPCON ANCHORS AS MANUFACTURED BY L.T.C. BANGS/ RED HEAD

- 1/4" x 3/4" ALL POINTS SOLID-SET ANCHORS AS DISTRIBUTED BY ALL POINTS SQUAD-BOLT & SPECIALTY COMPANY

- 1/4" WEDGE-BOLT W/ 1/4" BLOCK-PLUS ANCHORS AS MANUFACTURED BY FORDS FASTENING, INC.

NOTES:

B.1) MINIMUM EMBEDMENT INTO CONCRETE BLOCK OF TAPCON & WEDGE-BOLT ANCHORS IS 1 1/4".

B.2) MINIMUM EMBEDMENT OF 1/4" x 3/4" ALL POINTS SOLID-SET ANCHORS SHALL BE ENTIRELY EMBEDDED INTO THE CONCRETE BLOCK. NO EMBEDMENT INTO STUCCO SHALL BE PERMITTED. 1/4" #20 U.S. MACHINE SCREW USED SHALL BE 1 1/2" LONG MINIMUM SHOULD STUCCO EXIST, AND 1" MINIMUM FOR WALLS WITH NO STUCCO.

B.3) IN CASE THAT PRECAST STONE, PRECAST CONCRETE PANELS, PAVES OR ANY VENEER BE FOUND ON THE EXISTING WALL OR FLOOR, ANCHORS SHALL BE LONG ENOUGH TO REACH THE MAIN STRUCTURE BEHIND SAID WALL, FINISHES, ANCHORAGE SHALL BE AS INDICATED ON NOTES B.1) & B.2) ABOVE. FOR INSTALLATIONS ON VINYL SIDING OR EPS CONSULT THE ENGINEER.

(C) ANCHORS SHALL BE INSTALLED FOLLOWING ALL OF THE RECOMMENDATIONS AND SPECIFICATIONS OF THE ANCHOR'S MANUFACTURER.

(D) SEE SCHEDULE BUGH FOR EDGE DISTANCE (ED) VERSUS SPACING RELATIONSHIP FOR ANCHORS.

EDGE DISTANCE VERSUS SPACING FOR ANCHORS

MINIMUM ANCHOR SPACINGS ARE VALID FOR 3 1/2" EDGE DISTANCE FOR 1/4" TAPCON AND SOLID SET AND WEDGE-BOLT FOR E. D. LESS THAN THE ABOVE MENTIONED. REDUCE ANCHOR SPACING BY MULTIPLYING SPACING SHOWN ON SCHEDULE BY THE FOLLOWING FACTORS. (NOTE: MIN. E. D. FOR ALL POINT SOLID SET ANCHORS IS 3"). REDUCED SPACING OBTAINED USING FACTOR SHALL NOT BE LESS THAN MINIMUM SPACING INDICATED FOR EACH ANCHOR TYPE.

ACTUAL E. D.	FACTOR		
	WEDGE-BOLT	1/4" TAPCON	SOLID SET
3"	0.83	0.83	0.78
3 1/2"	0.88	0.88	-
4"	0.90	0.90	-

F.B.C. (Non High Velocity Hurricane Zone)

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BERTHA HV™
Accordion Shutter System

REV. NO.	DESCRIPTION	DATE	Drawn By: J.S.S.
1	AS SHOWN	DATE: 7/12/05	
2			DRAWING No
3			05-196
4			SHEET
5			11 OF 14



TILTECO TESTING & ENGINEERING COMPANY
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ALTER A. TILTECO, P.E.
FLORIDA No. 64137

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West Palm Beach, FL 33407

**MAXIMUM DESIGN PRESSURE RATING (p.s.f.) AND CORRESPONDING MAXIMUM ANCHOR* SPACING (in.) SCHEDULE FOR
A GIVEN MAX. SHUTTER SPAN, NEGATIVE DESIGN LOAD AND A CORRESPONDING MOUNTING TYPE.**

(SEE MOUNTINGS ON SHEETS 13 & 14).

MAXIMUM SPAN (ft)	SUBSTRATE																																											
	SOUTHERN PINE No. 2 W/ MIN. G=0.55, DOUGLAS FIR W/ MIN. G=0.50												SPRUCE PINE FIR NORTH W/ MIN. G=0.43										SPRUCE PINE FIR SOUTH W/ MIN. G=0.36										1/2" MIN. CDX PLYWOOD											
	WALL MOUNTINGS 1W, 2W, 3W & 6W						CEILING MOUNT 8W FLOOR MOUNTINGS 9W & 10W						WALL MOUNTINGS 1W, 2W, 3W & 6W						CEILING MOUNT 8W FLOOR MOUNTINGS 9W & 10W						WALL MOUNTINGS 1W, 2W, 3W & 6W						CEILING MOUNT 8W FLOOR MOUNTINGS 9W & 10W						WALL MOUNTINGS 4W & 5W							
	NEGATIVE DESIGN LOAD (psf)						NEGATIVE DESIGN LOAD (psf)						NEGATIVE DESIGN LOAD (psf)						NEGATIVE DESIGN LOAD (psf)						NEGATIVE DESIGN LOAD (psf)						NEGATIVE DESIGN LOAD (psf)						NEGATIVE DESIGN LOAD (psf)							
	30	40	60	75	90	105	30	40	60	75	90	105	30	40	60	75	90	105	30	40	60	75	90	105	30	40	60	75	90	105	30	40	60	75	90	105								
3'-0" OR LESS	12	10	10	9	8	8	8	6	6	6	6	6	12	10	10	9	8	8	8	6	6	6	6	6	6	12	10	10	9	8	8	8	6	6	6	6	6	6	5	4	4	4	3	
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6'-0"	12	10	10	9	8	6.5	6	6	6	6	5.5	4.5	12	10	10	9	8	8	6	6	6	5.5	4.5	3.5	2.5	12	10	10	9	8	6	3.5	6	6	6	6	5	4.5	6	5	4	4	3.5	-
7'-0"	12	10	10	9	6.5	4	6	6	6	5.5	4.5	4	12	10	10	9	5	3	6	6	6	5	4	3.5	2.5	12	10	10	7	3.5	-	6	6	6	5	4	3.5	5	5	4	4	-	-	
8'-0"	12	10	10	7.5	4.5	3	8	6	6	5	4	3.5	12	10	10	6	3.5	-	6	6	5	4.5	3.5	3	2.5	12	10	10	4	-	-	6	6	5	4.5	3.5	3	6	5	4	-	-	-	
9'-0"	12	10	10	5	3.5	-	6	6	5.5	4.5	3.5	3	12	10	9	4	-	-	6	6	4.5	4	3	3	2.5	12	10	10	4	3	-	-	6	6	4.5	4	3	3	6	5	3.5	-	-	-
10'-0"	12	10	7.5	4	-	-	6	6	5	4	3	-	12	10	6	3	-	-	6	6	4	3.5	3	-	12	10	3	-	-	-	-	6	6	4	3.5	3	-	6	5	-	-	-	-	
11'-0"	12	10	5.5	-	-	-	6	6	4.5	-	-	-	12	10	4.5	-	-	-	6	5.5	3.5	-	-	-	12	10	-	-	-	-	-	6	5.5	4	-	-	-	6	5	-	-	-	-	
12'-0"	12	10	4.5	-	-	-	6	5	4	-	-	-	12	10	3.5	-	-	-	6	5	3.5	-	-	-	12	10	-	-	-	-	-	6	5	3.5	-	-	-	6	5	-	-	-	-	
13'-0"	12	10	-	-	-	-	6	6	-	-	-	-	12	10	-	-	-	-	6	4.5	-	-	-	-	12	7.5	-	-	-	-	-	6	4.5	-	-	-	-	6	4.5	-	-	-	-	
14'-0"	12	10	-	-	-	-	6	5.5	-	-	-	-	12	7	-	-	-	-	5.5	4.5	-	-	-	-	12	7	-	-	-	-	-	5.5	4.5	-	-	-	-	6	3	-	-	-	-	
15'-0"	12	-	-	-	-	-	6	-	-	-	-	-	12	-	-	-	-	-	5	-	-	-	-	-	12	-	-	-	-	-	-	5	-	-	-	-	-	5	-	-	-	-	-	
16'-0"	12	-	-	-	-	-	6	-	-	-	-	-	12	-	-	-	-	-	5	-	-	-	-	-	11.5	-	-	-	-	-	-	5	-	-	-	-	-	6	-	-	-	-	-	

+ NOTES ON ANCHORS REQUIREMENTS: WOOD SUBSTRATES.

1. ANCHORS TO WALL, FLOOR OR SOLID WOOD JOIST SHALL BE AS FOLLOWS:

- 1/4" TYPICAL ANCHORS, AS MANUFACTURED BY L.T.V. BUILDING W/ MINIMUM EMBEDMENT AS INDICATED AT EACH SECTION ON SHEETS 13 & 14.
EXCEPTION: SEE NOTE 2 BELOW.

2. ANCHORS TO HOLLOW CEILING SHALL BE AS PER MOUNTING TYPE TW, SHEET 14. AND SHALL CONSIST OF LAG BOLDS AS PER NDS 1987 SPECIFICATIONS.

3. SUBSTRATE SHALL CONSIST OF THE FOLLOWING:

- SOUTHERN PINE No. 2 W/ G=0.55 (N.D.S.)
- DOUGLAS FIR W/ G=0.50 (N.D.S.)
- SPRUCE PINE FIR NORTH W/ G=0.43 (N.D.S.)
- SPRUCE PINE FIR SOUTH W/ G=0.36 (N.D.S.)
- W/ 1/2" CDX PLYWOOD (1988 APA)

4. ANCHORS SHALL BE INSTALLED FOLLOWING ALL OF THE RECOMMENDATIONS AND SPECIFICATIONS OF THE ANCHORS MANUFACTURER.

F.B.C. (Non High Velocity Hurricane Zone)

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**BERTHA HV™
Accordion Shutter System**

REV. NO.	DESCRIPTION	DATE	Drawn by J.A.S.
1	NO. 1000	7/13/05	DATE: 7/13/05
2			DRAWING No
3			05-196
4			SHEET
5			12 OF 14

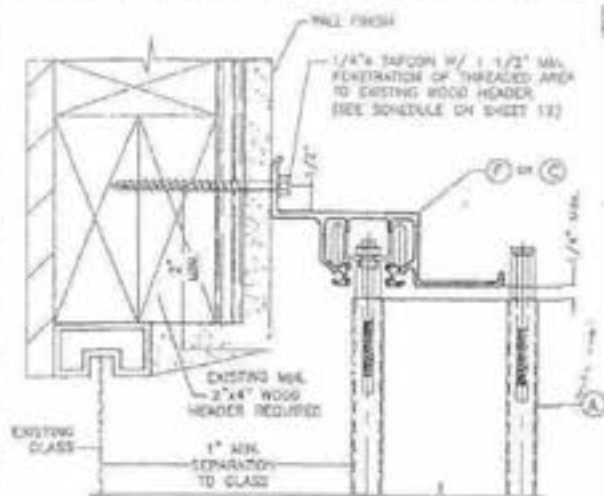


TILTECO INC.
TILT TESTING & ENGINEERING COMPANY
4258 W. 30th, Bldg. 300, Miramar Gardens, Miramar, FL 32550
Phone: (305) 671-1530, Fax: (305) 671-1531
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813-000-7115
WALTER A. TILLY, Jr., P.E.
FLORIDA Lic. # 94751

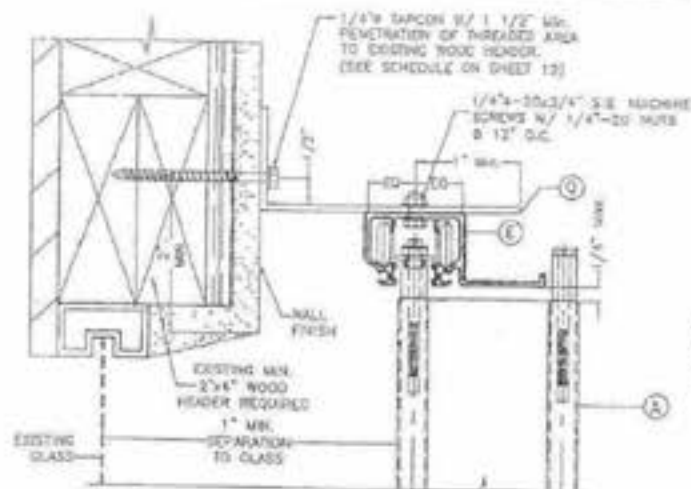
A.S.S.A.
American Shutter Systems
Association, Inc.
4258 Westroads Drive
West Palm Beach, FL 33407

INSTALLATION DETAILS ON EXISTING WOOD BUILDINGS

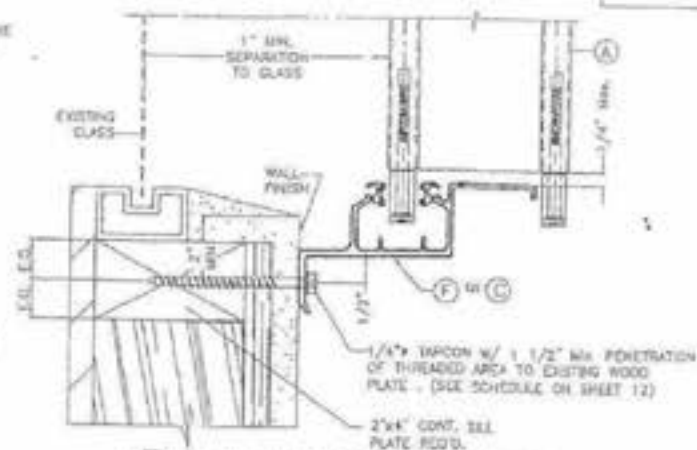
NOTE:
1-SEE SHEET 3 FOR TYPICAL SHUTTER
ASSEMBLY
2-MOUNTING SECTIONS CAN BE
COMBINED IN ANY WAY TO SUIT ANY
INSTALLATION.



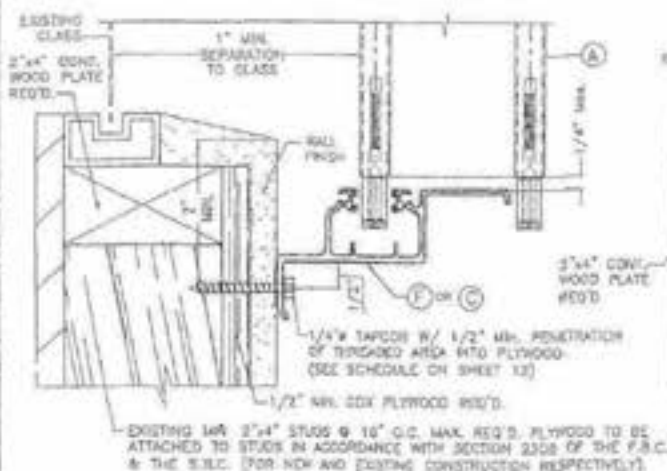
1W WALL CONNECTION AT TOP



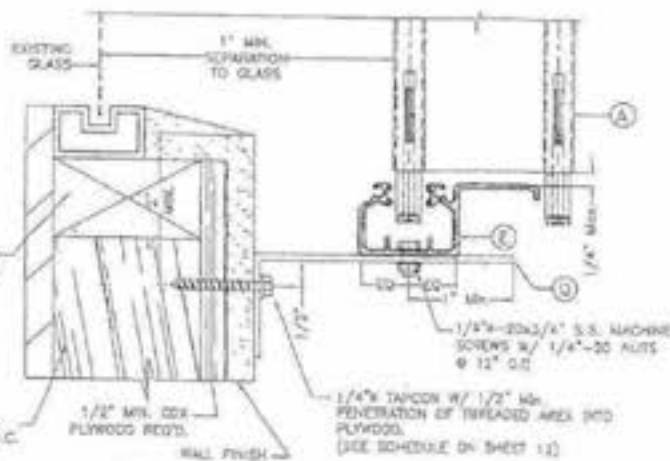
2W WALL CONNECTION AT TOP



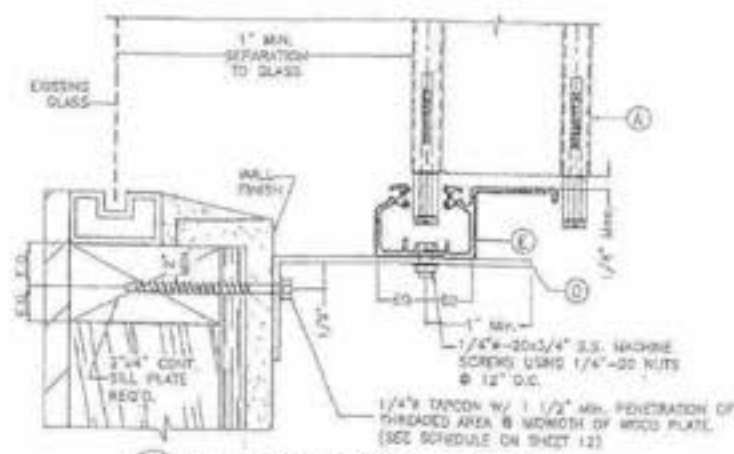
3W WALL CONNECTION AT BOTTOM



4W WALL CONNECTION AT BOTTOM



5W WALL CONNECTION AT BOTTOM



6W WALL CONNECTION AT BOTTOM

F.B.C. (Non High Velocity Hurricane Zone)

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TILECO INC.
TULLY TESTING & ENGINEERING COMPANY
2825 S.W. 38th St., Suite 300, Fort Lauderdale, Florida 33308
Phone: (305) 571-1530, Fax: (305) 571-1531
www.tileco.com
BB-0004718
WALTER A. TULL, P.E.
FLORIDA Lic. # 12418

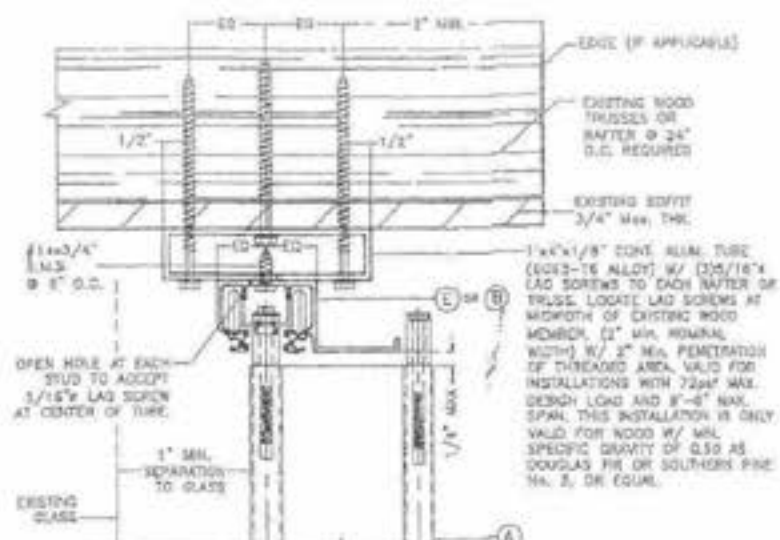
A.S.S.A.
American Shutter Systems
Association, Inc.
4265 Westroads Drive
West Palm Beach, FL 33407

BERTHA HV™
Accordion Shutter System

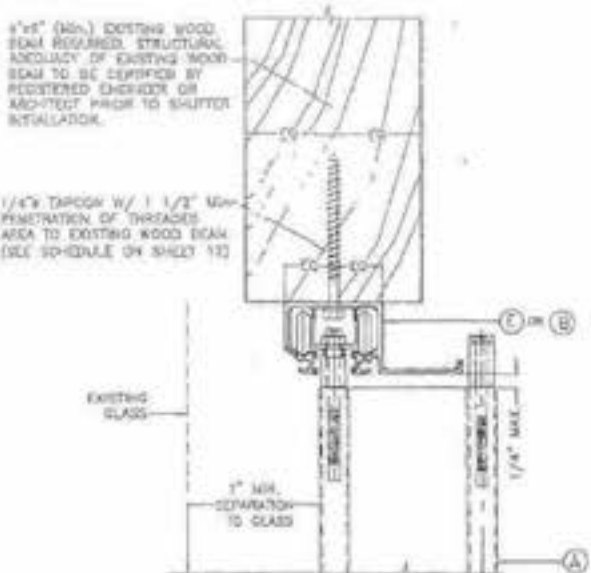
REV. NO.	DESCRIPTION	DATE	Drawn By/Checked By
1	REV. 10-01	7/13/05	
2			
3			
4			
5			
6			

DRAWING NO.
05-196

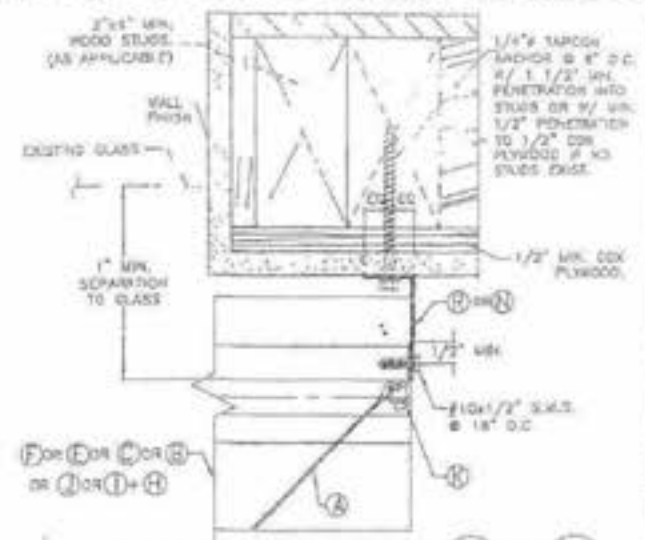
SHEET
13 OF 14



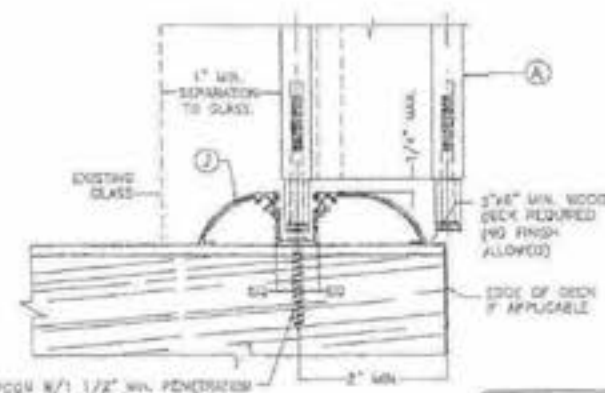
7W HOLLOW CEILING MOUNTING INSTALLATION



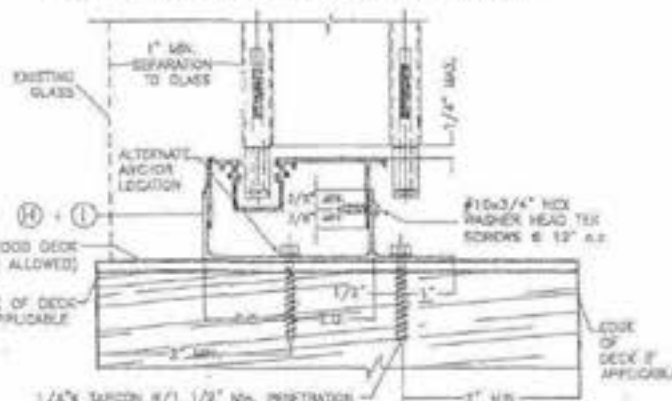
8W SOFFIT MOUNTING INSTALLATION



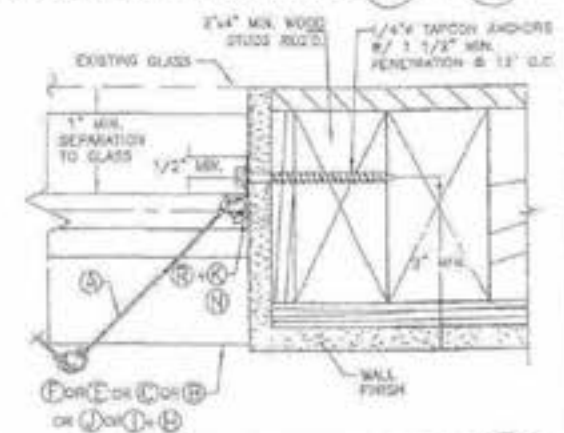
END CONNECTION DETAIL 1W & 4W



9W FLOOR CONNECTION



10W FLOOR CONNECTION



END CONNECTION DETAIL 5W

NOTES:
1-SEE SHEET 3 FOR TYPICAL SHUTTER ASSEMBLY
2-MOUNTING SECTIONS CAN BE COMBINED
IN ANY WAY TO SUIT ANY INSTALLATION.

F.B.C. (Non High Velocity Hurricane Zone)

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JOHN A. TILLEY & SONS
P.E. SEAL/SIGNATURE/DATE

TILTECO INC.
TILIT TESTING & ENGINEERING COMPANY
4500 N.W. 30th St., Ft. Lauderdale, Florida 33309
Phone: (305) 771-1520 Fax: (305) 771-1521
e-mail: tiliteco@aol.com
CR-2008712
WALTER A. TILIT, J.E.
FLORIDA Lic. # 44197

A.S.S.A.
American Shutter Systems
Association, Inc.
4255 Westroads Drive
West Palm Beach, FL 33407

BERTHA HV™ Accordion Shutter System			
REV. NO.	DESCRIPTION	DATE	Drawn By: J.A.S.
1	REV. 01-03	1/11/04	DATE: 7/13/03
2			DRAWING NO.
3			05-196
4			SHEET
5			14 OF 16
6			

BUILDING'S PEAK ROOF HEIGHT vs DESIGN LOAD FOR COMPONENTS & CLADDING (P.S.F.) FOR ASCE 7-02 - PEAK ROOF HEIGHT LESS THAN OR EQUAL TO 60 FEET *

BASIC WIND SPEED = 140 mph CATEGORY II BUILDINGS (I=1.00)
RESIDENTIAL, APARTMENT BUILDINGS & COMMERCIAL BUILDINGS (RETAIL, WHOLESALE)

DESIGN LOADS INDICATED BELOW SHALL ONLY BE USED FOR THE DETERMINATION OF POSITIVE OR NEGATIVE PRESSURES FOR A GIVEN OPENING OR OPENINGS OF A BUILDING WHERE SAID OPENINGS ARE SURROUNDED BY OTHER OPENINGS LOCATED WITHIN THE LOWEST 60 FEET OF BUILDING, AND WHICH ARE PROTECTED AGAINST WIND BORNE DEBRIS BY MEAN OF IMPACT RESISTANT GLASS OR ADEQUATE HURRICANE SHUTTERS. ***

DESIGN LOADS INDICATED BELOW SHALL ONLY BE USED FOR THE DETERMINATION OF POSITIVE OR NEGATIVE PRESSURES FOR A GIVEN OPENING OR OPENINGS OF A BUILDING WHERE SAID OPENINGS ARE SURROUNDED BY OTHER OPENINGS LOCATED WITHIN THE LOWEST 60 FEET OF BUILDING, AND WHICH ARE NOT PROTECTED AGAINST WIND BORNE DEBRIS BY MEAN OF IMPACT RESISTANT GLASS OR ADEQUATE HURRICANE SHUTTERS. ***

PEAK ROOF HEIGHT (ft)	1** LESS THAN 7'-0"		1** GREATER OR EQUAL THAN 7'-0" AND LESS THAN 12'-0"		1** GREATER OR EQUAL THAN 12'-0"		1** LESS THAN 7'-0"		1** GREATER OR EQUAL THAN 7'-0" AND LESS THAN 12'-0"		1** GREATER OR EQUAL THAN 12'-0"	
	POSITIVE	NEGATIVE	POSITIVE	NEGATIVE	POSITIVE	NEGATIVE	POSITIVE	NEGATIVE	POSITIVE	NEGATIVE	POSITIVE	NEGATIVE
0' - 15'	47.7	57.9	40.5	52.4	37.8	46.8	58.1	70.8	53.9	65.8	51.2	60.2
> 15'-20'	45.4	60.8	43.0	55.7	40.1	49.7	59.8	75.0	57.3	69.9	54.4	63.9
> 20'-25'	47.8	63.7	45.1	58.4	42.1	52.1	62.5	78.6	60.0	73.3	57.0	67.0
> 25'-30'	43.4	66.2	46.8	60.7	43.7	54.1	64.3	81.7	62.4	76.2	59.2	69.8
> 30'-40'	52.5	70.3	49.8	64.4	46.4	57.5	69.0	86.8	68.3	80.9	62.9	74.0
> 40'-50'	55.0	73.7	52.2	67.5	48.7	60.3	72.3	91.0	69.4	84.8	65.9	77.5
> 50'-60'	57.2	76.6	54.2	70.2	50.8	62.6	75.1	94.5	72.2	88.1	68.5	80.8

BUILDING'S PEAK ROOF HEIGHT vs DESIGN LOAD FOR COMPONENTS & CLADDING (P.S.F.) FOR ASCE 7-02 - PEAK ROOF HEIGHT GREATER THAN 60 FEET *

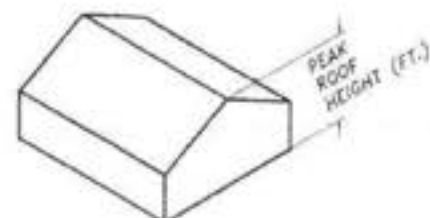
BASIC WIND SPEED = 140 mph CATEGORY II BUILDINGS (I=1.00)
RESIDENTIAL, APARTMENT BUILDINGS & COMMERCIAL BUILDINGS (RETAIL, WHOLESALE)

DESIGN LOADS INDICATED BELOW SHALL ONLY BE USED FOR THE DETERMINATION OF POSITIVE OR NEGATIVE PRESSURES FOR A GIVEN OPENING OR OPENINGS OF A BUILDING WHERE SAID OPENINGS ARE SURROUNDED BY OTHER OPENINGS LOCATED WITHIN THE LOWEST 60 FEET OF BUILDING, AND WHICH ARE PROTECTED AGAINST WIND BORNE DEBRIS BY MEAN OF IMPACT RESISTANT GLASS OR ADEQUATE HURRICANE SHUTTERS. ***

DESIGN LOADS INDICATED BELOW SHALL ONLY BE USED FOR THE DETERMINATION OF POSITIVE OR NEGATIVE PRESSURES FOR A GIVEN OPENING OR OPENINGS OF A BUILDING WHERE SAID OPENINGS ARE SURROUNDED BY OTHER OPENINGS LOCATED WITHIN THE LOWEST 60 FEET OF BUILDING, AND WHICH ARE NOT PROTECTED AGAINST WIND BORNE DEBRIS BY MEAN OF IMPACT RESISTANT GLASS OR ADEQUATE HURRICANE SHUTTERS. ***

PEAK ROOF HEIGHT (ft)	1** LESS THAN 12'-0"		1** GREATER OR EQUAL THAN 12'-0"		1** LESS THAN 12'-0"		1** GREATER OR EQUAL THAN 12'-0"	
	POSITIVE	NEGATIVE	POSITIVE	NEGATIVE	POSITIVE	NEGATIVE	POSITIVE	NEGATIVE
> 60' - 70'	54.1	59.1	49.2	65.3	72.6	117.7	67.7	103.8
> 70' - 80'	55.6	102.0	50.6	87.7	74.7	123.0	69.6	106.7
> 80' - 90'	57.0	104.5	51.8	89.8	76.6	124.1	71.4	109.4
> 90' - 100'	58.3	106.9	53.0	91.9	78.3	126.8	73.0	111.9
> 100' - 120'	60.6	111.7	55.1	95.5	81.3	131.8	75.8	116.3
> 120' - 140'	62.6	114.7	56.9	98.7	84.0	136.2	78.3	120.1
> 140' - 160'	64.4	118.0	58.5	101.5	86.4	140.0	80.6	123.5
> 160' - 180'	66.0	121.0	60.0	104.0	88.6	143.6	82.6	126.6
> 180' - 200'	67.5	123.7	61.3	106.3	90.6	146.8	84.4	129.5
> 200' - 250'	70.7	129.6	64.3	111.5	94.9	153.8	88.5	135.7
> 250' - 300'	73.5	134.7	66.8	115.8	98.6	158.9	92.0	141.0
> 300' - 350'	75.9	139.1	69.0	119.6	101.9	165.1	95.0	145.8
> 350' - 400'	78.1	143.1	71.0	123.1	104.8	169.8	97.7	149.8
> 400' - 450'	80.0	146.7	72.7	126.1	107.4	174.1	100.2	153.6
> 450' - 500'	81.8	150.0	74.4	129.0	109.8	178.0	102.4	157.0

* INDICATED PRESSURES CORRESPOND TO THE WINDWARD END ZONE AND INTERIOR ZONE CONDITIONS AND FOR ALL ROOF SLOPES.
** L = STORM PANEL LENGTH ACCORDING SHUTTER BLADE LENGTH OR BAHAMA SHUTTER HEIGHT
*** PRODUCTS WHICH HAVE A MAIN-STATE COUNTY OR A F.E.C. APPROVAL



BUILDING SCHEMATIC

NOTES:

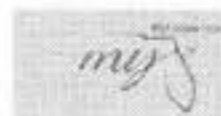
- 1.) THIS SCHEDULE SHOULD BE USED AND LOAD SCHEDULE PROVIDED BY THE ENGINEER, ARCHITECT OR ASCE 7-02, SECTION 6 SHOULD BE USED FOR A GIVEN WIND SPEED OF 140 MPH AND APPLICABLE WINDWAVE FACTORS & APPROPRIATE & SCHEMATICALLY CORRECT AND DOES NOT PROVIDE INFORMATION FOR A SITE SPECIFIC PROJECT.
- 2.) THE WIND LOAD SCHEDULE MUST BE REVIEWED, SIGNED AND DATED BY THE ENGINEER IF USED TO DESIGN A ONE STORY BUILDING PERMIT.
- 3.) CONTRACTOR OR PERMIT HOLDER TO BE RESPONSIBLE FOR THE SELECTION OF DESIGN AND LOAD WIND SPEED ON THE SCHEDULE AND LOAD SCHEDULE.
- 4.) THE WIND LOAD SCHEDULE SHALL BE ENDORSED WITH A SIGNATURE BY THE ENGINEER.
- 5.) DESIGN AND LOAD SCHEDULES BY THE CONTRACTOR OR PERMIT HOLDER FROM THIS WIND LOAD SCHEDULE SHALL BE INDICATED ON A SEPARATE DOCUMENT PREPARED BY THE CONTRACTOR OR PERMIT HOLDER (e.g., SPECIFICATIONS, OPENING DETAILS, ETC.).

2004 FLORIDA BUILDING CODE (WITH HIGH WIND VELOCITY HURRICANE ZONES)

ASCE 7-02 / WIND LOAD SCHEDULES STORM PANELS ACCORDING TO BAHAMA SHUTTERS						AS DESIGN SCALE
WIND LOAD SCHEDULE						DATE
BASIC WIND SPEED = 140 mph EXPOSURE C, K _f = 0.85						02 - 080
USE IN	DESCRIPTION	USE	USE IN	DESCRIPTION	USE	DATE
1	STORM PANELS	1	STORM PANELS	1	STORM PANELS	02-080
2	STORM PANELS	2	STORM PANELS	2	STORM PANELS	02-080

TILECO INC.
TILE TESTING & ENGINEERING COMPANY
1001 N. W. 10th St. Suite 100, Ft. Lauderdale, FL 33304
(954) 566-7777
FAX: (954) 566-7778
WWW.TILECO.COM

Community Affairs



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Product Approval
USER: Public User

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[Application Detail](#)

- BUILDING CODES
- FLORIDA COMMUNITY TRUST
- FROM PUNCH FLOOR

- SEAS
- FREQUENTLY ASKED QUESTIONS
- ABOUT OUR SECRETARY
- E-MAIL THE SECRETARY
- INSPECTOR GENERAL
- WEB ASSISTANCE
- CONTACT US
- OUR LOGO
- DCA EMPLOYEE ROSTER

FI	FL2668-R1
Application Type	Revision
Code Version	2004
Application Status	Approved
Comments	
Archived	☐
Product Manufacturer	Town and Country Industries, Inc.
Address Phone Email	400 West McNab Road Ft. Lauderdale, FL 33309 (954) 970-7700 townj@tc-alum.com
Authorized Signatory	Vladimir Knezevich vjk@knezevich.com
Technical Representative	Thomas B. Johnston
Address Phone Email	400 west McNab Rd. Ft. Lauderdale, FL 33309 (954) 970-9999 townj@tc-alum.com
Quality Assurance Representative	
Address Phone Email	
Category	Shutters
Subcategory	Bahama
Compliance Method	Evaluation Report from a Florida Registered Architect or a Licensed Florida Professional Engineer ☐ Evaluation Report - Hardcopy Received
Florida Engineer or Architect Name who developed the Evaluation Report	John W. Knezevich
Florida License	PE-41961
Quality Assurance Entity	National Accreditation and Management Institute
Validated By	ORLANDO L. BLANCO, P.E.
Certificate of Independence	

Reference(Standard and Year of Standard)	Standard	Year
	ASTM E1886 and E1996	2002
	ASTM E330	2002

Equivalence of Product Standards
Certified By:

Sections from the Code:

Product Approval Method Method 2 Option B

Date Submitted 9/23/2005
Date Validated 9/23/2005
Date Pending FBC Approval 10/1/2005
Date Approved 10/11/2005

Summary of Products

FL #	Model, Number or Name	Description
2668.1	Bahama Shutter	Aluminum Bahama Shutter with operable blades
Limits of Use: Approved for use in HVHZ: Approved for use outside HVHZ: Impact Resistant: Design Pressure: + Other: Maximum allowable design pressure = +/- 30 PSF for 6'-11" span shutter, and maximum allowable design pressure = +/- 37.5 PSF for 8'-0" span shutter. The Bahama Shutter does not provide impact protection in the open position or in the closed position with open louver blades. To provide impact protection, the shutter must be in the closed position and the louver blades must be locked closed. Maximum Bahama Shutter projection = 8'-0". See reference drawing for allowable angles of operation for varying projections. Maximum Bahama Shutter louver blade span = 3'-2". This product is NOT suitable for installation in the High Velocity Hurricane Zone. This product may only be installed on concrete, hollow concrete block or wood substrates. For all other conditions site specific design shall be by this office or our delegated engineer.		Installation Instructions Verified By: Evaluation Reports PTID 2668_R1_T 05-353 DWGs_Bahama.pdf PTID 2668_R1_T 05-355 Report_Bahama.pdf

[Back](#) [Next](#)

DCA Administration
Department of Community Affairs
Florida Building Code Online
Codes and Standards
2333 Shumard Oak Boulevard
Tallahassee, Florida 32309-2100
1830 4th-1824, Suncom 777-1824, Fax 1830-474-8456
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Product Approval Accepts:





09/23/05 10:37

09/23/2005 10:37:32 AM

ANCHOR NOTE 1:

CONNECTION IN MIN. 3 KSI CONCRETE:
 1/4" DIA. TAPCONS @ 12" O.C. WITH 1-1/2" MIN. EMBED AND 2" MIN. EDGE DIST.
 1/4" DIA. EPO PANELMATE @ 12" O.C. WITH 1-1/2" MIN. EMBED AND 2" MIN. EDGE DIST.
 1/4" DIA. ALL POINTS SOLID SET ANCHOR @ 12" O.C. WITH 1-1/2" MIN. EMBED AND 2" MIN. EDGE DIST.
 CONNECTION IN HOLLOW CONCRETE BLOCK:
 1/4" DIA. TAPCONS @ 12" O.C. WITH 1-1/2" MIN. EMBED AND 2" MIN. EDGE DIST.
 1/4" DIA. EPO PANELMATE @ 12" O.C. WITH 1-1/2" MIN. EMBED AND 2" MIN. EDGE DIST.
 1/4" DIA. ALL POINTS SOLID SET ANCHOR @ 12" O.C. WITH 1-1/2" MIN. EMBED AND 2" MIN. EDGE DIST.
 CONNECTION IN WOOD:
 3/16" WOOD SCREW @ 12" O.C. WITH 1-1/2" MIN. EMBED AND 3/4" MIN. EDGE DIST.

ANCHOR NOTE 2:

CONNECTION IN MIN. 3 KSI CONCRETE:
 1/4" DIA. TAPCONS WITH 1-1/2" MIN. EMBED AND 2" MIN. EDGE DIST.
 1/4" DIA. EPO PANELMATE WITH 1-1/2" MIN. EMBED AND 2" MIN. EDGE DIST.
 1/4" DIA. ALL POINTS SOLID SET ANCHORS WITH 1-1/2" MIN. EMBED AND 2" MIN. EDGE DIST.
 CONNECTION IN HOLLOW CONCRETE BLOCK:
 1/4" DIA. TAPCONS WITH 1-1/2" MIN. EMBED AND 2" MIN. EDGE DIST.
 1/4" DIA. EPO PANELMATE WITH 1-1/2" MIN. EMBED AND 2" MIN. EDGE DIST.
 1/4" DIA. ALL POINTS SOLID SET ANCHORS WITH 1-1/2" MIN. EMBED AND 2" MIN. EDGE DIST.
 CONNECTION IN WOOD:
 3/16" WOOD SCREWS WITH 1-1/2" MIN. EMBED AND 3/4" MIN. EDGE DIST.

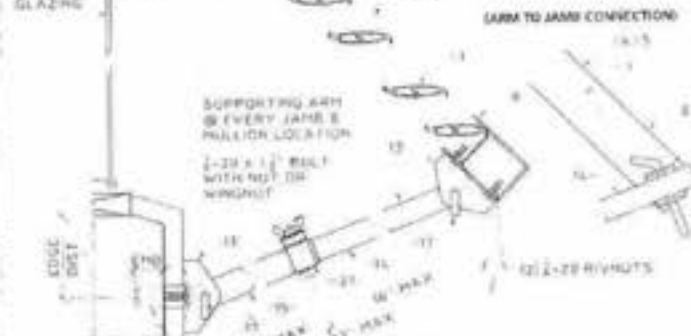
ANCHOR NOTE 3:

CONNECTION IN MIN. 3 KSI CONCRETE:
 1/4" DIA. TAPCONS WITH 1-1/2" MIN. EMBED AND 2-1/2" MIN. EDGE DIST.
 CONNECTION IN HOLLOW CONCRETE BLOCK:
 1/4" DIA. TAPCONS WITH 1-1/2" MIN. EMBED AND 2-1/2" MIN. EDGE DIST.
 1/4" DIA. EPO PANELMATE WITH 1-1/2" MIN. EMBED AND 2-1/2" MIN. EDGE DIST.
 1/4" DIA. ALL POINTS SOLID SET ANCHORS WITH 1-1/2" MIN. EMBED AND 2-1/2" MIN. EDGE DIST.
 CONNECTION IN WOOD:
 3/16" WOOD SCREWS WITH 1-1/2" MIN. EMBED AND 3/4" MIN. EDGE DIST.



OPENING ANGLE NOTE:

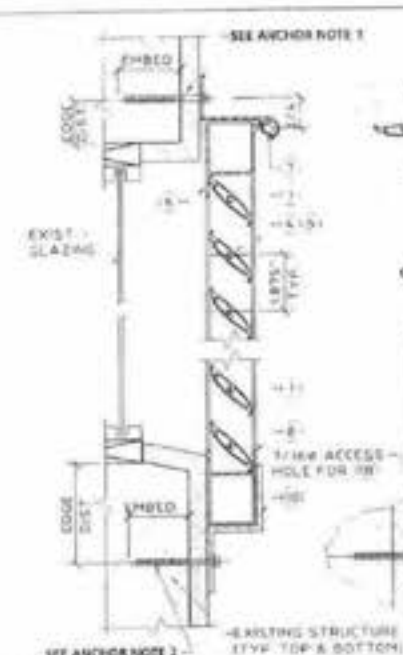
FOR PANEL LENGTHS UP TO 5' MAX., OPENING ANGLE IS 30°.
 FOR PANEL LENGTHS UP TO 12' MAX., OPENING ANGLE IS 25°.
 FOR PANEL LENGTHS GREATER THAN 12' UP TO 55' MAX., OPENING ANGLE IS 25°.



SEE ANCHOR NOTE 1
 EXISTING STRUCTURE (TYP. TOP & BOTTOM)

6 TYPICAL OPEN POSITION SECTION
 SCALE: 3/4" = 1'-0"

NOTE:
 SHUTTER MUST BE CLOSED DURING A HURRICANE WARNING. SHUTTER IS NOT DESIGNED TO REMAIN IN THE OPEN POSITION DURING A HURRICANE.



7 TYPICAL CLOSED POSITION SECTION
 SCALE: 3/4" = 1'-0"



8 TYPICAL BUILD-OUT CLOSED POSITION SECTION
 SCALE: 3/4" = 1'-0"



9 TYPICAL PLAN VIEW
 SCALE: HALF SIZE

THORNTON-TOMASETTI
 10000 N. W. 10th Ave., Suite 200, Fort Lauderdale, FL 33309
 (954) 571-1100
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BAHAMA SHUTTER
 Tom E. Country
 A Division of B&B Shutter Co., Inc.
 400 West Atlantic Road, Fort Lauderdale, FL 33309
 (954) 571-1100

FLORIDA BUILDING CODE, 2004
 05-355
 SHEET 3 OF 3

Date: 4/27/07



**Town of Sewall's Point
BUILDING PERMIT APPLICATION**

Permit Number: _____

OWNER/TITLEHOLDER NAME ROBERT CUSHING Phone (Day) 772-283-5016 (Fax) _____

Job Site Address: 102 ABILE CT. City: SEWALLS POINT State: FL Zip: 34996

Legal Desc. Property (Subd/Lot/Block) LOT 22 SEWALLS MEADOW (PDN # 52) Parcel Number: 13-38.41-013.000.002200

Owner Address (if different): _____ City: _____ State: _____ Zip: _____

Description of Work To Be Done: add accordion shutters to 20 openings, Bahama's for 3 openings

WILL OWNER BE THE CONTRACTOR?:

YES ☐ NO ☒

COST AND VALUES:

Estimated Cost of Construction or Improvements: \$ 18,665.00
(Notice of Commencement needed over \$2500)

Estimated Fair Market Value prior to improvement: \$ _____

Is improvement cost 50% or more of Fair Market Value? YES ☐ NO ☐

Method of Determining Fair Market Value: _____

(If no, fill out the Contractor & Subcontractor sections below)

(If yes, Owner Builder Affidavit must accompany application)

CONTRACTOR/Company: COASTAL SHUTTERS OF THE TC Phone: 772-335-9198 Fax: 772-335-7077

Street: 1386 SE HUFFMAN RD City: PORT ST LUCIE State: FL Zip: 34952

State Registration Number: _____ State Certification Number: _____ Martin County License Number: CSCL131149646

SUBCONTRACTOR INFORMATION:

Electrical: _____ State: _____ License Number: _____

Mechanical: _____ State: _____ License Number: _____

Plumbing: _____ State: _____ License Number: _____

Roofing: _____ State: _____ License Number: _____

ARCHITECT _____ Lic. #: _____ Phone Number: _____

Street: _____ City: _____ State: _____ Zip: _____

ENGINEER _____ Lic. #: _____ Phone Number: _____

Street: _____ City: _____ State: _____ Zip: _____

AREA SQUARE FOOTAGE - SEWER - ELECTRIC Living: _____ Garage: _____ Covered Patio: _____ Screened Porch: _____

Carport: _____ Total Under Roof _____ Wood Deck: _____ Accessory Building: _____

NOTICE: In addition to the requirements of this permit, there may be additional restrictions applicable to this property that may be found in the public records of this county, and there may be additional permits required from other governmental entities such as water management districts, state agencies, or federal agencies.

CODE EDITIONS IN EFFECT AT TIME OF APPLICATION: Florida Building Code (Structural, Mechanical, Plumbing, Gas): 2004
National Electrical Code: 2002 Florida Energy Code: 2004 Florida Accessibility Code: 2004 Florida Fire Code 2004

I HEREBY CERTIFY THAT THE INFORMATION I HAVE FURNISHED ON THIS APPLICATION IS TRUE AND CORRECT TO THE BEST OF MY KNOWLEDGE AND I AGREE TO COMPLY WITH ALL APPLICABLE CODES, LAWS AND ORDINANCES DURING THE BUILDING PROCESS.

OWNER OR AGENT SIGNATURE (required)

State of Florida, County of: Martin

This the 27th day of April, 2007

by Robert Cushing who is personally

known to me or produced

as Identification: C25E-762-42-140-0

My Commission Expires: _____

Notary Public: Alicia A. Seitz

Seal

CONTRACTOR SIGNATURE (required)

James C. Koorich

On State of Florida, County of: Martin

This the 27th day of April, 2007

by James C. Koorich who is personally

known to me or produced

as Identification: Alicia A. Seitz

My Commission Expires: 4/2/10

Notary Public: _____

Seal

PERMIT APPLICATIONS VALID 30 DAYS FROM APPROVAL NOTIFICATION - PLEASE PICK UP YOUR PERMIT PROMPTLY



Martin County, Florida **Laurel Kelly, C.F.A**

Site Provided by...
 governmax.com T1.14

Summary

print Owner 4 of 5

Parcel Info

Summary

Land

Residential

Improvement

Commercial

Image

Sales & Transfers

Assessments →

Taxes →

Parcel Map →

Full Legal →

Parcel ID 13-38-41-013-000-00220-0
Unit Address 102 ABBIE CT

Serial ID 119126
Index Order Owner
Commercial 0
Residential 1

Summary

Property Location 102 ABBIE CT
Tax District 2200 Sewall's Point
Account # 119126
Land Use 101 0100 Single Family
Neighborhood 120300
Acres 0.577

Legal Description

Property Information
 LOT 22 SEWALL'S MEADOW (PB 14 PG 32)

Did call -
 this is to
 be pulled
 in Sewall's
 point
 Jim

Search By

Parcel ID

Owner

Address

Account #

Use Code

Legal Description

Neighborhood

Sales

Map →

Owner Information

Owner Information
CUSHING, ROBERT B & PEARL C

Mail Information

102 ABBIE COURT
 STUART FL 34996

Assessment Info

Front Ft. 0.00

Market Land Value \$340,000

Market Impr Value \$529,540

Market Total Value \$869,540

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Sale Amount \$552,100

Sale Date 9/14/2001

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STATE OF FLORIDA

DEPARTMENT OF BUSINESS AND PROFESSIONAL REGULATION
CONSTRUCTION INDUSTRY LICENSING BOARD

SEQ# L06071100705

DATE	BATCH NUMBER	LICENSE NBR
07/11/2006	058087220	SCC131149646

The SPECIALTY STRUCTURE CONTRACTOR
Named below IS CERTIFIED
Under the provisions of Chapter 489 FS.
Expiration date: AUG 31, 2008

KOERICK, JAMES CHARLES JR
COASTAL SHUTTERS OF THE TREASURE COAST INC
1386 SE HUFFMAN ROAD
PORT SAINT LUCIE FL 34952

JEB BUSH
GOVERNOR

DISPLAY AS REQUIRED BY LAW

SIMONE MARSTILLER
SECRETARY

2006-2007

ST. LUCIE COUNTY OCCUPATIONAL LICENSE

BOB DAVIS, CPA, CGFO, CFC, ST. LUCIE COUNTY TAX COLLECTOR

ACCOUNT: 1799-18990033

EXPIRES SEP 30, 2007

FACILITIES
OR
MACHINES

ROOMS

SEATS

EMPLOYEES 1-10

TYPE OF BUSINESS 1799-SPECIALTY CONTRACTOR

BUSINESS LOCATION 1386 SE Hoffman Rd
City of Pt St Lucie

NAME James Charles Koerick Jr SCC131149646
MAILING Coastal Shutter of the TC Inc
ADDRESS Koerick, James C
1386 SE Hoffman Rd
Port St Lucie FL 34952

X RENEWAL
NEW LICENSE
TRANSFER-
ORIGINAL TAX 11.80

AMOUNT
PENALTY
COLLECTION COST
TOTAL 11.80

Please see back for additional information

P00000012111

PAID 08/15/2006 99-20060815-087307 11.80

TO BE COMPLETED WHEN CONSTRUCTION VALUE EXCEEDS \$2500.00

PERMIT # _____

TAX FOLIO # 13-38-41-013-000-00220-0**NOTICE OF COMMENCEMENT**STATE OF FLORIDACOUNTY OF SEWALL'S POINT

THE UNDERSIGNED HEREBY GIVES NOTICE THAT IMPROVEMENT WILL BE MADE TO CERTAIN REAL PROPERTY, AND IN ACCORDANCE WITH CHAPTER 713, FLORIDA STATUTES, THE FOLLOWING INFORMATION IS PROVIDED IN THIS NOTICE OF COMMENCEMENT.

LEGAL DESCRIPTION OF PROPERTY (INCLUDE STREET ADDRESS IF AVAILABLE):LOT 22 SEWALL'S MEADOW (PB 14 PG 32)**GENERAL DESCRIPTION OF IMPROVEMENT:** adding shutters TO 23 openings**OWNER:** ROBERT B CUSHING**ADDRESS:** 102 ABBIE CT. SEWALL'S POINT, FL. 34996**PHONE #:** 72-283-5016**FAX #:** _____**CONTRACTOR:** COASTAL SHUTTERS OF THE TREASURE COAST**ADDRESS:** 1386 SE HUFFMAN ROAD. PORT ST LUCIE, FL 34952**PHONE #:** 72-335-9198**FAX #:** 72-335-7077**SURETY COMPANY (IF ANY):** _____**ADDRESS:** _____**PHONE #:** _____**FAX #:** _____**BOND AMOUNT:** _____**LENDER:** _____**ADDRESS:** _____**PHONE #:** _____**FAX #:** _____STATE OF FLORIDA
HARTIN COUNTY

THIS IS TO CERTIFY THAT THE
FOREGOING _____ PAGES IS A TRUE
AND CORRECT COPY OF THE ORIGINAL
MARSHA EWING CLERK

BY [Signature] D.C.
DATE 5.17.07



PERSONS WITHIN THE STATE OF FLORIDA DESIGNATED BY OWNER UPON WHOM NOTICES OR OTHER DOCUMENTS MAY BE SERVED AS PROVIDED BY SECTION 713.13(1)(A)7, FLORIDA STATUTES:

NAME: _____**ADDRESS:** _____**PHONE #:** _____**FAX #:** _____

IN ADDITION TO HIMSELF, OWNER DESIGNATES _____

OF _____ TO RECEIVE A COPY OF THE LIENOR'S NOTICE AS PROVIDED IN SECTION 713.13(1)(B), FLORIDA STATUTES.

PHONE #: _____**FAX #:** _____**EXPIRATION DATE OF NOTICE OF COMMENCEMENT:** _____

THE EXPIRATION DATE IS ONE (1) YEAR FROM THE DATE OF RECORDING UNLESS A DIFFERENT DATE IS SPECIFIED ABOVE.

SIGNATURE OF OWNER

SWORN TO AND SUBSCRIBED BEFORE ME THIS 3rd May 2007 BY Robert Cushing

PERSONALLY KNOWN

PRODUCED ID

TYPE OF ID C2276242400

NOTARY SIGNATURE

/data/gnd/bnd/bldg_forms/Noc.aw

12/01/99

INSTR # 2013938 OR BK 02248 PG 1898 RECD 05/17/2007 12:30:39 PM
MARSHA EWING MARTIN COUNTY DEPUTY CLERK L. Wood



A/C #2 15 ✓ 45 ✓
A/C #1 30 ✓ 30 ✓

Date of Issuance

TOWN OF SEWALL'S POINT

One South Sewall's Point Road
Sewall's Point, Florida 34996
(772) 287-2455

CORRECTION NOTICE

ADDRESS: 102 ABBIE CT

I have this day inspected this structure and these premises and have found the following violations of the City, County, and/or State laws governing same.

SHUTTERS

PAPERWORK IS NOT USABLE -
DIRECT CONTRACTOR TO REPLACE
w/ NEW & PROTECT FROM
WEATHER & RESCHEDULE
INSPECTION

You are hereby notified that no work shall be concealed upon these premises until the above violations are corrected. When corrections have been made, call for an inspection.

DATE: 6/8

INSPECTOR

DO NOT REMOVE THIS TAG

COMMENTS:

OR:

COMMENTS:

USE

OR:

COMMENTS:

LOSE

OR:

COMMENTS:

OR:

COMMENTS:

OR:

COMMENTS:

OR:

COMMENTS:

INSPECTOR:

PERMIT

8613

PERMIT

8614

3

PERMIT

8611

2

PERMIT

8616

2

PERMIT

8608

4

PERMIT

7819

1

PERMIT

0027

5

OTHER:

3 TUSCAN
MASTERPIECE

INSULATION PASS

TOWN OF SEWALL'S POINT

Building Department - Inspection Log

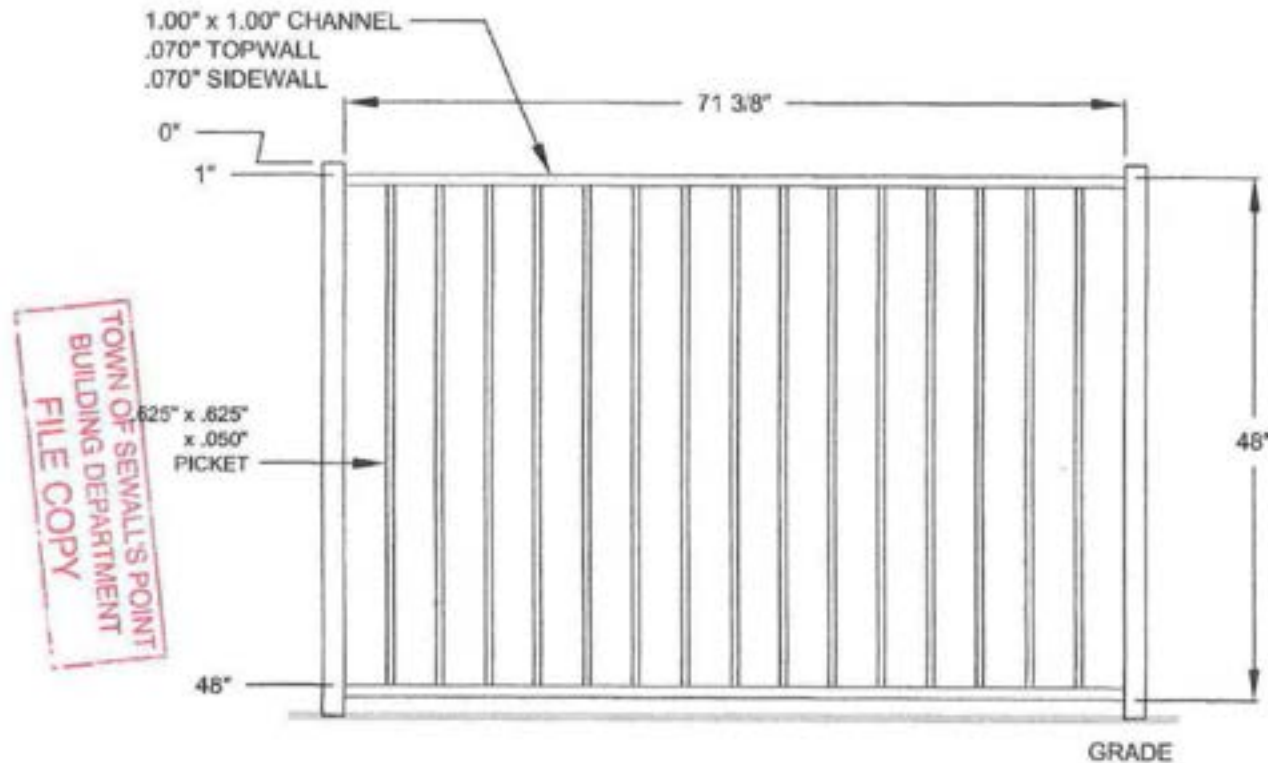
Date of Inspection: ☐ Mon ☐ Wed ☒ Fri 7-20, 2007 Page 1 of 1

PERMIT	OWNER/ADDRESS/CONTR.	INSPECTION TYPE	RESULTS	NOTES/COMMENTS:
3608	Cushing	Final	PASS	CLOSE
1	102 Abbie Ct Coastal Shutters			INSPECTOR: <i>[Signature]</i>
8647	Tompeck	Final	PASS	CLOSE
3	20 Emarieta Coastal Shutters			INSPECTOR: <i>[Signature]</i>
7682	Olsen	Final-dock	FAIL	
4	19 N River Rd O/B	repair		INSPECTOR: <i>[Signature]</i>
8401	Olsen	Final-secured	PASS	CLOSE
4	19 N River Rd 1001 (Olsen)			INSPECTOR: <i>[Signature]</i>
8638	McKinley	Final-	PASS	CLOSE
2	48 Rio Vista Monterey Glass			INSPECTOR: <i>[Signature]</i>
8528	Masterpiece Sys.	Dry in	PASS	
	S. mandalay Masterpiece Bldg.			INSPECTOR: <i>[Signature]</i>
PERMIT	OWNER/ADDRESS/CONTR.	INSPECTION TYPE	RESULTS	NOTES/COMMENTS:
				INSPECTOR:
OTHER:				

10178
FENCE

71.375" PANEL SERIES-A
2-CH 48" RESIDENTIAL FLUSH BTM

STEPHENS PIPE & STEEL (MEL)	DATE: ~~6-20-12~~ 7-31-12
PO/JOB NAME:	SWING: PANEL /white
QUOTE NO: Q10061102	HINGE TYPE: self closing
COLOR: -- white	DAYLIGHT OPENING:
APPROVAL SIGNATURE (REQUIRED):	



* Gate with childproof MagnaLatch.

**TOWN OF SEWALL'S POINT BUILDING DEPARTMENT**

One S. Sewall's Point Road
Sewall's Point, Florida 34996
Tel 772-287-2455 Fax 772-220-4765

BUILDING PERMIT CARD

**THIS CARD MUST BE POSTED IN A CONSPICUOUS PLACE IN PLAIN
VIEW FROM THE STREET PRIOR TO BEGINNING ANY WORK**

A FINAL INSPECTION IS REQUIRED FOR ALL PERMITS

PERMIT NUMBER:	10178	DATE ISSUED:	AUGUST 2, 2012
SCOPE OF WORK:	FENCE		
CONTRACTOR:	OB		
PARCEL CONTROL NUMBER:	133841013000-002200	SUBDIVISION	SEWALLS MEADOW - L22
CONSTRUCTION ADDRESS:	102 ABBIE CT		
OWNER NAME:	CUSHING		
QUALIFIER:	OB	CONTACT PHONE NUMBER:	283-5016

WARNING TO OWNER: YOUR FAILURE TO RECORD A NOTICE OF COMMENCEMENT MAY RESULT IN YOUR PAYING TWICE FOR IMPROVEMENTS TO YOUR PROPERTY. IF YOU INTEND TO OBTAIN FINANCING, CONSULT WITH YOUR LENDER OR AN ATTORNEY BEFORE RECORDING YOUR NOTICE OF COMMENCEMENT. A CERTIFIED COPY OF THE RECORDED NOTICE OF COMMENCEMENT MUST BE SUBMITTED TO THE BUILDING DEPARTMENT PRIOR TO THE FIRST REQUESTED INSPECTION.

NOTICE: IN ADDITION TO THE REQUIREMENTS OF THIS PERMIT, THERE MAY BE ADDITIONAL RESTRICTIONS APPLICABLE TO THIS PROPERTY THAT MAY BE FOUND IN PUBLIC RECORDS OF THIS COUNTY, AND THERE MAY BE ADDITIONAL PERMITS REQUIRED FROM OTHER GOVERNMENTAL ENTITIES SUCH AS WATER MANAGEMENT DISTRICTS, STATE AGENCIES, OR FEDERAL AGENCIES.

**24 HOUR NOTICE REQUIRED FOR INSPECTIONS - ALL CONSTRUCTION DOCUMENTS MUST BE AVAILABLE ON SITE
CALL 287-2455 - 8:00AM TO 4:00PM INSPECTIONS: 9:00AM TO 3:00PM - MONDAY THROUGH FRIDAY**

INSPECTIONS

UNDERGROUND PLUMBING _____
UNDERGROUND MECHANICAL _____
STEM-WALL FOOTING _____
SLAB _____
ROOF SHEATHING _____
TIE DOWN /TRUSS ENG _____
WINDOW/DOOR BUCKS _____
ROOF DRY-IN/METAL _____
PLUMBING ROUGH-IN _____
MECHANICAL ROUGH-IN _____
FRAMING _____
FINAL PLUMBING _____
FINAL MECHANICAL _____
FINAL ROOF _____

UNDERGROUND GAS _____
UNDERGROUND ELECTRICAL _____
FOOTING _____
TIE BEAM/COLUMNS _____
WALL SHEATHING _____
INSULATION _____
LATH _____
ROOF TILE IN-PROGRESS _____
ELECTRICAL ROUGH-IN _____
GAS ROUGH-IN _____
METER FINAL _____
FINAL ELECTRICAL _____
FINAL GAS _____
BUILDING FINAL _____

ALL RE-INSPECTION FEES AND ADDITIONAL INSPECTION REQUESTS WILL BE CHARGED TO THE PERMIT HOLDER. THE CONTRACTOR OR OWNER /BUILDER MUST SCHEDULE A FINAL INSPECTION. FAILURE TO RECEIVE A SUCCESSFUL FINAL INSPECTION WILL RESULT IN PERMIT RENEWAL FEES, FINES, AND OR DENIAL OF FUTURE BUILDING PERMITS TO THE CONTRACTOR OR OWNER /BUILDER.

Town of Sewall's Point

BUILDING PERMIT APPLICATION

Permit Number:

10178

Date: 7-31-12

OWNER/LESSEE NAME: Robert & Pearl Cushing

Phone (Day) 772-283-5016 (Fax)

Job Site Address: 102 Abbie Court

City: Stuart

State: FL

Zip: 34996

Legal Description: LOT 22 Sewall's Meadow

Parcel Control Number: 13-38-41-013-000-00220-0

Fee Simple Holder Name:

Address:

City:

State:

Zip:

Telephone:

*SCOPE OF WORK (PLEASE BE SPECIFIC):

Fence

WILL OWNER BE THE CONTRACTOR?

(If yes, Owner/Builder questionnaire must accompany application)

YES

NO

Has a Zoning Variance ever been granted on this property?

YES

(YEAR)

NO

(Must include a copy of all variance approvals with application)

COST AND VALUES: (Required on ALL permit applications)

Estimated Value of Improvements: \$ 1873.00

(Notice of Commencement required when over \$2500 prior to first inspection, \$7,500 on HVAC change out)

Is subject property located in flood hazard area? VE10 AE9 AE8 X

FOR ADDITIONS, REMODELS AND RE-ROOF APPLICATIONS ONLY:

Estimated Fair Market Value prior to improvement: \$

(Fair Market Value of the Primary Structure only, Minus the land value)

PRIVATE APPRAISALS MUST BE SUBMITTED WITH PERMIT APPLICATION

Construction Company:

Phone:

Fax:

Qualifiers name:

Street:

City:

State:

Zip:

State License Number:

OR: Municipality:

License Number:

LOCAL CONTACT:

Phone Number:

DESIGN PROFESSIONAL:

Fla. License#

Street:

City:

State:

Zip:

Phone Number:

AREAS SQUARE FOOTAGE: Living:

Garage:

Covered Patios/Porches:

Enclosed Storage:

Carport:

Total under Roof

Elevated Deck:

Enclosed area below BFE*

* Enclosed non-habitable areas below the Base Flood Elevation greater than 300 sq. ft. require a Non-Conversion Covenant Agreement.

CODE EDITIONS IN EFFECT THIS APPLICATION: Florida Building Code (Structural, Mechanical, Plumbing, Existing, Gas): 2010

National Electrical Code: 2008, Florida Energy Code: 2010, Florida Accessibility Code: 2010, Florida Fire Prevention Code: 2010

WARNINGS TO OWNERS AND CONTRACTORS:

1. YOUR FAILURE TO RECORD A NOTICE OF COMMENCEMENT MAY RESULT IN YOUR PAYING TWICE FOR IMPROVEMENTS TO YOUR PROPERTY. WHEN FINANCING, CONSULT WITH YOUR LENDER OR AN ATTORNEY BEFORE RECORDING YOUR NOTICE OF COMMENCEMENT. A NOTICE OF COMMENCEMENT MUST BE RECORDED AND POSTED ON THE JOB SITE BEFORE THE FIRST INSPECTION.
2. IT IS YOUR RESPONSIBILITY TO DETERMINE IF YOUR PROPERTY IS ENCUMBERED BY ANY DEED RESTRICTIONS. SOME RESTRICTIONS APPLICABLE TO THIS PROPERTY MAY BE FOUND IN THE PUBLIC RECORDS OF MARTIN COUNTY OR THE TOWN OF SEWALL'S POINT. THERE MAY BE ADDITIONAL PERMITS REQUIRED FROM OTHER GOVERNMENTAL ENTITIES SUCH AS WATER MANAGEMENT DISTRICTS, STATE AGENCIES, OR FEDERAL AGENCIES.
3. BUILDING PERMITS FOR SINGLE FAMILY RESIDENCES AND SUBSTANTIAL IMPROVEMENTS TO SINGLE FAMILY RESIDENCES ARE VALID FOR A PERIOD OF 24 MONTHS. RENEWAL FEES WILL BE ASSESSED AFTER 24 MONTHS PER TOWN ORDINANCE 50-95.
4. THIS PERMIT WILL BECOME NULL AND VOID IF THE WORK AUTHORIZED BY THIS PERMIT IS NOT COMMENCED WITHIN 180 DAYS, OR IF WORK IS SUSPENDED OR ABANDONED FOR A PERIOD OF 180 DAYS AT ANY TIME AFTER THE WORK IS COMMENCED. ADDITIONAL FEES WILL BE ASSESSED ON ANY PERMIT THAT BECOMES NULL AND VOID. REF. FBC 2007 SECT. 105.4.1, 105.4.1.1 - .5.

*****A FINAL INSPECTION IS REQUIRED ON ALL BUILDING PERMITS*****

AFFIDAVIT: APPLICATION IS HEREBY MADE TO OBTAIN A PERMIT TO DO THE WORK AS SPECIFICALLY INDICATED ABOVE. I CERTIFY THAT NO WORK OR INSTALLATION HAS COMMENCED PRIOR TO THE ISSUANCE OF A PERMIT AND THAT THE INFORMATION I HAVE FURNISHED ON THIS APPLICATION IS TRUE AND CORRECT TO THE BEST OF MY KNOWLEDGE. I AGREE TO COMPLY WITH ALL APPLICABLE CODES, LAWS, AND ORDINANCES OF THE TOWN OF SEWALL'S POINT DURING THE BUILDING PROCESS.

OWNER/AGENT/LESSEE - NOTARIZED SIGNATURE:

X

State of Florida, County of: Martin

On This the 1 day of Aug 2012

by Dr. Robert Cushing who is personally

known to me or produced

As identification:

My Commission Expires: Feb 11, 2014

CONTRACTOR/LICENSEE NOTARIZED SIGNATURE:

X

State of Florida, County of:

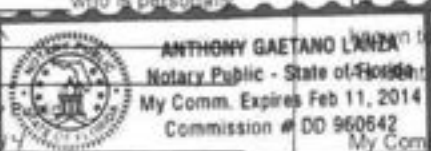
On This the day of 20

by who is personally

known to me or produced

As identification:

My Commission Expires:



Notary Public

SINGLE FAMILY PERMIT APPLICATIONS MUST BE ISSUED WITHIN 30 DAYS OF APPROVAL NOTIFICATION (FBC 105.3.4) ALL OTHER APPLICATIONS WILL BE CONSIDERED ABANDONED AFTER 180 DAYS (FBC 105.3.2) - PLEASE PICK UP YOUR PERMIT PROMPTLY!

**Martin County, Florida
Laurel Kelly, C.F.A***generated on 8/1/2012 10:52:26 AM EDT***Summary**

Parcel ID	Account #	Unit Address	Market Total Value	Website Updated
13-38-41-013-000-00220-0	119126	102 ABBIE COURT, SEWALL'S POINT	\$537,440	7/28/2012

Owner Information

Owner(Current)	CUSHING ROBERT B & PEARL C
Owner/Mail Address	102 ABBIE CT STUART FL 34996
Sale Date	9/14/2001
Document Book/Page	1582 0288
Document No.	JMB
Sale Price	552100

Location/Description

Account #	119126	Map Page No.	SP-05
Tax District	2200	Legal Description	LOT 22 SEWALL'S MEADOW (PB 14 PG 32)
Parcel Address	102 ABBIE COURT, SEWALL'S POINT		
Acres	.5770		

Parcel Type

Use Code	0100 Single Family
Neighborhood	120300 Sewall's Meadow

Assessment Information

Market Land Value	\$242,000
Market Improvement Value	\$295,440
Market Total Value	\$537,440



TOWN OF SEWALL'S POINT BUILDING DEPARTMENT

One S. Sewall's Point Road
Sewall's Point, Florida 34996
Tel 772-287-2455 Fax 772-220-4765

OWNER/BUILDER QUESTIONNAIRE AND DISCLOSURE STATEMENT
MUST BE COMPLETED AND REVIEWED PRIOR TO PERMIT ISSUANCE

APPLICABLE ONLY TO OWNER-OCCUPIED SINGLE FAMILY RESIDENCES
AND COMMERCIAL IMPROVEMENTS LESS THAN \$75,000 IN VALUE

NOTICE: FLORIDA STATUTE 489 REQUIRING CONSTRUCTION TO BE DONE ONLY BY LICENSED CONTRACTORS PROVIDES AN EXEMPTION FROM LICENSING FOR A PROPERTY OWNER WHO ACTS AS HIS/HER OWN CONTRACTOR UNDER SPECIFIC CONDITIONS. ANSWERS TO THE FOLLOWING QUESTIONS ARE ESSENTIAL TO DETERMINE IF THOSE STATE QUALIFICATIONS ARE SATISFIED BY AN OWNER/BUILDER APPLICANT.

ALL QUESTIONS MUST BE ANSWERED. IF A QUESTION DOES NOT APPLY, INDICATE BY WRITING "N/A"

Owner/Builder Applicant Name: Dr. Robert B. Cushing

Site address of the proposed building work: 102 Abbie Ct. Sewall's Point, FL 34996

Name of legal title owner of the address above: Dr. Robert B. Cushing

Describe the scope of work for the proposed new construction: Fence around Pool -

Name of Architect of Record: NA Structural Engineer of Record: NA

Who will supervise the trade work to meet the applicable code? Owner

What provisions have you made for Liability and Property Damage Insurance? Homeowners Ins

What provisions exist for withholding Social Security and Federal Income Taxes, as required by Federal Law, from wages paid to people you hire who are not licensed? Self

What previous Owner/Builder improvements have you done in the State of Florida?

Location: None Scope of Work Done: _____ Year: _____

Location: _____ Scope of Work Done: _____ Year: _____

What code books do you have available for reference? Building: NA

Electric: _____ Plumbing: _____ HVAC: _____

Other: _____

I have internet access and will view The Florida Building code at www.floridabuilding.org YES ☒ NO ☐

Do you understand that as the permit holder you are liable for following all Local, County, State and Federal codes, laws and requirements, and you are also liable for anyone injured on the construction site? Yes (yes/no)

Have you consulted with your Homeowner's Insurance Agent? NA Lender? _____ Attorney? _____

In order to assure your success in this project, please signify your awareness that the function of the building department is to issue you a building permit and verify code compliance through plan review and the inspection process. I am aware that town staff is not obligated to offer supervision, design or instructional advice prior or during my project. NA (Initials).



TOWN OF SEWALL'S POINT BUILDING DEPARTMENT

One S. Sewall's Point Road
Sewall's Point, Florida 34996
Tel 772-287-2455 Fax 772-220-4765

OWNER/BUILDER DISCLOSURE STATEMENT

NOTICE: STATE LAW REQUIRES THAT ALL PERMITTING AGENCIES PROVIDE INDIVIDUALS SUBMITTING APPLICATIONS FOR OWNER/BUILDER PERMITS THE FOLLOWING INFORMATION:

1. THE TOWN OF SEWALL'S POINT CODE OF LAWS AND ORDINANCES REQUIRES THAT ANY PERSON DESIRING TO ENGAGE IN BUSINESS AS A CONTRACTOR IN THE TOWN OF SEWALL'S POINT BE A HOLDER OF A CERTIFICATE OF COMPETENCY.
2. FLORIDA STATUTES 489.103 (7) ALLOWS YOU, AS A PROPERTY OWNER, AN EXEMPTION TO CONSTRUCT OR IMPROVE A SINGLE FAMILY DWELLING AND ACCESSORY-USE STRUCTURES ON SAID PROPERTY FOR YOUR OWN USE OR OCCUPANCY WITHOUT HAVING A CERTIFICATE OF COMPETENCY.
3. AS AN OWNER/BUILDER, YOU MUST PHYSICALLY PERFORM OR MATERIALLY SUPERVISE ALL CONSTRUCTION/IMPROVEMENTS SPECIFIED ON YOUR OWNER/BUILDER PERMIT AND YOU ARE TOTALLY RESPONSIBLE FOR ALL ACTIVITIES ASSOCIATED THEREWITH. OWNER/BUILDERS WHO WISH TO DO ELECTRICAL OR PLUMBING WORK MUST PASS A SHORT OPEN BOOK QUIZ ADMINISTERED BY THE BUILDING DEPARTMENT.
4. IF YOU DO NOT PHYSICALLY PERFORM A SPECIFIC PHASE OF SAID CONSTRUCTION/IMPROVEMENT, BUT CHOOSE TO SUB-CONTRACT IT, THEN SUCH CONSTRUCTION/IMPROVEMENT MUST BE SUB-CONTRACTED TO A LOCALLY LICENSED OR STATE CERTIFIED CONTRACTOR.
5. YOU MAY NOT HIRE AN UNLICENSED INDIVIDUAL WHO IN ANY MANNER ACTS IN THE CAPACITY OF A GENERAL CONTRACTOR, THAT IS, ONE WHO FULFILLS YOUR DUTIES UNDER PARAGRAPH (3) ABOVE.
6. UNDER AN OWNER/BUILDER PERMIT YOU MAY NOT CONSTRUCT A BUILDING WHICH YOU INTEND TO SELL OR LEASE. THE SALE OR LEASE, OR OFFERING FOR SALE OR LEASE, OF ANY SUCH STRUCTURE BY THE OWNER-BUILDER WITHIN 1 YEAR AFTER COMPLETION OF SAME CREATES A PRESUMPTION THAT THE CONSTRUCTION WAS UNDERTAKEN FOR PURPOSES OF SALE OR LEASE WHICH IS A VIOLATION OF THIS EXEMPTION.
7. THIS EXEMPTION SHALL NOT APPLY TO ANY PERSON WHO HAS LEASED, SOLD, OR OFFERED FOR SALE MORE THAN 1 STRUCTURE BUILT UNDER AN OWNER-BUILDER PERMIT IN ANY JURISDICTION WITHIN THE 5 YEARS IMMEDIATELY PRECEDING THE APPLICATION FOR A PERMIT.
8. THERE MUST BE A THIRTY-SIX (36) MONTH PERIOD BETWEEN THE ISSUANCE OF A CERTIFICATE OF OCCUPANCY FOR THE INITIAL DWELLING AND THE SUBMITTAL OF AN OWNER/BUILDER APPLICATION FOR A SUBSEQUENT DWELLING. NO OTHER BUILDING PERMIT FOR A HOME SHALL BE ISSUED TO THAT OWNER/BUILDER, THE OWNER/BUILDER'S SPOUSE OR ANY MEMBER OF THE OWNER/BUILDER'S IMMEDIATE FAMILY UNDER EIGHTEEN YEARS OF AGE UNTIL THREE YEARS AFTER THE HOME BUILT UNDER THE FIRST BUILDING PERMIT HAS RECEIVED A CERTIFICATE OF OCCUPANCY.
9. ALL CONSTRUCTION IMPROVEMENTS PERFORMED UNDER YOUR OWNER/BUILDER PERMIT MUST BE IN STRICT COMPLIANCE WITH FLORIDA STATE STATUTE SECTION 489, SEWALL'S POINTS CODE OF LAWS AND ORDINANCES, ALL BUILDING & ZONING CODES AND REGULATIONS PERTAINING TO SINGLE FAMILY DWELLINGS, ACCESSORY USE STRUCTURES AS APPLICABLE.
10. YOUR OWNER/BUILDER EXEMPTION DOES NOT WAIVE ANY PERMIT CONDITIONS OR REQUIREMENTS OR WAIVE ANY PORTION OR PORTIONS OF ANY APPLICABLE BUILDING OR SWIMMING POOL CODES OR TOWN ORDINANCES.
11. ALL CONSTRUCTION/IMPROVEMENTS PERFORMED UNDER YOUR OWNER/BUILDING PERMIT MUST ALSO BE IN STRICT COMPLIANCE WITH ALL APPLICABLE ZONING REGULATIONS (QUESTIONS REGARDING ZONING REGULATIONS SHOULD BE DIRECTED TO THE TOWN OF SEWALL'S POINT AT 772-287-2455.)



TOWN OF SEWALL'S POINT BUILDING DEPARTMENT

One S. Sewall's Point Road
Sewall's Point, Florida 34996
Tel 772-287-2455 Fax 772-220-4765

12. YOU MUST BE FAMILIAR WITH AND CALL FOR THE REQUIRED INSPECTIONS OF ALL CONSTRUCTION/IMPROVEMENTS PERFORMED UNDER YOUR OWNER/BUILDER PERMIT.

13. AS AN OWNER/BUILDER YOU MUST VERIFY THAT ALL INDIVIDUALS OR FIRMS ENGAGED IN CONSTRUCTION/IMPROVEMENTS ACTIVITIES UNDER YOUR OWNER/BUILDER PERMIT ARE PROPERLY LICENSED AS REQUIRED BY STATE LAW OR LOCAL ORDINANCE.

14. ANY PERSON WORKING ON YOUR BUILDING WHO IS NOT LICENSED MUST WORK UNDER YOUR DIRECT SUPERVISION AND MUST BE EMPLOYED BY YOU, WHICH MEANS THAT YOU MUST DEDUCT F.I.C.A. AND WITHHOLDING TAX AND PROVIDE WORKERS' COMPENSATION FOR THAT EMPLOYEE, ALL AS PRESCRIBED BY LAW.

15 I, AS AN OWNER/BUILDER, IN CONSIDERATION OF A BUILDING PERMIT ISSUED BY SEWALL'S POINT, FLORIDA, AGREE TO INDEMNIFY AND HOLD HARMLESS SAID SEWALL'S POINT, FLORIDA, ITS OFFICERS AND AGENTS FROM ANY AND ALL CLAIMS, DAMAGES, OR EXPENSES THAT SEWALL'S POINT MAY BE LIABLE FOR WHICH ARISE FROM THE CONSTRUCTION/IMPROVEMENTS ACCOMPLISHED IN CONNECTION WITH SAID BUILDING PERMIT.

I HEREBY ACKNOWLEDGE THAT I HAVE THOROUGHLY READ AND COMPLETELY UNDERSTAND THE PRECEDING PAGE OF THE OWNER/BUILDER DISCLOSURE STATEMENT.

ON THIS 1 DAY OF August 2014
PROPERTY ADDRESS 102 Abbie Ct. Sewalls Pt FL 34996
CITY Seaside STATE FL ZIP 32184

SIGNATURE OF OWNER/BUILDER

SWORN TO AND SUBSCRIBED BEFORE ME THIS 1 DAY OF Aug 2012

BY Robert Cushing

PERSONALLY KNOWN _____

OR PRODUCED ID ☒

TYPE/OF ID FLDL#C252-762 10-0

NOTARY SIGNATURE



R4101.17.1 Outdoor swimming pools. Outdoor swimming pools shall be provided with a barrier complying with R4101.17.1.1 through R4101.17.1.14.

R4101.17.1.1 The top of the barrier shall be at least 48 inches (1219 mm) above grade measured on the side of the barrier which faces away from the swimming pool. The maximum vertical clearance between grade and the bottom of the barrier shall be 2 inches (51 mm) measured on the side of the barrier which faces away from the swimming pool. Where the top of the pool structure is above grade the barrier may be at ground level or mounted on top of the pool structure. Where the barrier is mounted on top of the pool structure, the maximum vertical clearance between the top of the pool structure and the bottom of the barrier shall be 4 inches (102 mm).

R4101.17.1.2 The barrier may not have any gaps, openings, indentations, protrusions, or structural components that could allow a young child to crawl under, squeeze through, or climb over the barrier as herein described below. One end of a removable child barrier shall not be removable without the aid of tools. Openings in any barrier shall not allow passage of a 4-inch-diameter (102 mm) sphere.

R4101.17.1.3 Solid barriers which do not have openings shall not contain indentations or protrusions except for normal construction tolerances and tooled masonry joints.

R4101.17.1.4 Where the barrier is composed of horizontal and vertical members and the distance between the tops of the horizontal members is less than 45 inches (1143 mm), the horizontal members shall be located on the swimming pool side of the fence. Spacing between vertical members shall not exceed 13/4 inches (44 mm) in width. Where there are decorative cutouts within vertical members, spacing within the cutouts shall not exceed 13/4 inches (44 mm) in width.

R4101.17.1.5 Where the barrier is composed of horizontal and vertical members and the distance between the tops of the horizontal members is 45 inches (1143 mm) or more, spacing between vertical members shall not exceed 4 inches (102 mm). Where there are decorative cutouts within vertical members, spacing within the cutouts shall not exceed 13/4 inches (44 mm) in width.

R4101.17.1.6 Maximum mesh size for chain link fences shall be a 2 1/4 inch square (57 mm) unless the fence is provided with slats fastened at the top or bottom which reduce the openings to no more than 1 3/4 inches (44 mm).

R4101.17.1.7 Where the barrier is composed of diagonal members, the maximum opening formed by the diagonal members shall be no more than 1 3/4 inches (44 mm).

R4101.17.1.8 Access gates, when provided, shall be self-closing and shall comply with the requirements of Sections R4101.17.1.1 through R4101.17.1.7 and shall be equipped with a self-latching locking device located on the pool side of the gate. Where the device release is located no less than 54 inches (1372 mm) from the bottom of the gate, the device release mechanism may be located on either side of the gate and so placed that it cannot be reached by a young child over the top or through any opening or gap from the outside. Gates that provide access to the swimming pool must open outward away from the pool. The gates and barrier shall have no opening greater than 1/2 inch (12.7 mm) within 18 inches (457 mm) of the release mechanism.



TOWN OF SEWALL'S POINT BUILDING DEPARTMENT
One S. Sewall's Point Road
Sewall's Point, Florida 34996
Tel 772-287-2455 Fax 772-2204765

FENCE and or POOL BARRIER CHECKLIST

A document review will be performed on the following items prior to the submittal of a permit application. Failure to submit these items will result in the application package returned to the applicant until the deficient documents are included. This review sheet must accompany the application submittal.

Please make sure you have ALL required copies before submitting permit application

1 Copy Completed permit application

2 Copies Survey or site plan showing the following:

- All existing structures on property
- Location of proposed fence
- Setbacks from the fence to property lines
- Height & type of fence
- Location of all easements
- Street & house number on site plans

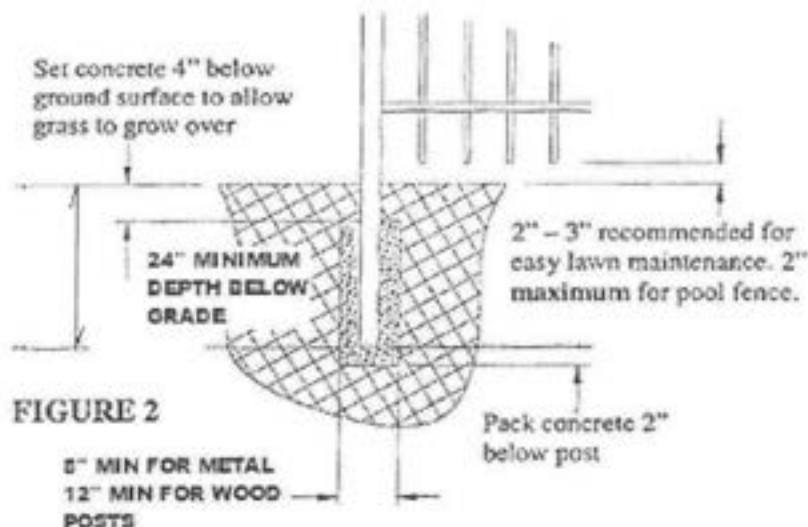
4' ALUMINUM -

DO NOT SUBMIT PREVIOUSLY STAMPED SITE PLANS

2 Copies support post footer sketch indicating size of footers. Fences to be used as a Pool Barrier (other than chain link fence) must include an accurate sketch or drawing indicating barrier requirement compliance.

2 Copies, if fence crosses any easement, Easement agreement from all utility Companies are required. Agreement form included in permit package.

Typical Fence Footer



TOWN OF SEWALLS POINT

BUILDING DEPARTMENT - INSPECTION LOG

Date of Inspection ☐ Mon ☐ Tue ☐ Wed ☒ Thur ☐ Fri 8-9-12 Page 1 of 1

PERMIT #	OWNER/ADDRESS/CONTRACTOR	INSPECTION TYPE	RESULTS	COMMENTS
9904	Howley			
	14 CRANES NEST	FINAL REMODEL	PASS	CLOSE
	O/B			INSPECTOR
PERMIT #	OWNER/ADDRESS/CONTRACTOR	INSPECTION TYPE	RESULTS	COMMENTS
10148	BOTWITNK			
	27 EMALITA WAY	ROOF FINAL	PASS	CLOSE
	CARDINAL			INSPECTOR <i>AF</i>
PERMIT #	OWNER/ADDRESS/CONTRACTOR	INSPECTION TYPE	RESULTS	COMMENTS
10131	ANTONELLI			
	10 ISLAND RD	WINDOW	PASS	CLOSE
	HAMILTON	FINAL		INSPECTOR <i>AF</i>
PERMIT #	OWNER/ADDRESS/CONTRACTOR	INSPECTION TYPE	RESULTS	COMMENTS
10178	CUSHING			
	102 ADDIE CT	FENCE FINAL	PASS	CLOSE
	O/B			INSPECTOR <i>AF</i>
PERMIT #	OWNER/ADDRESS/CONTRACTOR	INSPECTION TYPE	RESULTS	COMMENTS
		PROTECTOR TREE		NEED
	11 PRM		NG	ARTIST OPINION
				INSPECTOR
PERMIT #	OWNER/ADDRESS/CONTRACTOR	INSPECTION TYPE	RESULTS	COMMENTS
				INSPECTOR
PERMIT #	OWNER/ADDRESS/CONTRACTOR	INSPECTION TYPE	RESULTS	COMMENTS
				INSPECTOR

10604

A/C CHANGE OUT

Town of Sewall's Point
BUILDING PERMIT APPLICATION

Date: 09-19-2013

Permit Number: 10604

OWNER/LESSEE NAME: Robert & Pearl Rushing Phone (Day) 283-5016 (Fax) _____
Job Site Address: 102 Abbie Court City: Stuart State: FL Zip: 34996
Legal Description: 13-38-41-017-000-0000-0 Parcel Control Number: Sewall's Point
Fee Simple Holder Name: James Address: _____
City: _____ State: _____ Zip: _____ Telephone: _____

***SCOPE OF WORK (PLEASE BE SPECIFIC):** Replace HVAC system

WILL OWNER BE THE CONTRACTOR?

(If yes, Owner Builder questionnaire must accompany application)

YES _____ NO X

Has a Zoning Variance ever been granted on this property?

YES _____ (YEAR) _____ NO _____

(Must include a copy of all variance approvals with application)

COST AND VALUES: (Required on ALL permit applications)

Estimated Value of Improvements: \$ 6460.00

(Notice of Commencement required when over \$2500 prior to first inspection, \$7,500 on HVAC change out)

Is subject property located in flood hazard area? VE10 AE8 AE8 X

FOR ADDITIONS, REMODELS AND RE-ROOF APPLICATIONS ONLY:

Estimated Fair Market Value prior to improvement: \$ _____

(Fair Market Value of the Primary Structure only, Minus the land value)

PRIVATE APPRAISALS MUST BE SUBMITTED WITH PERMIT APPLICATION

Construction Company: Stephen K. Denny, Inc Phone: 743-9554 Fax: 746-2581

Qualifiers name: Steve Sanders Street: 406 Commerce Way City: Jupiter State: FL Zip: 33458

State License Number: PAC 1813800 OR: Municipality: _____ License Number: _____

LOCAL CONTACT: Deborah Davis-Acker Phone Number: 561-743-9554

DESIGN PROFESSIONAL: NA Fla. License# _____

Street: _____ City: _____ State: _____ Zip: _____ Phone Number: _____

AREAS SQUARE FOOTAGE: Living: 3280 Garage: _____ Covered Patios/ Porches: _____ Enclosed Storage: _____

Carport: _____ Total under Roof _____ Elevated Deck: SEP 20 2013 Enclosed area below BFE*: _____

* Enclosed non-habitable areas below the Base Flood Elevation greater than 300 sq. ft. require a Non-Conversion Covenant Agreement.

CODE EDITIONS IN EFFECT THIS APPLICATION: Florida Building Code (Structural, Mechanical, Plumbing, Existing, Gas): 2010
National Electrical Code: 2008, Florida Energy Code: 2010, Florida Accessibility Code: 2010, Florida Fire Prevention Code: 2010

WARNINGS TO OWNERS AND CONTRACTORS:

1. YOUR FAILURE TO RECORD A NOTICE OF COMMENCEMENT MAY RESULT IN YOUR PAYING TWICE FOR IMPROVEMENTS TO YOUR PROPERTY. WHEN FINANCING, CONSULT WITH YOUR LENDER OR AN ATTORNEY BEFORE RECORDING YOUR NOTICE OF COMMENCEMENT. A NOTICE OF COMMENCEMENT MUST BE RECORDED AND POSTED ON THE JOB SITE BEFORE THE FIRST INSPECTION.
2. IT IS YOUR RESPONSIBILITY TO DETERMINE IF YOUR PROPERTY IS ENCUMBERED BY ANY DEED RESTRICTIONS. SOME RESTRICTIONS APPLICABLE TO THIS PROPERTY MAY BE FOUND IN THE PUBLIC RECORDS OF MARTIN COUNTY OR THE TOWN OF SEWALL'S POINT. THERE MAY BE ADDITIONAL PERMITS REQUIRED FROM OTHER GOVERNMENTAL ENTITIES SUCH AS WATER MANAGEMENT DISTRICTS, STATE AGENCIES, OR FEDERAL AGENCIES.
3. BUILDING PERMITS FOR SINGLE FAMILY RESIDENCES AND SUBSTANTIAL IMPROVEMENTS TO SINGLE FAMILY RESIDENCES ARE VALID FOR A PERIOD OF 24 MONTHS. RENEWAL FEES WILL BE ASSESSED AFTER 24 MONTHS PER TOWN ORDINANCE 50-95.
4. THIS PERMIT WILL BECOME NULL AND VOID IF THE WORK AUTHORIZED BY THIS PERMIT IS NOT COMMENCED WITHIN 180 DAYS, OR IF WORK IS SUSPENDED OR ABANDONED FOR A PERIOD OF 180 DAYS AT ANY TIME AFTER THE WORK IS COMMENCED. ADDITIONAL FEES WILL BE ASSESSED ON ANY PERMIT THAT BECOMES NULL AND VOID. REF. FBC 2007 SECT. 105.4.1, 105.4.1.1 - .5.

******* A FINAL INSPECTION IS REQUIRED ON ALL BUILDING PERMITS *******

AFFIDAVIT: APPLICATION IS HEREBY MADE TO OBTAIN A PERMIT TO DO THE WORK AS SPECIFICALLY INDICATED ABOVE. I CERTIFY THAT NO WORK OR INSTALLATION HAS COMMENCED PRIOR TO THE ISSUANCE OF A PERMIT AND THAT THE INFORMATION I HAVE FURNISHED ON THIS APPLICATION IS TRUE AND CORRECT TO THE BEST OF MY KNOWLEDGE. I AGREE TO COMPLY WITH ALL APPLICABLE CODES, LAWS, AND ORDINANCES OF THE TOWN OF SEWALL'S POINT DURING THE BUILDING PROCESS.

OWNER /AGENT/LESSEE - NOTARIZED SIGNATURE:

X [Signature]
State of Florida, County of: Florida
On This the 19 day of September, 2013
by Robert Rushing who is personally
known to me or produced License
As identification: [Signature]

Notary Public

My Commission Expires: _____

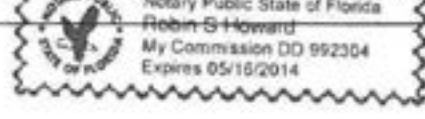
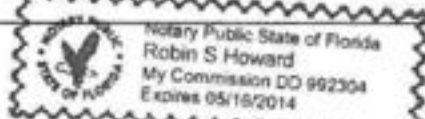
CONTRACTOR/LICENSEE NOTARIZED SIGNATURE:

X [Signature]
State of Florida, County of: _____
On This the 19 day of Sept, 2013
by Steve Sanders who is personally
known to me or produced _____
As identification: [Signature]

Notary Public

My Commission Expires: _____

SINGLE FAMILY PERMIT APPLICATIONS MUST BE ISSUED WITHIN 30 DAYS OF APPROVAL. NOTIFICATION (FBC 105.3.4) ALL OTHER APPLICATIONS WILL BE CONSIDERED ABANDONED AFTER 180 DAYS (FBC 105.3.2) - PLEASE PICK UP YOUR PERMIT PROMPTLY!





Martin County, Florida

Laurel Kelly, C.F.A

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Summary

One of 3 of

Tabs

Summary

[Print View](#)
[Land](#)
[Improvements](#)
[Assessments & Exemptions](#)
[Sales](#)
[Taxes →](#)
[NEW: Navigator](#)
[Parcel Map →](#)
[Notice of Prop.](#)
[Taxes →](#)

Searches

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Functions

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[County Login](#)

Parcel ID	Account #	Unit Address	Market Total Value	Webs Update
13-38-41-013-000-00220-0	119126	102 ABBIE COURT, SEWALL'S POINT	\$608,670	9/14/2

Owner Information	
Owner(Current)	CUSHING ROBERT B & PEARL C
Owner/Mail Address	102 ABBIE CT STUART FL 34996
Sale Date	9/14/2001
Document Book/Page	1582 0288
Document No.	JMB
Sale Price	552100

Account #	Location/Description	Map Page No.	SP-01
119126			
Tax District	2200	Legal Description	LOT 22 SEWALL'S MEADOW (PB 1 32)
Parcel Address	102 ABBIE COURT, SEWALL'S POINT		
Acres	.5770		

Parcel Type	
Use Code	0100 Single Family
Neighborhood	120300 Sewall's Meadow

Assessment Information	
Market Land Value	\$247,500
Market Improvement Value	\$361,170
Market Total Value	\$608,670

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TOWN OF SEWALL'S POINT BUILDING DEPARTMENT
One S. Sewall's Point Road
Sewall's Point, Florida 34996
Tel 772-287-2455 Fax 772-220-4765

BUILDING PERMIT CARD

THIS CARD MUST BE POSTED IN A CONSPICUOUS PLACE IN PLAIN VIEW FROM THE STREET PRIOR TO BEGINNING ANY WORK

A FINAL INSPECTION IS REQUIRED FOR ALL PERMITS

PERMIT NUMBER:	10604	DATE ISSUED:	SEPTEMBER 24, 2013
SCOPE OF WORK:	AC CHANGEOUT		
CONTRACTOR:	STEPHEN K DENNY INC		
PARCEL CONTROL NUMBER:	133841013-000-002200	SUBDIVISION	SEWALLS MEADOW - L 22
CONSTRUCTION ADDRESS:	102 ABBIE CT		
OWNER NAME:	CUSHING		
QUALIFIER:	STEVE SAUNDERS	CONTACT PHONE NUMBER:	561-743-9554

WARNING TO OWNER: YOUR FAILURE TO RECORD A NOTICE OF COMMENCEMENT MAY RESULT IN YOUR PAYING TWICE FOR IMPROVEMENTS TO YOUR PROPERTY. IF YOU INTEND TO OBTAIN FINANCING, CONSULT WITH YOUR LENDER OR AN ATTORNEY BEFORE RECORDING YOUR NOTICE OF COMMENCEMENT. A CERTIFIED COPY OF THE RECORDED NOTICE OF COMMENCEMENT MUST BE SUBMITTED TO THE BUILDING DEPARTMENT PRIOR TO THE FIRST REQUESTED INSPECTION.

NOTICE: IN ADDITION TO THE REQUIREMENTS OF THIS PERMIT, THERE MAY BE ADDITIONAL RESTRICTIONS APPLICABLE TO THIS PROPERTY THAT MAY BE FOUND IN PUBLIC RECORDS OF THIS COUNTY, AND THERE MAY BE ADDITIONAL PERMITS REQUIRED FROM OTHER GOVERNMENTAL ENTITIES SUCH AS WATER MANAGEMENT DISTRICTS, STATE AGENCIES, OR FEDERAL AGENCIES.

24 HOUR NOTICE REQUIRED FOR INSPECTIONS - ALL CONSTRUCTION DOCUMENTS MUST BE AVAILABLE ON SITE
CALL 287-2455 - 8:00AM TO 4:00PM INSPECTIONS: 9:00AM TO 3:00PM - MONDAY THROUGH FRIDAY

INSPECTIONS

UNDERGROUND PLUMBING	_____	UNDERGROUND GAS	_____
UNDERGROUND MECHANICAL	_____	UNDERGROUND ELECTRICAL	_____
STEM-WALL FOOTING	_____	FOOTING	_____
SLAB	_____	TIE BEAM/COLUMNS	_____
ROOF SHEATHING	_____	WALL SHEATHING	_____
TIE DOWN /TRUSS ENG	_____	INSULATION	_____
WINDOW/DOOR BUCKS	_____	LATH	_____
ROOF DRY-IN/METAL	_____	ROOF TILE IN-PROGRESS	_____
PLUMBING ROUGH-IN	_____	ELECTRICAL ROUGH-IN	_____
MECHANICAL ROUGH-IN	_____	GAS ROUGH-IN	_____
FRAMING	_____	METER FINAL	_____
FINAL PLUMBING	_____	FINAL ELECTRICAL	_____
FINAL MECHANICAL	_____	FINAL GAS	_____
FINAL ROOF	_____	BUILDING FINAL	_____

ALL RE-INSPECTION FEES AND ADDITIONAL INSPECTION REQUESTS WILL BE CHARGED TO THE PERMIT HOLDER. THE CONTRACTOR OR OWNER /BUILDER MUST SCHEDULE A FINAL INSPECTION. FAILURE TO RECEIVE A SUCCESSFUL FINAL INSPECTION WILL RESULT IN PERMIT RENEWAL FEES, FINES, AND OR DENIAL OF FUTURE BUILDING PERMITS TO THE CONTRACTOR OR OWNER /BUILDER.

TOWN OF SEWALLS POINT

BUILDING DEPARTMENT - INSPECTION LOG

Date of Inspection ☒ Mon ☐ Tue ☐ Wed ☐ Thur ☐ Fri 1-13-14 Page 1 of 1

PERMIT #	OWNER/ADDRESS/CONTRACTOR	INSPECTION TYPE	RESULTS	COMMENTS
0604	CUSIANG	FINAL A/C		
	102 ABBIE CT		PASS	CLOSE
	S. K Denny A/C			INSPECTOR <i>JD</i>
PERMIT #	OWNER/ADDRESS/CONTRACTOR	INSPECTION TYPE	RESULTS	COMMENTS
	GARY	DRIVEWAY		
	RIO VISTA			
				INSPECTOR
PERMIT #	OWNER/ADDRESS/CONTRACTOR	INSPECTION TYPE	RESULTS	COMMENTS
	MANDRAJ d RIVER	LIGHT POLE		LANTERN
				INSPECTOR
PERMIT #	OWNER/ADDRESS/CONTRACTOR	INSPECTION TYPE	RESULTS	COMMENTS
				INSPECTOR
PERMIT #	OWNER/ADDRESS/CONTRACTOR	INSPECTION TYPE	RESULTS	COMMENTS
				INSPECTOR
PERMIT #	OWNER/ADDRESS/CONTRACTOR	INSPECTION TYPE	RESULTS	COMMENTS
				INSPECTOR
PERMIT #	OWNER/ADDRESS/CONTRACTOR	INSPECTION TYPE	RESULTS	COMMENTS
				INSPECTOR



TOWN OF SEWALL'S POINT BUILDING DEPARTMENT

One S. Sewall's Point Road
Sewall's Point, Florida 34996
Tel 772-287-2455 Fax 772-2204765

Air Conditioning Change out Affidavit

Residential ☒ Commercial ☐

Package Unit ☐ Yes ☒ No (Use Condenser side of form below for equipment listing)

Duct Replacement ☐ Yes ☒ No - Refrigerant line replacement ☐ Yes ☒ No

Flushing Existing Refrigerant lines ☐ Yes ☒ No - Adding Refrigerant Drier ☐ Yes ☒ No

Rooftop A/C Stand Installation ☐ Yes ☒ No - Curb Installation ☐ Yes ☒ No

Smoke Detector in Supply (over 2000 CFM) ☐ Yes ☒ No

One form required for each A/C system installed

REPLACEMENT SYSTEM COMPONENTS

Air handler: Mfg: Lennox Model# 0556

Volts 240 CFM's Heat Strip 4 Kw

Min. Circuit Amps 35 Wire gauge

Max. Breaker size Min. Breaker size

Ref. line size: Liquid 3/8 Suction 3/4

Refrigerant type 410A

Location: Existing ☒ New ☐

Attic/Garage/Closet (specify)

Access:

Condenser: Mfg: Lennox Model# K21-036

Volts SEER/EER 19 BTU's

Min. Circuit Amps 35 Wire gauge

Max. Breaker size Min. Breaker size

Ref. line size: Liquid 3/8 Suction 3/4

Refrigerant type 410A

Location: Existing ☒ New ☐

Left/Right/Rear/Front/Roof ground

Condensate Location

NOTE: CONTRACTOR MUST SUPPLY A PROPER LADDER IF REQUIRED FOR INSPECTION

EXISTING SYSTEM COMPONENTS

Air handler: Mfg: Trane Model# TW1024

Volts 240 CFM's Heat Strip 5 Kw

Min. Circuit Amps 35 Wire gauge

Max. Breaker size Min. Breaker size

Ref. line size: Liquid Suction

Refrigerant type R-22

Location: Ext. ☒ New ☐

Attic/Garage/Closet (specify) attic

Access:

Condenser: Mfg: Trane Model# TT1024

Volts SEER/EER 11A BTU's

Min. Circuit Amps Wire gauge

Max. Breaker size 35 Min. Breaker size

Ref. line size: Liquid 3/8 Suction 3/4

Refrigerant type R-22

Location: Ext. ☒ New ☐

Left/Right/Rear/Front/Roof ground

Condensate Location

Certification:

I hereby certify that the information entered on this form accurately represents the equipment installed and further that this equipment is considered matched as required by FBC - R (N) 1107 & 1108

Signature

Date 09-19-2013



TOWN OF SEWALL'S POINT BUILDING DEPARTMENT
One S. Sewall's Point Road
Sewall's Point, Florida 34996
Tel 772-287-2455 Fax 772-2204765

Air Conditioning Change out Affidavit

Residential X Commercial _____

Package Unit ____ Yes X No (Use Condenser side of form below for equipment listing)

Duct Replacement ____ Yes X No - Refrigerant line replacement ____ Yes X No

Flushing Existing Refrigerant lines ____ Yes X No - Adding Refrigerant Drier ____ Yes X No

Rooftop A/C Stand Installation ____ Yes X No - Curb Installation ____ Yes X No

Smoke Detector in Supply (over 2000 CFM) ____ Yes X No

One form required for each A/C system installed

REPLACEMENT SYSTEM COMPONENTS

Air handler: Mfg: Lennox Model# 10132MV-1

Volts 240 CFM's _____ Heat Strip 4 Kw _____

Min. Circuit Amps 35 Wire gauge _____

Max. Breaker size _____ Min. Breaker size _____

Ref. line size: Liquid 3/8 Suction 3/4

Refrigerant type 410 A

Location: Existing X New _____

Attic/Garage/Closet (specify) _____

Access: _____

Condenser: Mfg: Lennox Model# XCA1-036

Volts _____ SEER/EER 19 BTU's _____

Min. Circuit Amps 35 Wire gauge _____

Max. Breaker size _____ Min. Breaker size _____

Ref. line size: Liquid 3/8 Suction 3/4

Refrigerant type 410 A

Location: Existing X New _____

Left/Right/Rear/Front/Roof ground

Condensate Location _____

NOTE: CONTRACTOR MUST SUPPLY A PROPER LADDER IF REQUIRED FOR INSPECTION

EXISTING SYSTEM COMPONENTS

Air handler: Mfg: Trane Model# TW024

Volts 240 CFM's _____ Heat Strip 5 Kw _____

Min. Circuit Amps 35 Wire gauge _____

Max. Breaker size _____ Min. Breaker size _____

Ref. line size: Liquid _____ Suction _____

Refrigerant type R-22

Location: Ext. X New _____

Attic/Garage/Closet (specify) attic

Access: _____

Condenser: Mfg: Trane Model# TTN024

Volts _____ SEER/EER 11A BTU's _____

Min. Circuit Amps _____ Wire gauge _____

Max. Breaker size 35 Min. Breaker size _____

Ref. line size: Liquid 3/8 Suction 3/4

Refrigerant type R-22

Location: Ext. X New _____

Left/Right/Rear/Front/Roof ground

Condensate Location _____

Certification:

I hereby certify that the information entered on this form accurately represents the equipment installed and further that this equipment is considered matched as required by FBC - R (N)1107 & 1108

Signature

Date

[Signature]

09-19-2013



TOWN OF SEWALL'S POINT BUILDING DEPARTMENT

One S. Sewall's Point Road
Sewall's Point, Florida 34996
Tel 772-287-2455 Fax 772-2204765

Air Conditioning Change out AffidavitResidential ☒ Commercial _____Package Unit _____ Yes ☒ No (Use Condenser side of form below for equipment listing)Duct Replacement _____ Yes ☒ No - Refrigerant line replacement _____ Yes ☒ NoFlushing Existing Refrigerant lines _____ Yes ☒ No - Adding Refrigerant Drier _____ Yes ☒ NoRooftop A/C Stand Installation _____ Yes ☒ No - Curb Installation _____ Yes ☒ NoSmoke Detector in Supply (over 2000 CFM) _____ Yes ☒ NoOne form required for each A/C system installedREPLACEMENT SYSTEM COMPONENTSAir handler: Mfg: Lennox Model# 10632muVolts 240 CFM's _____ Heat Strip 4 Kw _____Min. Circuit Amps 35 Wire gauge _____

Max. Breaker size _____ Min. Breaker size _____

Ref. line size: Liquid 3/8 Suction 3/4Refrigerant type 410 ALocation: Existing ☒ New _____Attic/Garage/Closet (specify) located above garageAccess: (need ladder)Condenser: Mfg: Lennox Model# 1021-036Volts _____ SEER/EER 19 BTU's _____Min. Circuit Amps 35 Wire gauge _____

Max. Breaker size _____ Min. Breaker size _____

Ref. line size: Liquid 3/8 Suction 3/4Refrigerant type 410 ALocation: Existing ☒ New _____Left/Right/Rear/Front/Roof ground

Condensate Location _____

NOTE: CONTRACTOR MUST SUPPLY A PROPER LADDER IF REQUIRED FOR INSPECTIONEXISTING SYSTEM COMPONENTSAir handler: Mfg: Trane Model# TTN024Volts 240 CFM's _____ Heat Strip 5 Kw _____Min. Circuit Amps 35 Wire gauge _____

Max. Breaker size _____ Min. Breaker size _____

Ref. line size: Liquid _____ Suction _____

Refrigerant type R-22Location: Ext. ☒ New _____Attic/Garage/Closet (specify) attic

Access: _____

Condenser: Mfg: Trane Model# TTN024Volts _____ SEER/EER 11A BTU's _____

Min. Circuit Amps _____ Wire gauge _____

Max. Breaker size 35 Min. Breaker size _____Ref. line size: Liquid 3/8 Suction 3/4Refrigerant type R-22Location: Ext. ☒ New _____Left/Right/Rear/Front/Roof ground

Condensate Location _____

Certification:

I hereby certify that the information entered on this form accurately represents the equipment installed and further that this equipment is considered matched as required by FBC - R (N) 1107 & 1108

[Signature]
Signature

09-19-2013
Date

Project Information

For: CUSHING RESIDENCE
102 ABBIE COURT, STUART, FL 34996

Notes:

Design Information

Weather: W Palm Beach, FL, US

Winter Design Conditions

Outside db 47 °F
Inside db 70 °F
Design TD 23 °F

Summer Design Conditions

Outside db 90 °F
Inside db 75 °F
Design TD 15 °F
Daily range L
Relative humidity 50 %
Moisture difference 59 gr/lb

Heating Summary

Structure 15810 Btuh
Ducts 4248 Btuh
Central vent (0 cfm) 0 Btuh
Humidification 0 Btuh
Piping 0 Btuh
Equipment load 20058 Btuh

Sensible Cooling Equipment Load Sizing

Structure 25229 Btuh
Ducts 6912 Btuh
Central vent (0 cfm) 0 Btuh
Blower 0 Btuh
Use manufacturer's data n
Rate/swing multiplier 0.95
Equipment sensible load 30598 Btuh

Infiltration

Method Simplified
Construction quality Tight
Fireplaces 0

	Heating	Cooling
Area (ft ²)	1221	1221
Volume (ft ³)	12210	12210
Air changes/hour	0.16	0.08
Equiv. AVF (cfm)	33	16

Latent Cooling Equipment Load Sizing

Structure 647 Btuh
Ducts 1893 Btuh
Central vent (0 cfm) 0 Btuh
Equipment latent load 2540 Btuh
Equipment total load 33139 Btuh
Req. total capacity at 0.70 SHR 3.6 ton

Heating Equipment Summary

Make
Trade
Model
AHRI ref
Efficiency 80 AFUE
Heating input 0 MBtuh
Heating output 0 Btuh
Temperature rise 0 °F
Actual air flow 1187 cfm
Air flow factor 0.059 cfm/Btuh
Static pressure 0 in H₂O
Space thermostat

Cooling Equipment Summary

Make Lennox
Trade XC21 SERIES
Cond XC21-036-230-10
Coil CBX32MV-036*++TDR
AHRI ref 5991578
Efficiency 13.5 EER, 19 SEER
Sensible cooling 24920 Btuh
Latent cooling 10680 Btuh
Total cooling 35600 Btuh
Actual air flow 1187 cfm
Air flow factor 0.037 cfm/Btuh
Static pressure 0 in H₂O
Load sensible heat ratio 0.93

Calculations approved by ACCA to meet all requirements of Manual J 8th Ed.

CLIPS



Florida Department of
Business & Professional
Regulation

BCIS Home Log In User Registration Hot Topics Submit Surcharge Stats & Facts Publications FBC Staff BCIS Site Map Links Search



Product Approval
USER: Public User

Online Product Approval

Product Approval Menu > Product or Application Search > Application List > Application Detail

FL #	FL15750
Application Type	New
Code Version	2010
Application Status	Approved
Comments	
Archived	
Product Manufacturer	RM Enterprises, Inc.
Address/Phone/Email	352 Tall Pines Rd. Suite A-1 West Palm Beach, FL 33413 (561) 682-1225 raymanucy@aol.com
Authorized Signature	Ray Manucy raymanucy@aol.com
Technical Representative	
Address/Phone/Email	
Quality Assurance Representative	
Address/Phone/Email	
Category	Structural Components
Subcategory	Anchors
Compliance Method	Evaluation Report from a Florida Registered Architect or a Licensed Florida Professional Engineer - Evaluation Report - Hardcopy Received
Florida Engineer or Architect Name who developed the Evaluation Report	Denis K. Solano
Florida License	PE-PE-56902
Quality Assurance Entity	Keystone Certifications, Inc.
Quality Assurance Contract Expiration Date	12/31/2016
Validated By	Steven M. Ulrich, PE Validation Checklist - Hardcopy Received
Certificate of Independence	FL15750_RQ_COI_Certification_of_Independence.pdf
Referenced Standard and Year (of Standard)	
Equivalence of Product Standards	
Certified By	
Sections from the Code	1609 1620

TABLE # 1. CLIP SELECTION (GROUND APPLICATION)					
RM-1"x4" / 1"x6" 8" GALV. STEEL CLIP TIE-DOWN SCHEDULE FOR GROUND APPLICATION ONLY					
MAXIMUM SURFACE AREA OF UNITS LARGEST FACE	MAXIMUM UNITS WIDTH	MAXIMUM UNITS HEIGHT	CORRELATION DETAIL APPLICABLE TO EACH SUBSTRATE CONDITION		
			CLIP AT 1" CORRELATION OF 8" CLIP	CLIP AT 1" CORRELATION OF 8" CLIP	CLIP AT 1" CORRELATION OF 8" CLIP
10 SQ FT	32" MAX	48" MAX	SEE DETAIL "D" ON 5-2	NA	NA

TABLE # 2. CLIP SELECTION (GROUND & ROOF TOP APPLICATIONS H<15FT)					
RM-FB-2x4 & FB-2x6 GALV. STEEL CLIP TIE-DOWN SCHEDULE FOR GROUND & ROOF TOP APPLICATIONS (H<15FT)					
MAXIMUM SURFACE AREA OF UNITS LARGEST FACE	MAXIMUM UNITS WIDTH	MAXIMUM UNITS HEIGHT	CORRELATION DETAIL APPLICABLE TO EACH SUBSTRATE CONDITION		
			CLIP AT 1" CORRELATION OF 8" CLIP	CLIP AT 1" CORRELATION OF 8" CLIP	CLIP AT 1" CORRELATION OF 8" CLIP
10 SQ FT	48" MAX	48" MAX	SEE DETAIL "D" ON 5-2	SEE DETAIL "C" ON 5-3	SEE DETAIL "D" ON 5-3

NOTE: 1" MAX UNIT HEIGHT

TABLE # 3. CONNECTION TO BASE SUBSTRATE (GROUND & ROOF TOP APPLICATIONS H<15 FT)	
BASE ANCHOR SCHEDULE FOR RM-FB-2x4 & FB-2x6 TIE DOWN CLIPS	
SUBSTRATE	ANCHOR TYPE
ALUMINUM (1/2" THICK, MAX 12" MAX. ALUM.)	1/2" DIA. 30 GALV. KNUX-FLUX SELF DRILLING SCREWS BY HULT. SEE DETAIL "D" ON 5-2.
STEEL (1/2" THICK, MAX 12" MAX. ALUM.)	1/2" DIA. 30 GALV. KNUX-FLUX SELF DRILLING SCREWS BY HULT. SEE DETAIL "D" ON 5-2.
CONCRETE (4" THICK, MAX, 3000 PSI MAX)	1/2" DIA. 30 GALV. KNUX-FLUX SELF DRILLING SCREWS BY HULT. SEE DETAIL "D" ON 5-2.

TABLE # 4. CONN. TO CONC. BASE SUBSTRATE (GROUND APPLICATIONS ONLY)	
ANCHOR SCHEDULE FOR CONC. SUBSTRATE APPLICATIONS RM-1"x4" & 1"x6" 8" CLIPS	
SUBSTRATE	ANCHOR TYPE
CONCRETE (4" THICK, MAX, 3000 PSI MAX)	1/2" DIA. 30 GALV. KNUX-FLUX SELF DRILLING SCREWS BY HULT. SEE DETAIL "D" ON 5-2.

TABLE # 5			
WIND DESIGN CRITERIA BASED ON BLDG HEIGHT & WIND VELOCITY.			
(H) HEIGHT FROM GRADE TO TOP OF ROOF LEVEL OR UNIT BASE			
WIND VELOCITIES (W)	V=140 MPH	V=170 MPH	
LOCATION	ASCE-10	ASCE-10	
BLIND	LATERAL	65.9 PSF (41.54 PSF)	77.35 PSF (46.41 PSF)
	UP/LIFT	50.57 PSF (30.34 PSF)	NA
COASTAL (1)	LATERAL	104	94.0 PSF (56.4 PSF)
	UP/LIFT	104	NA

1) COASTAL CONSIDERATIONS APPLY TO BUILDING LOCATED WITHIN 1 MILE OF THE SHORE LINE FOR A DISTANCE UP TO 660 FT OR AS GOVERNED BY LOCAL ORDINANCES.

GENERAL NOTES:

- THIS PRODUCT HAS BEEN DESIGNED AND SHALL BE FABRICATED IN ACCORDANCE WITH THE REQUIREMENTS OF THE 2010 F.B.C. & ASCE 7-10 USING WIND PRESSURES AS SHOWN ON TABLE #5 FOR D.W. AND FACTORED BY 1.5 WHEN APPLICABLE. SEE STAND SELECTION PROTOCOL ON THIS SHEET.
- LIMITATION OF USE:
 - GROUND APPLICATION: USE RM-FB-2x4 & FB-2x6 AND RM-1"x4" & RM-1"x6" AT COASTAL & BLIND GROUND APPLICATIONS WITH WIND VELOCITIES V<170 mph. SEE GROUND APPLICATION DETAILS ON 5-2.
 - ROOF TOP APPLICATION: USE RM-FB-2x4 & FB-2x6 AT ROOF TOP APPLICATIONS LIMITED TO BLDG ROOF HEIGHTS <15 FT LOCATED ON BLIND AREAS ONLY. WIND VELOCITIES V<140 mph. SEE TYPICAL ROOF TOP CONNECTION DETAILS "B", "C", "D" ON 5-3.
- MAXIMUM & MINIMUM DIMENSIONS AND MINIMUM WEIGHT OF MECHANICAL UNIT SHALL CONFORM TO SPECIFICATIONS SHOWN HEREIN ON THE TYP. MECH. UNIT TIE DOWN CONNECTIONS ON 5-2 AND CONNECTION DETAILS ON 5-3.
- ALLOWABLE DESIGN PRESSURES USED TO DETERMINE CAPACITY OF CLIPS AS LISTED HEREIN IN TABLE #6 AND DETERMINED THROUGH RATIONALLY ANALYSIS. CLIPS SHALL BE TESTED TO CHECK FOR CONSISTENCY IN EACH CALCULATION PERFORMED.
- ALL CONCRETE SPECIFIED HEREIN IS NOT PART OF THIS CERTIFICATION, AS A MINIMUM, ALL CONCRETE SHALL BE STRUCTURAL CONCRETE 4" MAX. THICK AND SHALL HAVE MINIMUM COMPRESSIVE STRENGTH OF 3000 PSI, UNLESS NOTED OTHERWISE.
- ALL STEEL CLIPS SHALL BE A500 50S STEEL (GRADE 50 W/ F_y=50 KSI) OR BETTER. ALL STEEL MEMBER SHALL BE PROTECTED AGAINST CORROSION BY AN APPROVED COAT OF PAINT, ENAMEL, OR OTHER APPROVED PROTECTION IN ACCORDANCE WITH THE 2010 F.B.C. SECTION 2203.2 AND 2220 600-RATED COATING REQUIRED FOR ALL COASTAL INSTALLATIONS.
- SEE ANCHOR SCHEDULES TABLE #3, & TABLE #4 FOR ANCHOR REQUIREMENTS. ALL FASTENERS SHALL HAVE APPROPRIATE CORROSION PROTECTION TO PREVENT ELECTROLYSIS.
- THE CONTRACTOR IS RESPONSIBLE TO INSULATE ALL MEMBERS FROM DISSIMILAR MATERIALS TO PREVENT ELECTROLYSIS.
- THE ADEQUACY OF ANY EXISTING STRUCTURE TO WITHSTAND SUPERIMPOSED LOADS SHALL BE VERIFIED BY THE ON SITE DESIGN PROFESSIONAL AND IS NOT INCLUDED IN THIS CERTIFICATION. CONTRACTOR SHALL HIRE THE SERVICES OF FLORIDA PROFESSIONAL ENGINEER.
- WATERPROOFING OF EXISTING SUBSTRATE SHALL BE THE FULL RESPONSIBILITY OF THE CONTRACTOR.

DESIGN CRITERIA:

- WIND CALC AS PER ASCE 7-10.
- WIND VELOCITIES V<170 mph & 140 mph. SEE LIMITATION OF USE ON GENERAL NOTES #2.
- GCF=0.20
- GCF=3.3 (LATERAL)
- GCF=1.5 (UP/LIFT)

STAND SELECTION PROTOCOL:

- DETERMINE WHETHER YOUR UNIT WILL BE LOCATED ON A GROUND CONCRETE SLAB, GROUND MOUNTED ALUM./STEEL/CONC. STAND OR OVER A ROOF IN A MAX. HEIGHT, <15 FT. SEE TABLE #1 & #2 FOR APPLICABLE CLIP SELECTION BASED ON UNIT SIZE & TYPE OF ALUM./STEEL/CONC. SUBSTRATE.
- FOR GROUND CONC. SLAB APPLICATIONS ONLY YOU MAY SELECT ONE OF THE TWO TIE DOWN CLIP PATTERN AS SHOWN ON 5-2 DETAIL "D" & "E" BASED ON THE TYPE OF CLIP SELECTED ON STEP #1.
- FOR ROOF TOP AND GROUND MOUNTED STAND APPLICATION, SELECT THE CORRESPONDING APPLICABLE DETAIL "B", "C" OR "D" FROM TABLE #2. (SEE ALSO INSTALLATION DETAIL ON 5-3).

CLIP TIE-DOWN SCHEDULE NOTES:

- TIE DOWN CLIPS SHALL BE FASTENED TO MECH. HOLDING UNIT WITH (1) OR MAXIMUM (2) GALV. KNUX-FLUX SELF DRILLING SCREWS BY HULT. SEE DETAILS ON 5-3.
- MECH. HOLDING UNIT SHALL CONFORM TO THE FOLLOWING:
 - ALUM. HOLDING UNITS SHALL BE 5052-H32 MAX. ALUM. SHEET WITH F_y=30 KSI, 0.008" MIN. THICKNESS (1/8" O.D.)
 - STEEL HOLDING UNITS SHALL BE 33 KSI MIN. STEEL, GRADE 42 GA. W/ F_y=40 KSI.
- RM-FB-2x4 & FB-2x6 TIE DOWN CLIPS SHALL NOT BE USED FOR BLDG HEIGHTS IN EXCESS OF 15 FT. SEE LIMITATION OF USE IN GENERAL NOTE #2.
- RM-1"x4" & 1"x6" 8" TIE DOWN CLIPS SHALL BE USED ONLY ON GROUND APPLICATIONS WITH WIND VELOCITIES V<170 mph. SEE LIMITATION OF USE IN GENERAL NOTE #2.

Denis K. Solano
P.E. NRE

CONSULTANTS:

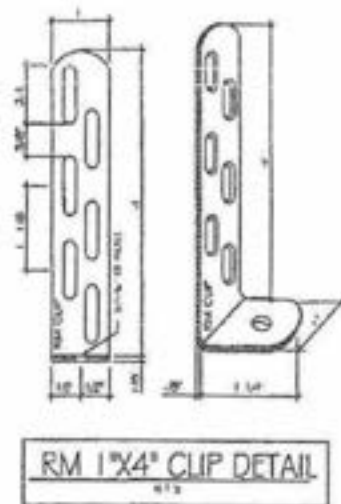
Robert S. Scharf, P.E.
352 TALL PINES ROAD, SUITE 4-1
WEST PALM BEACH, FLORIDA 33413
(561) 833-1111
www.scharf-engineering.com

By: R.M. ENTERPRISES OF THE SE, INC.
352 TALL PINES ROAD, SUITE 4-1
WEST PALM BEACH, FLORIDA 33413
(561) 833-1111
www.rm-engineering.com

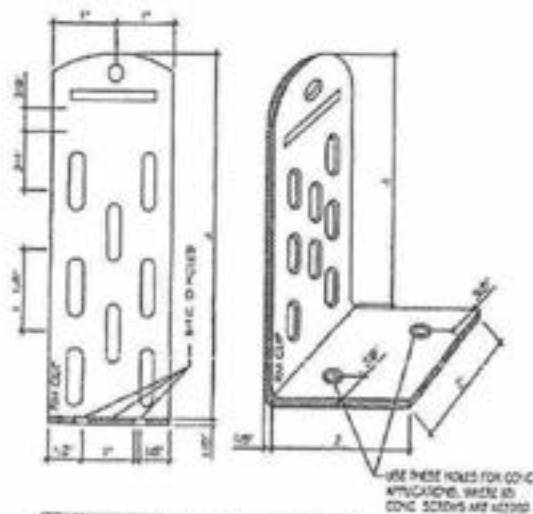
DATE: 6/20/20
REVISED: 7/2/20
DRAWN: R.S.S.
CHECKED: R.S.S.
PROJECT: _____

Sheet No. 1
1 of 3

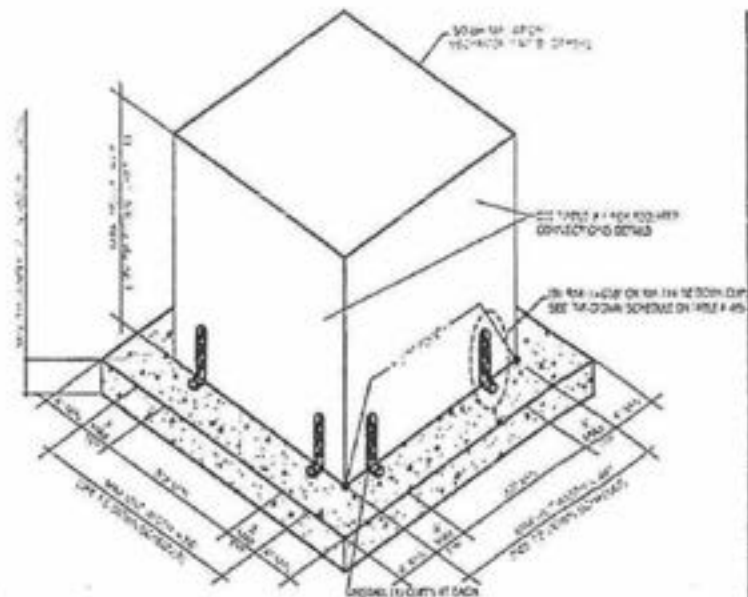
RM FB2X6 CLIP DETAIL



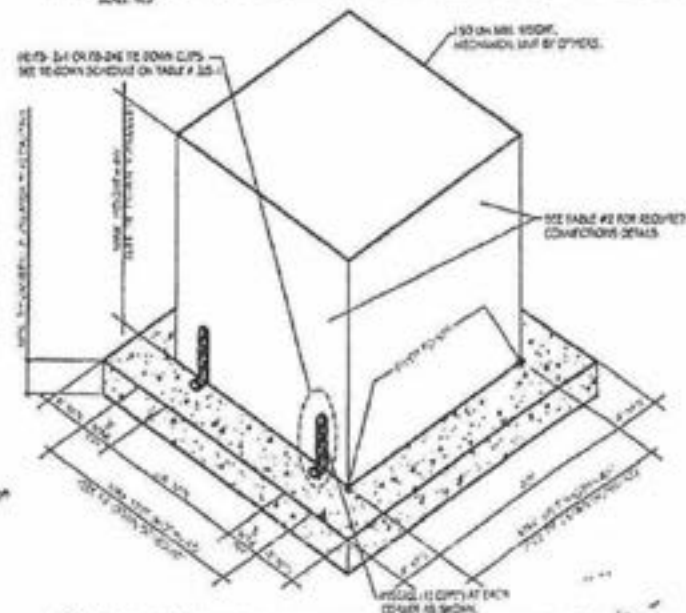
RM 1"X8" CLIP DETAIL



RM FB2X4 CLIP DETAIL



- ① TYP. MECHANICAL UNIT TIE-DOWN ISOMETRIC FOR RM CLIPS:
RM-(1"x4") AND RM-(1"x6"x8") ON GROUND CONC. SLAB APPLICATIONS
SCALE: NED



- ② TYP. MECHANICAL UNIT TIE-DOWN ISOMETRIC FOR RM CUPS (1/2" FB-(2"x4") AND FB-(2"x6") ON GROUND CONC. SLAB APPLICATIONS

Denis K. Solano

P.E. 56902 S.I. 2046

CONSULTANTS:



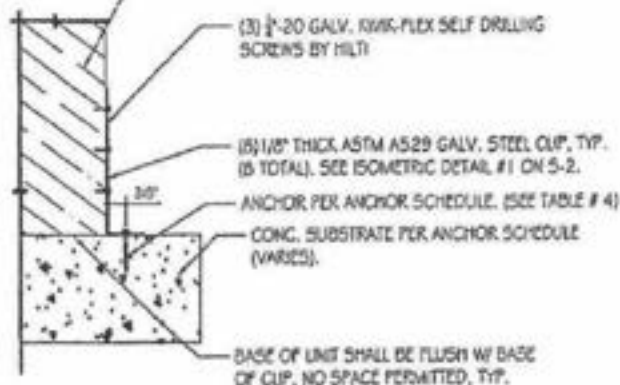
Solman Structural Partners, Inc.
 1000 West 10th St.
 Suite 200
 Denver, CO 80202
 Tel: (303) 733-1111
 Fax: (303) 733-1111
 E-Mail: info@solman.com

Curran:
R.M. ENTERPRISES OF THE SE, INC.
352 TALL PINES ROAD SUITE A-1
WEST PALM BEACH, FLORIDA 33413

Date 7-19-90
 Page 70-2
 Name Allen
 Address 1-17
 City Am

$$\frac{2}{2 \pm 3}$$

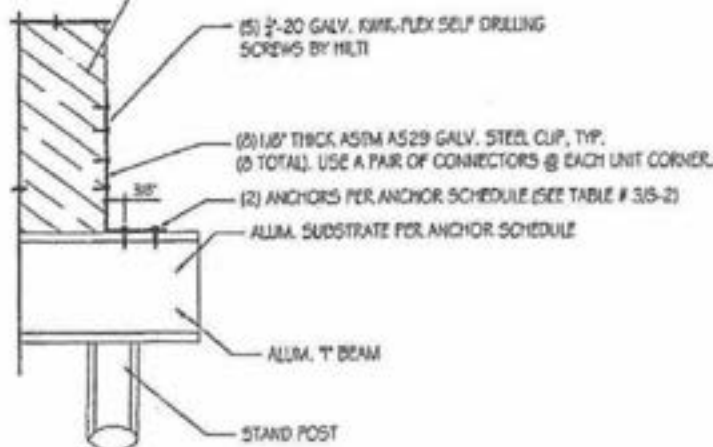
MECH UNIT BY OTHERS (MAX. UNIT DIMS: 30"X30"X48").
ALUMINUM HOUSING UNITS SHALL BE 5052-H32 ALLOY
MIN. ALUM. SHEET W/ $F_{ty}=30$ KSI, 16 GA. THICKNESS,
STEEL HOUSING UNITS SHALL BE 33 KSI MIN. STEEL, GRADE 33,
22 GA MIN. ($t=0.0299$), MIN. UNIT WEIGHT = 150 LB.



A 1" TIE-DOWN CLIP ANCHOR DETAIL
TO CONC. (GROUND APPLICATION ONLY).

SCALE: 3" = 1'

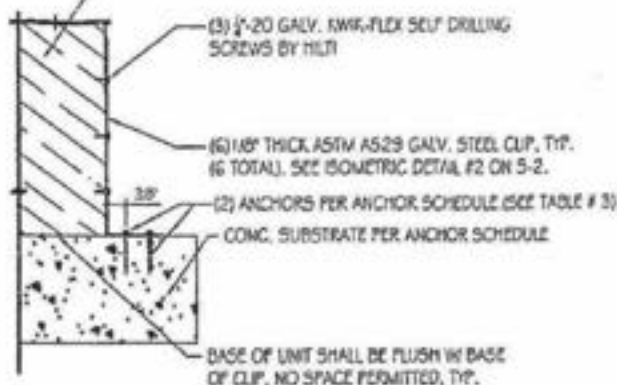
MECH UNIT BY OTHERS (MAX. UNIT DIMS: 30"X30"X48").
ALUMINUM HOUSING UNITS SHALL BE 5052-H32 ALLOY
MIN. ALUM. SHEET W/ $F_{ty}=30$ KSI, 16 GA. THICKNESS,
STEEL HOUSING UNITS SHALL BE 33 KSI MIN. STEEL, GRADE 33,
22 GA MIN. ($t=0.0299$), MIN. UNIT WEIGHT = 150 LB.



C 2" TIE-DOWN CLIP ANCHOR DETAIL TO ALUM. SUBSTRATE
FOR GROUND APPLICATION AND BLDG HEIGHT ≤ 15 FT.

SCALE: 3" = 1'

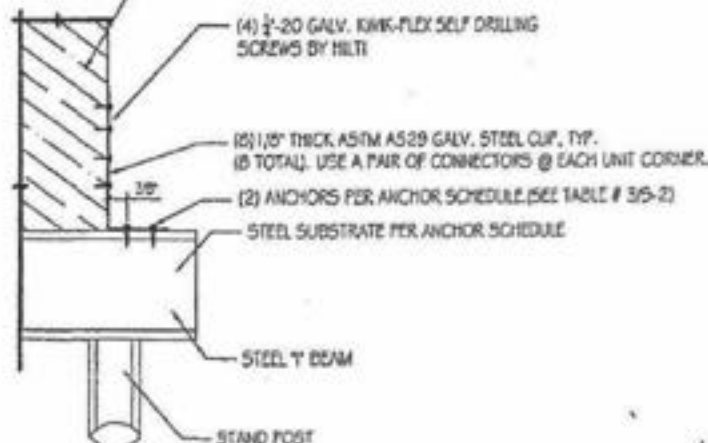
MECH UNIT BY OTHERS (MAX. UNIT DIMS: 30"X30"X48").
ALUMINUM HOUSING UNITS SHALL BE 5052-H32 ALLOY
MIN. ALUM. SHEET W/ $F_{ty}=30$ KSI, 16 GA. THICKNESS,
STEEL HOUSING UNITS SHALL BE 33 KSI MIN. STEEL, GRADE 33,
22 GA MIN. ($t=0.0299$), MIN. UNIT WEIGHT = 150 LB.



B 2" TIE-DOWN CLIP ANCHOR DETAIL TO CONC. SUBSTRATE.

SCALE: 3" = 1'

MECH UNIT BY OTHERS (MAX. UNIT DIMS: 30"X30"X48").
ALUMINUM HOUSING UNITS SHALL BE 5052-H32 ALLOY
MIN. ALUM. SHEET W/ $F_{ty}=30$ KSI, 16 GA. THICKNESS,
STEEL HOUSING UNITS SHALL BE 33 KSI MIN. STEEL, GRADE 33,
22 GA MIN. ($t=0.0299$), MIN. UNIT WEIGHT = 150 LB.



D 2" TIE-DOWN CLIP ANCHOR DETAIL TO STEEL SUBSTRATE
FOR GROUND APPLICATION AND BLDG HEIGHT ≤ 15 FT.

SCALE: 3" = 1'

CONSULTANTS:



R.M. ENTERPRISES OF THE SE, INC.
352 TALL PINES ROAD SUITE 4-1
WEST PALM BEACH, FLORIDA 33413
MECHANICAL UNIT STEEL TIE-DOWN CLIPS

Rev: 0-20-2
Date: 12-2
By: A-1
Check: P-2
Project No:

3
3 of 3

Denis K. Solano
P.E. 56702 S.I. 2046

Certificate of Product Ratings

AHRI Certified Reference Number: 5991578

Date: 9/19/2013

Product: Split System: Air-Cooled Condensing Unit, Coil with Blower

Outdoor Unit Model Number: XC21-036-230-10

Indoor Unit Model Number: CBX32MV-036*+TDR

Manufacturer: LENNOX INDUSTRIES, INC.

Trade/Brand name: XC21 SERIES

Manufacturer responsible for the rating of this system combination is LENNOX INDUSTRIES, INC.

Rated as follows in accordance with AHRI Standard 210/240-2008 for Unitary Air-Conditioning and Air-Source Heat Pump Equipment and subject to verification of rating accuracy by AHRI-sponsored, independent, third party testing:

Cooling Capacity (Btuh): 35600

EER Rating (Cooling): 13.50

SEER Rating (Cooling): 19.00

FootNote 11 - The AHRI 210/240 certified EER ratings are calculated under the same methodology as the EER ratings at T1 conditions of ISO 5151:2010 and ISO 13253:2011.

* Ratings followed by an asterisk (*) indicate a voluntary rerate of previously published data, unless accompanied with a VAS, which indicates an involuntary rerate.

DISCLAIMER

AHRI does not endorse the product(s) listed on this Certificate and makes no representations, warranties or guarantees as to, and assumes no responsibility for, the product(s) listed on this Certificate. AHRI expressly disclaims all liability for damages of any kind arising out of the use or performance of the product(s), or the unauthorized alteration of data listed on this Certificate. Certified ratings are valid only for models and configurations listed in the directory at www.ahridirectory.org.

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CERTIFICATE VERIFICATION

The information for the model cited on this certificate can be verified at www.ahridirectory.org, click on "Verify Certificate" link and enter the AHRI Certified Reference Number and the date on which the certificate was issued, which is listed above, and the Certificate No., which is listed below.



Air-Conditioning, Heating,
and Refrigeration Institute

HORIZONTAL AIR HANDLER DETAIL

ROPE HOOK WITH
2 - 1" SCREWS (MIN.)
TO TOP CORD OF
TRUSS (TYPICAL)

#2
CHAIN

AIR
HANDLER
UNIT

METAL SUPPORT
STRUTS IN PAN

AUX DRAIN PAN

AUX DRIAN LINE SHALL TERMINATE
TO A CONSPICUOUS POINT

REVISION

ALL-CONCRETE DRIVING FOR

STAPLETON K. GENTRY, INC.
(903) 344-0000
107 107 STREET
ARLINGTON, TEXAS 76010



406 Commerce Way
Jupiter, FL 33458

743-9554
Lic# CAC-1813800

stephen k. denny, inc.
AIR CONDITIONING & POOL HEATING
Sales & Service
1-561-743-9554 phone
1-561-746-2581 fax

July 8, 2013

Re: Duct Certification at 102 Abbie Court, Stuart FL 34996

I do hereby certify that the all accessible (a minimum of 30 inches clearance) joints and seams in the Air Distribution System have been inspected and sealed where needed using reinforced mastic per Section 101.4.7.1.1 of the Florida Building Code, Energy Conservation at the above referenced location.

Sincerely,

Steve Sanders
Qualifier for stephen k. denny, inc.
License # CAC1813800

SDS/hd



TOWN OF SEWALL'S POINT BUILDING DEPARTMENT
One S. Sewall's Point Road
Sewall's Point, Florida 34996
Tel: 772-287-2455 Fax 772-220-4765

FLORIDA ENERGY CONSERVATION CODE

Mandatory Duct Inspection Certification for HVAC change-out

For use when part of the duct and/or HVAC system has been replaced (Section 101.4.7.1.1 & FS 553.912)

Owner: Robert + Pearl Rushing Contractor name: Stephen K. Denny, Inc
Street address: 102 Abbie Court Jurisdiction: _____
City: Stuart Permit No.: _____
Zip: 34996 Final inspection date: _____

I certify that I have inspected the duct work associated with the HVAC unit referenced by the permit listed above and found it complies with the requirements of Section 101.4.7.1.1 as indicated below:

- ☒ Where needed, the existing ducts have been sealed using reinforced mastic or code-approved equivalent.
☐ Ducts are located within conditioned space. (Section 101.4.7.1.1 exception 1)
☐ The joints or seams are already sealed with fabric and mastic (Section 101.4.7.1.1 exception 2)
☐ System was tested (see below) and repairs were made as necessary – (Section 101.4.7.1.1 exception 3)

Signature: Steve Sanders Date: 09-19-2013

Printed Name: Steve Sanders

Contractor License #: CAC 1813800

I certified I have tested the replaced air distribution system(s) referenced by the permit listed above at a pressure differential of 25 Pascals (0.10 in. w.c.).

Signature: _____ Date: _____

Printed Name: _____

TREE

TOWN OF SEWALL'S POINT

APPLICATION FOR TREE REMOVAL, RELOCATION, REPLACEMENT

1/17/01 SCHED
10SP. ✓
APPR: 1/17
ISSUED 1/17



Permit # 0399
Date Issued 1/17/01

This application shall include a written statement giving reasons for removal, relocation or replacement and a site plan which shall include the dimensional location on a survey, scale drawing, or aerial photograph, superimposed with lot lines to scale, of all existing or proposed structures, improvements and site uses, location of affected trees identified with an estimated size and number, etc.

Owner REGULA CONTRACTING CORP Address 7418 WILSON BL. GAITHERSBURG Phone 954-755-8808

Contractor REGULA CONTRACTING Address " " " Phone " " "

Number of trees to be removed (list kinds of trees) 102 ABBIE CANY.

(1) 24" OAK & (1) TREE CLUSTER

Number of trees to be relocated within 30 days (no fee) (list kinds of trees):

(INCIDENT TO DEVELOPMENT - BPN 5228 1/16/01)

Number of trees to be replaced (list kinds of trees):

Permit Fee \$ 15.00 (~~\$25.00~~ first tree plus \$10.00 - each additional tree - not to exceed \$100.00. \$15.00)

(No permit fee for trees which are relocated on property or lie within a utility easement & are required to be removed in order to provide utility service, nor for a tree which is dead, diseased, injured or hazardous to life or property.)

Plans approved as submitted ✓ Plans approved as marked " "

Permit good for one year. Fee for renewal of expired permit is \$5.00

Signature of applicant [signature] Date submitted 1-16-01

Approved by Building Inspector [signature] Date 1/17/01

Approved by Building Commissioner " " Date " "

Completed " " Date " " Checked by " "

FEE

THE FOLLOWING TREES MAY BE REMOVED OR DESTROYED WITHOUT ~~OBSTRUCTING~~ BRAZILIAN PEPPER, FLORIDA HOLLY TREE, AUSTRALIAN PINE AND STRANGLER FIG. FOR THE PURPOSE OF THIS PERMIT, A TREE IS DEFINED AS ANY SELF-SUPPORTING WOODY OR FIBROUS PERENNIAL PLANT WHICH HAS A MINIMUM HEIGHT OF TWELVE (12) FEET.

THE FOLLOWING TREES MUST BE REMOVED BEFORE CONSTRUCTION BEGINS: BRAZILIAN PEPPER, FLORIDA HOLLY TREE, AUSTRALIAN PINE AND MELALEUCA?

TOWN OF SEWALL'S POINT

Building Department - Inspection Log

Date of Inspection: ☐ Mon ☒ Wed ☐ Fri 1/17/01 ~~7/2000~~ Page 2 of 2

PERMIT	OWNER/ADDRESS/CONTR.	INSPECTION TYPE	RESULTS	REMARKS
✓ 5075	VASQUES	T/T # MTL	Passed	1/17
5 (6)	82 S. SEWALL'S POINT RD GROZA (536-7563 x7)			
PERMIT	OWNER/ADDRESS/CONTR.	INSPECTION TYPE	RESULTS	REMARKS
5 T/R	FOGLIA	FIELD VERIF.	Passed	BPN 5228 (1/16/01)
(5)	102 ABBIE CT FOGLIA		Ok to remove as per plan	
PERMIT	OWNER/ADDRESS/CONTR.	INSPECTION TYPE	RESULTS	REMARKS
5214	Rohloff	Shedding	Passed	1/17
(9)	20 Rivermore RTS		Partial	west slope
PERMIT	OWNER/ADDRESS/CONTR.	INSPECTION TYPE	RESULTS	REMARKS
PERMIT	OWNER/ADDRESS/CONTR.	INSPECTION TYPE	RESULTS	REMARKS
PERMIT	OWNER/ADDRESS/CONTR.	INSPECTION TYPE	RESULTS	REMARKS

OTHER: _____

INSPECTOR (Name/Signature): _____

FILE

TOWN OF SEWALL'S POINT, FLORIDA

Date 1/17/01 17 TREE REMOVAL PERMIT No 0399
APPLIED FOR BY FOGLIA CONTRACTING CORP (Contractor or Owner)
Owner 102 ABBA COURT
Sub-division _____, Lot _____, Block _____
Kind of Trees (1) "cypress" ; (1) oak
No. Of Trees: REMOVE 2
No. Of Trees: RELOCATE -0- WITHIN 30 DAYS (NO FEE)
No. Of Trees: REPLACE -0- WITHIN 30 DAYS
REMARKS SEE SURVEY SKETCH FOR LOCATION
Signed, [Signature] Applicant
Signed, [Signature] Town Clerk
FEE \$ 15.00 CASH
RECEIVED WRP IF 1/17/01
REMOVAL IN ORDER
TO CONSTRUCTION
PN 5228 (1/16/01)
S.V.D.
BEN OFF.

TOWN OF SEWALL'S POINT

Call 287-2455 - 8:00 A.M.-12:00 Noon for Inspection

WORK HOURS 8:00 A.M. - 5:00 P.M. - NO SUNDAY WORK.

TREE REMOVAL PERMIT

RE: ORDINANCE 103

OK to remove as per plan
1/17/01
Add'l. tree removal permit issued
5/23/01 to remove
dead oak tree
5/23/01

PROJECT DESCRIPTION

REMARKS

TOWN OF SEWALL'S POINT

Building Department - Inspection Log

Date of Inspection: ☐ Mon ☒ Wed ☐ Fri MAY 23, 2001; Page 1 of 2.

PERMIT	OWNER/ADDRESS/CONTR.	INSPECTION TYPE	RESULTS	NOTES/COMMENTS:
✓ 5318	Koenke	Deck	Passed	
6	66 S. Sewall's Pt Rd. Advantage Pools			INSPECTOR: J 5/23
PERMIT	OWNER/ADDRESS/CONTR.	INSPECTION TYPE	RESULTS	NOTES/COMMENTS:
✓ 5143	GILFORD	TIE BM/COL	Passed	
7	85 N. SEWALL'S POINT RD SCOTT HOLMES (JERRY)	(Highway) 201-9689		INSPECTOR: J 5/23
PERMIT	OWNER/ADDRESS/CONTR.	INSPECTION TYPE	RESULTS	NOTES/COMMENTS:
✓ 5344	HEURY	SHEATHING	Passed	EARLY AS POSSIBLE 900
1	B E. HIGH POINT HEATON REFR. CO.	(North slope 'DAN' 287-0116)		INSPECTOR: J 5/23
PERMIT	OWNER/ADDRESS/CONTR.	INSPECTION TYPE	RESULTS	NOTES/COMMENTS:
✓ T/R	FOGLIA	FIELD VERIF.	Passed	SUPPL. TO PN 0399
2	102 ABBIE COURT FOGLIA CONST.			Oak tree dead INSPECTOR: J 5/23
PERMIT	OWNER/ADDRESS/CONTR.	INSPECTION TYPE	RESULTS	NOTES/COMMENTS:
✓ 5378	COMBS	ROOFING -	Passed	
4	MANDALAY ISLAND CARPS & HUFF	FINAL		INSPECTOR: J 5/23
PERMIT	OWNER/ADDRESS/CONTR.	INSPECTION TYPE	RESULTS	NOTES/COMMENTS:
✓ 5374	PRAWNBROKER	A/C UNIT - ^{ROOF}	Postponed	due to rain
8	3754 SE OCEAN SUPERIOR REFRIG.	FINAL 201-1340 - GREG	Will call + reschedule	INSPECTOR: J 5/23
PERMIT	OWNER/ADDRESS/CONTR.	INSPECTION TYPE	RESULTS	NOTES/COMMENTS:
✓ T/R	REILLY	FIELD VERIF.	Passed	dead tree
5	785 SEWALL'S POINT RD. FERRY LAWN SERVICE			INSPECTOR: J 5/23

OTHER: _____

TOWN OF SEWALL'S POINT, FLORIDA

Date 5/23/01 ☒ TREE REMOVAL PERMIT No 0449
APPLIED FOR BY Paglia Contr. Co. 62 Abbie Ct. (Contractor or Owner)
Owner _____

Sub-division _____, Lot _____, Block _____

Kind of Trees Oak

No. Of Trees: REMOVE 1

No. Of Trees: RELOCATE _____ WITHIN 30 DAYS (NO FEE)

No. Of Trees: REPLACE _____ WITHIN 30 DAYS

REMARKS Decayed tree to be removed

Signed, Sign. on file Applicant
Signed, [Signature] Town Clerk
Bldg. Insp.

FEE \$ 0.-

TOWN OF SEWALL'S POINT

Call 287-2455 - 8:00 A.M.-12:00 Noon for Inspection

WORK HOURS 8:00 A.M. - 5:00 P.M.—NO SUNDAY WORK.

TREE REMOVAL PERMIT

RE: ORDINANCE 103

PROJECT DESCRIPTION _____

REMARKS _____

Original tree removal permit
issued 1/17/01

TOWN OF SEWALL'S POINT

APPLICATION FOR TREE REMOVAL, RELOCATION, REPLACEMENT

5/23/01 SCHED. (NSP)



Permit # 0449

Date Issued 5/23/01

102 ABBIE COURT

This application shall include a written statement giving reasons for removal, relocation or replacement and a site plan which shall include the dimensional location on a survey, scale drawing, or aerial photograph, superimposed with lot lines to scale, of all existing or proposed structures, improvements and site uses, location of affected trees identified with an estimated size and number, etc.

Owner KEENA CONTRACTING CORP. Address 7428 WILKS RD Phone 954-255-8808Contractor " Address " Phone "Number of trees to be removed (list kinds of trees) (1) OAK APPROX 24"Ø

Number of trees to be relocated within 30 days (no fee) (list kinds of trees):

Number of trees to be replaced (list kinds of trees):

TREE IS DEAD (NO FEE)Permit Fee \$ 0 (~~\$25.00~~ first tree plus \$10.00 - each additional tree - not to exceed \$100.00. \$15.00)

(No permit fee for trees which are relocated on property or lie within a utility easement & are required to be removed in order to provide utility service, nor for a tree which is dead, diseased, injured or hazardous to life or property.)

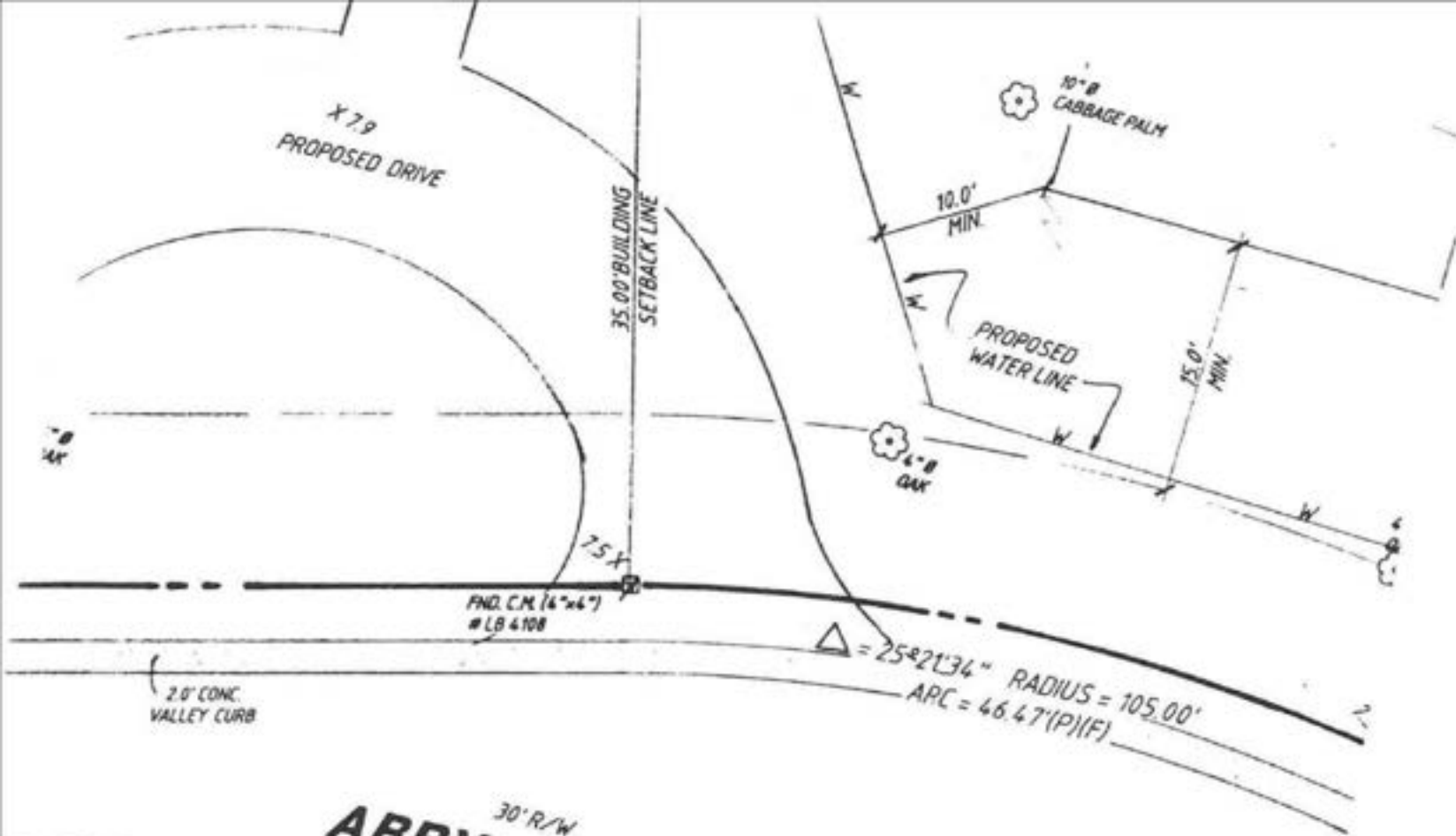
Plans approved as submitted ✓ Plans approved, as marked "

Permit good for one year. Fee for renewal of expired permit is \$5.00

Signature of applicant [signature] AGENT Date submitted 5-21-01Approved by Building Inspector [signature] Date 5/23/01Approved by Building Commissioner " Date "Completed " Date " Checked by "

THE FOLLOWING TREES MAY BE REMOVED OR DESTROYED WITHOUT ~~OBTAINING A PERMIT~~ FEE. BRAZILIAN PEPPER, FLORIDA HOLLY TREE, AUSTRALIAN PINE AND STRANGLER FIG. FOR THE PURPOSE OF THIS PERMIT, A TREE IS DEFINED AS ANY SELF-SUPPORTING WOODY OR FIBROUS PERENNIAL PLANT WHICH HAS A MINIMUM HEIGHT OF TWELVE (12) FEET.

THE FOLLOWING TREES MUST BE REMOVED BEFORE CONSTRUCTION BEGINS: BRAZILIAN PEPPER, FLORIDA HOLLY TREE, AUSTRALIAN PINE AND MELALEUCA?



ABBY COURT
30' R/W

NOTE: SEE TR P0399
FOR FULL SITE SURVEY.

SYMBOLS:

SURVEYOR'S NOTE

1. - SURVEY OF DESCRIPTION AS FURNISHED
2. - LANDS SHOWN HEREON WERE NOT ABSI AND/OR RIGHTS-OF-WAY OF RECORD B

ROBERT M. WIENKE
Mayor

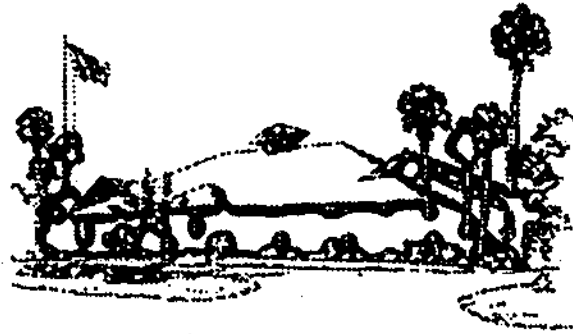
MARC S. TEPLITZ
Vice Mayor

DAWSON C. GLOVER, III
Commissioner

THOMAS P. BAUSCH
Commissioner

E. DANIEL MORRIS
Commissioner

TOWN OF SEWALL'S POINT



JOSEPH C. DORSKY
Town Manager

JOAN H. BARROW
Town Clerk

LARRY McCARTY
Chief of Police

EDWIN B. ARNOLD
Building Official

JOSE TORRES, JR.
Maintenance

NOTICE OF RESIDENTIAL POOL SAFETY REQUIREMENTS

To: All Pool/Spa Contractors
From: Edwin B. Arnold, Building Official *EA*
Subj: Preston de Ibern/McKenzie Merriam
Residential Swimming Pool Safety Act
Date: Sept. 1, 2000

Section 515.27 of the subject law provides in part as follows:

(1) In order to pass final inspection and receive a certificate of completion, a residential swimming pool must meet at least one of the following requirements relating to pool safety features:

(a) The pool must be isolated from access to a home by an enclosure that meets the pool barrier requirements of s. 515.29;

(b) The pool must be equipped with an approved safety pool cover;

(c) All doors and windows providing direct access from the home to the pool must be equipped with an exit alarm that has a minimum sound pressure rating of 85 dB A at 10 feet; or

(d) All doors and windows providing direct access from the home to the pool must be equipped with a self-closing, self-latching device with a release mechanism placed no lower than 54 inches from the floor.

The effective date of this statute is October 1, 2000. All pools completed on or after that date will be required to fully comply with the provisions of the statute. The statute also mandates specific information which must be furnished to buyers on entering into an agreement to build a residential swimming pool. Evidence of compliance with these requirements will be required as part of the building permit application submittal. Please contact me if you have any questions.

*** SUBMIT DETAILS OF PROPOSED METHOD
OF COMPLIANCE FOR REVIEW PRIOR
TO INSTALLATION. FIRST**



One South Sewall's Point Road, Sewall's Point, Florida 34996
Town Hall (561) 287-2455 • Fax (561) 220-4765 • E-Mail: clerk@sewallspoint.org
Police Department (561) 781-3378 • Fax (561) 288-7669 • E-Mail: police@sewallspoint.org

NOTICE OF REQUIREMENTS
RESIDENTIAL SWIMMING POOL, SPA AND HOT TUBS
SAFETY ACT
CHAPTER 515 FLORIDA STATUTES

In signing this affidavit, the property owner and commissioned pool contractor hereby affirm that the proposed new swimming pool, spa and/or hot permitted by building permit # _____ for construction or installation at 102 Abbie Court shall be provided with at least one of the following safety features installed and maintained in compliance with Chapter 515, Florida Statutes.

Initial selected safety feature(s)

[Signature]

The swimming pool, spa, or hot tub will be isolated from access to the home by an enclosure that meets the barrier requirements of Florida 515.29;

_____ The swimming pool, spa, or hot tub will be equipped with an approved safety pool cover that complies with ASTM F1346-91 (Standard Performance Specifications for Safety Covers for Swimming Pools, Spas and Hot Tubs);

_____ All doors and windows providing direct access from the home to the swimming pool, spa, or hot tub shall be equipped and maintained with an exit alarm that has a minimum sound pressure rating of 85 decibels at 10 feet;

_____ All doors providing direct access from the home to the swimming pool, spa, or hot tub shall be equipped and maintained with self-closing, self-latching devices with release mechanisms placed no lower than 54" above the floor or deck.

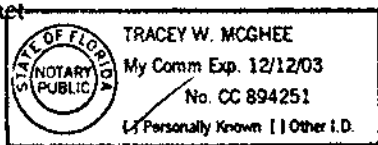
I understand that not having at least one of the above devices installed at the time of final inspection or thereafter will constitute a violation of Chapter 525, F.S. Furthermore, such violation will be considered a misdemeanor of the second degree, punishable by fines up to \$500.0 and/or up to 60 days in jail as established in Chapter 775, F.S.

[Signature]
Property owner's signature and date

JOE FOGLIA
Property owner's name (please print)

Notary as to Owner: *[Signature]*

poolact



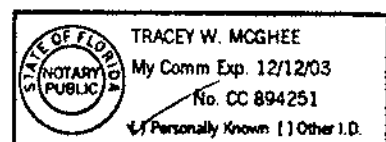
[Signature]
Contractor's signature and date

A+G Concrete Pools
ART ALLEN

Contractor name, business name, and license number (please print)

CPL 057200

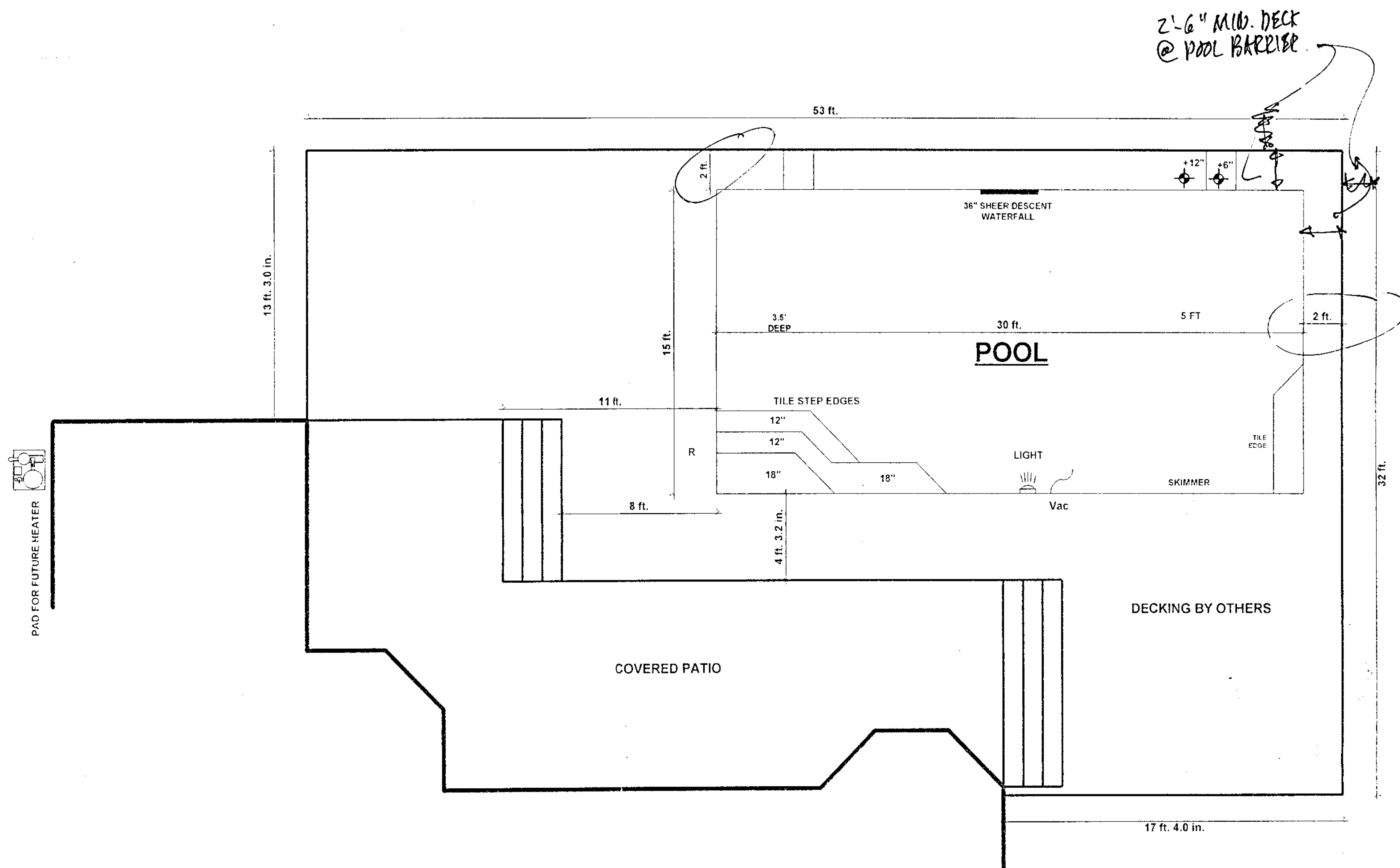
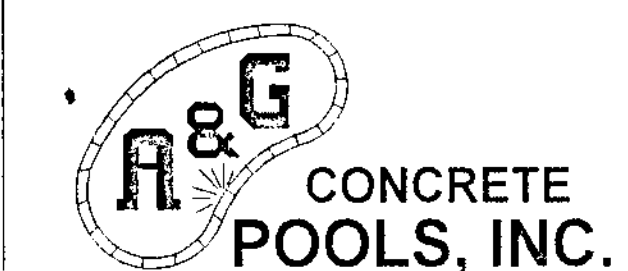
Notary as to Contractor: *[Signature]*



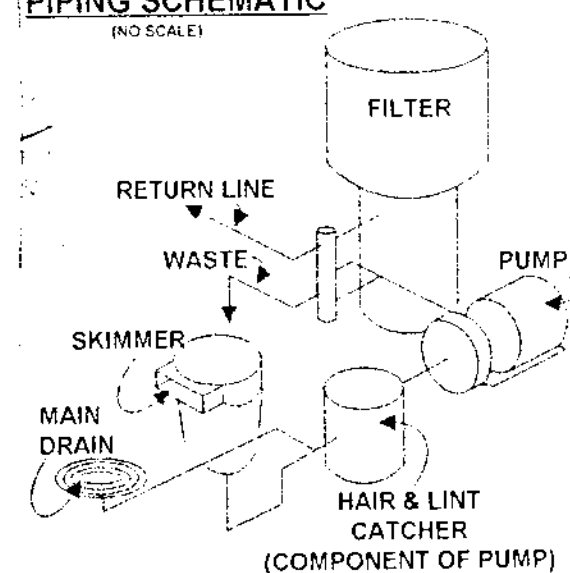
MAX. WIDTH	15'	MAX. LENGTH	30'
MIN. DEPTH	3'	MAX. DEPTH	5'
SHAPE	CUST ELEVATION T.B.D		
FILL FROM EXCAVATION	STOCKPILE		
TILE	TBD	INTERIOR FINISH	GEM
FILTER	C900	PUMP	HAYWARD
PUMP HP:	POOL 1HP	SPA	NA
	INCL		
LIGHTS	POOL 300 WATT	SPA	NA
TRNFORM:	POOL 300 WATT	SPA	NA
MAIN DRAIN:	POOL 1 INCL	SPA	NA
SKIMMER	1		
RETURNS:	POOL 3	SPA	NA
JETS:	POOL NA	SPA	NA
VALVED VAC LINE	INCL	FILL LINE	NA
DISCHARGE LINE	TO GROUND		
CHLORINATOR	INLINE	VISION	NA
HANDRAIL	NA	LADDER	NA
DIVING BOARD	NA	SLIDE	NA
HEATER	NA	LP TANK	NA
VACUUM HEAD	INCL	POLE	INCL
VACUUM HOSE	INCL	BRUSH	INCL
LEAF SKIMMER	INCL	TEST KIT	INCL
TREE / STUMP REMOVAL	NA		
REMOVE EXISTING CONCRETE	NA		
PUMP POOL PATIO	NA		
SCREEN ENCLOSURE	NA		
ELECTRICAL	NA		
POOL PATIO	NA		
EXISTING PATIO	NA		
DECK DRAIN	NA		
GALLON CAPACITY-POOL			
GALLON CAPACITY-SPA			
TURN-OVER RATE	6 hrs		
TERMITE TREAT	NA	BARRIER	NA

[illegible]

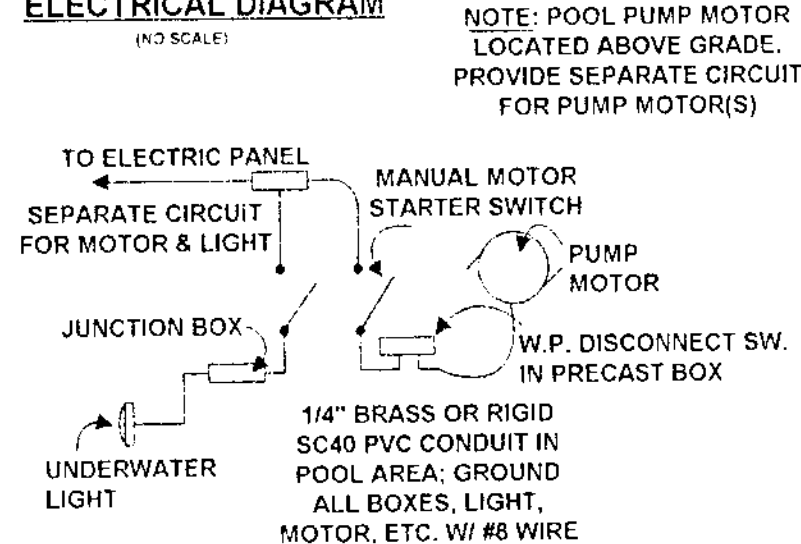
JOB NAME: DR. CUSHING
ADDRESS: LOT 22 SEWALL'S MADOW
SEWALL'S POINT
PERMIT NO.:
DATE DRAWN: APRIL 17, 2001
DRAWN BY: F.DELROCCO
DESIGNER APPROVAL:
CUSTOMER APPROVAL:
SCALE: 1/4" = 1'



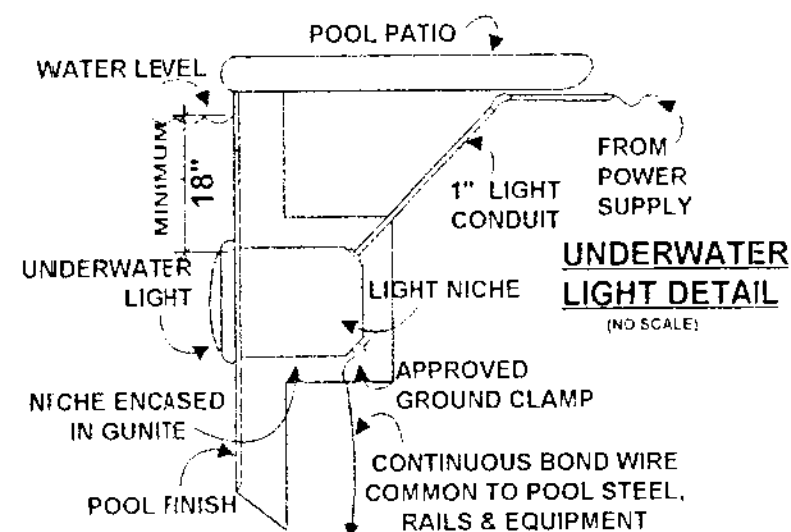
PIPING SCHEMATIC



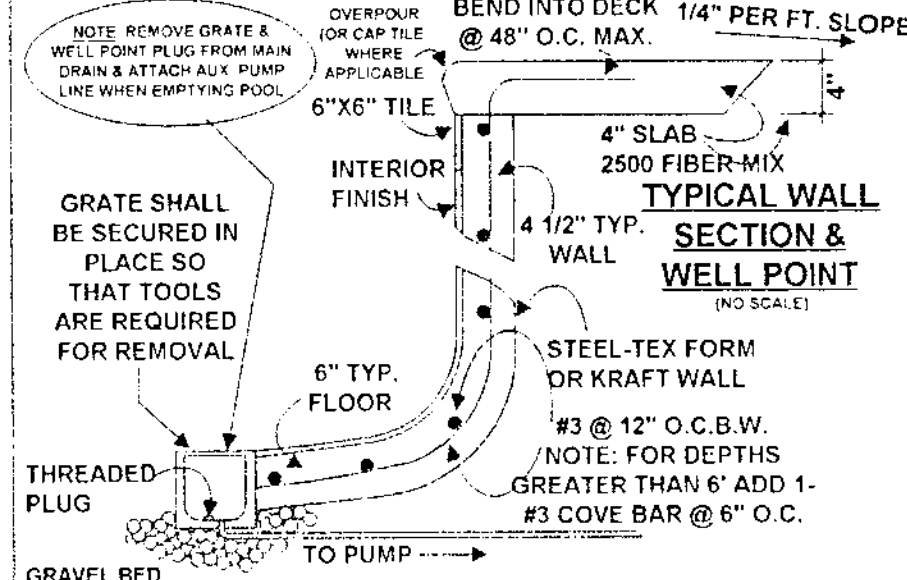
ELECTRICAL DIAGRAM



**NOTE: POOL PUMP MOTOR
LOCATED ABOVE GRADE.
PROVIDE SEPARATE CIRCUIT
FOR PUMP MOTOR(S)**



UNDERWATER
LIGHT DETAIL



TYPICAL SECTION THRU POOL

(NO SCALE)
ALL SHEETS STRUCTURE ONLY
WALTER MARSHALL PE40605
11403 N 172 PL, APT. FL 83478
(561) 740-1400

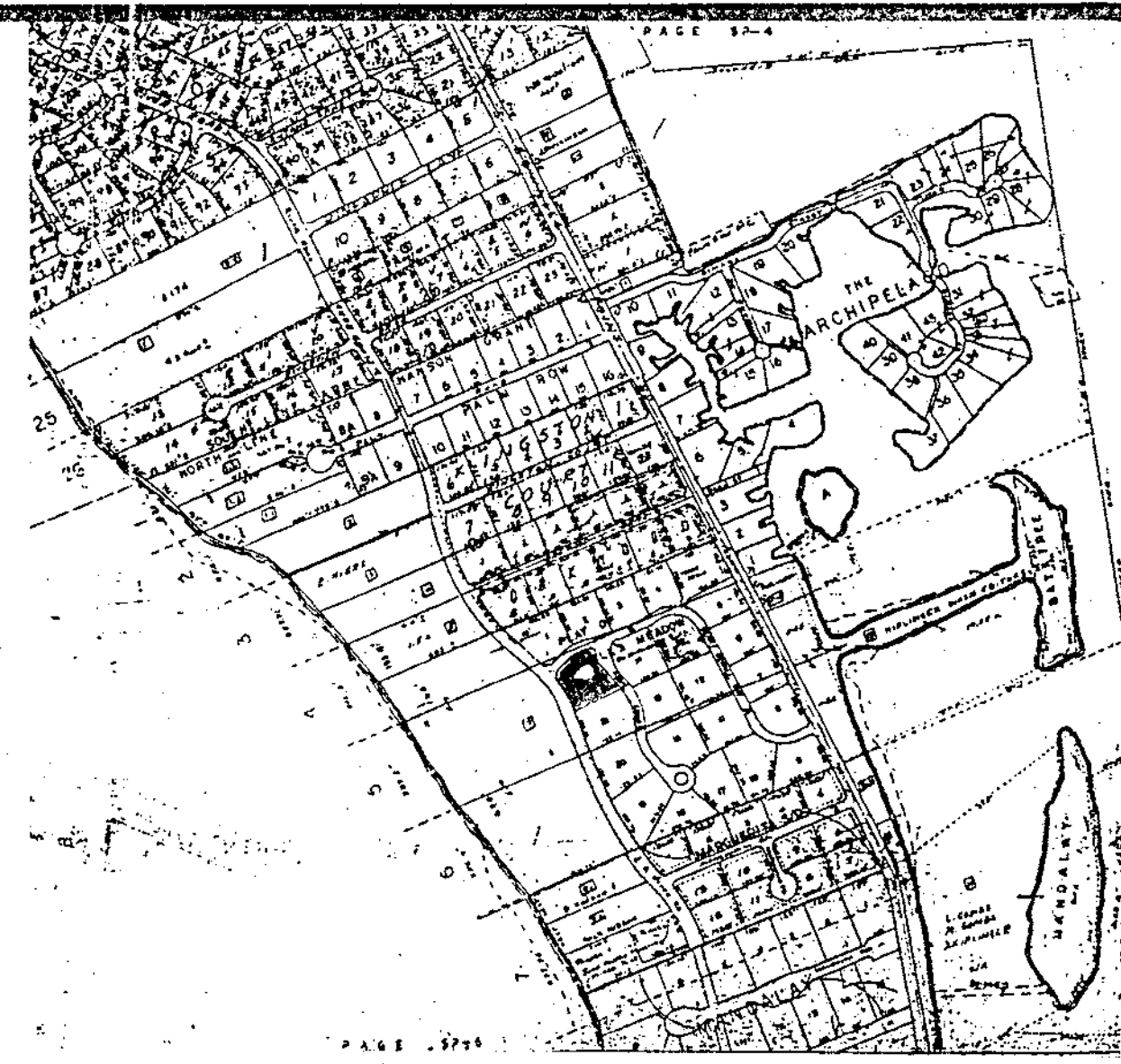
REVISIONS	BY
ADD SITE DATA 01/13/2001	W.G.H.
TIE-IN 02/10/2001	W.G.H.
FORM BOARD TIE-IN 02/24/2001	W.G.H.
SLAB VERIFICATION 03/05/2001	W.G.H.
REVISED SEPTIC 03/11/2001	W.G.H.

PREPARED FOR:
FOGLIA CONTRACTING CORPORATION
 BOUNDARY SURVEY

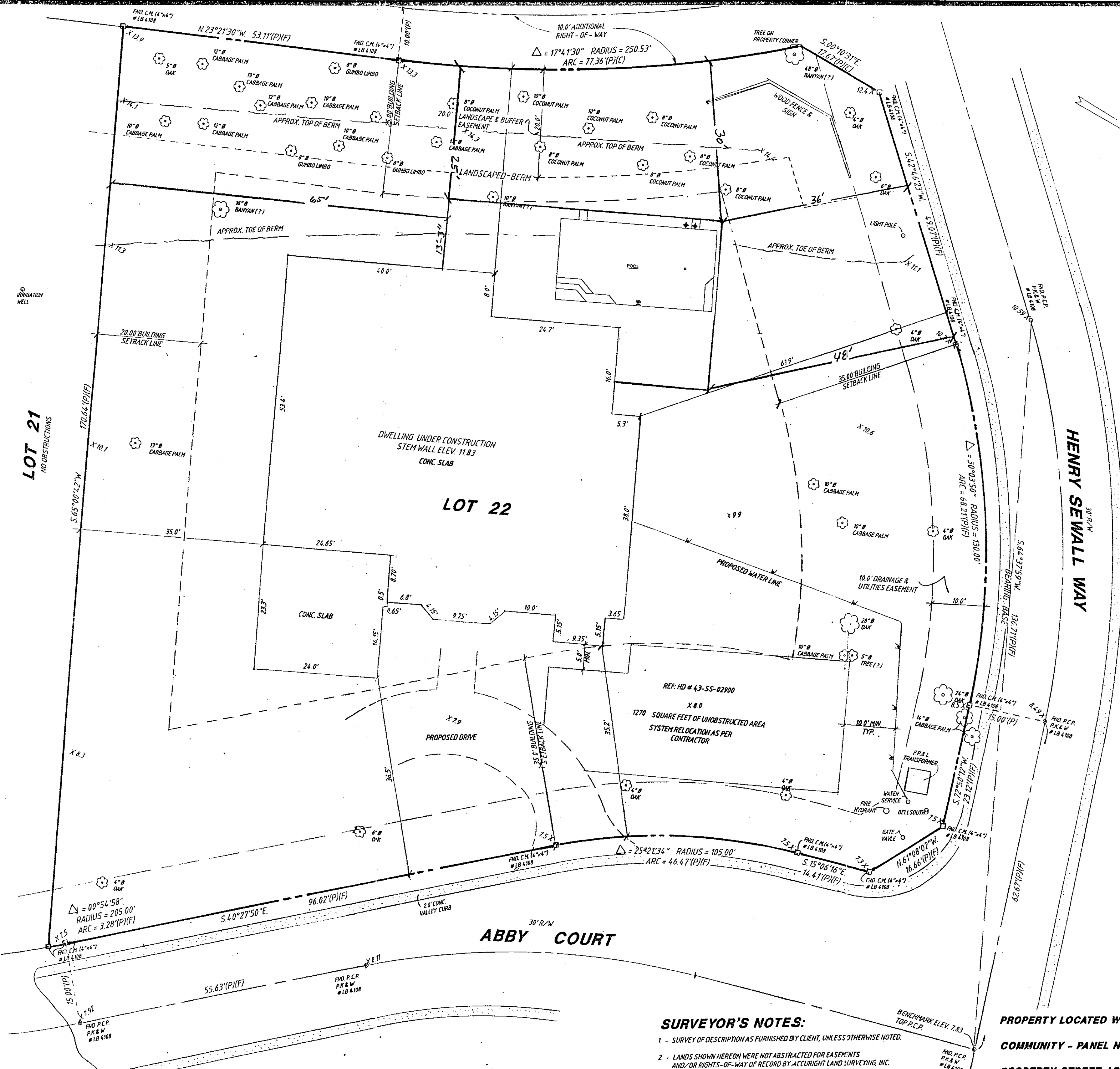
ACCURIGHT LAND SURVEYING, INC.
 OFFICE PHONE # (561) 286-7694
 OFFICE FAX # (561) 220-7993

1501 DECKER AVENUE, SUITE 419-D
 STUART, FL 34994

DRAWN W.G.H.
CHECKED E.R.S.
DATE (FIELD) 10/1/2000
SCALE 1" = 10.00'
JOB NO. 690-06-010
SHEET 1
OF ONE SHEETS



LOCATION MAP



HENRY SEWALL WAY

ABBY COURT

LEGAL DESCRIPTION
 KNOWN AS LOT 22, "SEWALL'S MEADOW", ACCORDING TO THE PLAT THEREOF, AS RECORDED IN PLAT BOOK 14, PAGE 32, PUBLIC RECORDS OF MARTIN COUNTY, FLORIDA.

THE GENERAL SLOPE OF THE PROPERTY, RECORDED EASEMENT(S) FROM THE RECORD PLAT, FILLED AREA, AND DRAINAGE FEATURES ARE AS SHOWN.

FORM BOARD
 & FIELD SURVEY
 REQUIRED

5/4/01 TOWN OF SEWALL'S POINT
 REVIEW: [Signature]

FILE TOWN COPY
 102 ABBIE CT.

PN 5373

SITE DATA
 TOTAL LOT SIZE - 23,130.3 S.F. - 100%
 IMPERVIOUS AREA BREAKDOWN
 A/C, GARAGE, ENTRY & PATIO - 4,470 S.F. - 17.8%
 POOL AREA, WALKS & PATIO - 1,515 S.F. - 6%
 DRIVEWAY - 2615 S.F. - 10.4%
 TOTAL IMPERVIOUS AREA - 8,600 S.F. - 34.2%

SURVEYOR'S NOTES:

1. - SURVEY OF DESCRIPTION AS FURNISHED BY CLIENT, UNLESS OTHERWISE NOTED.
2. - LANDS SHOWN HEREON WERE NOT ABSTRACTED FOR EASEMENTS AND/OR RIGHTS-OF-WAY OF RECORD BY ACCURIGHT LAND SURVEYING, INC.
3. - ALL BEARINGS ARE REFERENCED TO THE INSTRUMENT OF RECORD AS SHOWN HEREON, UNLESS OTHERWISE NOTED.
4. - ELEVATIONS SHOWN HEREON ARE RELATIVE TO NATIONAL GEODETIC VERTICAL DATUM OF 1929, UNLESS OTHERWISE NOTED. SEE ABOVE FOR REFERENCE BENCH MARK IF APPLICABLE.
5. - THERE ARE NO ABOVE GROUND ENCROACHMENTS, UNLESS OTHERWISE NOTED.

PROPERTY LOCATED WITHIN FLOOD ZONE: "C"

COMMUNITY - PANEL NUMBER: 120164 0002 D

PROPERTY STREET ADDRESS: 102 ABBIE COURT
STUART, FL 34996

CERTIFIED TO: FOGLIA CONTRACTING CORP., A FLORIDA CORPORATION;
 MCCARTHY, SUMMERS, BOBKO, McKEY, WOOD & SAWYER, P.A.;
 ATTORNEY'S TITLE INSURANCE FUND, INC.;
 FIDELITY FEDERAL SAVINGS BANK OF FLORIDA;
 TOWN OF SEWALL'S POINT

SURVEYOR'S CERTIFICATE

WE HEREBY CERTIFY THAT THE BOUNDARY SURVEY AS SHOWN HEREON IS TRUE AND CORRECT TO THE BEST OF OUR KNOWLEDGE AND BELIEF AS SURVEYED UNDER OUR DIRECTION. WE FURTHER CERTIFY THAT IT MEETS THE MINIMUM TECHNICAL STANDARDS UNDER RULE 61G17 - 6 FLORIDA ADMINISTRATIVE CODE, PURSUANT TO CHAPTER 472.027 FLORIDA STATUTES.

NOT VALID WITHOUT THE SIGNATURE AND THE ORIGINAL RAISED SEAL OF A FLORIDA LICENSED SURVEYOR AND MAPPER

ACCURIGHT LAND SURVEYING, INC.
 EARLE R. STARKEY - PROFESSIONAL SURVEYOR & MAPPER
 REGISTRATION NO. 4459 - STATE OF FLORIDA

LEGEND:
 (P) - DENOTES DISTANCE, ANGLE OR BEARING BY FURNISHED DESCRIPTION
 (F) - DENOTES MEASURED DISTANCE, ANGLE OR BEARING
 (C) - CALCULATED DISTANCE, ANGLE OR BEARING
 CONC. - CONCRETE
 FND - FOUND
 CM - CONCRETE MONUMENT
 COVD - COVERED
 ELEV - ELEVATION
 C.B.S. - CONCRETE BLOCK STRUCTURE
 R/W - RIGHT OF WAY
 I.B. - IRON BAR

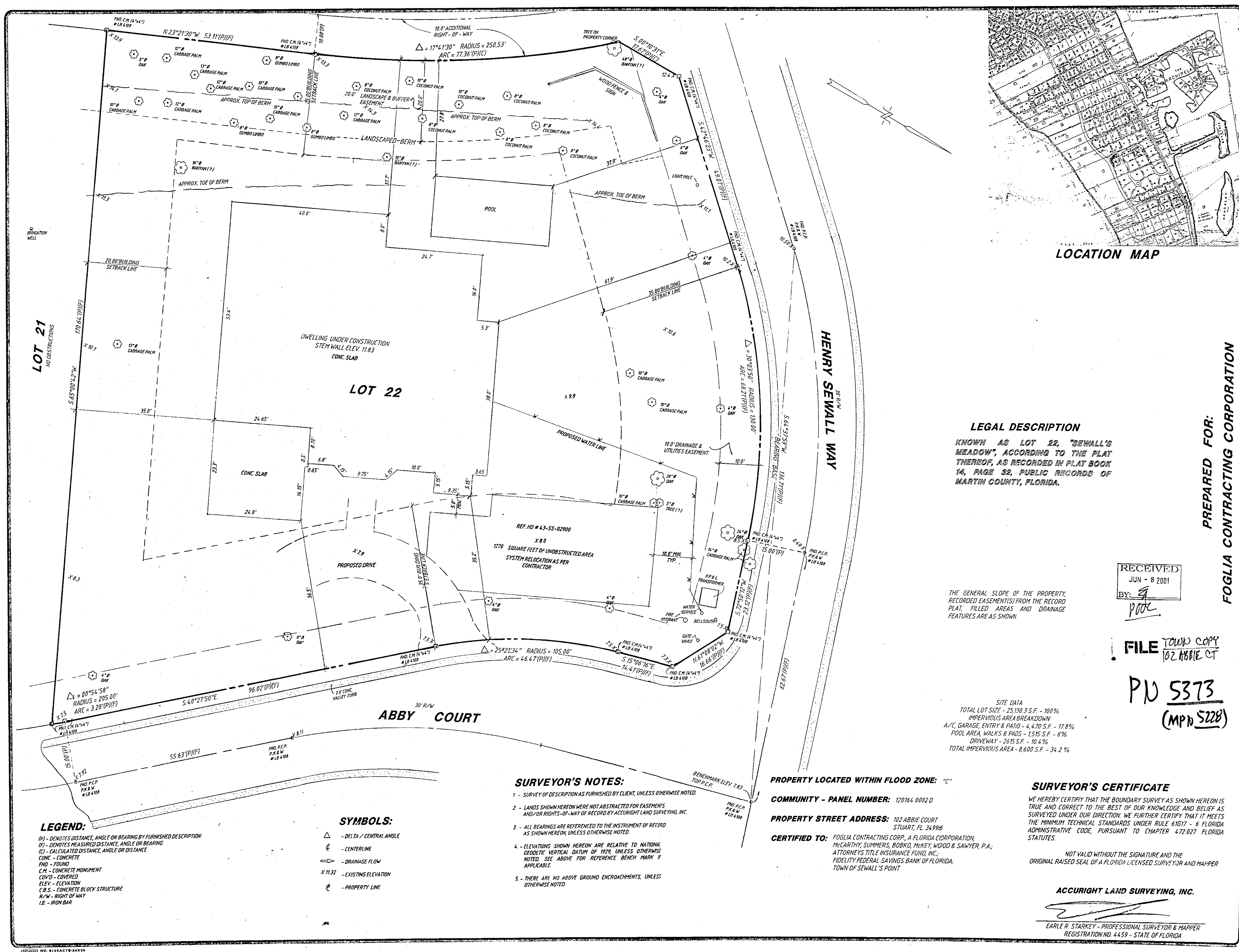
SYMBOLS:
 Δ - DELTA / CENTRAL ANGLE
 C - CENTERLINE
 D - DRAINAGE FLOW
 X 11.32 - EXISTING ELEVATION
 P - PROPERTY LINE

102

102

102

102



REVISIONS	BY
ADD SITE DATA 01/13/2001	W.G.H.
TIE-IN 02/10/2001	W.G.H.
FORWARD BOARD TIE-IN 02/24/2001	W.G.H.
SLAB VERIFICATION 03/06/2001	W.G.H.
REVISED SEPTIC 03/11/2001	W.G.H.
POOL TIE-IN 05/01/2001	W.G.H.

1501 DECKER AVENUE, SUITE 419-D
STUART, FL 34994
OFFICE PHONE # (561) 286-7694
OFFICE FAX # (561) 220-7993

FOGLIA CONTRACTING CORPORATION
BOUNDARY SURVEY

ACCURIGHT LAND SURVEYING, INC.

DRAWN W.G.H.
CHECKED E.R.S.
DATE (FIELD) 10/1/2000
SCALE 1" = 10.00'
JOB NO. 690-06-010
SHEET 1
OF ONE SHEETS

LOCATION MAP

LEGAL DESCRIPTION

KNOWN AS LOT 22, "SEWALL'S MEADOW", ACCORDING TO THE PLAT THEREOF, AS RECORDED IN PLAT BOOK 14, PAGE 32, PUBLIC RECORDS OF MARTIN COUNTY, FLORIDA.

RECEIVED
JUN - 8 2001
BY: [Signature]
POOL

FILE TOWN COPY
102 ABBIE CT

PN 5373
(MPD 5228)

THE GENERAL SLOPE OF THE PROPERTY, RECORDED EASEMENT(S) FROM THE RECORD PLAT, FILLED AREAS AND DRAINAGE FEATURES ARE AS SHOWN.

SITE DATA
TOTAL LOT SIZE - 25,130.3 S.F. - 100%
IMPERVIOUS AREA BREAKDOWN
A/C, GARAGE, ENTRY & PATIO - 4,470 S.F. - 17.8%
POOL AREA, WALKS & PATIOS - 1515 S.F. - 6%
DRIVEWAY - 2615 S.F. - 10.4%
TOTAL IMPERVIOUS AREA - 8,600 S.F. - 34.2%

SURVEYOR'S NOTES:

1. SURVEY OF DESCRIPTION AS FURNISHED BY CLIENT, UNLESS OTHERWISE NOTED.
2. LANDS SHOWN HEREON WERE NOT ABSTRACTED FOR EASEMENTS AND/OR RIGHTS-OF-WAY OF RECORD BY ACCURIGHT LAND SURVEYING, INC.
3. ALL BEARINGS ARE REFERENCED TO THE INSTRUMENT OF RECORD AS SHOWN HEREON, UNLESS OTHERWISE NOTED.
4. ELEVATIONS SHOWN HEREON ARE RELATIVE TO NATIONAL GEODETIC VERTICAL DATUM OF 1929, UNLESS OTHERWISE NOTED. SEE ABOVE FOR REFERENCE BENCHMARK IF APPLICABLE.
5. THERE ARE NO ABOVE GROUND ENCROACHMENTS, UNLESS OTHERWISE NOTED.

SYMBOLS:

- Δ - DELTA / CENTRAL ANGLE
- ⊕ - CENTERLINE
- - DRAINAGE FLOW
- X 11.32 - EXISTING ELEVATION
- ⌂ - PROPERTY LINE

LEGEND:

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- (F) - DENOTES MEASURED DISTANCE, ANGLE OR BEARING
- (C) - CALCULATED DISTANCE, ANGLE OR BEARING
- CONE - CONCRETE
- FND - FOUND
- C.M. - CONCRETE MONUMENT
- COVD - COVERED
- ELEV - ELEVATION
- C.B.S. - CONCRETE BLOCK STRUCTURE
- R/W - RIGHT OF WAY
- LB - IRON BAR

PROPERTY LOCATED WITHIN FLOOD ZONE: "C"

COMMUNITY - PANEL NUMBER: 120164.0002 D

PROPERTY STREET ADDRESS: 102 ABBIE COURT
STUART, FL 34996

CERTIFIED TO: FOGLIA CONTRACTING CORP., A FLORIDA CORPORATION;
McCARTHY, SUMMERS, BOBKO, McKEY, WOOD & SAWYER, P.A.;
ATTORNEYS TITLE INSURANCE FUND, INC.;
FIDELITY FEDERAL SAVINGS BANK OF FLORIDA;
TOWN OF SEWALL'S POINT

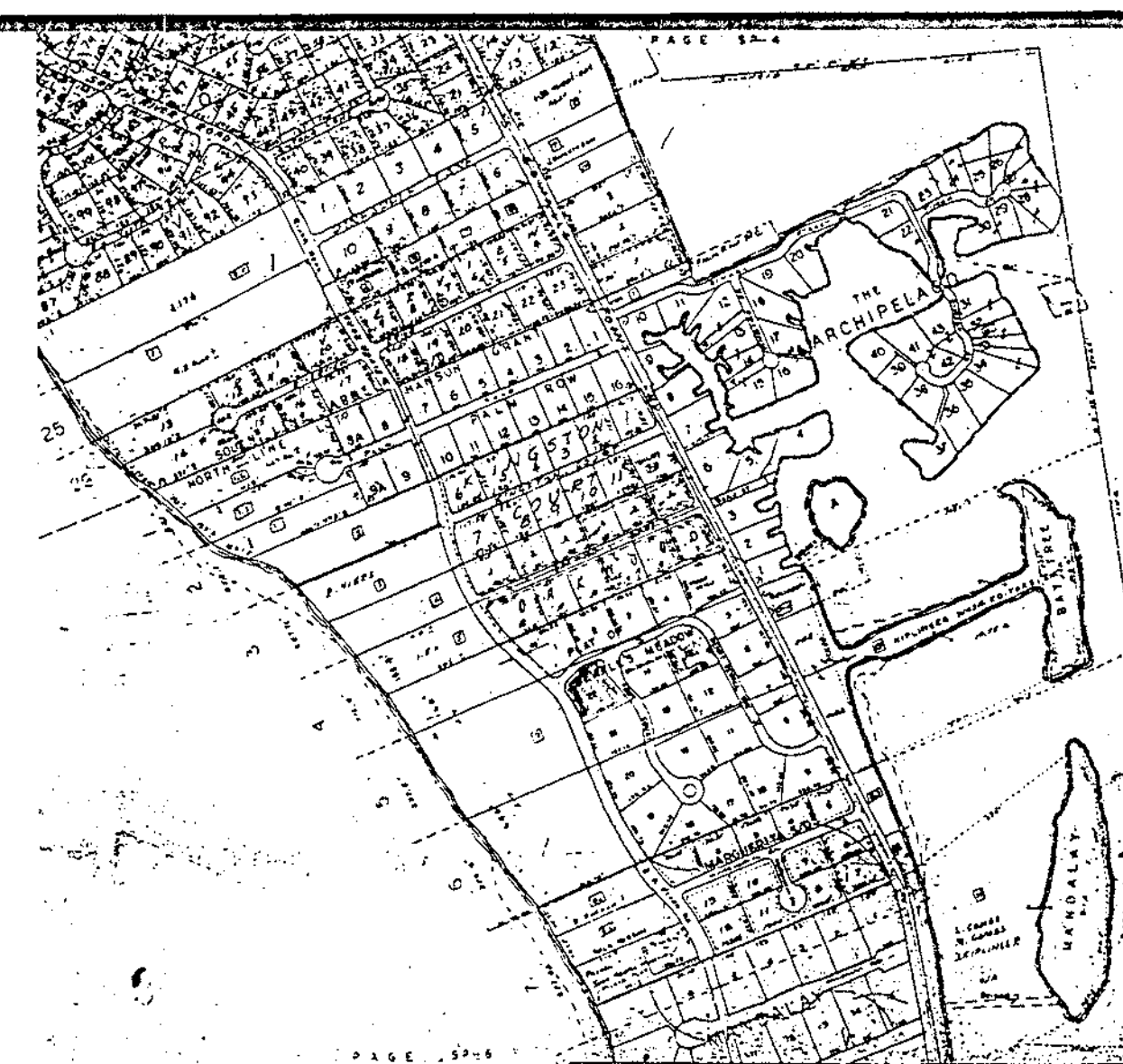
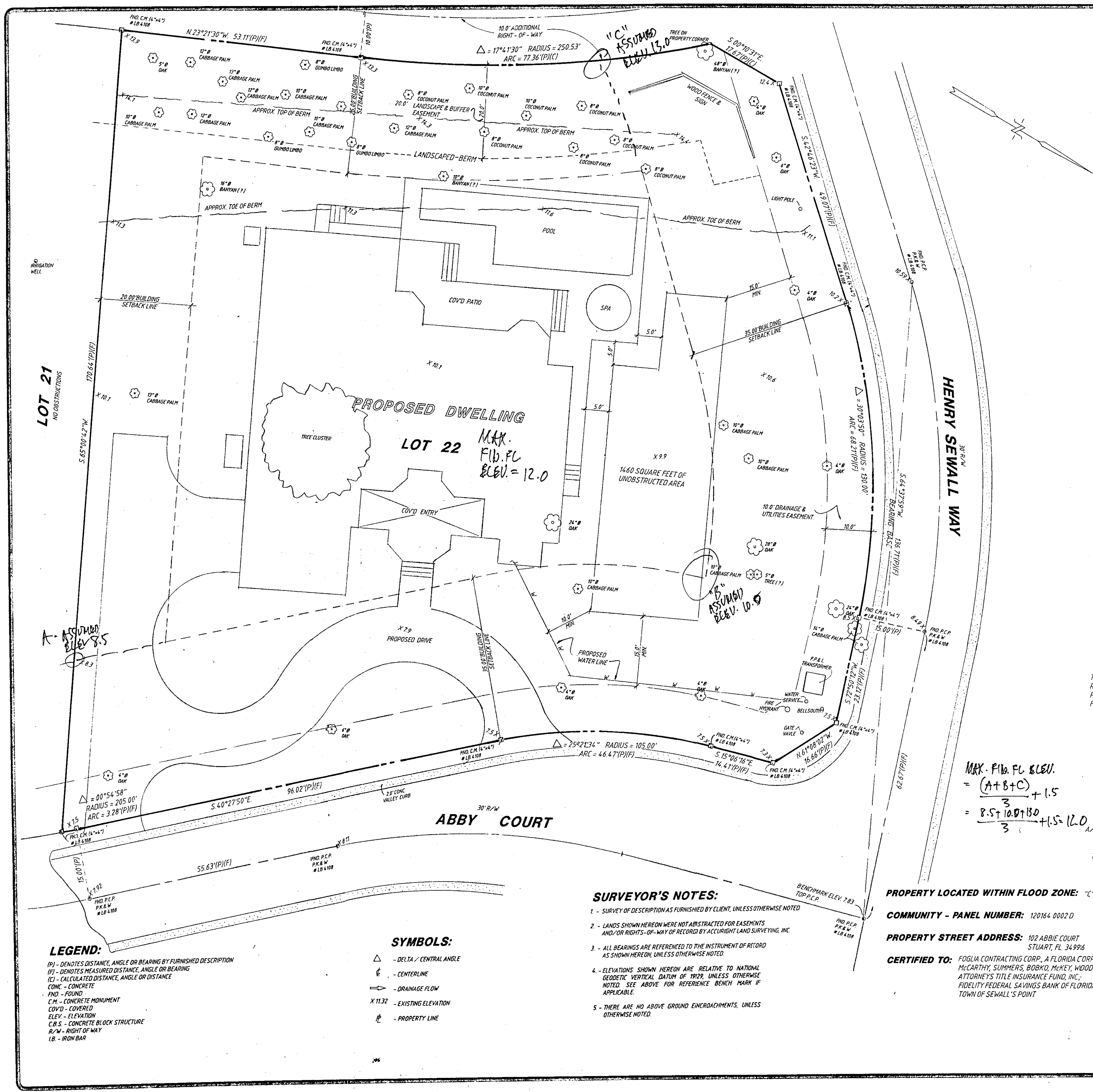
SURVEYOR'S CERTIFICATE

WE HEREBY CERTIFY THAT THE BOUNDARY SURVEY AS SHOWN HEREON IS TRUE AND CORRECT TO THE BEST OF OUR KNOWLEDGE AND BELIEF AS SURVEYED UNDER OUR DIRECTION. WE FURTHER CERTIFY THAT IT MEETS THE MINIMUM TECHNICAL STANDARDS UNDER RULE 61G17 - 6 FLORIDA ADMINISTRATIVE CODE, PURSUANT TO CHAPTER 472.027 FLORIDA STATUTES.

NOT VALID WITHOUT THE SIGNATURE AND THE ORIGINAL RAISED SEAL OF A FLORIDA LICENSED SURVEYOR AND MAPPER

ACCURIGHT LAND SURVEYING, INC.

EARLE R. STARKEY - PROFESSIONAL SURVEYOR & MAPPER
REGISTRATION NO. 4459 - STATE OF FLORIDA



LOCATION MAP

LEGAL DESCRIPTION

KNOWN AS LOT 22, "SEWALL'S MEADOW", ACCORDING TO THE PLAT THEREOF, AS RECORDED IN PLAT BOOK 16, PAGE 52, PUBLIC RECORDS OF MARTIN COUNTY, FLORIDA.

PREPARED FOR:
FOGLIA CONTRACTING CORPORATION
BOUNDARY SURVEY

THE GENERAL SLOPE OF THE PROPERTY, RECORDED EASEMENT(S) FROM THE RECORD PLAT, FILLED AREAS AND DRAINAGE FEATURES ARE AS SHOWN.

SITE PLAN / SURVEY

1/15/00 TOWN OF SEWALL'S POINT
REVIEW: [Signature]
BLDG. OFFICIAL

FILE TOWN COPY
102 ABBIE CT

PD 5228

$$\begin{aligned} \text{MAX. FID. FL. ELEV.} &= \frac{(A+B+C)}{3} + 1.5 \\ &= \frac{8.5 + 10.0 + 10.0}{3} + 1.5 = 12.0 \end{aligned}$$

SITE DATA
TOTAL LOT SIZE - 25,150.3 S.F. - 100%
IMPERVIOUS AREA BREAKDOWN
A/C, GARAGE, ENTRY & PATIO - 4,470 S.F. - 17.8%
POOL AREA, WALKS & PADS - 1,515 S.F. - 6%
DRIVEWAY - 2615 S.F. - 10.4%
TOTAL IMPERVIOUS AREA - 8,600 S.F. - 34.2%

SURVEYOR'S NOTES:

1. - SURVEY OF DESCRIPTION AS FURNISHED BY CLIENT, UNLESS OTHERWISE NOTED
2. - LANDS SHOWN HEREON WERE NOT ABSTRACTED FOR EASEMENTS AND/OR RIGHTS-OF-WAY OF RECORD BY ACCURIGHT LAND SURVEYING, INC.
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PROPERTY LOCATED WITHIN FLOOD ZONE: "C"

COMMUNITY - PANEL NUMBER: 120164 0002 D

PROPERTY STREET ADDRESS: 102 ABBIE COURT
STUART, FL 34996

CERTIFIED TO: FOGLIA CONTRACTING CORP., A FLORIDA CORPORATION;
MCARTHY, SUMMERS, BOBKO, MCKEY, WOOD & SAWYER, P.A.;
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NOT VALID WITHOUT THE SIGNATURE AND THE ORIGINAL RAISED SEAL OF A FLORIDA LICENSED SURVEYOR AND MAPPER

ACCURIGHT LAND SURVEYING, INC.

EARLE R. STARKER, PROFESSIONAL SURVEYOR & MAPPER
REGISTRATION NO. 2459 - STATE OF FLORIDA

LEGEND:

(P) - DENOTES DISTANCE, ANGLE OR BEARING BY FURNISHED DESCRIPTION
(F) - DENOTES MEASURED DISTANCE, ANGLE OR BEARING
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SYMBOLS:

Δ - DELTA / CENTRAL ANGLE
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— - DRAINAGE FLOW
X 11.32 - EXISTING ELEVATION
— - PROPERTY LINE

REVISIONS	BY
ADD SITE DATA 01/13/2001	W.G.H.

ACCURIGHT LAND SURVEYING, INC.

OFFICE PHONE # (561) 286 - 7694
OFFICE FAX # (561) 220 - 7993

150 LOCKER AVENUE, SUITE 419-D
STUART, FL 34994

RECEIVED
JAN 15 2001
BY: [Signature]

DRAWN W.G.H.
CHECKED E.R.S.
DATE (FIELD) 10/11/2000
SCALE 1" = 10.00'
JOB NO. 690-06-010
SHEET 1
OF ONE SHEETS

REVISIONS	BY
ADD SITE DATA 01/13/2001	W.G.H.
TIE-IN 02/10/2001	W.G.H.
FORM BOARD TIE-IN 02/24/2001	W.G.H.

OFFICE PHONE # (561) 286-7894
OFFICE FAX # (561) 220-7993

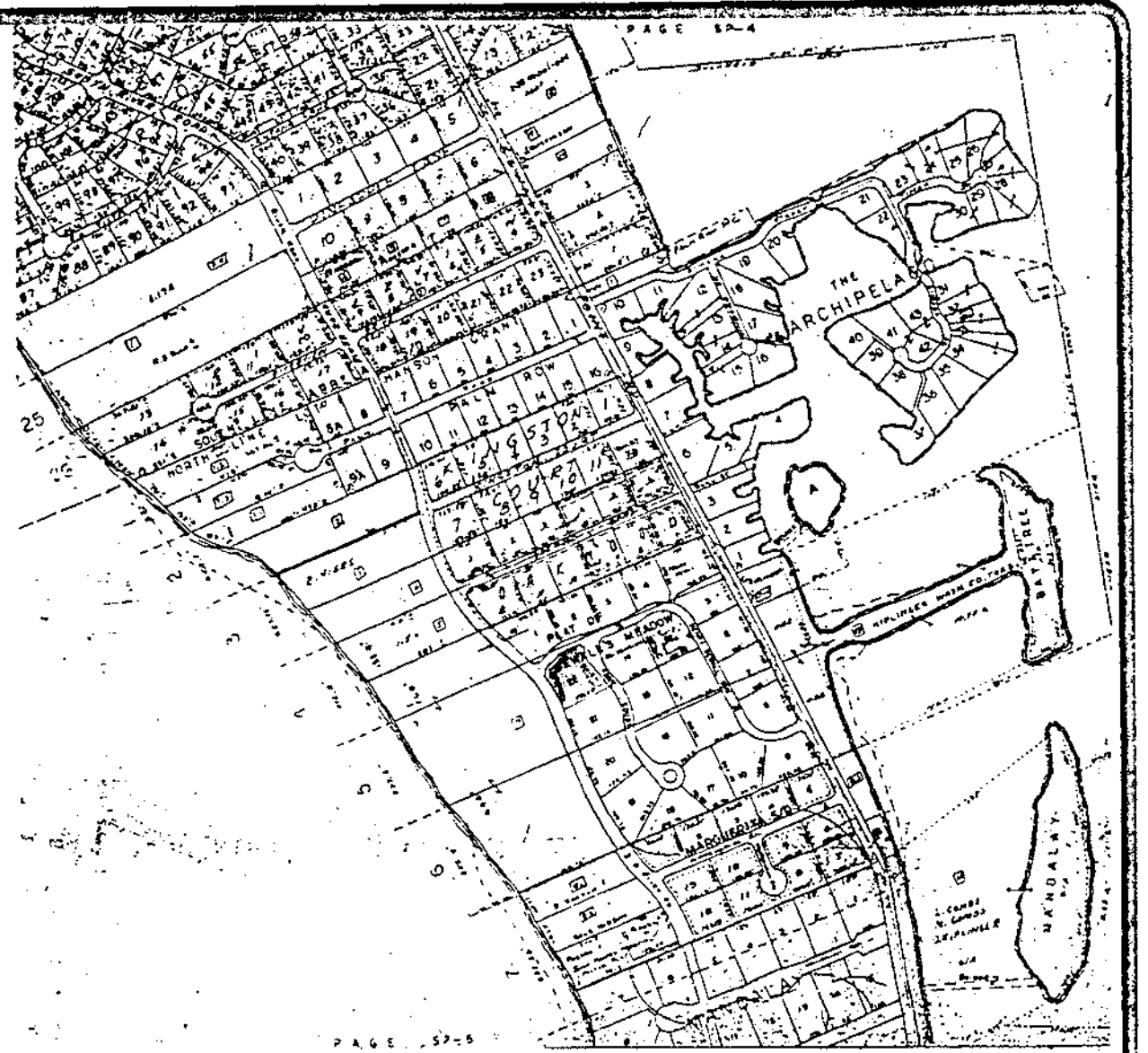


1501 DECKER AVENUE, SUITE 419-D
STUART, FL 34994

ACCURIGHT LAND SURVEYING, INC.

BOUNDARY SURVEY

PREPARED FOR:
FOGLIA CONTRACTING CORPORATION



LOCATION MAP

LEGAL DESCRIPTION

KNOWN AS LOT 22, "SEWALL'S MEADOW", ACCORDING TO THE PLAT THEREOF, AS RECORDED IN PLAT BOOK 14, PAGE 32, PUBLIC RECORDS OF MARTIN COUNTY, FLORIDA.

RECEIVED
FEB 26 2001

BY: *[Signature]*
STEMWELL (LAW) W/ GARAGE

FILE TOWN COPY
02 ABIE CT.

PN 5228

THE GENERAL SLOPE OF THE PROPERTY, RECORDED EASEMENT(S) FROM THE RECORD PLAT, FILLED AREAS AND DRAINAGE FEATURES ARE AS SHOWN.

SITE DATA
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DRIVEWAY - 2615 S.F. - 10.4%
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NOT VALID WITHOUT THE SIGNATURE AND THE ORIGINAL RAISED SEAL OF A FLORIDA LICENSED SURVEYOR AND MAPPER

ACCURIGHT LAND SURVEYING, INC.

EARLE R. SHARKEY, PROFESSIONAL SURVEYOR & MAPPER
REGISTRATION NO. 4468 - STATE OF FLORIDA

DRAWN W.G.H.
CHECKED E.R.S.
DATE (FIELD) 10/1/2000
SCALE 1" = 10.00'
JOB NO. 690-06-010'
SHEET 1
OF ONE SHEETS

PROPERTY LOCATED WITHIN FLOOD ZONE: "C"

COMMUNITY - PANEL NUMBER: 120164 0002 D

PROPERTY STREET ADDRESS: 102 ABBIE COURT
STUART, FL 34996

CERTIFIED TO: FOGLIA CONTRACTING CORP., A FLORIDA CORPORATION,
MC CARTHY, SUMMERS, BOBKO, McKEY, WOOD & SAWYER, P.A.;
ATTORNEYS TITLE INSURANCE FUND, INC.;
FIDELITY FEDERAL SAVINGS BANK OF FLORIDA;
TOWN OF SEWALL'S POINT

SURVEYOR'S NOTES:

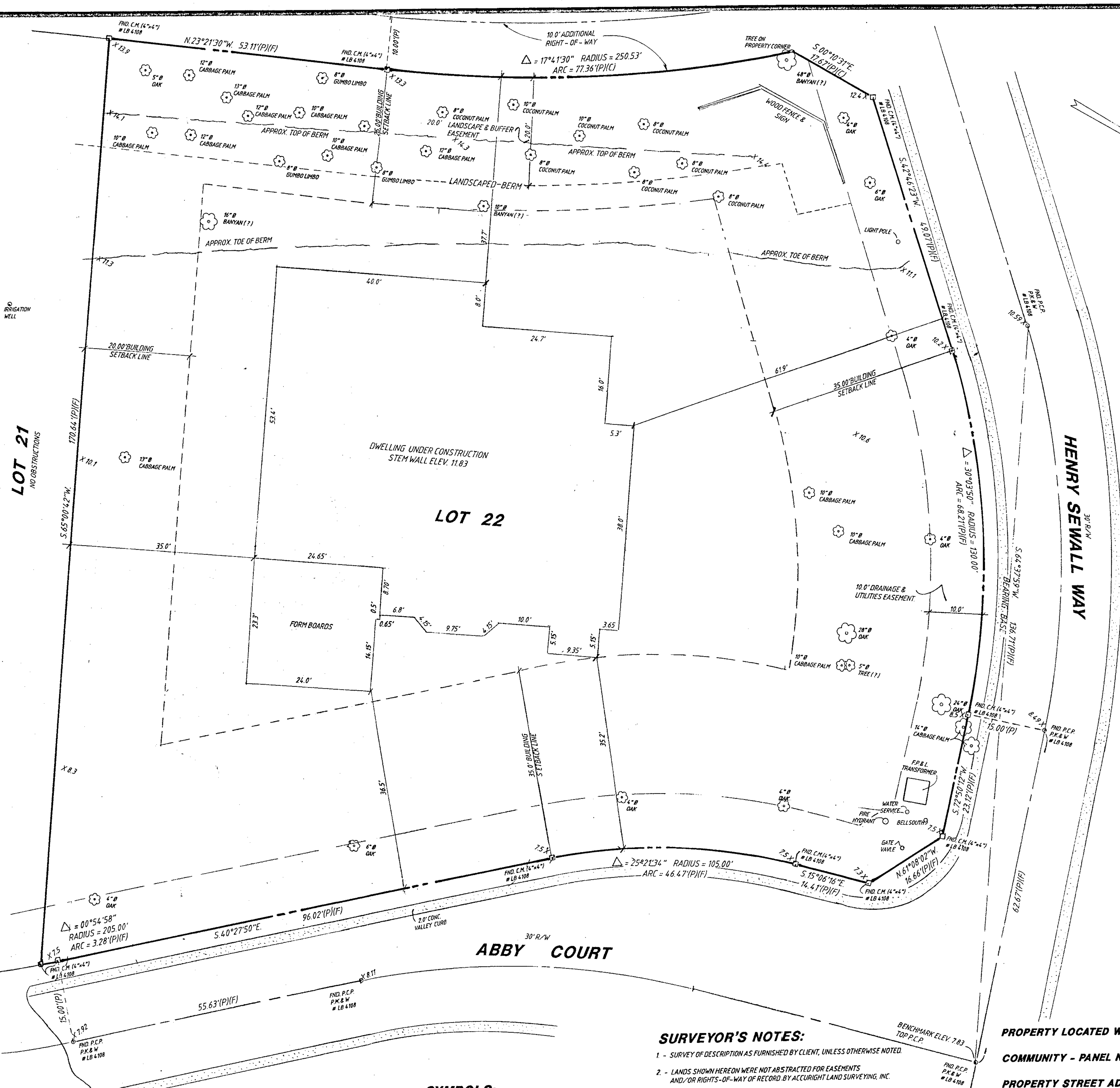
- 1 - SURVEY OF DESCRIPTION AS FURNISHED BY CLIENT, UNLESS OTHERWISE NOTED.
- 2 - LANDS SHOWN HEREON WERE NOT ABSTRACTED FOR EASEMENTS AND/OR RIGHTS-OF-WAY OF RECORD BY ACCURIGHT LAND SURVEYING, INC.
- 3 - ALL BEARINGS ARE REFERENCED TO THE INSTRUMENT OF RECORD AS SHOWN HEREON, UNLESS OTHERWISE NOTED.
- 4 - ELEVATIONS SHOWN HEREON ARE RELATIVE TO NATIONAL GEODETIC VERTICAL DATUM OF 1929, UNLESS OTHERWISE NOTED. SEE ABOVE FOR REFERENCE BENCH MARK IF APPLICABLE.
- 5 - THERE ARE NO ABOVE GROUND ENCROACHMENTS, UNLESS OTHERWISE NOTED.

SYMBOLS:

- Δ - DELTA / CENTRAL ANGLE
- ⊕ - CENTERLINE
- - DRAINAGE FLOW
- X 11.32 - EXISTING ELEVATION
- ℙ - PROPERTY LINE

LEGEND:

(P) - DENOTES DISTANCE, ANGLE OR BEARING BY FURNISHED DESCRIPTION
(F) - DENOTES MEASURED DISTANCE, ANGLE OR BEARING
(C) - CALCULATED DISTANCE, ANGLE OR BEARING
CONC. - CONCRETE
FND - FOUND
C.M. - CONCRETE MONUMENT
COVD - COVERED
ELEV - ELEVATION
C.B.S. - CONCRETE BLOCK STRUCTURE
R/W - RIGHT OF WAY
I.B. - IRON BAR

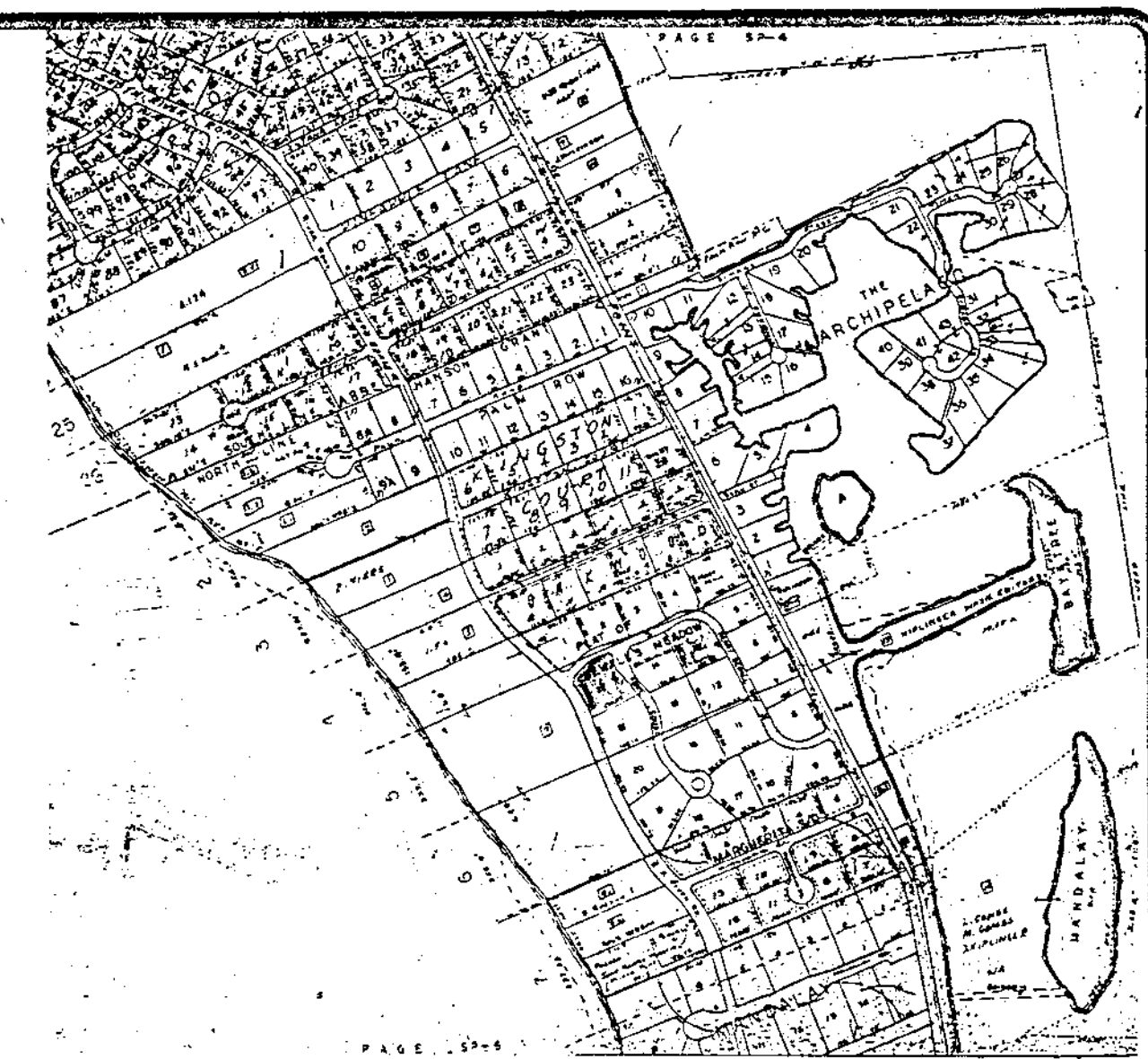
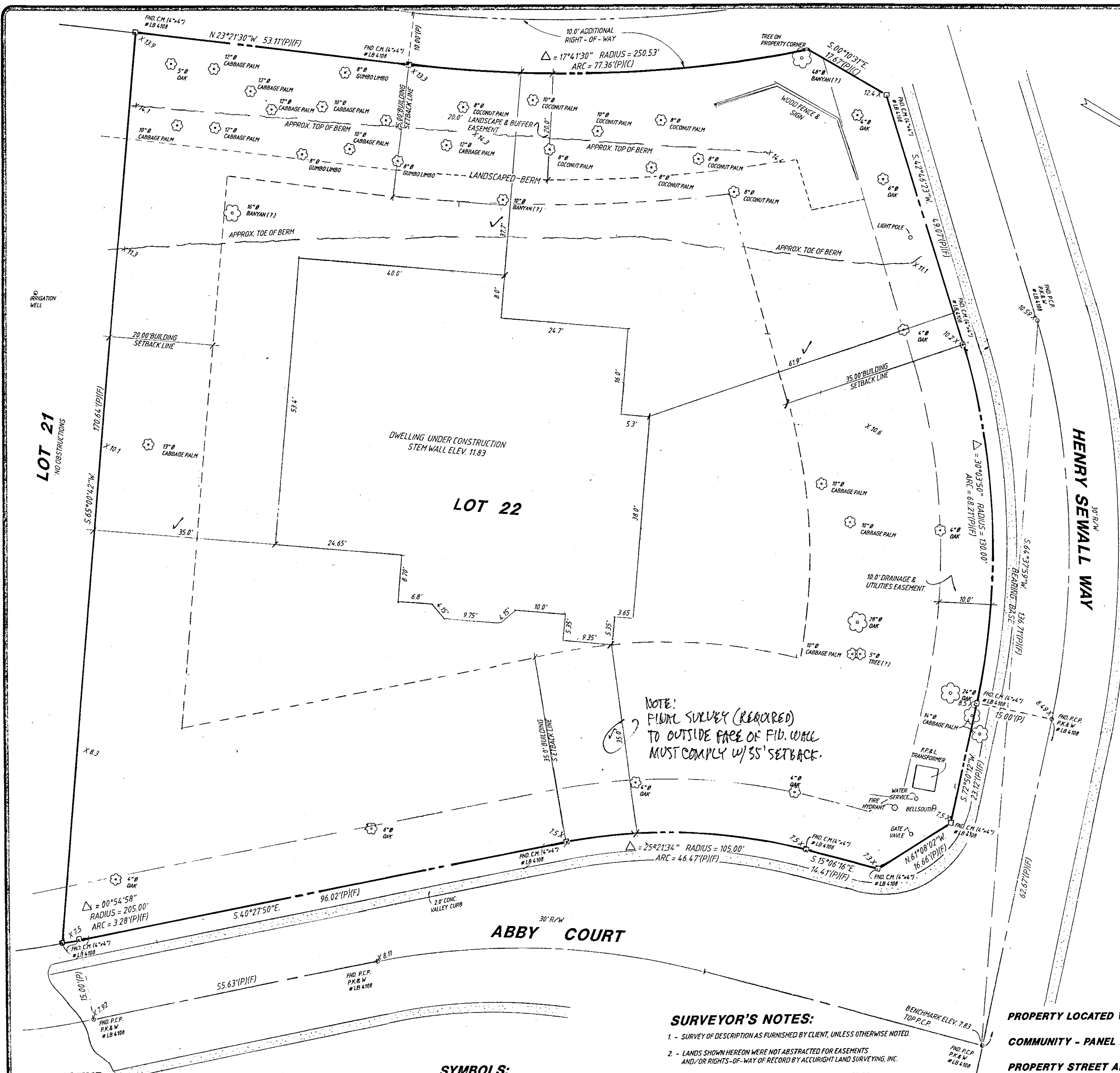


LOT 21
NO OBSTRUCTIONS

LOT 22

ABBY COURT

HENRY SEWALL WAY



LOCATION MAP

LEGAL DESCRIPTION

KNOWN AS LOT 22, "SEWALL'S MEADOW", ACCORDING TO THE PLAT THEREOF, AS RECORDED IN PLAT BOOK 16, PAGE 32, PUBLIC RECORDS OF MARTIN COUNTY, FLORIDA.

THE GENERAL SLOPE OF THE PROPERTY, RECORDED EASEMENT(S) FROM THE RECORD PLAT, FILLED AREAS AND DRAINAGE FEATURES ARE AS SHOWN.

RECEIVED
FEB 12 2001
BY: [Signature]

STEMWALL (FORMER) SURVEY

FILE TOWN COPY
LOZ ABIE OT

PN 5228

SITE DATA
TOTAL LOT SIZE - 25,130.3 S.F. - 100%
IMPERVIOUS AREA BREAKDOWN
A/C, GARAGE, ENTRY & PATIO - 4,470 S.F. - 17.8%
POOL AREA, WALKS & PADS - 1,515 S.F. - 6%
DRIVEWAY - 2615 S.F. - 10.4%
TOTAL IMPERVIOUS AREA - 8,600 S.F. - 34.2%

SURVEYOR'S NOTES:

1. SURVEY OF DESCRIPTION AS FURNISHED BY CLIENT, UNLESS OTHERWISE NOTED.
2. LANDS SHOWN HEREON WERE NOT ABSTRACTED FOR EASEMENTS AND/OR RIGHTS-OF-WAY OF RECORD BY ACCURIGHT LAND SURVEYING, INC.
3. ALL BEARINGS ARE REFERENCED TO THE INSTRUMENT OF RECORD AS SHOWN HEREON, UNLESS OTHERWISE NOTED.
4. ELEVATIONS SHOWN HEREON ARE RELATIVE TO NATIONAL GEODETIC VERTICAL DATUM OF 1929, UNLESS OTHERWISE NOTED. SEE ABOVE FOR REFERENCE BENCHMARK MARK IF APPLICABLE.
5. THERE ARE NO ABOVE GROUND ENCROACHMENTS, UNLESS OTHERWISE NOTED.

PROPERTY LOCATED WITHIN FLOOD ZONE: "C"

COMMUNITY - PANEL NUMBER: 120164 0002 D

PROPERTY STREET ADDRESS: 102 ABIE COURT
STUART, FL 34996

CERTIFIED TO: FOGLIA CONTRACTING CORP., A FLORIDA CORPORATION,
McCARHY, SUMMERS, BOBKO, McKEY, WOOD & SAWYER, P.A.;
ATTORNEYS TITLE INSURANCE FUND, INC.;
FIDELITY FEDERAL SAVINGS BANK OF FLORIDA;
TOWN OF SEWALL'S POINT

SURVEYOR'S CERTIFICATE

WE HEREBY CERTIFY THAT THE BOUNDARY SURVEY AS SHOWN HEREON IS TRUE AND CORRECT TO THE BEST OF OUR KNOWLEDGE AND BELIEF AS SURVEYED UNDER OUR DIRECTION. WE FURTHER CERTIFY THAT IT MEETS THE MINIMUM TECHNICAL STANDARDS UNDER RULE 61G17 - 6 FLORIDA ADMINISTRATIVE CODE, PURSUANT TO CHAPTER 472.027 FLORIDA STATUTES.

NOT VALID WITHOUT THE SIGNATURE AND THE ORIGINAL RAISED SEAL OF A FLORIDA LICENSED SURVEYOR AND MAPPER

ACCURIGHT LAND SURVEYING, INC.
[Signature]
EARLE R. STARKEY - PROFESSIONAL SURVEYOR & MAPPER
REGISTRATION NO. 4459 - STATE OF FLORIDA

LEGEND:

(P) - DENOTES DISTANCE, ANGLE OR BEARING BY FURNISHED DESCRIPTION
(F) - DENOTES MEASURED DISTANCE, ANGLE OR BEARING
(C) - CALCULATED DISTANCE, ANGLE OR BEARING
CONC. - CONCRETE
FND - FOUND
CM - CONCRETE MONUMENT
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I.B. - IRON BAR

SYMBOLS:

- Δ - DELTA / CENTRAL ANGLE
- - CENTERLINE
- - DRAINAGE FLOW
- X 11.32 - EXISTING ELEVATION
- - PROPERTY LINE

REVISIONS	BY
ADD SITE DATA 01/13/2001	W.G.H.
TIE-IN 02/10/2001	W.G.H.

PREPARED FOR:
FOGLIA CONTRACTING CORPORATION
BOUNDARY SURVEY

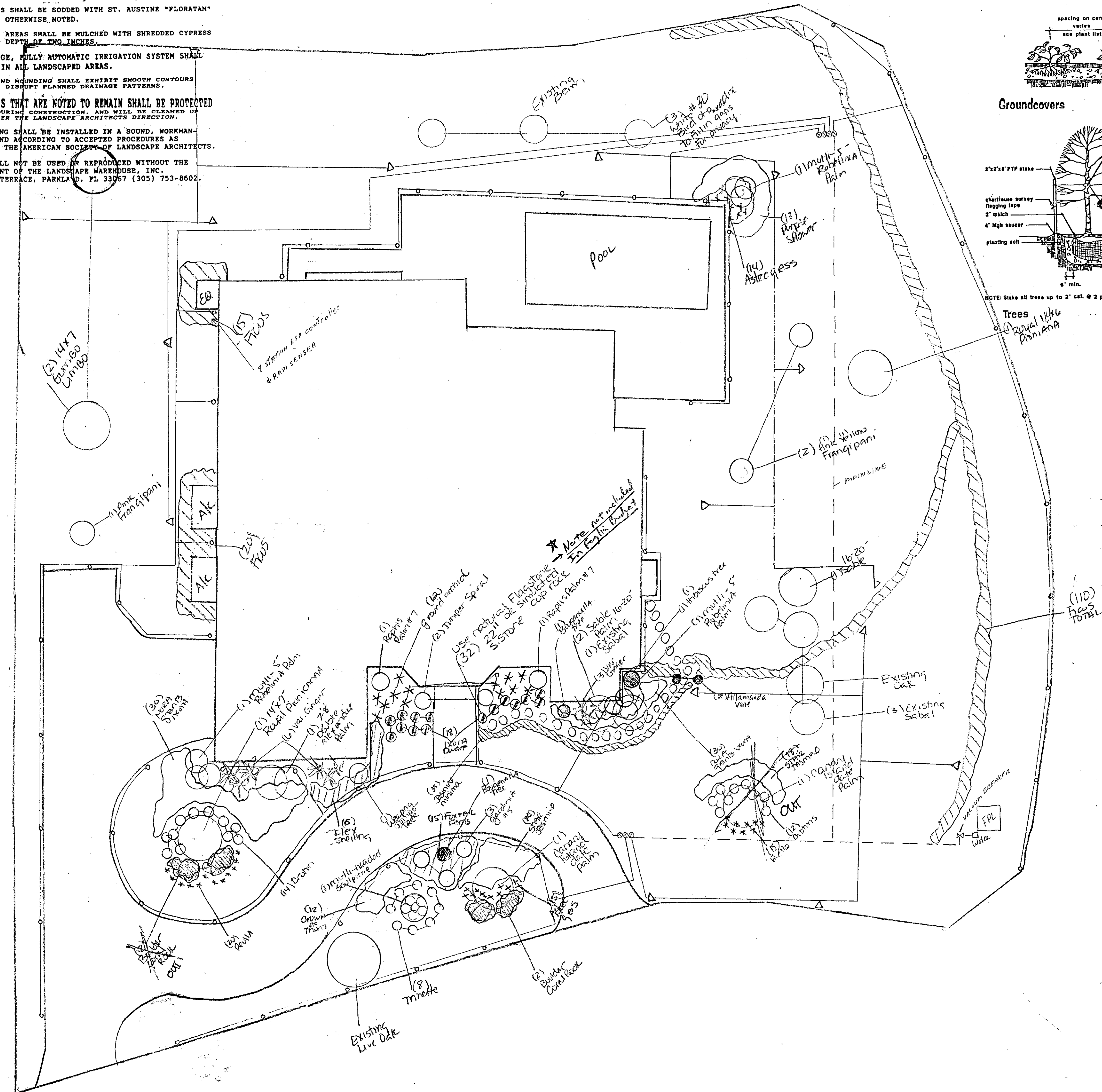
ACCURIGHT LAND SURVEYING, INC.

OFFICE PHONE # (561) 286 - 7894
OFFICE FAX # (561) 220 - 7993

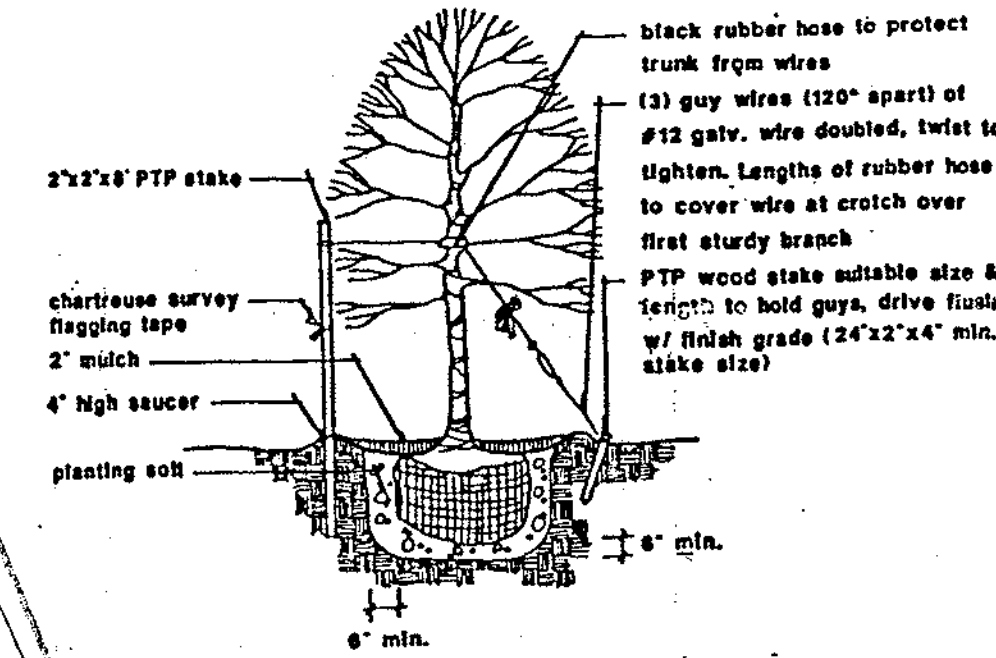
1501 DECKER AVENUE, SUITE 419-D
STUART, FL 34994

DRAWN W.G.H.	DATE (FIELD) 10/11/2000
CHECKED E.R.S.	SCALE 1" = 10.00'
JOB NO. 690-06-010	SHEET 1
OF ONE	SHEETS

- ALL MATERIALS SHALL BE GRADED FLORIDA #1, OR BETTER.
- ALL AREAS SHALL BE SODDED WITH ST. AUSTINE "FLORATAM" UNLESS OTHERWISE NOTED.
- PLANTINGS AREAS SHALL BE MULCHED WITH SHREDDED CYPRESS MULCH TO A DEPTH OF TWO INCHES.
- IRRIGATION COVERAGE, FULLY AUTOMATIC IRRIGATION SYSTEM SHALL BE INSTALLED IN ALL LANDSCAPED AREAS.
- ALL GRADING AND MOUNDING SHALL EXHIBIT SMOOTH CONTOURS AND SHALL NOT DISRUPT PLANNED DRAINAGE PATTERNS.
- EXISTING TREES THAT ARE NOTED TO REMAIN SHALL BE PROTECTED FROM DAMAGE DURING CONSTRUCTION, AND WILL BE CLEANED UP AND TRIMMED PER THE LANDSCAPE ARCHITECT'S DIRECTION.
- ALL LANDSCAPING SHALL BE INSTALLED IN A SOUND, WORKMAN-LIKE MANNER AND ACCORDING TO ACCEPTED PROCEDURES AS PRESCRIBED BY THE AMERICAN SOCIETY OF LANDSCAPE ARCHITECTS.
- THIS PLAN SHALL NOT BE USED OR REPRODUCED WITHOUT THE WRITTEN CONSENT OF THE LANDSCAPE WAREHOUSE, INC.
7250 N.W. 82 TERRACE, PARKLAND, FL 33067 (305) 753-8602.



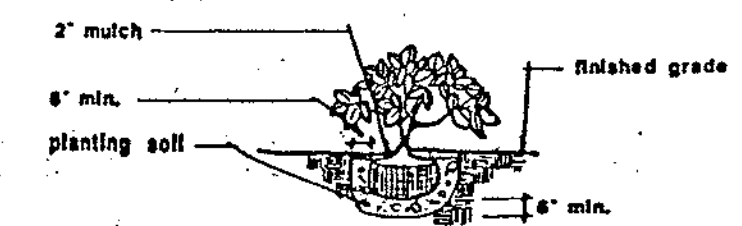
Groundcovers



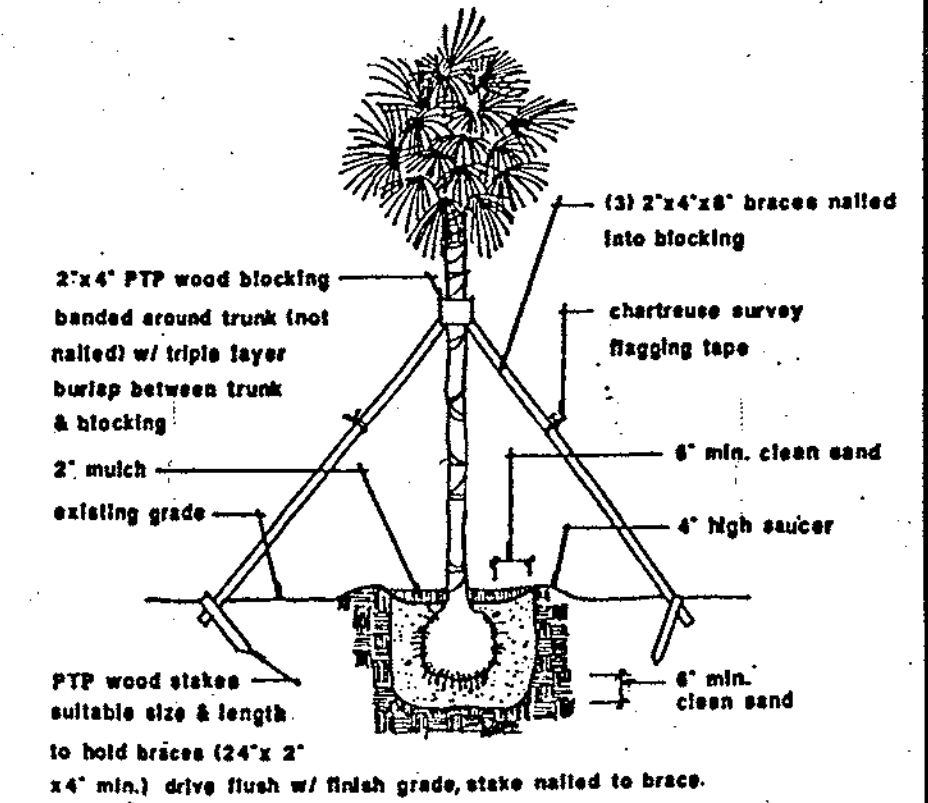
NOTE: Stake all trees up to 2" cal. @ 2 per tree. Guy all trees larger than 2" cal.

Trees

Planting Details



Shrubs



Straight Trunk Palms

QTY	SIZE	DESCRIPTION
2	14x7	Gumbo Limbo
2	14x7	Royal Poinciana
3	16-20	Sable Palm
1	45	Hibiscus tree
1	#10	Weeping Juniper tree
2	5'7	Raphis Palm
1	12'	Double Alexander Palm
2	#10	Bougainvillea tree
2	10'	Canary Island date Palm
3	#30	White Bird of Paradise
1	#10	Mutli-headed sculpture
3	5'	Mutli-headed Robelina Palm
2	#3	Amamanda-Vine Yellow
15	#3	Ilex Shelling
9	#3	Var. Ginger
8	#3	Trinette
3	#5	Cardinal
3	#10	Frangipani Pink (2) Yellow (1)
13	#3	Purple Shower
145	#3	Ficus
20	#3	Crotons
38	#3	Star Jasmine
100	#3	Nora Crab (Xona)
15	#3	Fortain Ferns
10	#3	Crown of Thorns
35	#1	Ruellia
29	#1	Aster grass
18	#3	Dwarf Ixora
2	#10	Juniper sculpture spiral
14	#3	Ground orchid
35	#1	Resmine minima

8/24/01 TOWN OF SEWELL'S POINT
REVIEWED: [Signature]
[Signature]

FILE TOWN COPY
102 ABIE COURT

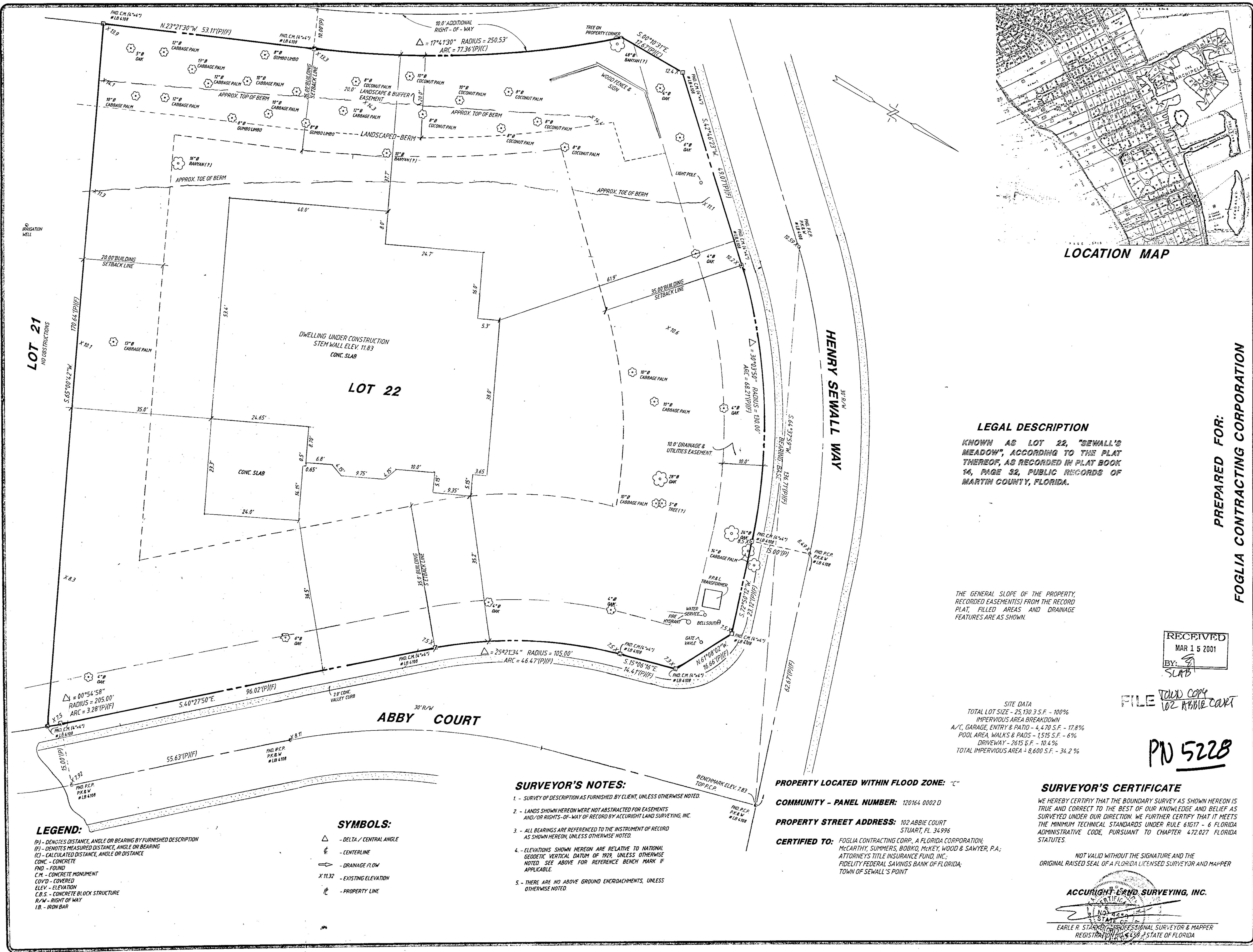
PN 5521

REVISIONS	BY

102 ABIE CT.
Sewell's Meadow
Lot 22 Abbie Court
FOGLIA CUSTOM HOMES

The landscape warehouse
7250 NW 82 Terrace
Parkland, FL 33067
(954) 753-8602

DRAWN 7-17-01	CHECKED 7-18-01
DATE 7-18-01	SCALE 1"=10'
JOB NO.	SHEET
OF	SHEETS



REVISIONS	BY
ADD SITE DATA	W.G.H.
01/12/2001	
TIE-IN	W.G.H.
02/10/2001	
FORM BOARD TIE-IN	W.G.H.
02/24/2001	
SLAB VERIFICATION	W.G.H.
03/06/2001	

FOGLIA CONTRACTING CORPORATION
BOUNDARY SURVEY

ACCURIGHT LAND SURVEYING, INC.
1501 DECKER AVENUE, SUITE 419-D
STUART, FL 34994
OFFICE PHONE # (561) 286-7694
OFFICE FAX # (561) 220-7993

DRAWN	W.G.H.
CHECKED	E.R.S.
DATE	10/1/2000
SCALE	1" = 10.00'
JOB NO.	690-06-010
SHEET	1
OF ONE	SHEETS

LOCATION MAP

LEGAL DESCRIPTION

KNOWN AS LOT 22, "SEWALL'S MEADOW", ACCORDING TO THE PLAT THEREOF, AS RECORDED IN PLAT BOOK 14, PAGE 32, PUBLIC RECORDS OF MARTIN COUNTY, FLORIDA.

THE GENERAL SLOPE OF THE PROPERTY, RECORDED EASEMENT(S) FROM THE RECORD PLAT, FILLED AREAS AND DRAINAGE FEATURES ARE AS SHOWN.

RECEIVED
MAR 15 2001
BY: [Signature]
SLAB

FILE TOWN COPY
102 ABILE CANY

PN 5228

SITE DATA
TOTAL LOT SIZE - 25,130.3 S.F. - 100%
IMPERVIOUS AREA BREAKDOWN
A/C, GARAGE, ENTRY & PATIO - 4,470 S.F. - 17.8%
POOL AREA, WALKS & PADS - 1515 S.F. - 6%
DRIVEWAY - 2615 S.F. - 10.4%
TOTAL IMPERVIOUS AREA - 8,600 S.F. - 34.2%

SURVEYOR'S NOTES:

1. SURVEY OF DESCRIPTION AS FURNISHED BY CLIENT, UNLESS OTHERWISE NOTED.
2. LANDS SHOWN HEREON WERE NOT ABSTRACTED FOR EASEMENTS AND/OR RIGHTS-OF-WAY OF RECORD BY ACCURIGHT LAND SURVEYING, INC.
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5. THERE ARE NO ABOVE GROUND ENCROACHMENTS, UNLESS OTHERWISE NOTED.

PROPERTY LOCATED WITHIN FLOOD ZONE: "C"

COMMUNITY - PANEL NUMBER: 120164 0002 D

PROPERTY STREET ADDRESS: 102 ABILE COURT
STUART, FL 34996

CERTIFIED TO: FOGLIA CONTRACTING CORP., A FLORIDA CORPORATION;
MCARTHY, SUMMERS, BOBKO, McKEY, WOOD & SAWYER, P.A.;
ATTORNEYS TITLE INSURANCE FUND, INC.;
FIDELITY FEDERAL SAVINGS BANK OF FLORIDA;
TOWN OF SEWALL'S POINT

SURVEYOR'S CERTIFICATE

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NOT VALID WITHOUT THE SIGNATURE AND THE ORIGINAL RAISED SEAL OF A FLORIDA LICENSED SURVEYOR AND MAPPER

ACCURIGHT LAND SURVEYING, INC.
EARLE R. STARK, PROFESSIONAL SURVEYOR & MAPPER
REGISTRATION NO. 4599 - STATE OF FLORIDA

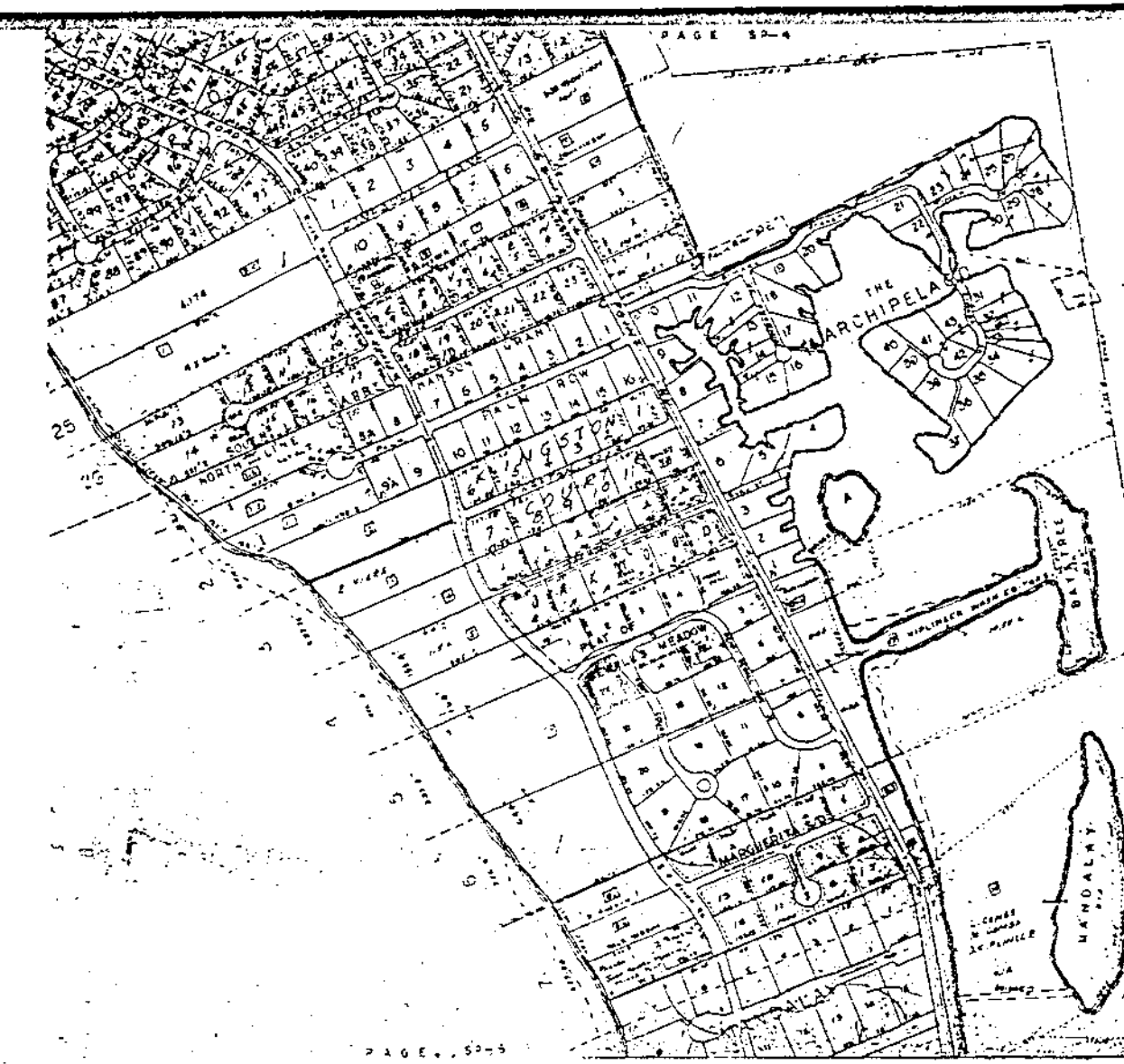
REVISIONS	BY
ADD SITE DATA 01/13/2001	W.G.H.
TIE-IN 02/10/2001	W.G.H.
FORM BOARD TIE-IN 02/26/2001	W.G.H.
SLAB VERIFICATION 03/06/2001	W.G.H.
REVISED SEPTIC 03/12/2001	W.G.H.
POOL TIE-IN 06/01/2001	W.G.H.
FINAL SURVEY 8/28/2001	W.G.H.
ADD ROOF HEIGHT 08/28/2001	W.G.H.

1501 DECKER AVENUE, SUITE 419-D
STUART, FL 34994

OFFICE PHONE # (561) 286-7694
OFFICE FAX # (561) 220-7993

ACCURIGHT LAND SURVEYING, INC.

DRAWN W.G.H.
CHECKED E.R.S.
DATE (FIELD) 10/1/2000
SCALE 1" = 10.00'
JOB NO. 690-06-010
SHEET 1



LOCATION MAP

PREPARED FOR:
FOGLIA CONTRACTING CORPORATION

BOUNDARY SURVEY

LEGAL DESCRIPTION

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RECEIVED
AUG 29 2001
BY: [Signature]
FILE "AS BUILT" SURVEY (REVISED) WITHIN 1447

FILE TOWN COPY
102 ABBIE CT

PN 5228

SITE DATA
TOTAL LOT SIZE - 25,130.3 S.F. - 100%
IMPERVIOUS AREA BREAKDOWN
A/C, GARAGE, ENTRY & PATIO - 4,470 S.F. - 17.8%
POOL AREA, WALKS & PATIOS - 1,515 S.F. - 6%
DRIVEWAY - 2615 S.F. - 10.4%
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PROPERTY STREET ADDRESS: 102 ABBIE COURT
STUART, FL 34996

CERTIFIED TO: FOGLIA CONTRACTING CORP., A FLORIDA CORPORATION;
MCARTHY, SUMMERS, BOBKO, McKEY, WOOD & SAWYER, P.A.;
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TOWN OF SEWALL'S POINT

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ACCURIGHT LAND SURVEYING, INC.

EARLE R. STARKEY, PROFESSIONAL SURVEYOR & MAPPER
REGISTRATION NO. 4458, STATE OF FLORIDA

SURVEYOR'S NOTES:

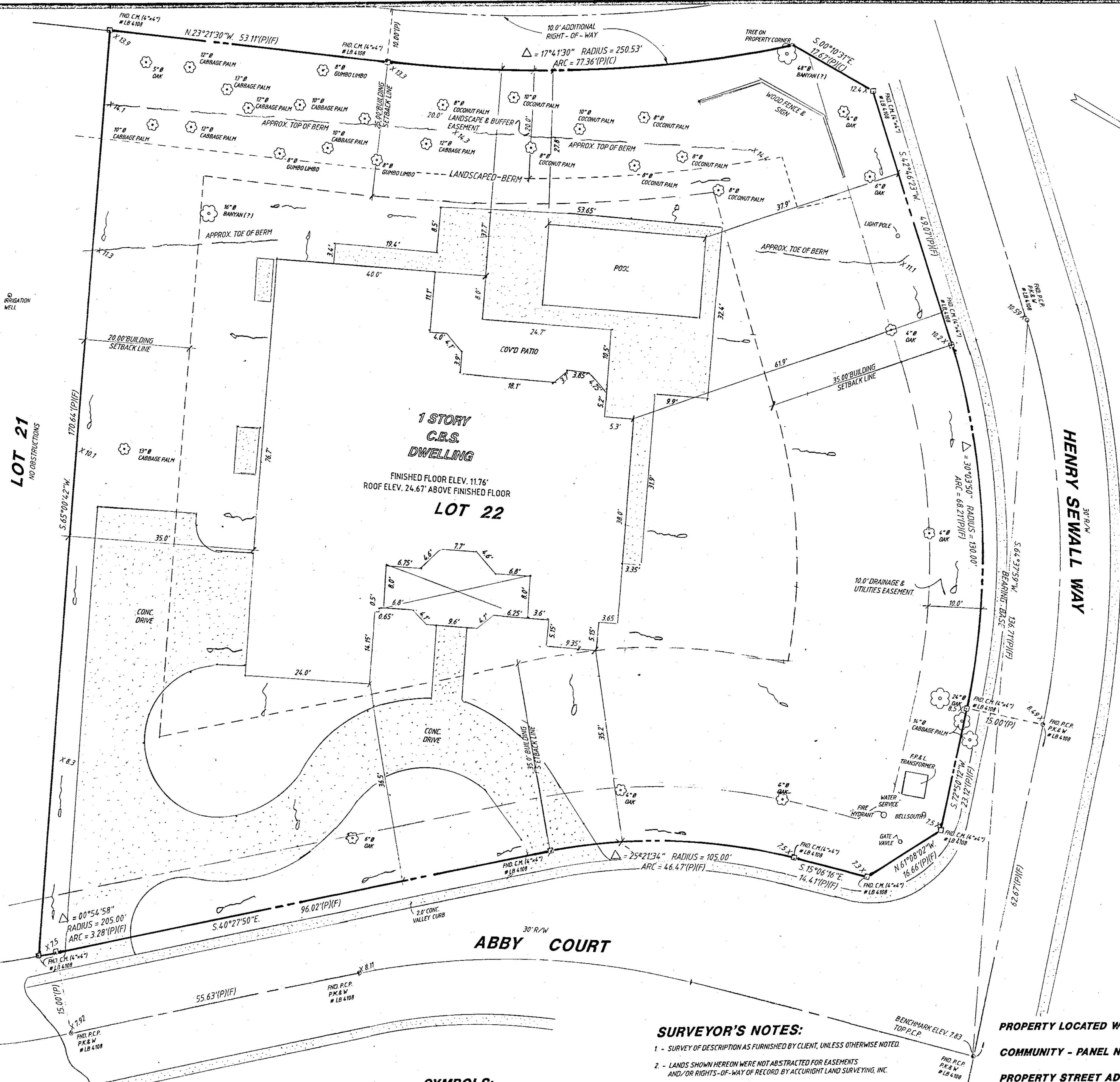
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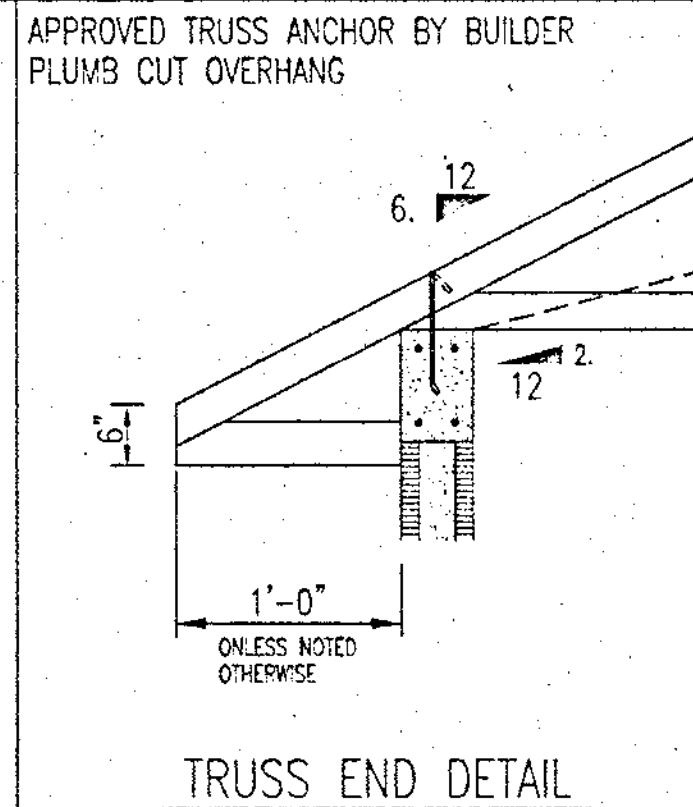
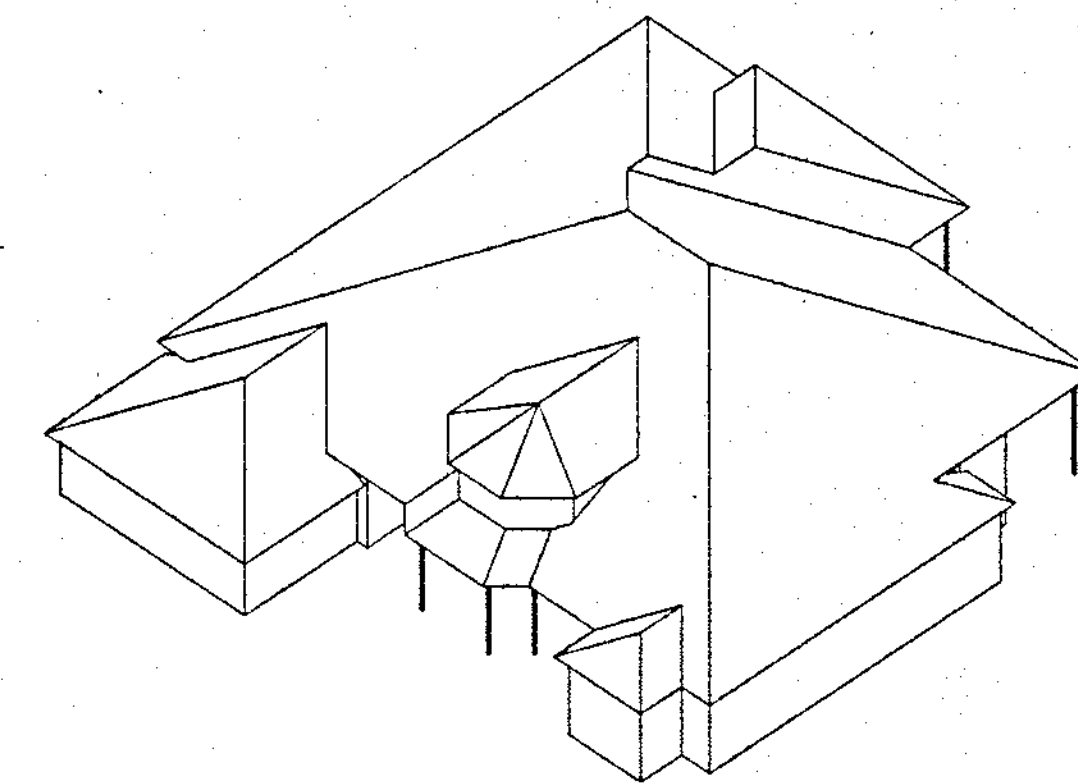
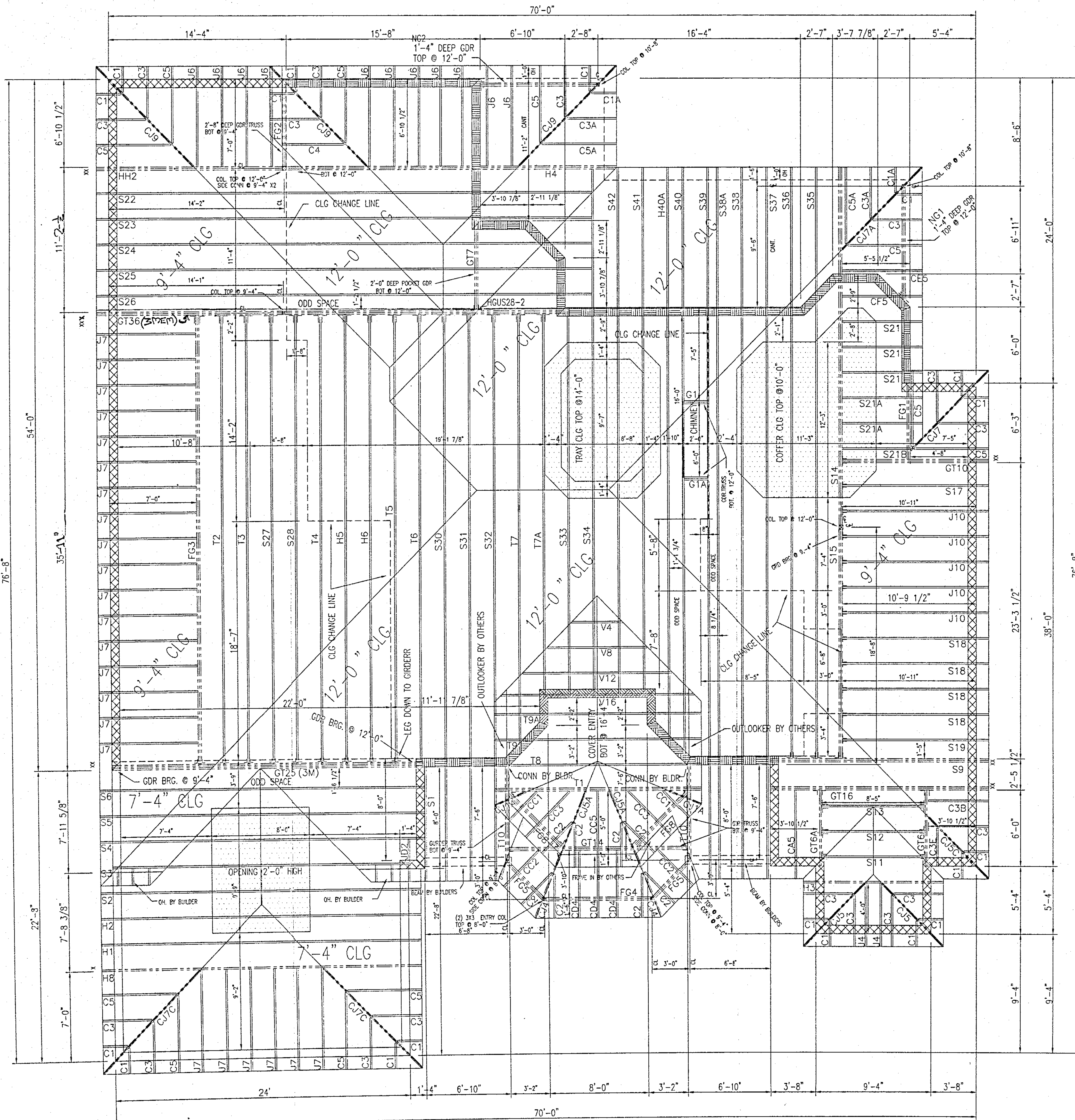
SYMBOLS:

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- C.B.S. - CONCRETE BLOCK STRUCTURE
- R/W - RIGHT-OF-WAY
- I.B. - IRON BAR





BEARING HEIGHT SCHEDULE	
	7'-4"
	9'-4"
	12'-0"

NOTICE

Field Bracing is not the responsibility of the truss fabricator, truss designer, nor plate manufacturer. Persons erecting trusses are cautioned to seek professional advice regarding erection bracing which is always required to prevent topping and dominating during erection, and permanent bracing which may be required in specific applications. Trusses shall be erected and fastened in a straight and plumb position. When no sheathing is applied directly to the top chords, they shall be braced at 24" o.c. max. Where no rigid ceiling is applied directly to the bottom chords, they shall be braced at 6'-0" o.c. max. Trusses shall be handled with reasonable care during erection to prevent excessive damage or personal injury.

See Engineering for connections, nailing schedules, etc.

NOTE: Labeling trusses is a service, not a requirement. Engineered drawings supersede labeling of trusses. It is the responsibility of the builder to utilize engineered drawings when erecting trusses.

Girder truss to girder truss connections which exceed the capacity of a standard hanger or truss to wall connections are the responsibility of the builder.

- GENERAL NOTES**
1. TRUSSES SPACED 24" ON CENTER, U.O.C.
 2. ALL WALL, BEAM & COLUMN MOUNTED TRUSS HANGERS BY BUILDER.
 3. SOME DOWN FRAMING BY BUILDER.
 4. SOME VALLEY FRAMING BY BUILDER.

EXPOSURE CATEGORY: C
BUILDING CATEGORY: I
WIND DESIGN VELOCITY: 110 MPH
MEAN ROOF HEIGHT: _____
IMPORTANCE FACTOR: 1.05
WIND LOAD DURATION FACTOR: 33%

FLORIDA QUALITY TRUSS

TEL: (954) 975-3384
FAX: (954) 978-8980
3120 N.W. 16TH TERRACE
POMPANO BEACH, FL. 33064
YEKTA KAVASOGLU, P.E.
FL. REG. # 41310
CIVIL-STRUCTURAL

P.N. 5228

TOWN COPY
102 ABIE CT

PN 5228

APPROVED FOR TRUSS LAYOUT ONLY

MAR 08 2001

SHOP DRAWING APPROVAL

THIS DRAWING IS THE SOLE SOURCE FOR FABRICATION OF TRUSSES AND VOIDS ALL PREVIOUS ARCHITECTURAL OR OTHER LAYOUTS. APPROVAL OF THIS LAYOUT IS NECESSARY BEFORE ANY WORK CAN BE DONE. PLEASE VERIFY TRUSS PLACEMENT, DIRECTIONS, DIMENSIONS, BEARING CONDITIONS, PITCHES, SHAPES, & OVERHANGS.

102 ABIE CRT.
Approved By: _____ Date: _____

CUSTOMER: FOGIA CUSTOM HOMES	
DATE: 11-30-00	REV: 01-25-01
SCALE: 1/4"=1'-0"	2-19-01 MH
SUBJECT: THE CAPTIVA	PROJ. NO.
LOT #: 22	SAC NO.
MARTIN COUNTY	00469

43 = HUS26
8 = HUS28-2
2 = DRL 45 LEFT
2 = DRL 45 RIGHT

(SAC/145.MH) REV: 3-2-01

